DESIGN DESIGNATION

A.A.D.T. - 2023 = 9,000 A.A.D.T. - 2043 = 10,000 D.H.V. = 800 T = 13% V = 65 M.P.H.

FUNCTIONAL CLASSIFICATION - PRINCIPAL ARTERIAL

NO R/W REQUIRED

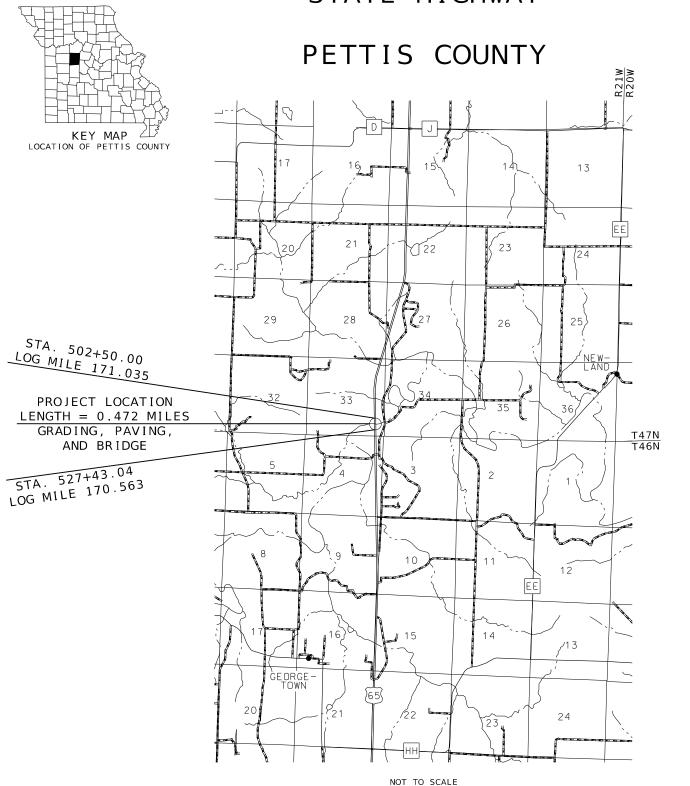
CONVENTIONAL SYMBOLS

(USED IN PLANS	5)	
	EXISTING	NEW
BUILDINGS AND STRUCTURES GUARD RAIL GUARD CABLE CONCRETE RIGHT-OF-WAY MARKER STEEL RIGHT-OF-WAY MARKER LOCATION SURVEY MARKER UTILITIES	0000 0000 0000	
FIBER OPTICS OVERHEAD CABLE TV UNDERGROUND CABLE TV OVERHEAD TELEPHONE UNDERGROUND TELEPHONE OVERHEAD POWER UNDERGROUND POWER SANITARY SEWER STORM SEWER GAS WATER	- FO - -OTV - -UTV - - OT - - UT - - OE - - UE - - SS - - G - - W -	-F0 -OTV -UTV -OT -UT -OE -UE -S -SS -G
MANHOLE	SAN HYD	€
FIRE HYDRANT	wv	3
WATER VALVE	ww.	€
WATER METER		₽
DROP INLET	Ī	
DITCH BLOCK	=	+
GROUND MOUNTED SIGN	SIGN	_
LIGHT POLE		
H-FRAME POWER POLE		
TELEPHONE PEDESTAL FENCE CHAIN LINK WOVEN WIRE GATE POST	——>	√—— ✓——
BENCHMARK	BM (3

NOTE: DASHED OR OPEN SYMBOLS INDICATE EXISTING FEATURES

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

PLANS FOR PROPOSED STATE HIGHWAY



THE EXISTENCE AND APPROXIMATE LOCATION OF UTILITY FACILITIES KNOWN TO EXIST, AS SHOWN ON THE PLANS, ARE BASED ON THE BEST INFORMATION AVAILABLE TO THE COMMISSION AT THIS TIME. THIS INFORMATION IS PROVIDED BY THE COMMISSION "AS-IS" AND THE COMMISSION EXPRESSLY DISCLAIMS ANY REPRESENTATION OR WARRANTY AS TO THE COMPLETENESS, ACCURACY, OR SUITABILITY OF THE INFORMATION FOR ANY USE. RELIANCE UPON THIS INFORMATION IS DONE AT THE RISK AND PERIL OF THE USER, AND THE COMMISSION SHALL NOT BE LIABLE FOR ANY DAMAGES THAT MAY ARISE FROM ANY ERROR IN THE INFORMATION. IT IS, THEREFORE, THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY THE EXISTENCE, LOCATION AND STATUS OF ANY FACILITY. SUCH VERIFICATION INCLUDES DIRECT CONTACT WITH THE LISTED UTILITIES.

INDEX OF SHEETS

SHEET DESCRIPTION NUMBER
TITLE SHEET 1
TYPICAL SECTIONS (TS) (1 SHEET) 2
QUANTITIES (QU) (4 SHEETS) 3
PLAN / PROFILE SHEETS (PL / PR) 4-13
REFERENCE POINTS (RP) 14
COORDINATE POINTS (CP) 15
TRAFFIC CONTROL SHEETS (TC) 16-24
EROSION CONTROL SHEETS (EC)25-34
CULVERT SECTIONS (CS) 35-36
BRIDGE DRAWINGS (B)
A30401 1-18
CROSS SECTIONS (XS) 1-8



LENGTH OF PROJECT

BEGINNING OF PROJECT STA. 502+50.00 END OF PROJECT STA. 527+43.04

APPARENT LENGTH 2493.04 FEET

EQUATIONS AND EXCEPTIONS:

TOTAL CORRECTIONS

NET LENGTH OF PROJECT





GEORGE BUTLER ASSOCIATES, INC. PRO. ENGINEER 000133 ARCHITECT 000212 PRO. LAND SURVEYOR 00005

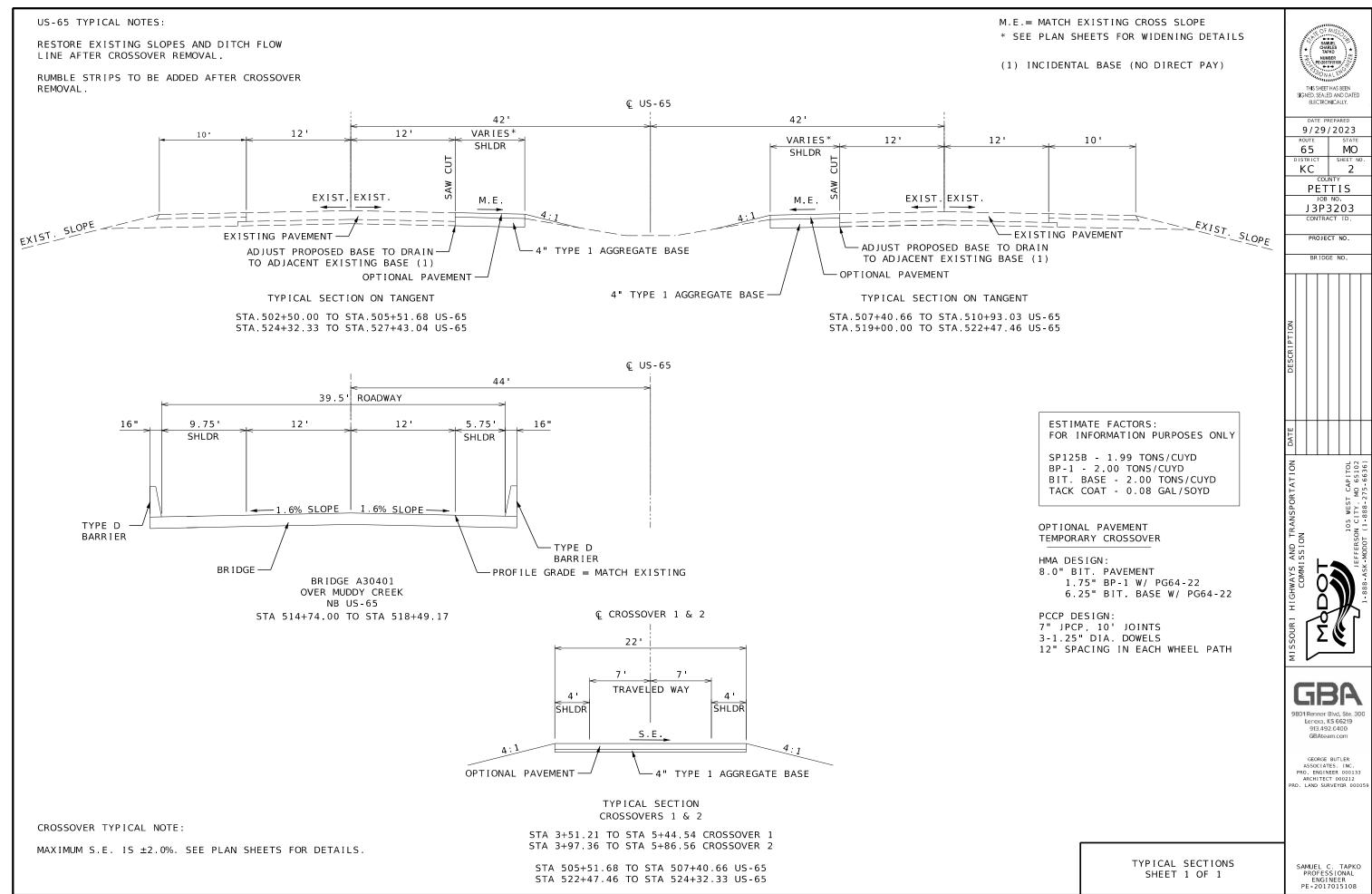
STATE LENGTH 0.472 MILES

FOR INFORMATION ONLY
ESTIMATED DISTURBED ACRES 1.2 ACRES

SAMUEL C. TAPKO PROFESSIONAL ENGINEER

0 FEET

2496.04 FEET



NO. 4 CL US-65 4 CL US-65 4 CL US-65 4,6 CL US-65 4,6 CL US-65 6 CL US-65 6 CL US-65 6 CL US-65 9 CL US-65 9,11 CL US-65 11 CL US-65 11,13 CL US-65 11,13 CL US-65 11,13 CL US-65 8 CL US-65 8 CL US-65 8 CL US-65	502+50.00	END	SIDE	ITEM	QUANTITY	UNIT
4 CL US-65 4 CL US-65 4,6 CL US-65 4,6 CL US-65 4,6 CL US-65 6 CL US-65 6 CL US-65 6 CL US-65 9,11 CL US-65 11 CL US-65 11,13 CL US-65		505+51.68	LT	SAWCUT EXISTING EOP	302	LF
4 CL US-65 4,6 CL US-65 4 CL US-65 4,6 CL US-65 6 CL US-65 6 CL US-65 6 CL US-65 9,11 CL US-65 11 CL US-65 11,13 CL US-65	502+50.00	505+51.68	LT	EXISTING SHOULDER	99	SY
4,6 CL US-65 4 CL US-65 4,6 CL US-65 6 CL US-65 6 CL US-65 6 CL US-65 9,11 CL US-65 11,13 CL US-65	502+50.00	505+51.68	LT	SAWCUT EOS	302	LF
4 CL US-65 4,6 CL US-65 6 CL US-65 6 CL US-65 6 CL US-65 6 CL US-65 9,11 CL US-65 9,11 CL US-65 9,11 CL US-65 9,11 CL US-65 11 CL US-65 11,13 CL US-65	502+50.00	510+93.03	LT/RT	CROSSOVER PAVEMENT	986	SY
4,6 CL US-65 6 CL US-65 6 CL US-65 6 CL US-65 6 CL US-65 9,11 CL US-65 9,11 CL US-65 9,11 CL US-65 9,11 CL US-65 11,13 CL US-65	505+03.11	-	CL	3' X 2' TYPE S-1 DROP INLET	1	EA
6 CL US-65 6 CL US-65 6 CL US-65 6 CL US-65 9,11 CL US-65 9 CL US-65 9,11 CL US-65 9,11 CL US-65 9,11 CL US-65 11,13 CL US-65	505+04.11	509+02.11	CL	18" RCP	398	LF
6 CL US-65 6 CL US-65 6 CL US-65 9,11 CL US-65 11 CL US-65 11,13 CL US-65	507+40.66	509+45.17	RT	SAWCUT EOS	205	LF
6 CL US-65 6 CL US-65 9,11 CL US-65 9 CL US-65 9,11 CL US-65 9,11 CL US-65 9,11 CL US-65 9,11,13 CL US-65 11 CL US-65 11,13 CL US-65	507+40.66	510+93.03	RT	SAWCUT EXISTING EOP	352	LF
6 CL US-65 9,11 CL US-65 9 CL US-65 9,11 CL US-65 9,11 CL US-65 9,11 CL US-65 9,11,13 CL US-65 11 CL US-65 11,13 CL US-65	507+40.66	510+93.03	RT	EXISTING SHOULDER	360	SY
9,11 CL US-65 9 CL US-65 9,11 CL US-65 9,11 CL US-65 9,11,13 CL US-65 9,11 CL US-65 11 CL US-65 11,13 CL US-65	509+02.11	-	CL	18" FES	1	EA
9 CL US-65 9,11 CL US-65 9,11 CL US-65 9,11,13 CL US-65 9,11 CL US-65 11 CL US-65 11,13 CL US-65 11 CL US-65	518+54.92	522+15.36	LT	GUARDRAIL	361	LF
9,11 CL US-65 9,11 CL US-65 9,11,13 CL US-65 9,11 CL US-65 11 CL US-65 11,13 CL US-65 11 CL US-65	518+55.14	519+87.74	LT	GUARDRAIL	133	LF
9,11 CL US-65 9,11,13 CL US-65 9,11 CL US-65 11 CL US-65 11,13 CL US-65 11,13 CL US-65 11,13 CL US-65 11,13 CL US-65 11 CL US-65 11 CL US-65	519+00.00	522+47.46	RT	SAWCUT EXISTING EOP	348	LF
9,11,13 CL US-65 9,11 CL US-65 11 CL US-65 11,13 CL US-65 11,13 CL US-65 11,13 CL US-65 11,13 CL US-65 11 CL US-65 8 CL US-65	519+00.00	522+47.46	RT	EXISTING SHOULDER	370	SY
9,11 CL US-65 11 CL US-65 11,13 CL US-65 11,13 CL US-65 11,13 CL US-65 11,13 CL US-65 11 CL US-65 8 CL US-65	519+00.00	527+43.04	LT/RT	CROSSOVER PAVEMENT	946	SY
11 CL US-65 11,13 CL US-65 11,13 CL US-65 11,13 CL US-65 11 CL US-65 8 CL US-65	520+47.87	522+47.46	RT	SAWCUT EOS	200	LF
11,13 CL US-65 11,13 CL US-65 11,13 CL US-65 11 CL US-65 8 CL US-65	521+86.46	525+82.46	CL	18" RCP	396	LF
11,13 CL US-65 11,13 CL US-65 11 CL US-65 8 CL US-65	524+32.33	527+43.04	LT	SAWCUT EXISTING EOP	310	LF
11,13 CL US-65 11 CL US-65 8 CL US-65	524+32.33	527+43.04	LT	EXISTING SHOULDER	148	SY
11 CL US-65 8 CL US-65	524+32.33	527+43.04	LT	SAWCUT EOS	310	LF
8 CL US-65	525+85.46	-	CL	3' X 2' TYPE S-1 DROP INLET	1	EA
	514+74.04	-	LT	BRIDGE APPROACH SLAB	1	EA
	514+74.04	-	LT	CONCRETE APPROACH PAVEMENT	1	EA
9 CL US-65	518+48.81	-	LT	BRIDGE APPROACH SLAB	1	EA
9 CL US-65	518+48.81	-	LT	CONCRETE APPROACH PAVEMENT	1	EA
		PROJECT T			1 LUMP S	

CULVERT CLEANOUT									
SHEET NO.	ROADWAY	STATION	CULVERT CLEANOUT (EACH)	REMARKS					
11	CL US-65	CL US-65 525+84.21		EX. 18" RCP					
	PROJECT	TOTAL	1						

	TEMPORARY PAVEMENT								
SHEET NO.	ROADWAY	STATION BEGIN	STATION END	SIDE	OPTIONAL PAVEMENT (SY)				
4,6	CROSSOVER 1	0+50.00	8+96.86	LT/RT	1,445				
9,11,13	CROSSOVER 2	0+50.00	8+96.87	LT/RT	1,465				
		PRO	2,910						

	AGGREGATE								
SHEET NO.	ROADWAY	STATION BEGIN	STATION END	SIDE	TYPE 1 AGGREGATE FOR BASE (4") (SY)				
4,6	CROSSOVER 1	0+50.00	8+96.86	LT/RT	1445				
9,11,13	CROSSOVER 2	0+50.00	8+96.87	LT/RT	1465				
		PRO	JECT TOTA	2,910					

	CONCRETE APPROACH PAVEMENT									
SHEET NO. ROADWAY STATION BEGIN STATION END SIDE CONCRETE APPROACH PAVEMENT (SY)						REMARKS				
8	CL US-65	514+19.00	514+54.00	LT	153.6	BRIDGE A30401				
9	CL US-65	518+69.17	519+04.17	LT	153.6	BRIDGE A30401				
		PROJEC	T TOTAL		307.2					

CONTRACTOR FURNISHED SURVEYING AND STALKING	
1 LUMP SUM	

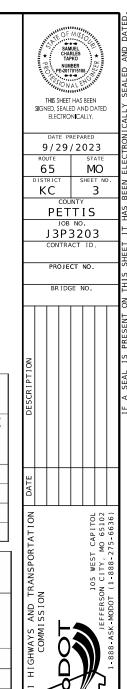
ADDITIONAL MOBLIZATION FOR SEEDING	MOBILIZATION
1 EACH	1 LUMP SUM

	EARTHWORK									
SHEET NO.	ROADWAY	STATION BEGIN	STATION END	CLASS A EXCAVATION (CY)	COMPACTING EMBANKMENT (CY)	EMBANKMENT IN PLACE (CY)	COMPACTING IN CUT (STA)	REMARKS		
4,6	CROSSOVER 1	2+19.08	7+26.29	108	108	214	5.1			
9,11,13	CROSSOVER 2	2+14.04	7+28.41	145	145	4	5.1			
4,6	CL US-65	502+50.00	509+46.70	322	108	-	_	CROSSOVER 1 REMOVAL. MEDIAN DITCH RECONSTRUCTION.		
9,11,13	CL US-65	520+45.04	527+36.64	149	145	-	-	CROSSOVER 2 REMOVAL. MEDIAN DITCH RECONSTRUCTION.		
		PROJEC	T TOTAL	724	507	218	10.2			

NOTE: UNADJUSTED VOLUMES. NO SHRINKAGE FACTOR INCLUDED.

NOTE: FOR INFORMATION ONLY. NO DIRECT PAY.

QUANTITY SHEET SHEET 1 OF 4





GEORGE BUTLER
ASSOCIATES, INC.
PRO. ENGINEER 000133
ARCHITECT 000212
PRO. LAND SURVEYOR 00005

SAMUEL C. TAPKO PROFESSIONAL ENGINEER PE-2017015108

	GUARDRAIL									
SHEET NO.	ROADWAY	STATION BEGIN	STATION END	SIDE	MGS GUARDRAIL 8 FT. POSTS, 6 FT 3 IN. SPACING (LF)	MGS BRIDGE APPROACH TRANSITION SECTION (REGULAR/NO CURB) (EACH)	TYPE A CRASHWORTHY END TERMINAL (MASH) (EACH)			
9,11	CL US-65	518+54.57	522+05.70	LT	263	1	1			
9,11	CL US-65	518+54.57	522+18.20	LT	275	1	1			
		PROJECT TOTAL			538	2	2			

	SHAPING SLOPES									
SHEET NO.	ROADWAY	STATION BEGIN	STATION END	SIDE	SHAPING SLOPES CLASS III (100F)					
9	CL US-65	518+54.57	521+05.70	LT	3					
9,11	CL US-65	518+54.57	521+43.20	LT	3					
		PRO	JECT TOTA	6						

NOTE: SEE STD 606.31B ALTERNATE GRADING LIMITS

	PIPES											
SHEET		STATION	STATION	GROUP A PIPE GROUP A FES CLASS 3 EXCAVATION ROO		ROCK						
NO.	ROADWAY	BEGIN	END	SIDE	18"	18" OR ALLOWED SUBSTITUTE		LINING	REMARKS			
					(LF)	(EACH)	, ,	(CY)				
4,6	CL US-65	505+03.11	509+02.11	CL	398	1	124	2				
11	CL US-65	521+85.46	525+82.46	CL	396	-	213	-	CONNECT TO EXIST. INLET			
	PROJEC TOTAL			-	794	1	337	2				

	INLETS											
SHEET NO.	ROADWAY STATION SIDE			PRECAST CONCRETE DROP INLET 3FT X 2FT (FT)	CLASS 3 EXCAVATION (CY)	REMARKS						
4	CL US-65	505+03.11	CL	5	3	TYPE S-1 DROP INLET						
11	CL US-65	525+85.46	CL	4	3	TYPE S-1 DROP INLET						
		PROJECT	TOTAL	9	6							

	RUMBLE STRIP										
SHEET NO.	ROADWAY	STATION BEGIN	STATION END	SIDE	OPTIONAL PAVEMENT RUMBLE STRIP (STA.)						
4	CL US-65	502+50.00	505+51.68	LT	3.0						
6	CL US-65	507+40.39	510+93.03	RT	3.5						
9,11	CL US-65	519+00.00	522+47.46	RT	3.5						
11,13	CL US-65	524+32.33	527+43.04	LT	3.1						
		PRO	JECT TOTA	<u> </u>	13.1						

	TEMPORARY SEEDING AND MULCHING										
SHEET NO.	ROADWAY	STATION STATION SIDE TEMPORARY SEEDING MUL (ACRE)									
25,26	CROSSOVER 1	0+50.00	5+44.54	RT	0.1	0.1					
25,26	CROSSOVER 1	3+51.21	7+48.14	LT	0.1	0.1					
27,28,29	CROSSOVER 2	2+12.95	5+86.56	LT	0.1	0.1					
27,28,29	CROSSOVER 2	3+97.36	8+90.44	RT	0.1	0.1					
		PROJECT TOTAL 0.4 0.4									

	PERMANENT SEEDING AND MULCHING										
SHEET NO.	ROADWAY										
30,31	CL US-65	502+50.00	509+46.70	LT/RT	0.4	0.4					
32,33,34	CL US-65	520+45.04	527+36.64	LT/RT	0.4	0.4					
	PROJECT TOTAL 0.8 0.8										

9801 Renner Blvd, Ste. 300 Lerexa, KS 66219 913.492.0400 GBAteam.com GEORGE BUTLER
ASSOCIATES, INC.
PRO. ENGINEER 000133
ARCHITECT 000212
PRO. LAND SURVEYOR 00005 QUANTITY SHEET SHEET 2 OF 4 SAMUEL C. TAPKO PROFESSIONAL ENGINEER PE-2017015108

THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY.

DATE PREPARED 9/29/2023 ROUTE 65

PETTIS

J3P3203 CONTRACT ID

PROJECT NO. BRIDGE NO.

DISTRICT KC

ятате МО

SHEET NO

					TEMPO	RARY EROS	SION CON	ΓROL			
SHEET NO.	ELI ROADWAY STATION STATION SIDE DITCH CHECK		ROCK DITCH CHECK (LF)	SEDIMENT TRAP ROCK (CY)	SILT FENCE (LF)	SEDIMENT REMOVAL (CY)	NO. OF DITCH CHECKS	REMARKS			
25	CROSSOVER 1	0+50.00	3+00.00	RT	-	-	-	248	3	-	
25	CROSSOVER 1	3+06.16	-	RT	14	-	=	-	1	1	AT DROP INLET
25,26	CROSSOVER 1	3+13.00	5+04.00	RT	-	-	-	191	2	-	
25,26	CROSSOVER 1	3+94.00	7+47.00	LT	-	-	-	352	4	-	
27,28	CROSSOVER 2	2+13.00	3+33.00	LT	-	-	-	119	2	-	
28	CROSSOVER 2	3+38.47	-	LT	14	-	-	1-1	1	1	AT DROP INLET
28	CROSSOVER 2	3+47.00	5+40.00	LT	-	-	-	193	2	-	
28,29	CROSSOVER 2	4+45.00	8+90.00	RT	-	-	-	443	5	-	
28	CROSSOVER 2	7+35.70		RT	20	-	-	-	1	1	AT EXISTING DROP INLET
30,31	CL US-65	505+00.00	508+70.00	CL	196	-	-	-	11	11	AT 37' SPACING
31	CL US-65	509+07.00	-	CL	-	18	-	-	1	1	LAST IN DITCH CHECK SYSTEM
31	CL US-65	509+44.00	-	CL	-	-	2.4	-	10	s=°	
32,33	CL US-65	522+00.00	525+00.00	CL	144	-	-	-	7	7	AT 50' SPACING
33	CL US-65	525+50.00	-	CL	-	21	-	-	1	1	LAST IN DITCH CHECK SYSTEM
34	CL US-65	526+00.00	-	CL	-	-	3.1	-	10	-	
		PRO	DJECT TOTA	L	387	38	5.5	1546	61		

	PERMANENT EROSION CONTROL										
SHEET NO.	ROADWAY	STATION	SIDE	FURNISHING TYPE 1 ROCK BLANKET (CY)	PLACING TYPE 1 ROCK BLANKET (CY)	PERMANENT EROSION CONTROL GEOTEXTILE (SY)	REMARKS				
32	CL US-65	518+70.67	LT	4	4	15	8" THICK DRAIN FLUME				
32	CL US-65	518+70.67	LT	2	2	7	8" THICK DRAIN FLUME				
		PROJECT	TOTAL	6	6	22					

	TEMPORARY PAVEMENT MARKING												
SHEET ROADWAY		TEMPORARY REMOVABLE MARKING TAPE, 4" WHITE (LF)		TEMPORARY PAVEMENT MARKING REMOVAL - BEFORE (LF)	TEMPORARY PAVEMENT MARKING REMOVAL - AFTER (LF)								
20 US 65 SB		-	1345	306	1345								
21 CROSSOVER 1		830	755	420	1585								
21 US 65 SB		123	1191	390	1314								
22 US 65 SB		447	1129	618	1576								
22	BRIDGE	375	750	469	750								
22	CROSSOVER 2	235	235	235	470								
23	US 65 SB	-	525	106	525								
23	CROSSOVER 2	545	498	275	1043								
23	US 65 NB	892	-	-7	892								
24	US 65 NB	408	-	-	408								
	SUBTOTAL	3855	6428	2819	9908								
PROJECT TOTAL 3855 6428 12727													

		PERMANENT PAVEMENT MARKING										
ER	ROADWAY	STATION BEGIN	STATION END	LENGTH (FT)	NO. OF LANES	6" WHITE HIGH BUILD (LF)	6" YELLOW HIGH BUILD (LF)					
_	US-65 NB	502+50.00	505+51.68	301.70	2	-	302					
_	US-65 SB	507+40.39	510+93.03	352.60	2	-	353					
-	US-65 NB	514+19.00	514+74.00	55.00	2	69	55					
\dashv	US-65 NB	514+74.00	518+49.17	375.20	2	469	375					
\dashv	US-65 NB	518+49.17	519+04.17	55.00	2	69	55					
\dashv	US-65 SB	519+00.00	522.47.46	347.50	2	-	348					
	US-65 NB	524+32.33	527+43.04	310.70	2	-	311					
	PROJECT TOTAL 607 1799											

QUANTITY SHEET SHEET 3 OF 4

		ELEC	SAN CHAI TAI NUM E-2017 ON SHEET SEALE	HAS D AN NICA	BEEN ID DV	ATED	
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			JOB	NO			
		CON	TRA	CT	0.		
		PRO	DJE	СТ	NO.		
H		BR	IDG	E M	10.		
DESCRIPTION							
DATE							
PORTATION					EST CAPITOL	Y, MO 65102	8-275-6636)

MODOT

105 WEST CA

105 WEST CA

9801 Renner Blvd, Ste. 300 Lenexa, KS 66219 913.492.0400 GBAteam.com

GEORGE BUTLER
ASSOCIATES, INC.
PRO. ENGINEER 000133
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PRO. LAND SURVEYOR 00005

SAMUEL C. TAPKO PROFESSIONAL ENGINEER PE-2017015108

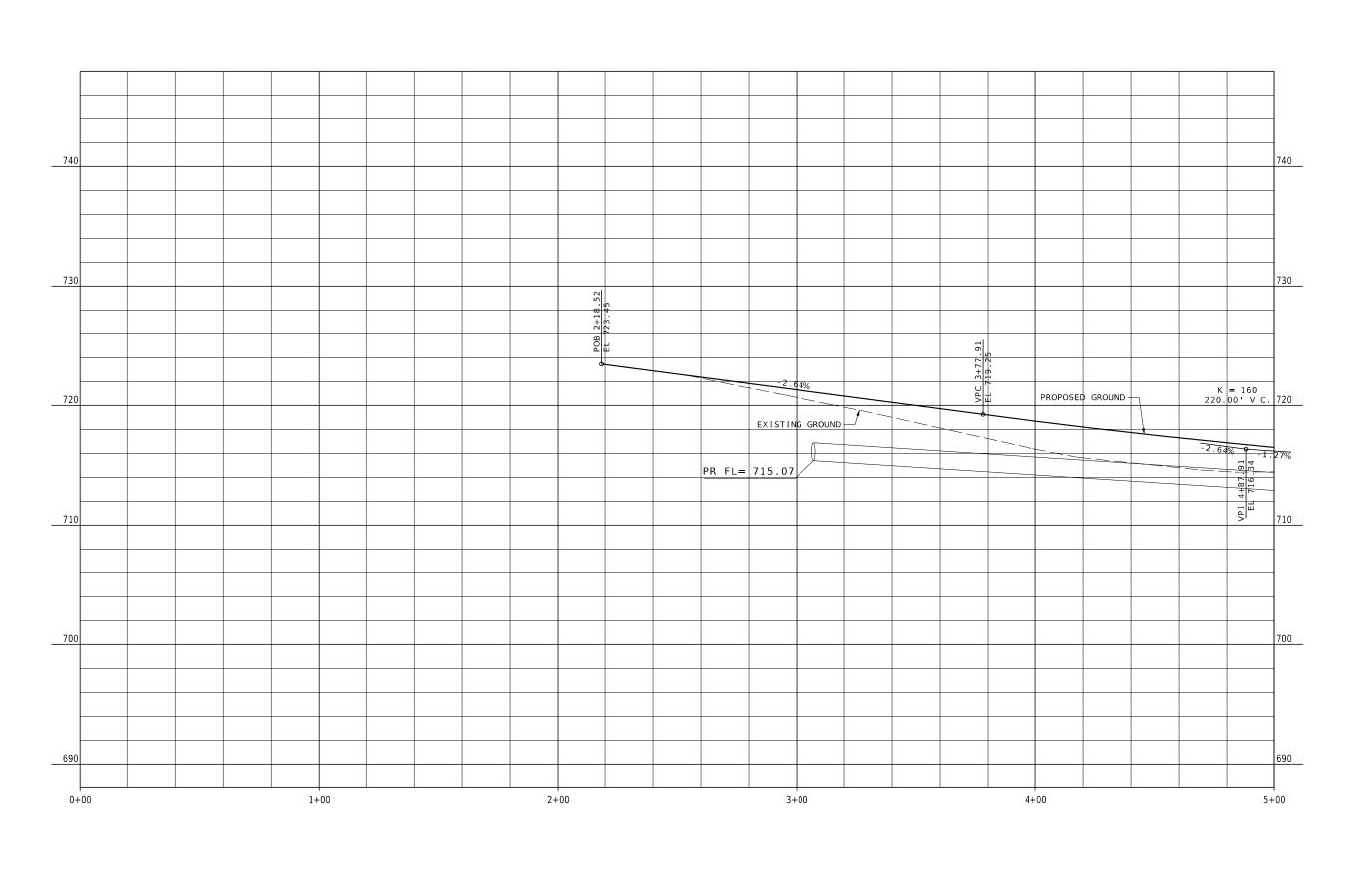
	ТОТА	L QTY TOTAL SIGN						QTY TOTAL	SIGN				
SIZE ARE			i. <mark> </mark>		SIZE		TY TOTA		NUM.			1	
IN. SQ.FT				SIGN	IN.		CH SQ.FT				ITEM	TOTAL	
100/40 140.00	WARNING S	SIGNS	DESCRIPTION		001/40		UIDE SIG	NS	1	DESCRIPTION	NUMBER	R QTY	DESCRIPTION
48X48 16.00 8 48X48 16.00			TURN (SYMBOL LEFT ARROW) TURN (SYMBOL RIGHT ARROW)	E05-1 E05-2		12.00 12.00				GORE EXIT EXIT OPEN	6122008 6122009		IMPACT ATTENUATOR 40 MPH (SAND BARRELS) IMPACT ATTENUATOR 45 MPH (SAND BARRELS)
48X48 16.00			CURVE (SYMBOL LEFT ARROW)	E05-2a		12.00				EXIT CLOSED	6122010		IMPACT ATTENUATOR 50 MPH (SAND BARRELS)
8 48X48 16.00			CURVE (SYMBOL RIGHT ARROW)	GO20-1			4 40.00		1	ROAD WORK NEXT XX MILES	6122012		IMPACT ATTENUATOR 55 MPH (SAND BARRELS)
48X48 16.00			REVERSE TURN (SYMBOL LEFT ARROW)	GO20-2	48X24	8.00	4 32.00		26	END ROAD WORK	6122014		IMPACT ATTENUATOR 60 MPH (SAND BARRELS)
8 48X48 16.00			REVERSE TURN (SYMBOL RIGHT ARROW)	GO20-4	36X18					PILOT CAR FOLLOW ME	6122017	4	IMPACT ATTENUATOR 65 MPH (SAND BARRELS)
48X48 16.00 8 48X48 16.00		15A 15B	REVERSE CURVE (SYMBOL LEFT ARROW) REVERSE CURVE (SYMBOL RIGHT ARROW)	GO20-4a GO20-4a	42X30	1.50				PILOT CAR IN USE WAIT & FOLLOW PILOT CAR IN USE WAIT & FOLLOW	6122019 6122020	8	IMPACT ATTENUATOR 70 MPH (SAND BARRELS) REPLACEMENT SAND BARREL
L 48X48 16.00		130	DOUBLE ARROW REVERSE CURVE (SYMBOL LT ARROWS)	GO20-5aP			2 72.00		54	WORK ZONE (PLAQUE)	6122030	- 0	IMPACT ATTENUATOR (RELOCATION)
R 48X48 16.00			DOUBLE ARROW REVERSE CURVE (SYMBOL RT ARROWS)	MO4-8a		3.00				END DETOUR	6123000A	3	TRUCK OR TRAILER MOUNTED ATTENUATOR (TMA)
L 48X48 16.00			TRIPLE ARROW REVERSE CURVE (SYMBOL LT ARROWS)	MO4-9L		12.00				DETOUR (LEFT ARROW)	6161008	4	ADVANCED WARNING RAIL SYSTEM
R 48X48 16.00		10	TRIPLE ARROW REVERSE CURVE (SYMBOL RT ARROWS)	MO4-9R		12.00				DETOUR (RIGHT ARROW)	6161012		BUOYS (BOATS KEEP OUT)
60X30 12.50 72X36 18.00		16	HORIZONTAL ARROW (SYMBOL) HORIZ. ARROW (SYMBOL ON PERMANENT BARRICADE)	MO4-9P MO4-10L	48X12 48X18					STREET NAME (PLAQUE) DETOUR (ARROW LEFT)	6161013 6161014		BUOYS (NO WAKE) SPECIAL SIGN ASSEMBLY (BOATS KEEP OUT)
60X30 12.50			DOUBLE HEAD HORIZONTAL ARROW (SYMBOL)	MO4-10E	48X18					DETOUR (ARROW RIGHT)	6161025	42	CHANNELIZER (TRIM LINE)
72X36 18.00			DOUBLE HEAD HORIZ. ARROW (SYMBOL ON PERM. BARR.)				<u>EGÜL</u> ATC	RY SIGNS			6161030	5	TYPE III MOVEABLE BARRICADE
18X24 3.00			CHEVRON (SYMBOL)	R1-1		13.25				STOP	6161033	32	DIRECTION INDICATOR BARRICADE
30X36 7.50			CHEVRON (SYMBOL FOR DIVIDED HIGHWAYS)	R1-2	48TRI.					YIELD	6161040	5	FLASHING ARROW PANEL
48X48 16.00 48X48 16.00			STOP AHEAD (SYMBOL) YIELD AHEAD (SYMBOL)	R1-2a R1-3P	36X36 30X12	9.00				TO ONCOMING TRAFFIC (PLAQUE) ALL WAY (PLAQUE)	6161047 6161055	32	TYPE III OBJECT MARKER SEQUENTIAL FLASHING WARNING LIGHT
48X48 16.00 48X48 16.00			SIGNAL AHEAD (SYMBOL)	R2-1		+	4 48.00		25	SPEED LIMIT 65 MPH	6161055	36	TUBULAR MARKER
48X48 16.00			BE PREPARED TO STOP	R2-1			8 96.00			SPEED LIMIT 55 MPH	6161095		RADAR SPEED ADVISORY SYSTEM
48X48 16.00	4 64.00	3	SPEED LIMIT AHEAD	R3-1	48X48	16.00				NO RIGHT TURN (SYMBOL)			CHANGEABLE MESSAGE SIGN,
48X48 16.00			MERGE (SYMBOL FROM LEFT)	R3-2		16.00				NO LEFT TURN (SYMBOL)	6161096		COMMISSION FURNISHED/RETAINED
R 48X48 16.00 L 48X48 16.00		6A	MERGE (SYMBOL FROM RIGHT) MERGE (ARROW SYMBOL)	R3-3 R3-4		9.00				NO TURNS NO U-TURN (SYMBOL)	6161098A	2	CHANGEABLE MESSAGE SIGN W/O COMM. INTERFACE - CONTRACTOR FURNISHED/RETAINED
R 48X48 16.00		6A 6B	MERGE (ARROW SYMBOL) MERGE (ARROW SYMBOL)	R3-7L	30X30					LEFT LANE MUST TURN LEFT	- HOROTO HORA	+	CHANGEABLE MESSAGE SIGN WITH COMM.
48X48 16.00			ROAD/BRIDGE/RAMP NARROWS	R3-7R	30X30					RIGHT LANE MUST TURN RIGHT	6161099		INTERFACE - CONTRACTOR FURNISHED/RETAINED
48X48 16.00			ONE LANE BRIDGE	R4-1	36X48	12.00	4 48.00		46	DO NOT PASS	6162000A		WORK ZONE TRAFFIC SIGNAL SYSTEM
48X48 16.00		34		R4-2		12.00				PASS WITH CARE	6162002		TEMPORARY LONG-TERM RUMBLE STRIPS
48X48 16.00 48X48 16.00			DIVIDED HIGHWAY (SYMBOL) DIVIDED HIGHWAY END (SYMBOL)	R4-8a R4-7a		12.00				KEEP LEFT (HORIZONTAL ARROW)		1140	TEMPORARY TRAFFIC BARRIER
48X48 16.00 48X48 16.00		31	TWO WAY TRAFFIC (SYMBOL)	R5-1		12.00 6.25	2 12.50		55	KEEP RIGHT (HORIZONTAL ARROW) DO NOT ENTER	6173600D		CONTRACTOR FURNISHED/RETAINED TEMPORARY TRAFFIC BARRIER
30X24 5.00			NEXT XX MILES (PLAQUE)	R5-1a	36X24		12.00			WRONG WAY	6173602B		CONTRACTOR FURNISHED/COMMISSION RETAINED
48X48 16.00			BUMP	R6-1L	54X18	6.75				ONE WAY ARROW (LEFT)	6174000A		TEMP. TRAFFIC BARRIER HEIGHT TRANSITION
48X48 16.00			DIP	R6-1R	54X18					ONE WAY ARROW (RIGHT)	6175010A		RELOCATING TEMPORARY TRAFFIC BARRIER
48X48 16.00			PAVEMENT ENDS	R6-2L	24X30					ONE WAY (LEFT)			TEMPORARY TRAFFIC BARRIER
48X48 16.00 48X48 16.00			SOFT SHOULDER SLIPPERY WHEN WET (SYMBOL)	R6-2R R9-9	24X30	2.00				ONE WAY (RIGHT) SIDEWALK CLOSED	6176000B		COMMISSION FURNISHED/RETAINED TEMP. TRAFFIC BARRIER HEIGHT TRANSITION
48X48 16.00			TRUCK CROSSING (WITH FLAGS)		21/(12	2.00				SIDEWALK CLOSED AHEAD,	6177000B		COMMISSION FURNISHED/RETAINED
48X48 16.00			TRUCK ENTRANCE	R9-11L	24X18	3.00				(ARROW LEFT) CROSS HERE	6208064A		TEMPORARY RAISED PAVEMENT MARKER
36X36 9.00			LOOSE GRAVEL							SIDEWALK CLOSED AHEAD,	9029400		TEMPORARY TRAFFIC SIGNALS
36X36 9.00			FRESH OIL/LOOSE GRAVEL	R9-11R	24X18					(ARROW RIGHT) CROSS HERE	9029401		TEMPORARY TRAFFIC SIGNALS AND LIGHTING
48X48 16.00 48X48 16.00			LOW SHOULDER UNEVEN LANES	R10-6 R11-2	24X36 48X30		1 10.00		29	STOP HERE ON RED (45^ ARROW) ROAD CLOSED	6205301B	3855*	TEMPORARY REMOVABLE MARKING TAPE 4 IN. WHITE
48X48 16.00			NO CENTER LINE		10/100	10.00	1 10.00		20	ROAD CLOSED XX MILES AHEAD			TEMPORARY REMOVABLE MARKING TAPE
48X48 16.00			GROOVED PAVEMENT	R11-3a	60X30	12.50				LOCAL TRAFFIC ONLY	6205303B	6428*	4 IN. YELLOW
P 30X24 5.00			MOTORCYCLE (PLAQUE)	R11-4	60X30					ROAD CLOSED TO THRU TRAFFIC	6207001	12,727*	PAVEMENT MARKING REMOVAL
48X48 16.00			SHOULDER DROP-OFF (SYMBOL)	CONST-3A						FINE SIGN			
P 30X24 5.00 42RND. 9.62			SHOULDER DROP-OFF (PLAQUE) RAILROAD CROSSING	CONST-3X	10012		ISCELLA!	NEOUS SIGNS	1	SPEEDING/PASSING (PLATE)		(*)· ∩	UUANTITES SUMMARIZED ON QUANTITY SHEET 3 OF 4
24X24 4.00			DOUBLE DOWN ARROW (SYMBOL)	CONST-5	48X36		- JOLLLAI			POINT OF PRESENCE		(). Q	CONTRACTOR OF CO
48X48 16.00			LOW CLEARANCE (SYMBOL)	CONST-5	96X48	32.00	2 64.00			POINT OF PRESENCE			
X 24X18 3.00			LOW CLEARANCE (PLAQUE)	CONST-7						RATE OUR WORK ZONE			
a 84X24 14.00			OVERHEAD LOW CLEARANCE (FEET AND INCHES)	CONST-7			2 36.00			RATE OUR WORK ZONE	_		
120X60 50.00 120X60 50.00			LOW CLEARANCE XX FT XX IN XX MILES AHEAD WIDTH RESTRICTION XX FT XX IN XX MILES AHEAD	CONST-8	48X36	12.00	4 48.00			WORK ZONE NO PHONE ZONE	_		
30X30 6.25		44	ADVISORY SPEED (PLAQUE)	$\dashv\vdash$	+						_		
30X24 5.00		 	XXX FEET (PLAQUE)	$\neg \Vdash$									
30X24 5.00			X MILE (PLAQUE)										
48X48 16.00		2									_		
48X48 16.00			DETOUR AHEAD	$-\parallel$	-						_		
48X48 16.00 48X48 16.00			ROAD CLOSED AHEAD ONE LANE ROAD AHEAD	-	+						_		
48X48 16.00		5	RIGHT/CENTER/LEFT LANE CLOSED AHEAD	616-10.0	5		TOTA	<u>.L</u>		ı			
a 48X48 16.00			2 RIGHT/CENTER/LEFT LANES CLOSED AHEAD			N SIGNS	1191						
a 48X48 16.00		6	RIGHT/CENTER/LEFT LANE CLOSED	616-10.1	0			TOTAL]				
a 48X48 16.00			FLAGGER (SYMBOL, WITH FLAGS)	RELOCA	ATED SI	GNS		0	_				
36X36 9.00			FRESH OIL										
48X48 16.00		21	SHOULDER WORK AHEAD										
48X48 16.00 42X36 10.50			BLASTING ZONE AHEAD TURN OFF 2-WAY RADIO AND PHONE										
12,100 10.00			END BLASTING ZONE										QUANTITY SHEET
42X36 10.50													Q0/111111 OTTEET

ENGINEER PE-2021011939

NOTE: € US-65 ANY ITEMS OF WORK BEYOND THE PROJECT LIMITS ARE INCIDENTAL TO, REGRADE TO EXISTING CONDITIONS AFTER STA. 505+03.11 TO STA. 509+02.11 AND A PART OF THE CONSTRUCTION OF THIS PROJECT. THE REHABILITATION OF THE BR. NO. A30401 INSTALL 3' x 2' TYPE S-1 DROP INLET W/ 398.0 (398) ' X 18" GROUP A PIPE THE INFORMATION SHOWN IN THESE PLANS CONCERNING TYPE AND LOCATION W/ 18" FLARED END SECTION OF UNDERGROUND UTILITIES IS NOT GUARANTEED TO BE ACCURATE OR ALL CL. 3 EXC. = 127 CYINCLUSIVE, THE CONTRACTOR IS RESPONSIBLE FOR MAKING THEIR OWN REMOVE AFTER BRIDGE CONSTRUCTION THIS SHEET HAS BEEN = SUPERELEVATION TRANSITION DETERMINATIONS AS TO THE TYPE AND LOCATION OF UNDERGROUND IS COMPLETE. UTILITIES AS MAY BE NECESSARY TO AVOID DAMAGE THERETO. 9/29/2023 65 МО KC 302' SAWCUT AT EXISTING EOP PETTIS REMOVE EXISTING SHOULDER EXISTING R/W WIDENING AT EOP FOR TEMPORARY CROSSOVER J3P3203 CONTRACT II PROJECT NO. 0 BRIDGE NO 1+90.19 0+50.00 3+30.00 PC PT 7°17'31.9' (RT) $\mathbf{\Omega}$ 280.00' (ARC) BEGIN PROJECT 140 19 0 - STA. 3+51.21 2,200.00 © US-65 STA 502+50.00, 36' LT 2 OFF 11.00 LT NB US-65 00+9 _S00°33'50"E \mathbb{C} CROSSOVER 1 STA. 0+00.00 = © US-65 STA 502+00.00, 36' LT +27.95 € US-65 +51.00 S MATCHL SB US-65 302' SAWCUT AT EOS LEAVE OPTIONAL PAVEMENT AT SHOULDER 9801 Renner Blvd. Ste. 30 Lenexa, KS 66219 913.492.0400 GBAteam.com GEORGE BUTLER
ASSOCIATES, INC.
PRO. ENGINEER 000133
ARCHITECT 000212
PRO. LAND SURVEYOR 0000 -EXISTING R/W SCALE CROSSOVER 1 SAMUEL C. TAPKO PROFESSIONAL PLAN

SHEET 1 OF 6

PE-2017015108



OF MASS
SAMUEL
SAMUEL
NAMEN
NA

DATE PREPARED
9/29/2023
ROUTE 5TATE
6.5 MO
DISTRICT SHEET NO
KC 5

PETTIS

JOB NO.

J3P3203

CONTRACT ID.

CONTRACT ID.

PROJECT NO.

BRIDGE NO.

DESCRIPTION

COMMISSION

COMMIS

GBA

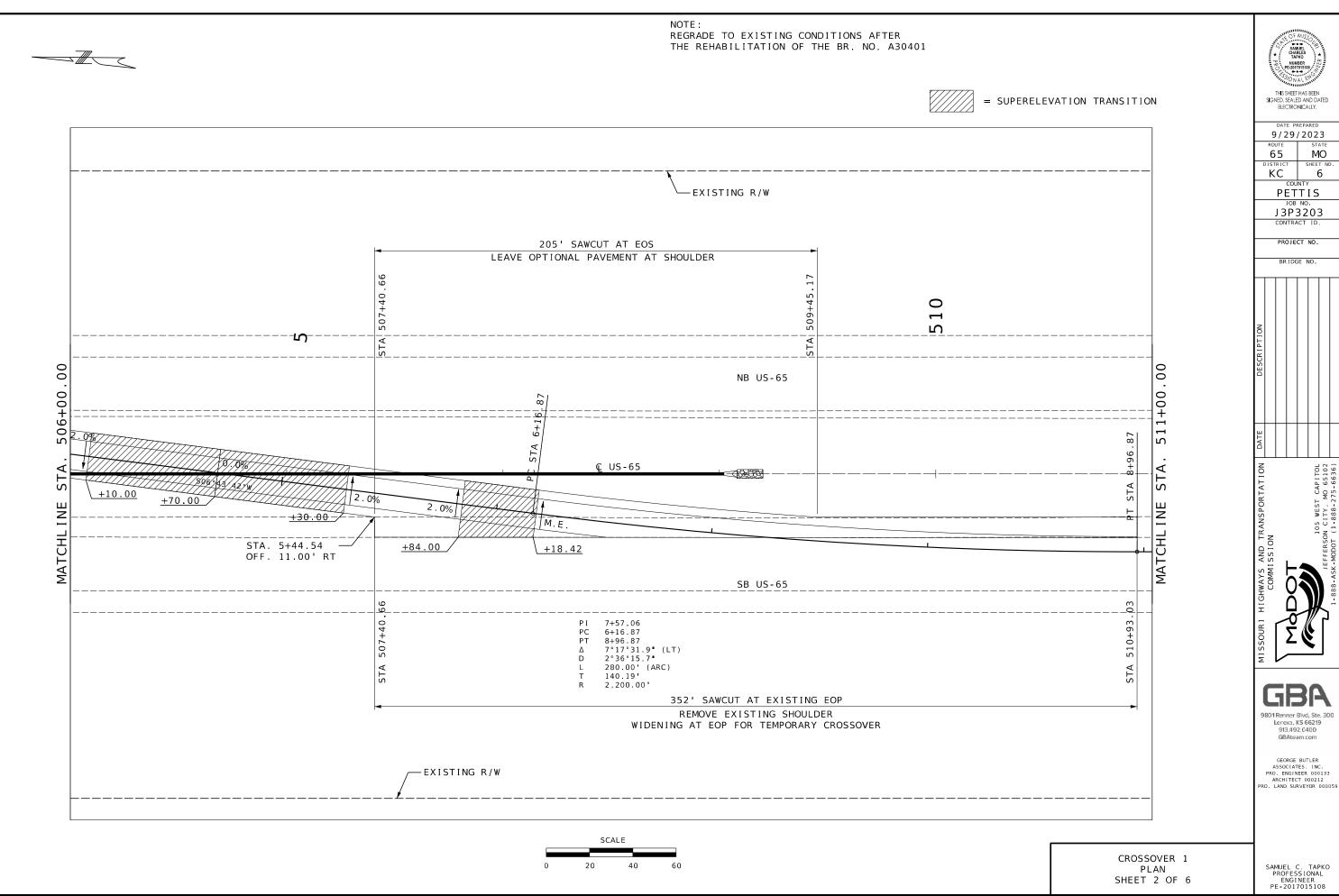
9801 Renner Blvd, Ste. 300 Lenexa, KS 66219 913.492.0400 GBAteam.com

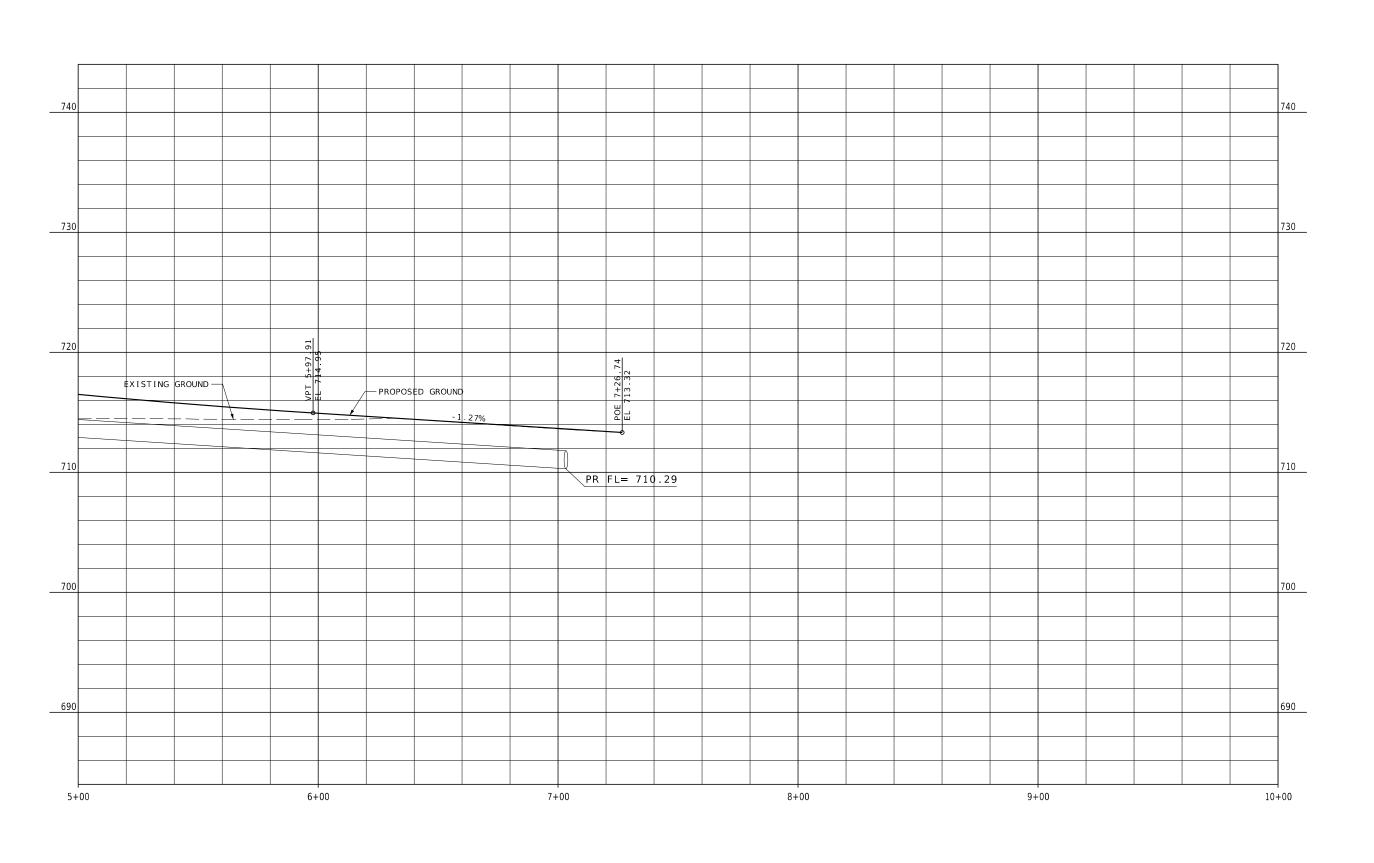
GEORGE BUTLER
ASSOCIATES, INC.
PRO. ENGINEER 000133
ARCHITECT 000212
PRO. LAND SURVEYOR 00005

SAMUEL C. TAPKO PROFESSIONAL ENGINEER PE-2017015108

CROSSOVER 1

PROFILE SHEET 1 OF 4





THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY.

DATE PREPARED 9/29/2023 ROUTE 65 ятате МО KC SHEET NO

PETTIS J3P3203 CONTRACT ID

PROJECT NO.

BRIDGE NO.

9801 Renner Blvd, Ste. 300 Lerexa, KS 66219 913.492.0400 GBAteam.com

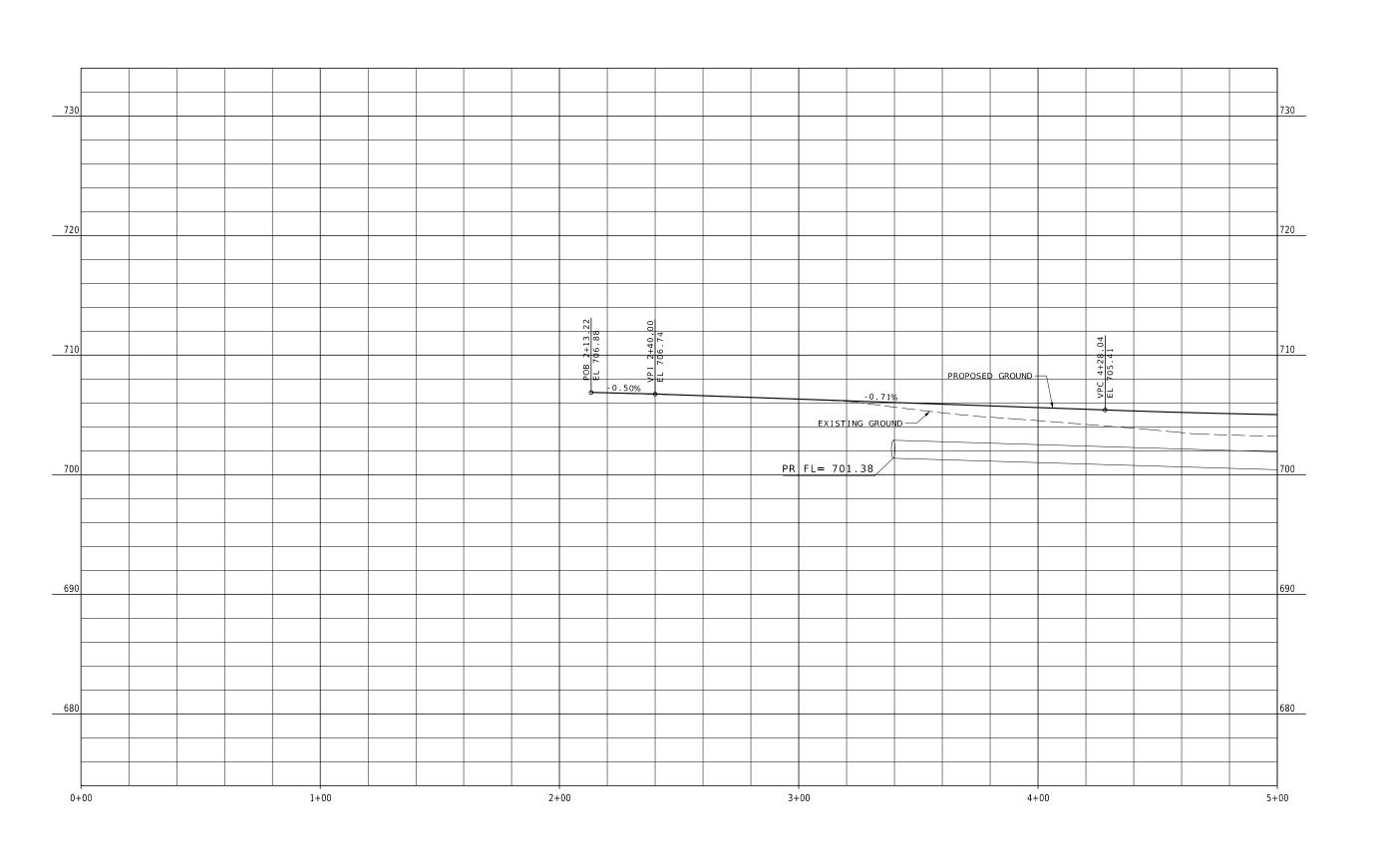
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ASSOCIATES, INC.
PRO. ENGINEER 000133
ARCHITECT 000212
PRO. LAND SURVEYOR 00005

SAMUEL C. TAPKO PROFESSIONAL ENGINEER PE-2017015108

NOTE: © US-65 STA. 514+19.00 TO STA. 514+54.00, LT CONSTRUCT 153.6 SY CONCRETE APPROACH PAVEMENT REGRADE TO EXISTING CONDITIONS AFTER
THE REHABILITATION OF THE BR. NO. A30401 (STD. PLAN 504.00L) THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY. 9/29/2023 MO 65 KC 8 PETTIS —EXISTING R/W J3P3203 PROJECT NO. -REDECK EXISTING BRIDGE A30401 (DND) BRIDGE NO. STA. 514+74.00 TO STA. 518+49.17 SEE BRIDGE PLANS FOR DETAILS CONCRETE APPROACH-PAVEMENT \mathbf{O} 2 00 NB US-65 BRIDGE A30401 +94. 35 ' 20 +00 0 2 5 BRIDGE APPROACH SLAB € US-65 (DND) ST ST, INE (DND) MATCHL I S00°33'50"E SB US-65 \mathbb{C} CROSSOVER 1 STA. $10+\underline{03.83} =$ © US-65 STA. 512+00.00, 36' RT Lenexa, KS 66219 913.492.0400 GBAteam.com GEORGE BUTLER ASSOCIATES, INC. PRO. ENGINEER 000133 ARCHITECT 000212 PRO. LAND SURVEYOR 0000 -EXISTING R/W CROSSOVER 1 SAMUEL C. TAPKO PROFESSIONAL ENGINEER PE-2017015108 PLAN SHEET 3 OF 6

¢ US-65 C US-65 NOTE: C US-65 STA. 518+54.57 TO STA. 522+05.70, LT STA. 518+54.57 TO STA. 522+18.20, LT STA. 518+69.17 TO STA. 519+04.17, LT REGRADE TO EXISTING CONDITIONS AFTER SAMUEL CHARLES TAPKO NUMBER PE-201701510 THE REHABILITATION OF THE BR. NO. A30401 INSTALL 1 MGS BRIDGE APPROACH TRANSITION INSTALL 1 MGS BRIDGE APPROACH TRANSITION CONSTRUCT 153.6 SY CONCRETE APPROACH PAVEMENT SECTION (REGULAR/NO CURB)
W/ 262.5' MGS GUARDRAIL (8' POSTS) SECTION (REGULAR/NO CURB) (STD. PLAN 504.00L) W/ 275.0' MGS GUARDRAIL (8' POSTS) & 1 TYPE A CRASHWORTHY END TERMINAL (MASH) & 1 TYPE A CRASHWORTHY END TERMINAL (MASH) THIS SHEET HAS BEEN 9/29/2023 МО 65 KC 9 PETTIS EXISTING R/W — J3P3203 PROJECT NO. 47' SAWCUT AT EOS LEAVE OPTIONAL BRIDGE NO PAVEMENT AT SHOULDER REDECK EXISTING BRIDGE A30401 CONCRETE APPROACH STA. 514+74.00 TO STA. 518+49.17 PAVEMENT 0 SEE BRIDGE PLANS FOR DETAILS BRIDGE APPROACH SLAB-2 BRIDGE A30401 NB US-65 20 35 49 2 € US-65 N90°00'00"E Ś S NE S00°33'50"E SB US-65 0+50.00 3+30.00 7°17'31.9" (LT) 2°36'15.7" \mathbb{C} CROSSOVER 2 STA. 0+00.00 = © US-65 STA 518+50 00, 36' RT 280.00' (ARC) 140.19' 2,200.00 195' SAWCUT AT EXISTING EOP 9801 Renner Blvd. Ste. 30 REMOVE EXISTING SHOULDER Lenexa, KS 66219 913.492.0400 WIDENING AT EOP FOR TEMPORARY CROSSOVER GBAteam.com GEORGE BUTLER
ASSOCIATES, INC.
PRO. ENGINEER 000133
ARCHITECT 000212
PRO. LAND SURVEYOR 0000 EXISTING R/W-CROSSOVER 2 SAMUEL C. TAPKO PROFESSIONAL PLAN SHEET 4 OF 6 PE-2017015108

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THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY.

DATE PREPARED 9/29/2023 ROUTE 65 STATE MO SHEET NO KC

PETTIS J3P3203 CONTRACT ID

PROJECT NO.

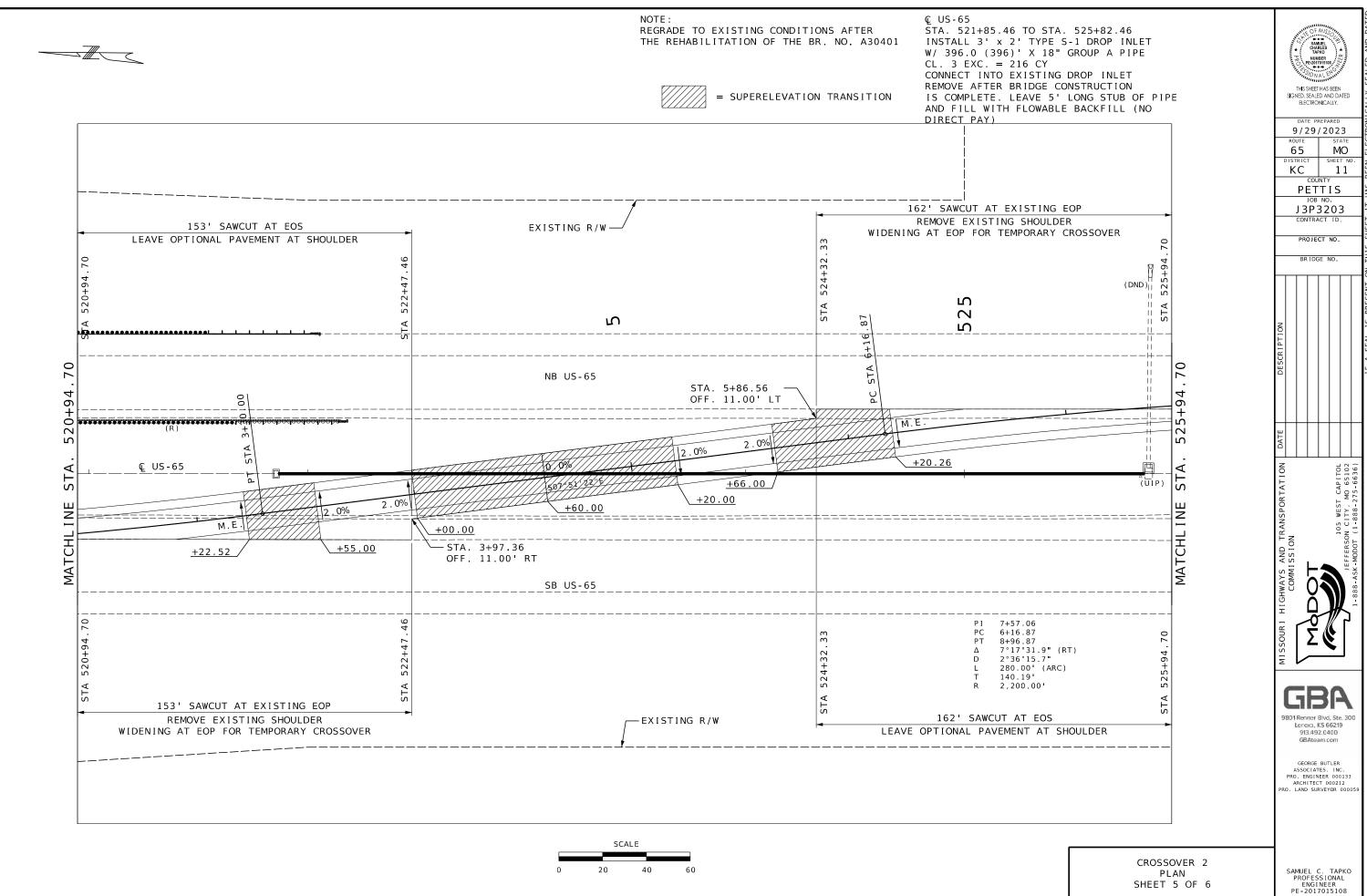
BRIDGE NO.

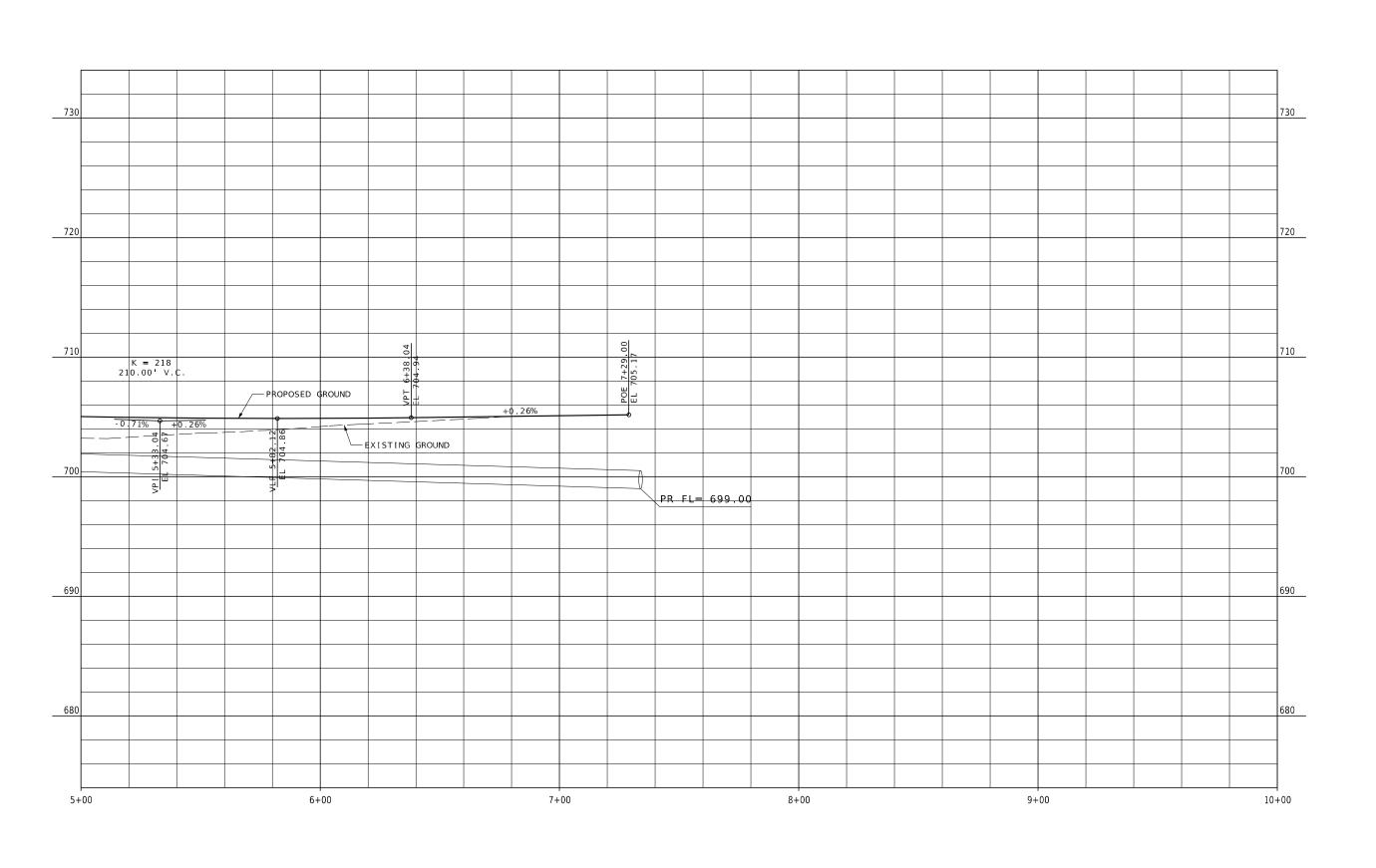
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ASSOCIATES, INC.
PRO. ENGINEER 000133
ARCHITECT 000212
PRO. LAND SURVEYOR 00005

SAMUEL C. TAPKO PROFESSIONAL ENGINEER PE-2017015108

CROSSOVER 2 PROFILE SHEET 3 OF 4





THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY.

DATE PREPARED

9/29/2023 ROUTE 65 STATE MO SHEET NO KC

PETTIS J3P3203 CONTRACT ID

PROJECT NO.

BRIDGE NO.

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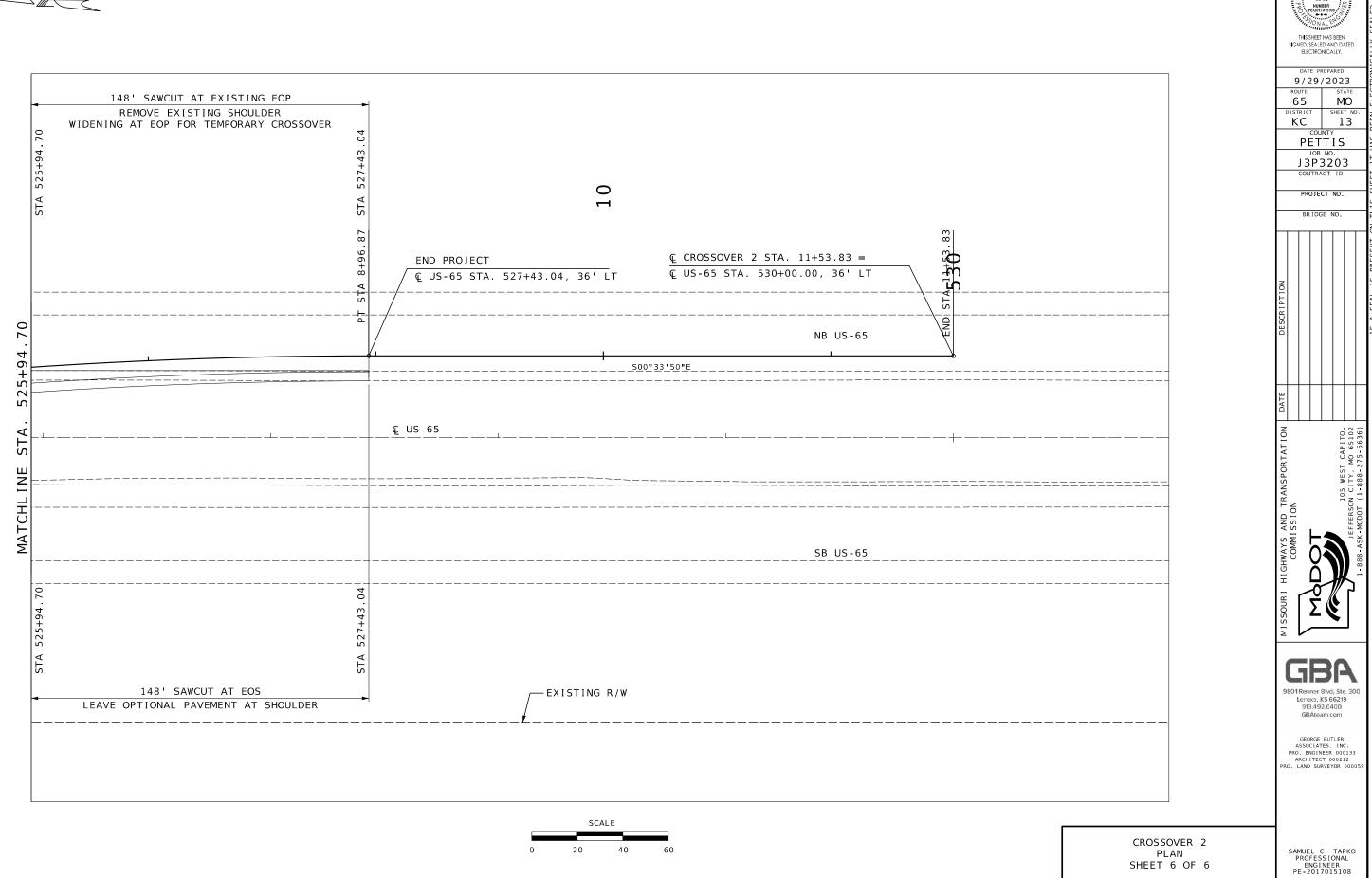
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ARCHITECT 000212
PRO. LAND SURVEYOR 00005

SAMUEL C. TAPKO PROFESSIONAL ENGINEER PE-2017015108

CROSSOVER 2 PROFILE SHEET 4 OF 4

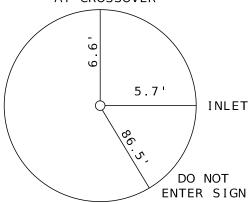
NOTE: REGRADE TO EXISTING CONDITIONS AFTER
THE REHABILITATION OF THE BR. NO. A30401



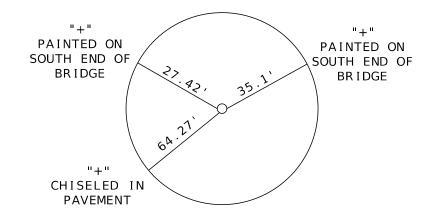


US 65 STA 493+38.80 CP#1 OFFSET 6.22'

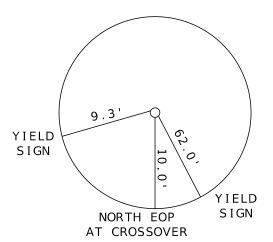
SOUTH EDGE OF PAVEMENT AT CROSSOVER



5/8" IRON BAR N: 1,084,427.721 E: 1,433,632.991 ELEV: 749.21' US 65 STA 518+59.92 CP#2 OFFSET 1.01'



5/8" IRON BAR N: 1,081,901.472 E: 1,433,663.070 ELEV: 706.07' US 65 STA 537+05.32 CP#3 OFFSET -1.09'



5/8" IRON BAR N: 1,080,056.186 E: 1,433,683.328 ELEV: 705.36'



THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY.

DATE PREPARED
9/29/2023
ROUTE STATE
65 MO
DISTRICT SHEET NO.
KC 14

PETTIS

JOB NO.

J3P3203

PROJECT NO.

BRIDGE NO.

ON DATE DESCRIPTION
OL 0.02
86)

MODOT

105 WEST CAPITO

105 WEST CAPITO

105 WEST CAPITO

106 WEST CAPITO

107 WEST CAPITO

108 WEST CAPITO

109 WEST CAPITO

100 WEST CAPITO

GBA
9801 Renner Blvd, Ste. 30

Lenexa, KS 66219
913.492.0400
GBAteam.com

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ASSOCIATES. INC.

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ASSOCIATES, INC.
PRO. ENGINEER 000133
ARCHITECT 000212
PRO. LAND SURVEYOR 0000

SAMUEL C. TAPKO PROFESSIONAL ENGINEER PE-2017015108

REFERENCE POINT SHEET 1 OF 1 ALL PROJECT COORDINATES HAVE BEEN PROJECTED FROM THE MISSOURI STATE PLANE COORDINATE (SPC) SYSTEM OF 1983 USING AN AVERAGE PROJECT PROJECTION (GRID TO GROUND) FACTOR. TO GET BACK TO STATE PLANE COORDINATES, MULTIPLY THE PROJECT COORDINATES BY THE AVERAGE GRID FACTOR AS SHOWN IN THE "REFERENCE CONTROL INFORMATION" PORTION OF THIS TABLE.

PROJECT COORDINATE INFORMATION									
COORDINATE SYSTEM	MODIFIED STATE PLANE (GROUND)								
HORIZONTAL DATUM	NAD83 (CORS96)								
VERTICAL DATUM	NAVD88								
GEOID MODEL	GEOID12								
ELEVATIONS	GPS OBSERVATION								
DETERMINED BY	GPS OBSERVATION								

PROJECT PROJECTION	1.0	0004680							
REFERENCE CONTROL INFORMATION									
COORDINATE SYSTE	м мо	MO COORDINATE SYSTEM OF 1983							
CONTROL STATION	МО	GEOGRAP	HIC	REFERENCE	SYSTEM				
DESIGNATION	PE-10								
CORS_ID	XX								
PID	15901	0							
LATITUDE	38°47'45.31704"								
LONGITUDE	46.3794	45"							
NORTHING (M)	32907	7.9420							
EASTING (M)	43083	3.0590							
ZONE	CENTR	AL							
PROJECT AVERAGE	GR I D	FACTOR	0.9	9995320					

\cap E	COORDINATE	$T \cap$	C	D	\sim

PROJECT NORTHING X AVERAGE GRID FACTOR = STATE PLANE NORTHING PROJECT EASTING X AVERAGE GRID FACTOR = STATE PLANE EASTING

EXAMPLE:	CONTROL POINT	#	
N	X	= N _	
E	Χ	= E	·

L	INEAR	U	NIT	CONVER	SIC	NC			
1	METER	=	3.28	0833333	US	SURVEY	FEET	(USFT)	

	COORDINATE POINT LISTING								
	MODIFIED STATE PLANE (GROUND)				GROUND)				
			OFFSET	NORTHING	EASTING	ELEVATION		GPK	
SHEET NO	STATION	LOCATION	(USFT)	(US SURVEY FT)	(US SURVEY FT)	(US SURVEY FT)	DESCRIPTION	POINT ID	
PROJECT CC	NTROL POINTS	5							
	493+38.80	EX US 65	6.22	1084427.72	1433632.99	749.21	CP#1 - 5/8 IRON BAR SET IN MEDIAN +/- 2140 FEET NORTH OF BRIDGES OVER MUDDY CREEK		
9	518+59.92	EX US 65	1.01	1081901.47	1433663.07	706.07	CP#2 - 5/8" IRON BAR SET IN MEDIAN AT SOUTH BRIDGE ENDS OF BRIDGES OVER MUDDY CREEK		
	537+05.32	EX US 65	-1 09	1080056.19	1433683.33	705.36	CP#3 - 5/8 IRON BAR SET IN MEDIAN +/ 1850 SOUTH OF BRIDGES OVER MUDDY CREEK		
			•		•				
				·			·		
		1			•				
					•				
·			•		•		·		
AL I GNMENTS	5								
	491+25.21	EX US 65		1084641.36	1433637.11		РОВ		
8	512+05.30	EX US 65		1082561.37	1433657.58		EQUATION BACK		
8	512+00.00	EX US 65		1082561.37	1433657.58		EQUATION AHEAD		
8	512+00.00	EX US 65		1082561.37	1433657.58		PI		
	550+00.00	EX US 65		1078761.55	1433694.98		POE		
4	0+00.00	TEMP CROSSOVER 1		1083566.98	1433683.69		РОВ		
4	0+50.00	TEMP CROSSOVER 1		1083516.98	1433684.18		P℃		
4	1+90.19	TEMP CROSSOVER 1		1083376.80	1433685.56		PI		
4	3+30.00	TEMP CROSSOVER 1		1083237.57	1433669.13		P·T		
6	6+16.87	TEMP CROSSOVER 1		1082952.68	1433635.52		P·C		
6	7+57.06	TEMP CROSSOVER 1		1082813.46	1433619.10		PI		
6	8+96.87	TEMP CROSSOVER 1		1082673.28	1433620.48		P.T		
8	10+03.83	TEMP CROSSOVER 1		1082566.32	1433621.53		POE		
9	0+00.00	TEMP CROSSOVER 2		1081911.05	1433627.98		РОВ		
9	0+50.00	TEMP CROSSOVER 2		1081861.05	1433628.47		PC		
9	1+90.19	TEMP CROSSOVER 2		1081720.87	1433629.85		PI		
1.1	3+30.00	TEMP CROSSOVER 2		1081581.99	1433649.01		P·T		
1.1	6+16.87	TEMP CROSSOVER 2		1081297.82	1433688.22		PC		
1.3	7+57.06	TEMP CROSSOVER 2		1081158.95	1433707.39		PI		
1.3	8+96.87	TEMP CROSSOVER 2		1081018.77	1433708.77		P·T		
1.3	11+53.83	TEMP CROSSOVER 2		1080761.81	1433711.29		POE		

THIS SHEET HAS BEEN
SIGNED, SEALED AND DATED
ELECTRONICALLY. 9/29/2023 ROUTE 65 ятате МО SHEET NO KC PETTIS J3P3203 CONTRACT ID PROJECT NO. BRIDGE NO.

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GEORGE BUTLER
ASSOCIATES, INC.
PRO. ENGINEER 000133
ARCHITECT 000212
PRO. LAND SURVEYOR 00001

SAMUEL C. TAPKO PROFESSIONAL ENGINEER PE-2017015108

SPACING AND TAPER LENGTHS FOR WORK ZONE SIGNS, CHANNELIZERS AND CONCRETE BARRIER

SIGN SPAC	ING FOR ADVANC	E SIGN SERIES (1) (2)				
PERMANENT	UNDIVIDED	DIVIDED				
POSTED SPEED	HIGHWAYS	HIGHWAYS				
MP-I	(S)	(S)				
0-35	200′	200′				
40-45	350′	500′				
50-55	500′	1000′				
60-70	10001	SA - 1000' SB - 1500' SC - 2640'				

TAPER LENGT	HS AND E	ND TREATE	EMENTS FO	R CONCRETE BARRIER
PERMANENT				
POSTED SPEED	MINIMUM L	ANE TAPER L	LENGTH (3)	
MPH	10'	11'	12′	END TREATMENT (4)
<40	160′	168′	176′	BARRIER HEIGHT TRANSITION
>40	160′	168′	176′	APPROVED CRASH CUSHION

TAPER LENGTHS AND SPACING FOR CHANNELIZERS									
PERMANENT	MINIMUM	LANE TAPE	R LENGTH	MINIMUM SHOULDER	BUFFER	MAXIMUM CHANNE	ELIZER SPACING		
POSTED SPEED		(∟)		TAPER LENGTH	LENGTH	THROUGH	THROUGH		
MP⊣	10′	11′	12′	BASED ON 10' SHOULDER	FT. (B)	TAPER	WORK AREA		
0-35	205′	225′	245′	70′	280'	35′	40'		
40-45	450'	495′	540′	150'	4001	40′	80'		
50-55	550′	605′	660′	185 '	560'	50′	80'		
60-70	700′	770′	840′	235 ′	840'	60′	120′		

NOTES:

- (1) SPACING BETWEEN SIGNS AND SPACING BETWEEN LAST SIGN AND FLAGGER, BEGINNING OF TAPER, OF SIGNED CONDITION.
- (2) SPACINGS MAY BE ADJUSTED AS NECESSARY TO MEET FIELD CONDITIONS.
- (3) TAPER LENGTHS SHOWN INCLUDE LENGTH REQUIRED FOR LANE AND 10' SHOULDER.
- (4) CONCRETE BARRIER MAY BE INSTALLED AT AND 8:1 FLARE RATE FROM THE SHOULDER POINT O THE LIMITS OF THE CLEAR ZONE WHERE THE SIDE SLOPE IS 6:1 OR FLATTER



MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

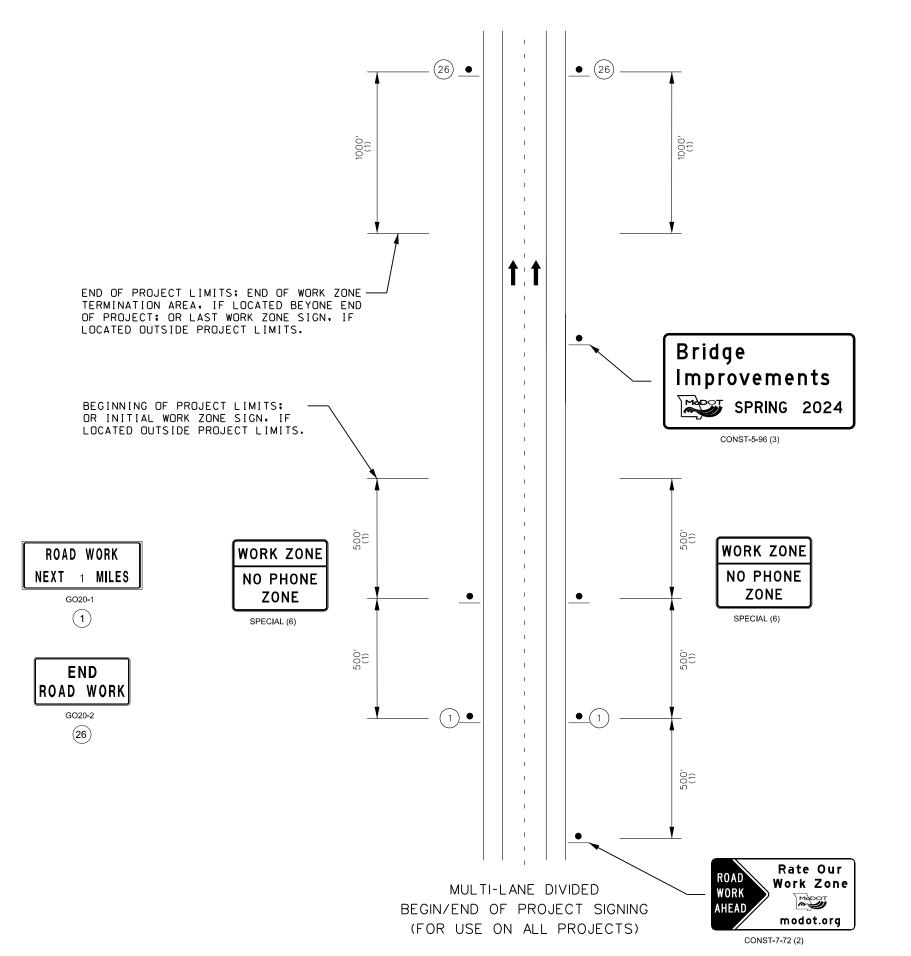
TOO DOT 106 WEST CAPITO JEFFERSON CITY, MO 65102

9801 Renner Blvd, Ste. 300 Lenexa, KS 66219 913.492.0400 GBAteam.com

GEORGE BUTLER ASSOCIATES, INC. PRO, ENGINEER 000133 ARCHITECT 000212 PRO, LAND SURVEYOR 00005

> JASON L. JARQUIO PROFESSIONAL ENGINEER PE-2021011939

TEMPORARY TRAFFIC CONTROL TYPICAL APPLICATION SHEET 1 OF 9



NOTES:

SIGN (1) IS REQUIRED PER EPG 616.6.56.

SIGN (26) IS USED ON ALL PROJECTS WHERE SIGN (1) IS USED.

OTHER SIGNS SUCH AS DETOUR OR ALTERNATE ROUTE SIGNING MAY BE USED OUTSIDE THE PROJECT LIMITS.

ANY EXISTING SIGNING THAT CONFLICTS WITH THE TRAFFIC CONTROL SIGNING SHALL BE COMPLETELY COVERED OR REMOVED.

- (1) DISTANCE MAY BE ADJUSTED ACCORDING TO FIELD CONDITIONS. WHERE TRAFFIC BACKUPS ARE EXPECTED BEYOND THE ADVANCE WARNING AREA. ADDITIONAL SIGNING MAY BE NEEDED.
- (2) SIGN CONST-7-72 OR CONST-7-48 IS PLACED 500 FEET BEFORE THE BEGINNING OF PROJECT LIMITS OR THE ROAD WORK AHEAD SIGN OR ROAD WORK NEXT XX MILES SIGN. IF USED. WHEN THESE SIGNS ARE LOCATED OUTSIDE THE PROJECT LIMITS.
- (3) SIGN CONST-5-96 IS PLACED IN A VISIBLE AREA WITHIN THE PROJECT LIMITS PROVIDED ITS PLACEMENT DOES NOT DISRUPT A SEQUENCE OF SIGNS. IF A VISIBLE LOCATION WITHIN THE PROJECT IS NOT AVAILABLE. THE SIGN MAY BE PLACED 500 FEET BEFORE SIGN CONST-7-72 OR CONST-7-48.
- (4) THE "RATE OUR WORK ZONE" SIGN IS AVAILABLE IN TWO SIZES DEPENDING ON ROADWAY CONDITIONS. THE 48 IN. X 24 IN. SIGN IS USED ON LOW VOLUME ROUTES THAT CARRY 400 AADT OR LESS. ON PROJECTS THAT HAVE A DURATION OF 4 WEEKS OR LESS OR IN URBAN AREAS WHERE THERE IS INSUFFICIENT RIGHT OF WAY FOR A SIX FOOT WIDE SIGN. THE 72 IN. X 36 IN. VERSION OF THE SIGN IS USED IN ALL OTHER APPLICATIONS. BUT CAN ALSO BE USED IN PLACE OF THE 48 IN. X 24 IN. SIGN IF ADDED EMPHASIS IS DEEMED NECESSARY.
- (5) THE "POINT OF PRESENCE" SIGN IS AVAILABLE IN TWO SIZES DEPENDING ON ROADWAY CONDITIONS. THE 48 IN. X 36 IN. SIGN IS USED ON LOW VOLUME ROUTES THAT CARRY 400 AADT OR LESS. ON PROJECTS THAT HAVE A DURATION OF 4 WEEKS OR LESS OR IN URBAN AREAS WHERE THERE IS INSUFFICIENT RIGHT OF WAY FOR A EIGHT FOOT WIDE SIGN. THE 96 IN. X 48 IN. VERSION OF THE SIGN IS USED IN ALL OTHER APPLICATIONS. BUT CAN ALSO BE USED IN PLACE OF THE 48 IN. X 36 IN. SIGN IF ADDED EMPHASIS IS DEEMED NECESSARY.
- (6) THE "WORK ZONE NO PHONE ZONE" SIGN IS PLACED 500 FEET BEFORE THE ROAD WORK AHEAD SIGN.



SIGNED, SEALED, AND DATED ELECTRONICALLY.

10/2/2023

ROUTE STATE
65 MO

KC 17

PETTIS
JOB NO.
J3P3203

CONTRACT ID.

PROJECT NO.

BRIDGE NO.

SSOURI HIGHWAYS AND TRANSPORTATIO
COMMISSION
COMMISSION
105 WEST CAF

9801 Renner Blvd, Ste. 30 Lenexa, KS 66219 913.492.0400

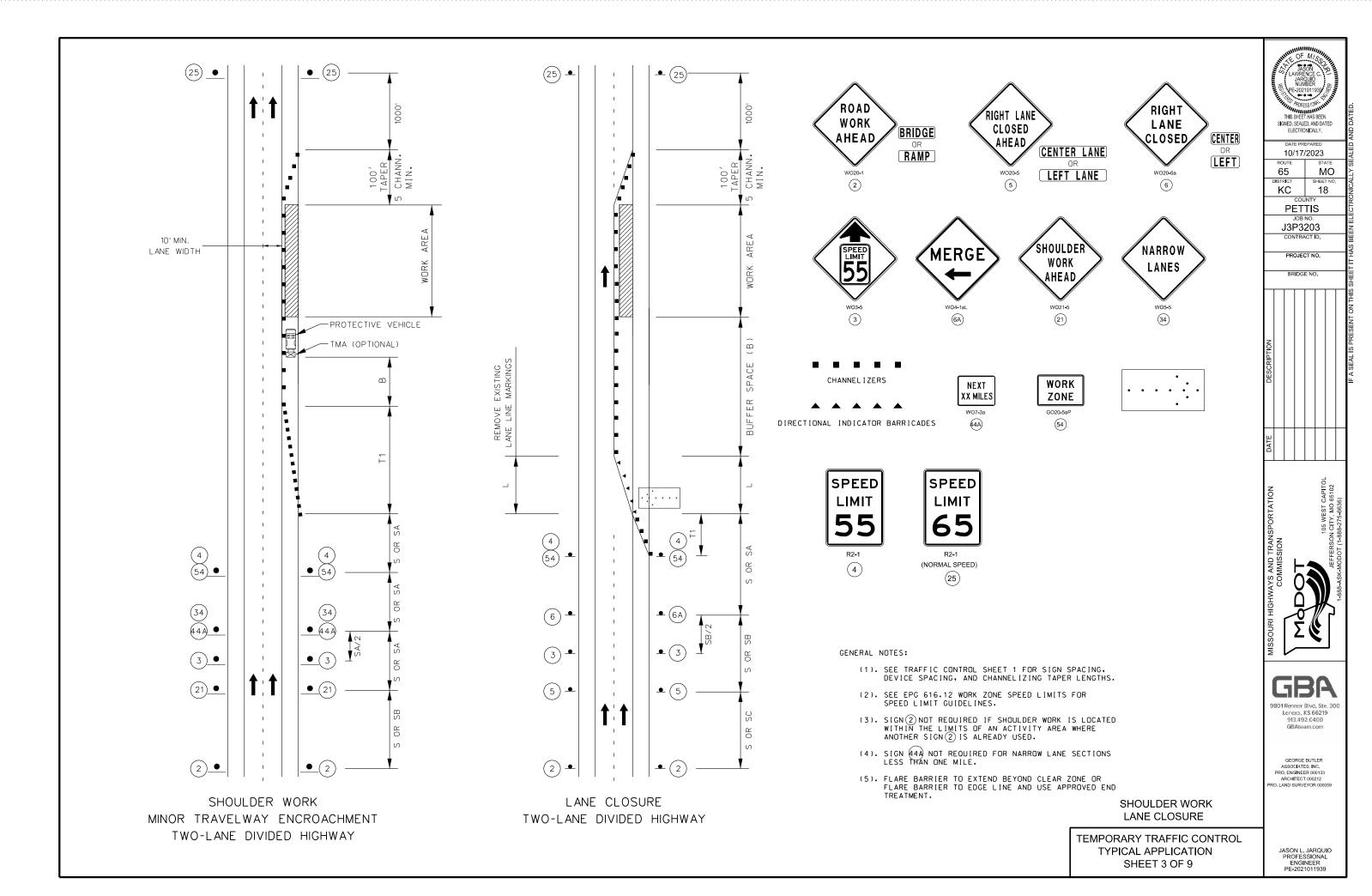
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ASSOCIATES, INC.

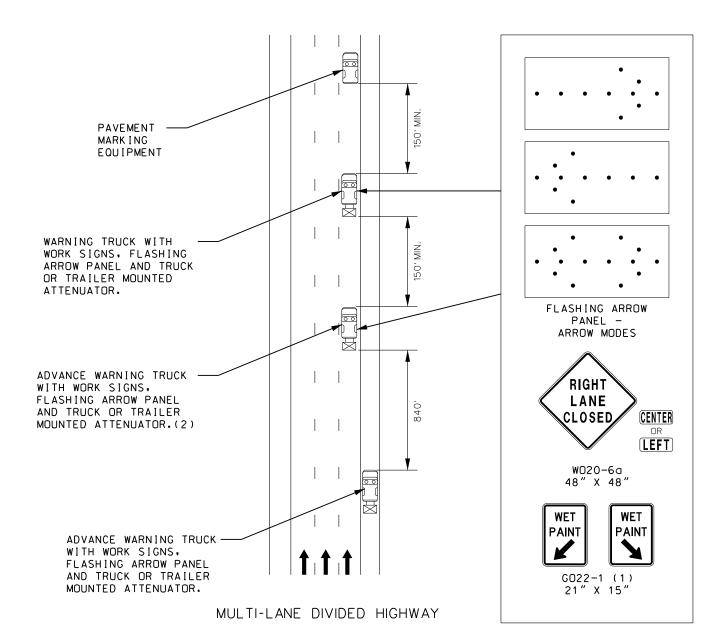
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ASSOCIATES, INC.
PRO, ENGINEER 000133
ARCHITECT 000212
PRO, LAND SURVEYOR 0000

TEMPORARY TRAFFIC CONTROL TYPICAL APPLICATION SHEET 2 OF 9

BEGIN/END OF PROJECT





MOBILE OPERATION ON HIGHWAY OPEN TO TRAFFIC

NOTES:

UPON ARRIVAL OF THE ENGINEER. THE CONTRACTOR MAY PROVIDE ADDITIONAL PROTECTIVE VEHICLES EQUIPPED WITH PROPER WARNING DEVICES.

PROTECTIVE TRUCK AND WORK VEHICLES SHALL DISPLAY HIGH-INTENSITY ROTATING, FLASHING, OSCILLATING, OR STROBE LIGHTS.

VEHICLE HAZARD WARNING SIGNALS SHALL NOT BE USED INSTEAD OF THE VEHICLE'S HGIH-INTENSITY ROTATING FLASHING, OSCILLATING, OR STROBE LIGHTS.

- (1). WET PAINT SIGNS ARE INSTALLED TO INDICATE THE SIDE IN WHICH THE PAVEMENT MARKING MATERIAL IS BEING APPLIED. AT THE CONTRACTOR'S OPTION. A FRONT FACING WET PAINT SIGN MAY BE INSTALLED ON THE LEFT SIDE OF THE PAVEMENT MARKING EQUIPMENT.
- (2). ADVANCE WARNING TRUCK IS POSITIONED AT THE NO TRACK POINT OF THE PAVEMENT MARKING MATERIAL OR SPACING SHOWN, WHICH EVER IS GREATER.

FLASHING ARROW PANELS SHALL. AS A MINIMUM, BE TYPE B. WITH A SIZE 60 X 30 INCHES.

VEHICLE-MOUNTED SIGNS SHALL BE MOUNTED IN A MANNER SUCH THAT THEY ARE NOT OBSCURED BY EQUIPMENT OR SUPPLIES. SIGN LEGENDS ON VEHICLE-MOUNTED SIGNS SHALL BE COVERED OR TURNED FROM VIEW WHEN WORK IS NOT IN PROGRESS.

A FLASHING ARROW BOARD SHALL BE USED WHEN A FREEWAY LANE IS CLOSED. WHEN MORE THAN ONE LANE IS CLOSED. A SEPARATE FLASHING ARROW BOARD SHALL BE USED FOR EACH CLOSED LANE.

SIGNED, SEALED, AND DATED ELECTRONICALLY. DATE PREPARED 10/2/2023 65 MO KC 19 **PETTIS** J3P3203 CONTRACT ID. PROJECT NO. BRIDGE NO.

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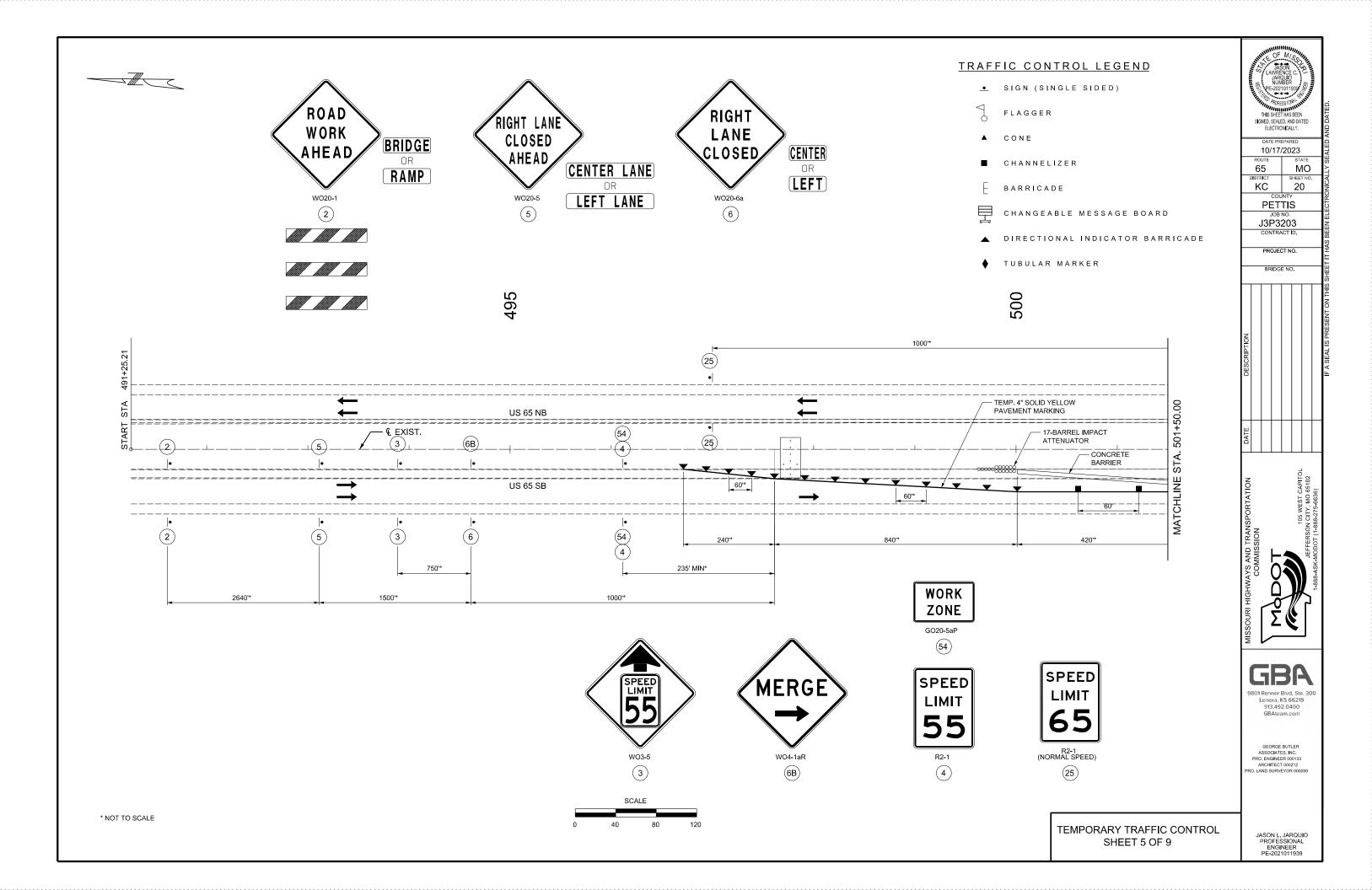
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MOBILE OPERATION OPEN TO TRAFFIC

TEMPORARY TRAFFIC CONTROL TYPICAL APPLICATION SHEET 4 OF 9





TRAFFIC CONTROL LEGEND

SIGN (SINGLE SIDED)

FLAGGER

▲ CONE

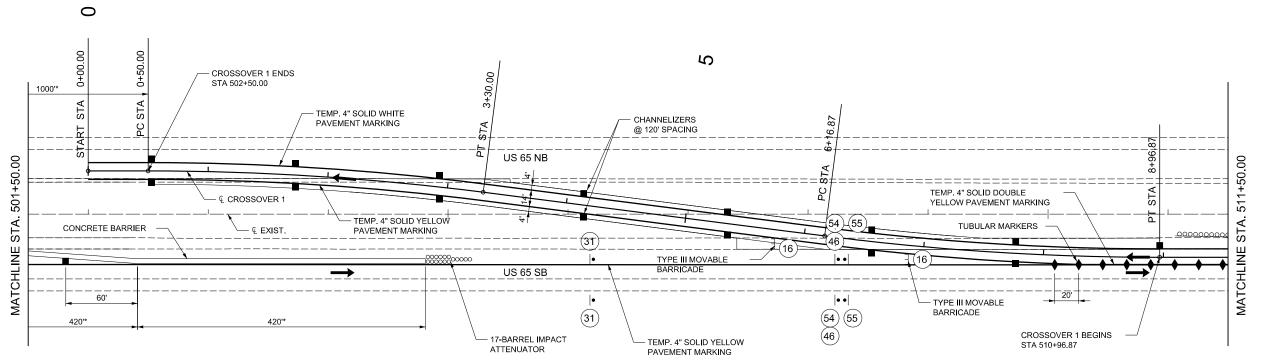
CHANNELIZER

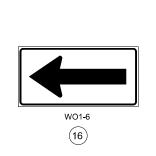
BARRICADE

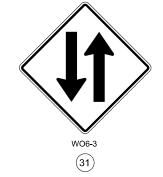
CHANGEABLE MESSAGE BOARD

▲ DIRECTIONAL INDICATOR BARRICADE

TUBULAR MARKER







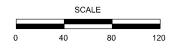




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| DATE PREPARED | 10/2/2023 | | ROUTE | STATE | 65 | MO | DISTRICT | SHEET NO. | KC | 21 |

PETTIS
JOB NO.
J3P3203

PROJECT NO.

BRIDGE NO.

DATE DESCRIPTION

MISSOURI HIGHWAYS AND I KANSPORTATIO
COMMISSION

MODOT

106 WEST CAF

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ASSOCIATES, INC.
PRO, ENGINEER 000133
ARCHITECT 000212
PRO, LAND SURVEYOR 00005











DO NOT PASS

46

GO20-5aP

WORK

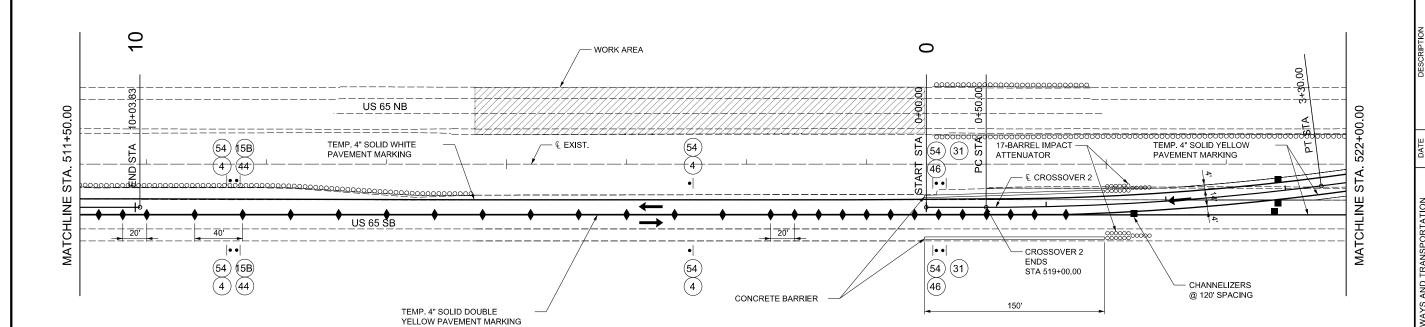
ZONE

TRAFFIC CONTROL LEGEND

- SIGN (SINGLE SIDED)
- FLAGGER
- ▲ CONE
- CHANNELIZER
- BARRICADE



- ▲ DIRECTIONAL INDICATOR BARRICADE
- ♦ TUBULAR MARKER



ASENCE C.

ANGUERO C.

NUMBER

PE-2021011939

THIS SHEET HAS BEEN
SINNED, SEALED, AND DATED
ELECTRONACALY.

DATE PREPARED

10/2/2023

ROUTE STATE
65 MO

DISTRICT SHEET NO.
KC 22

COUNTY

PETTIS

JOB NO.

J3P3203

CONTRACT ID.

PROJECT NO.

BRIDGE NO.

COMMISSION

COMMISSION

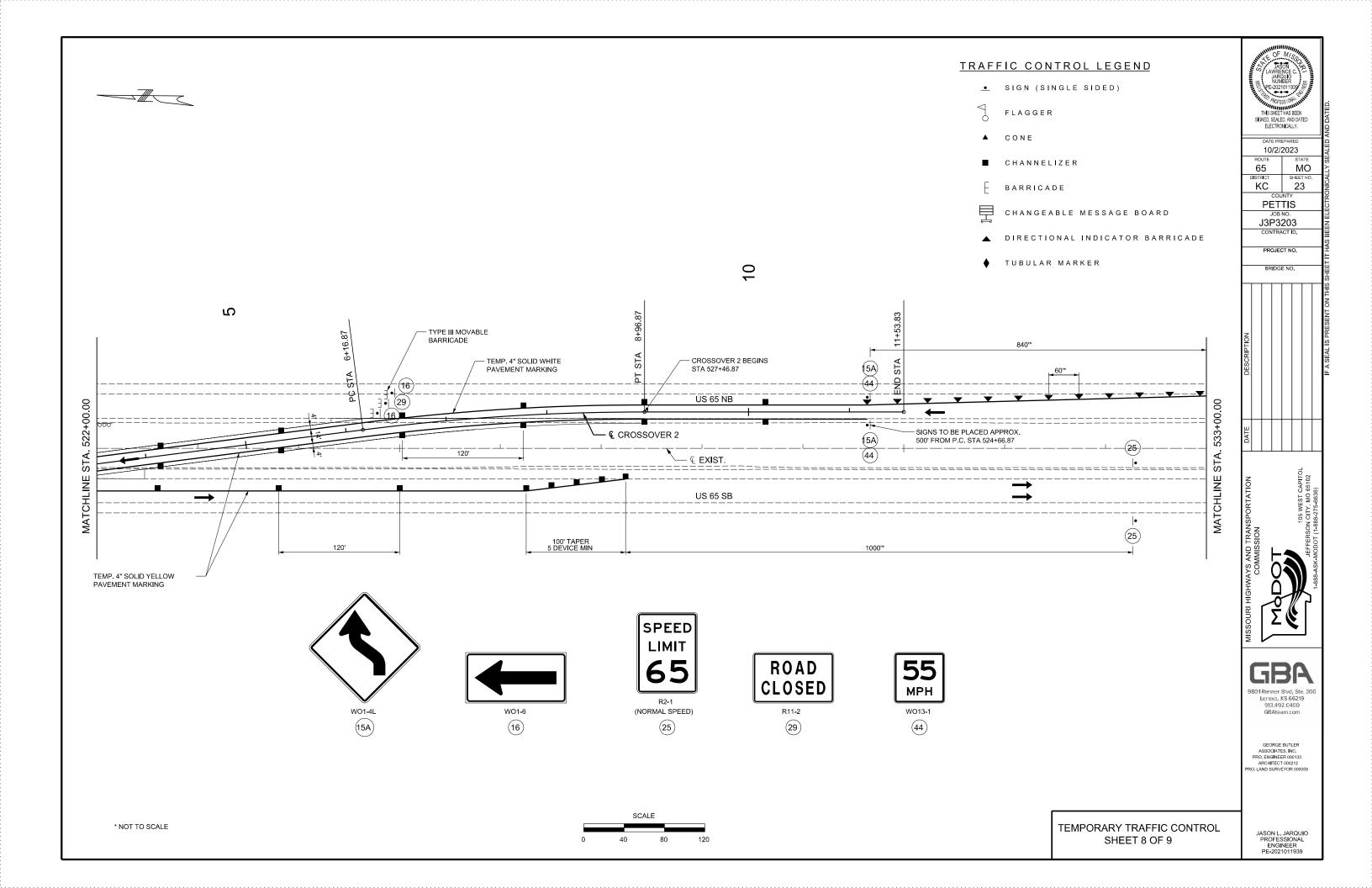
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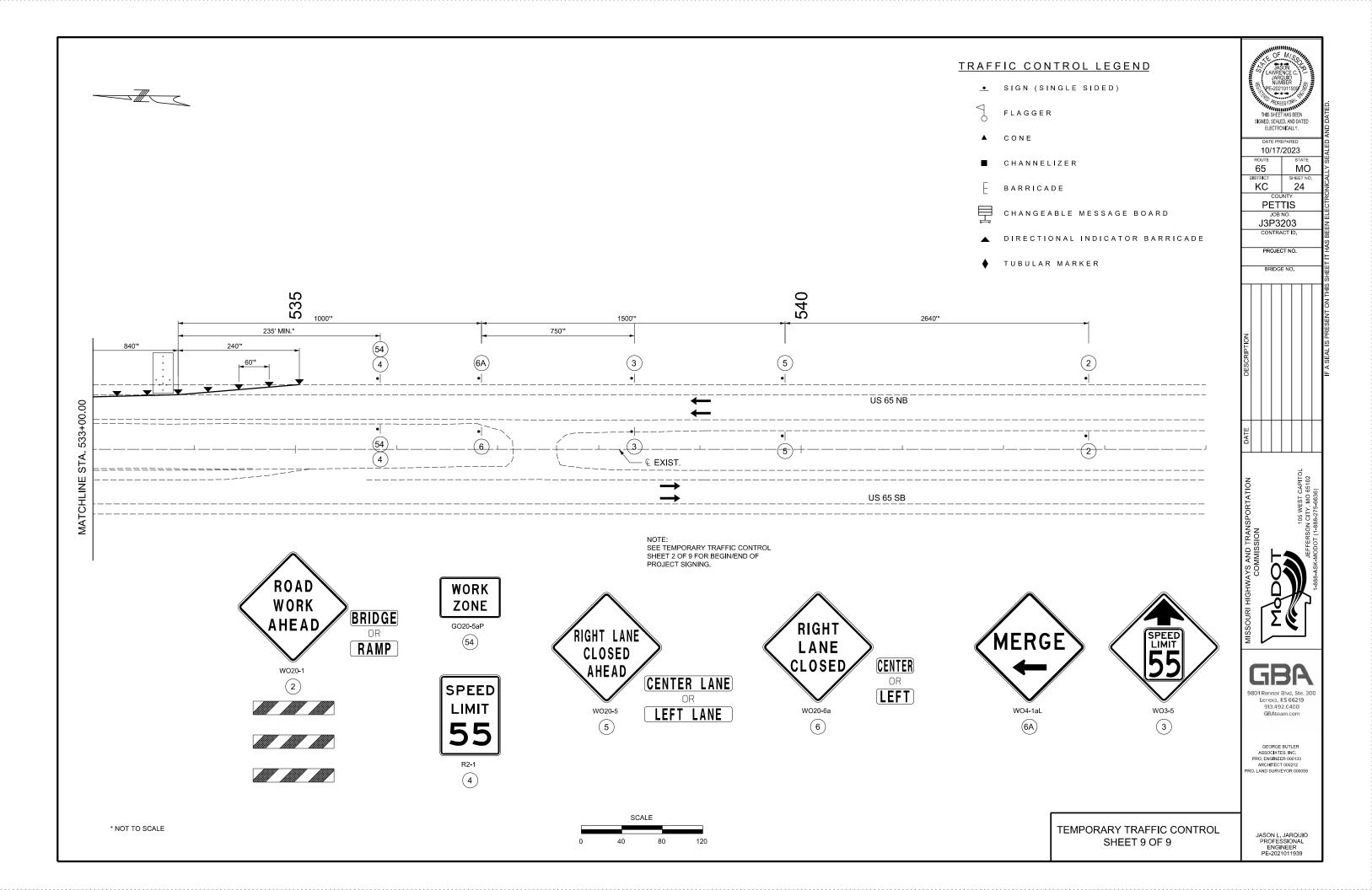
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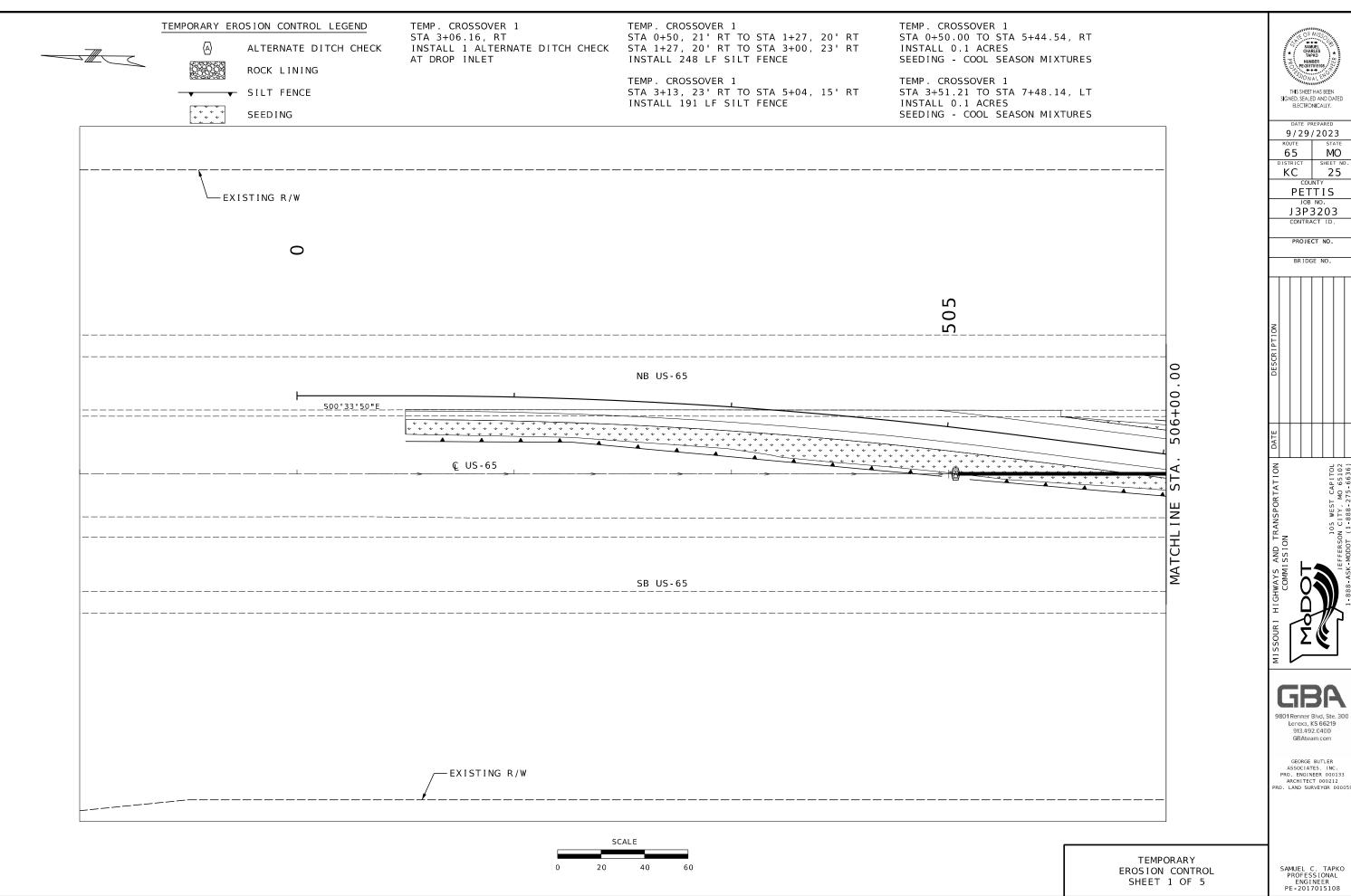
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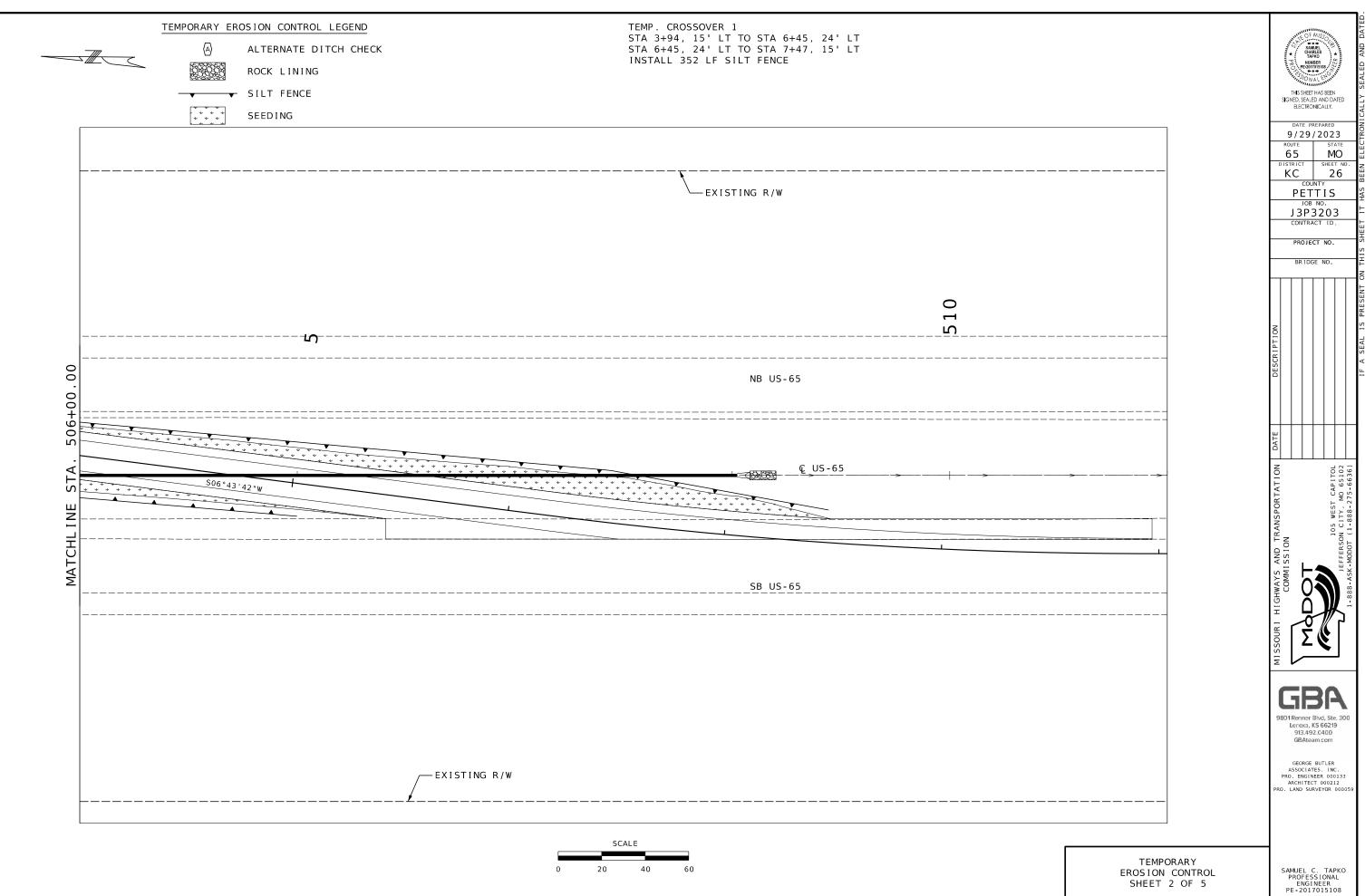
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ASSOCIATES, INC.
PRO, ENGINEER 000133
ARCHITECT 000212
PRO, LAND SURVEYOR 000058

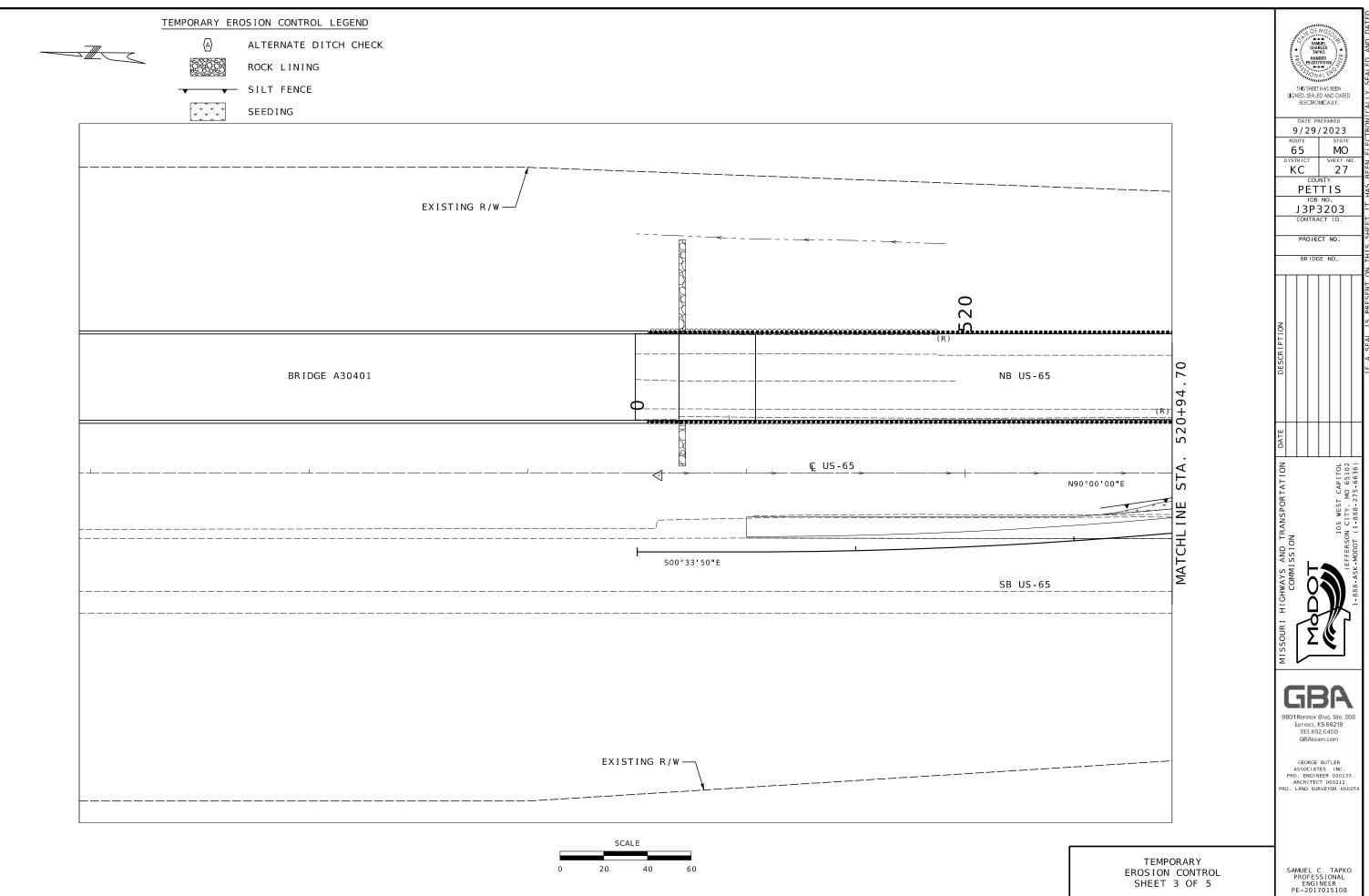
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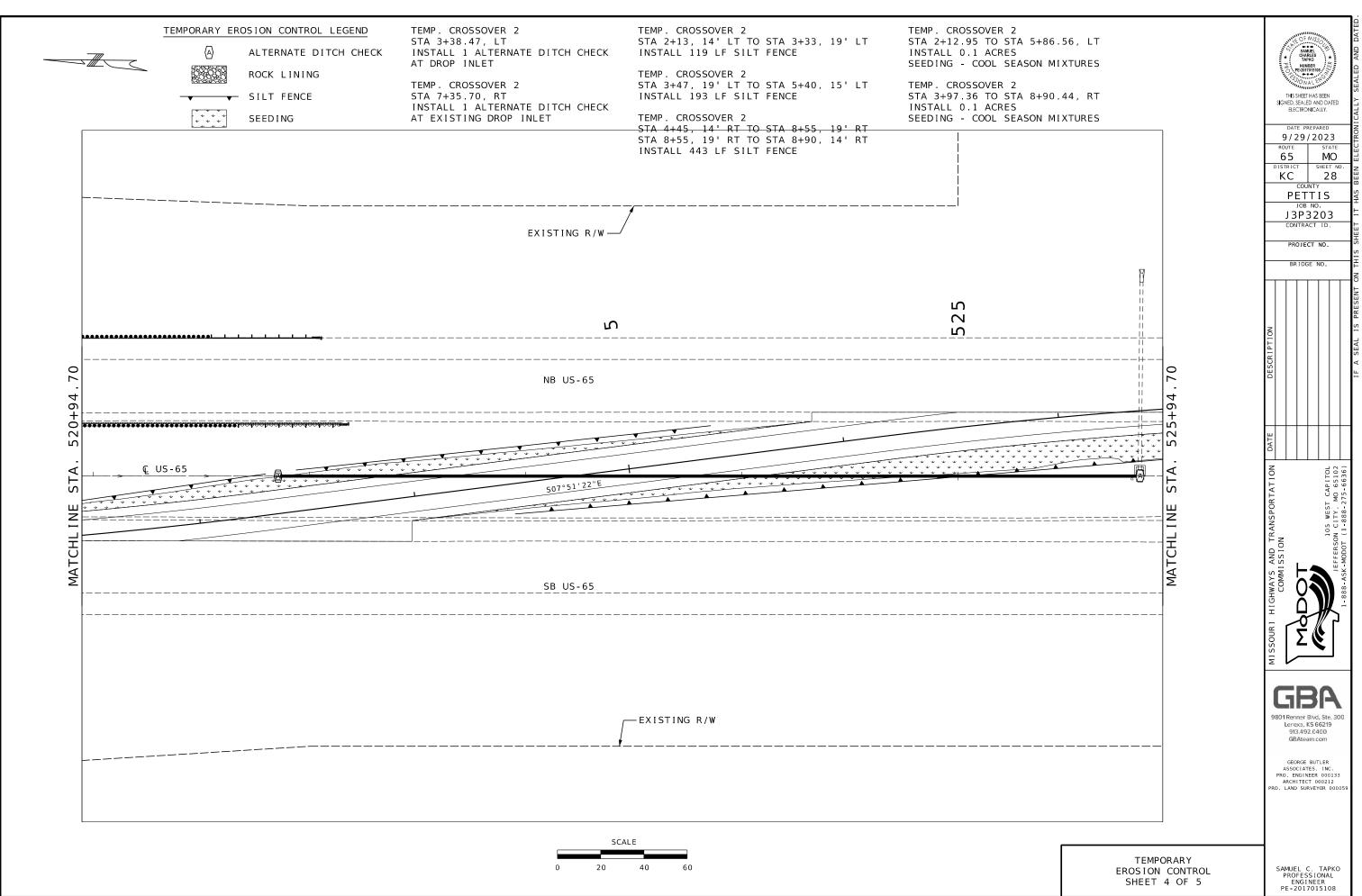


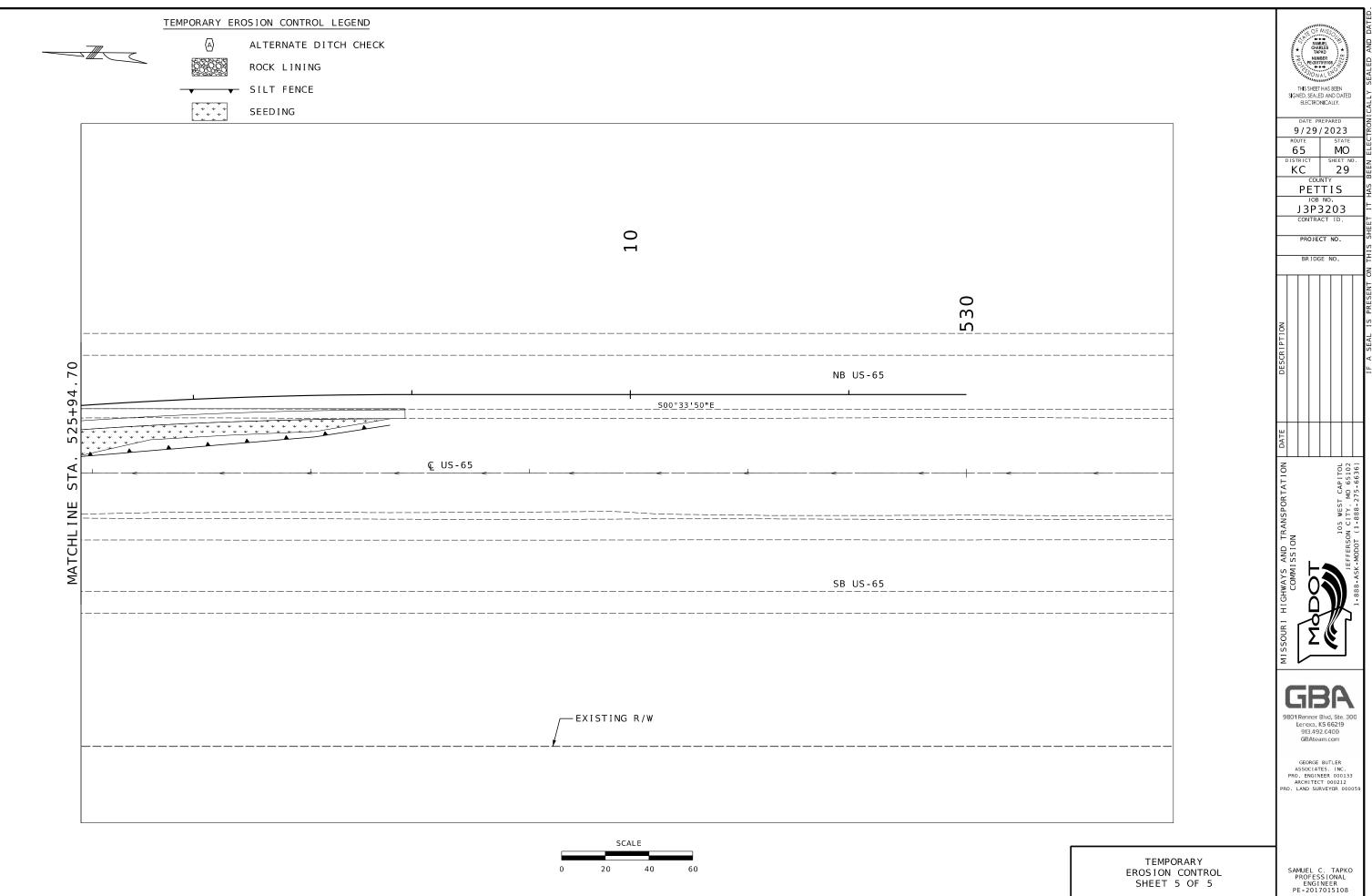




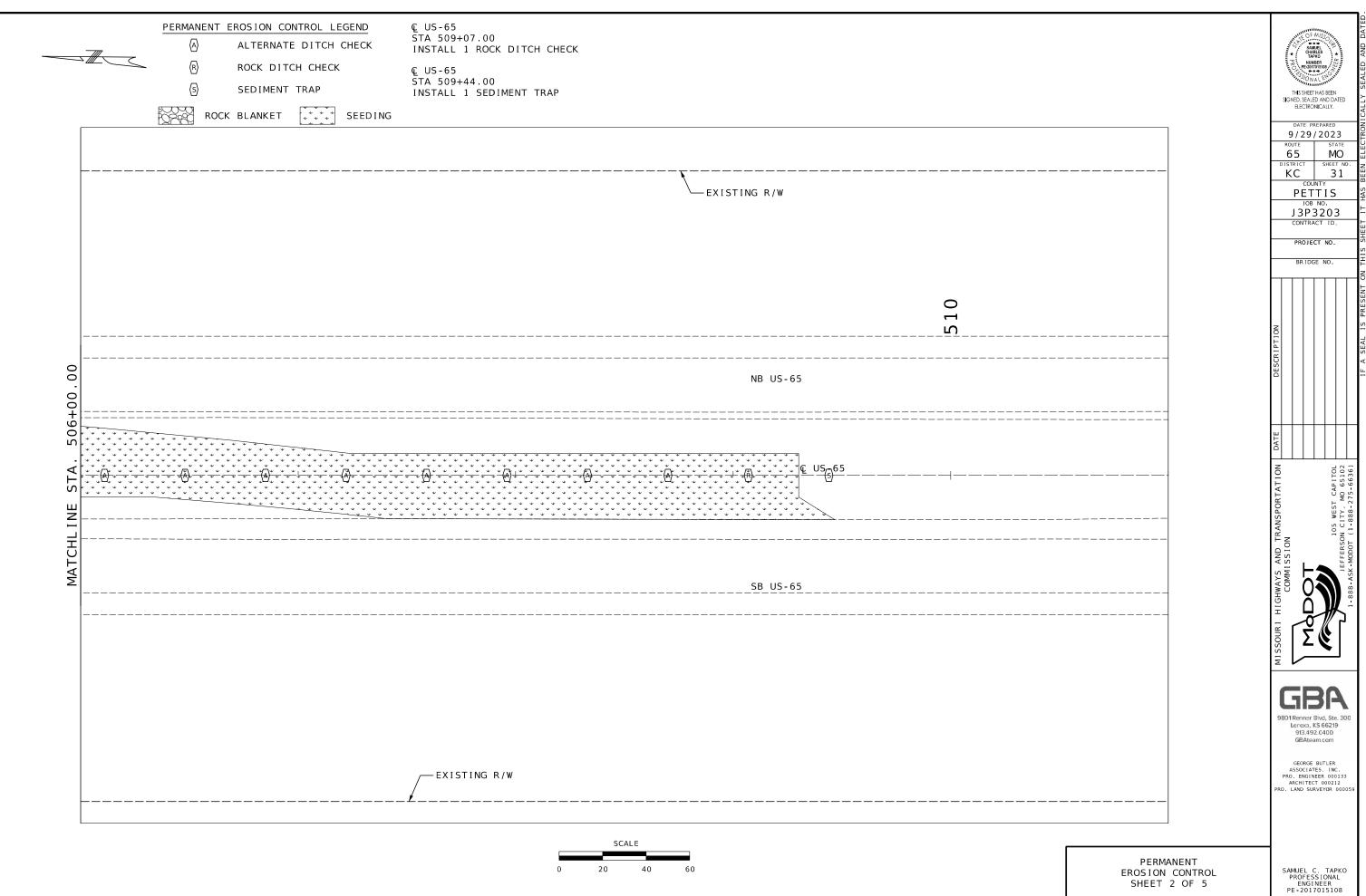


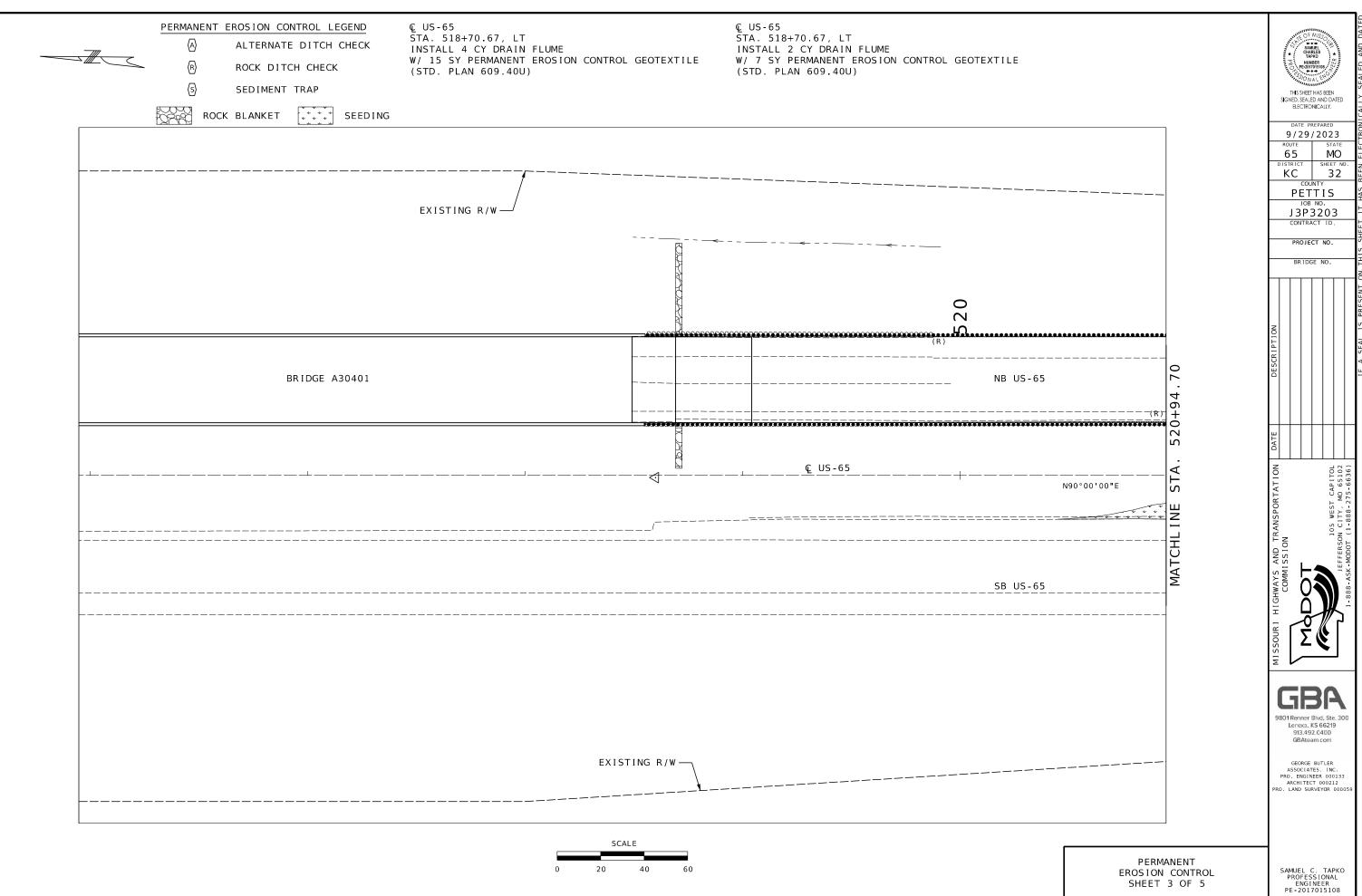


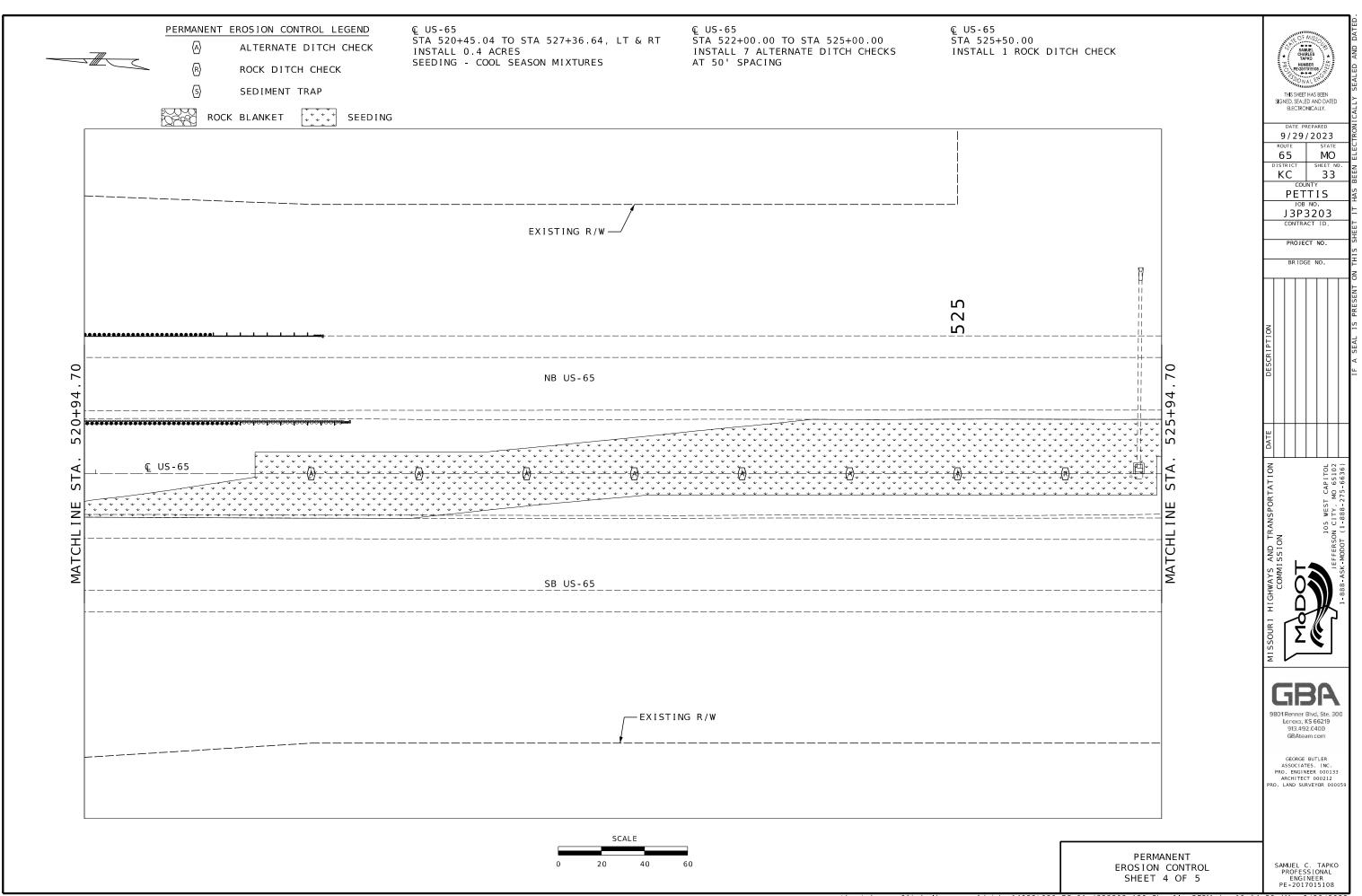


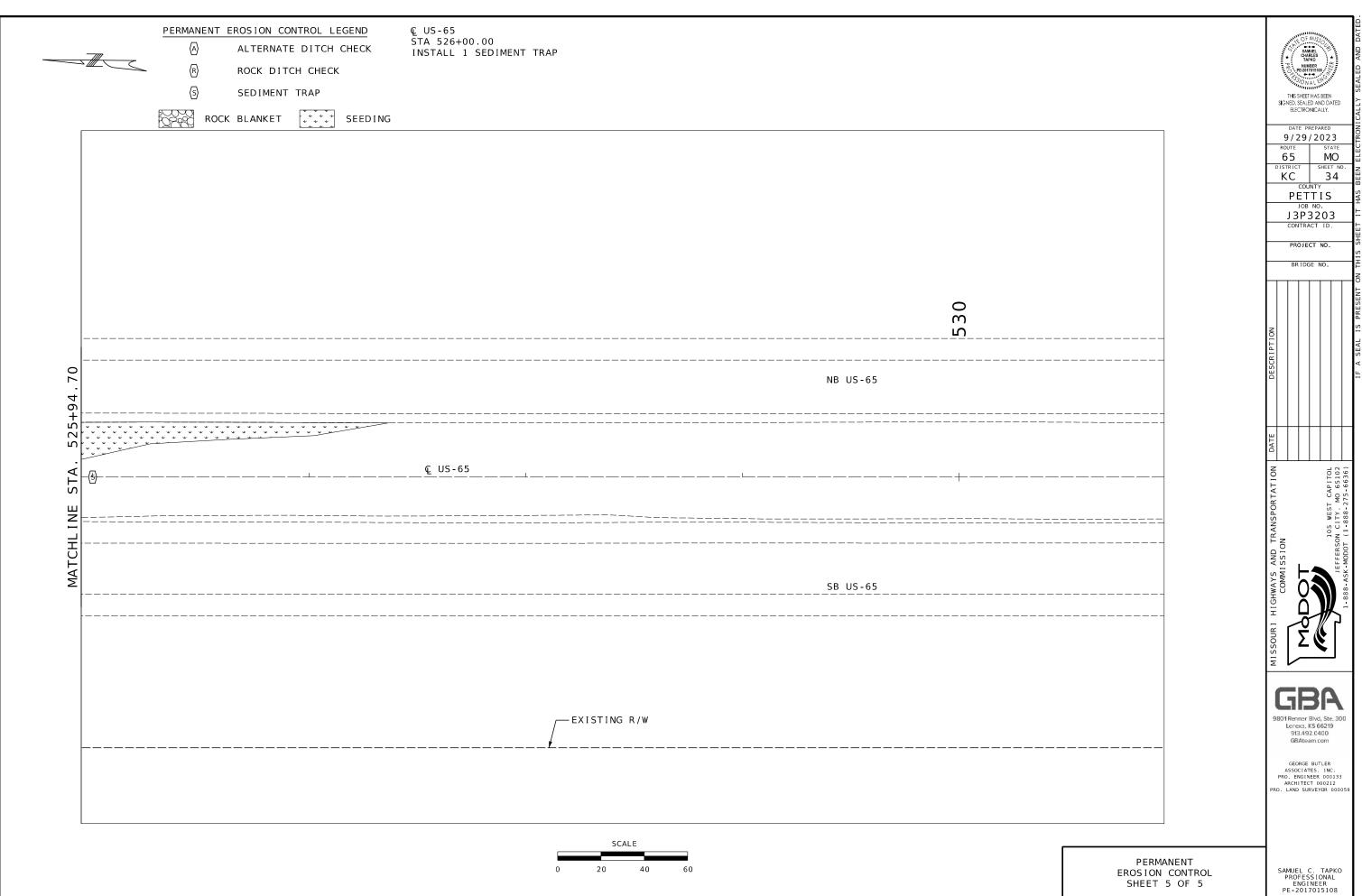


© US-65 STA 505+00.00 TO STA 508+70.00 PERMANENT EROSION CONTROL LEGEND © US-65 STA 502+50.00 TO STA 509+46.70, LT & RT INSTALL 0.4 ACRES ALTERNATE DITCH CHECK INSTALL 11 ALTERNATE DITCH CHECKS AT 37' SPACING SEEDING - COOL SEASON MIXTURES $\langle R \rangle$ ROCK DITCH CHECK $\langle s \rangle$ SEDIMENT TRAP THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY. ROCK BLANKET SEEDING 9/29/2023 65 ятате МО SHEET NO KC PETTIS -EXISTING R/W J3P3203 PROJECT NO. BRIDGE NO. 2 0 2 506+00.00 NB US-65 ₢ US-65 MATCHL 1 SB US-65 Lenexa, KS 66219 913.492.0400 GBAteam.com GEORGE BUTLER ASSOCIATES, INC. PRO. ENGINEER 000133 ARCHITECT 000212 PRO. LAND SURVEYOR 0000 — EXISTING R/W PERMANENT SAMUEL C. TAPKO PROFESSIONAL ENGINEER PE-2017015108 EROSION CONTROL SHEET 1 OF 5









↑												SAMUEL CHARGE CH
												THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY.
												DATE PREPARED 9/29/2023 ROUTE STATE 65 MO DISTRICT SHEET NO.
730 — EXISTING GROUND PROPOSED GROUND												COUNTY PETTIS JOB NO. J3P3203 CONTRACT ID.
720 710				////////		/////////		////////	//////	MATCHL INE	_	PROJECT NO. BRIDGE NO.
700 Q. US-65 STA. 505+03.11 INSTALL PRECAST CONCRETE DROP INLET TYPE S-1 (3FT. X 2FT.) D = 4.4 (5)' CL. 3 EXC. = 3 CY	© US-65 STA. 505+03.11 TO 398.00 (398)' X I W/ 18" GROUP A FE CL. 3 EXC. = 124 ROCK LINING = 2	STA. 509+02.1: 8" GROUP A PIPE SS CY	l E @ 1.20%								NOIL	
	REMOVE AFTER BRID	GE CONSTRUCTION	N IS COMPLE	TE.							DESCRIP	
730											DATE	
720 H											ORTATION	ST CAP1
710	FL OUT 710.29										AND TRANSP	105 WE 0N CITY (1-888
700					EXIS	TIN <mark>G GROUN</mark> E	D				HIGHWAYS	COMPANY TO SERVICE TO
											MISSOURI	Σ
												GBA 9801 Renner Blvd, Ste. 300 Lerexa, KS 66219 913.492.0400
									- CLASS 3	EXCAVAT		913.492.0400 GBAteam.com GEORGE BUTLER ASSOCIATES, INC. PRO, ENGINEER 000133 ARCHITECT 000212 RO, LAND SURVEYOR 000059
								NOTE: (#) = PAY I	.ENGTH/DE	PTH	.O. LAND SURVEYOR 000059
								CULVERT SHEET	SECTIC 1 OF 2	NS		SAMUEL C. TAPKO PROFESSIONAL ENGINEER PE-2017015108

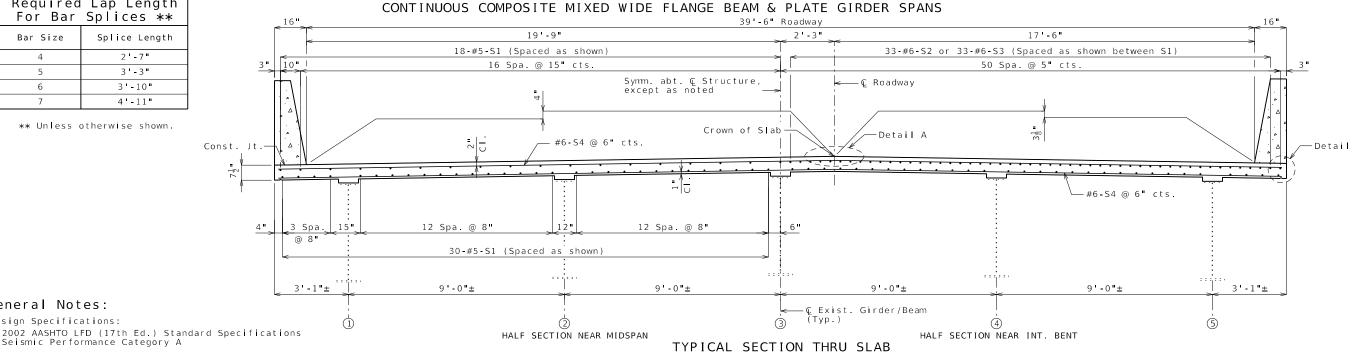
720 —EXISTING GROUND PROPOSED GROUND—							SIGNI 9 ROU 6 DISTR KU	STATE OF THE PROPERTY OF THE P
700 FL IN 701.38	TO STA. 525+82.	46 PE @ 0.60%				MATCHLINI		BRIDGE NO.
TYPE S-1 (3FT. X 2FT.) D = 3.9 (4)' CL. 3 EXC. = 3 CY CONNECT INTO E. CL. 3 EXC. = 2	(ISTING DROP INL	ET					DESCRIPTI	IF A SFA
720 710 E PROPOSED GROUND							NNSPORTATION DATE	5 WEST CAPITOL CITY, MO 65102 -888-275-6636)
700 FL OUT 699.4 FLOUT 699.4 REMOVE AFTER BRIDGE CONSTRUCTI LEAVE 5' STUB OF PIPE AND FILL BACKFILL (NO DIRECT PAY).	00/	EXISTING					RI HIGHWAYS AND TRA	JODOT 105 JEFFERSON C 1-888-ASK-MODOT (1-,
							9801 F	iBA Renner Blvd, Ste. 300 enexa, KS 66219
						EXCAVAT (ON AS	GEORGE BUTLER SSOCIATES, INC. ENGINEER 000133 RCHITECT 000212 AND SURVEYOR 000059
				CULVERT SHEET	SECTIC 2 OF 2	NS	SAM PI PE	NUEL C. TAPKO ROFESSIONAL ENGINEER -2017015108

SEC/SUR 33 TWP 47N RGE 21W

Required Lap Length For Bar Splices ** Bar Size Splice Length 2'-7" 5 3'-3" 3'-10" 4 - 11 "

** Unless otherwise shown.

U.I.P., REDECK & RECONFIGURE EXISTING SUPERSTRUCTURE TO (31'- 35'- 74'- 92'- 74'- 35'- 31')



9 - 0 "

Seismic Performance Category A Design Loading:

General Notes:

Design Specifications:

HS20-44 (1973) (Existing) HS20-44 (New Construction) 35 lb/sf Future Wearing Surface Earth - 120 lb/cf, Equivalent Fluid Pressure 45 lb/cf Fatigue Stress - Case III

Design Unit Stresses:

Class B-1 Concrete (Barrier) f'c = 4,000 psiClass B-2 Concrete (End Bents & Superstructure, except Barrier)
Reinforcing Steel (Grade 60) f'c = 4,000 psifv = 60.000 psiStructural Steel (ASTM A709 Grade 50) fy = 50,000 psi

All joint filler shall be in accordance with Sec 1057 for preformed sponge rubber expansion and partition joint filler, except as noted.

Minimum clearance to reinforcing steel shall be 1 1/2", unless

Fabricated Steel Connections:

Field connections shall be made with 3/4-inch diameter ASTM F3125 Grade A325 Type 1 bolts and 13\16-inch diameter holes, except as noted.

High Strength bolts, nuts and washers will be sampled for quality assurance as specified in Sec 106.

Protective coating for concrete bents and piers (Epoxy) shall be applied as shown on the bridge plans and in accordance with Sec 711.

Bars bonded in existing concrete not removed shall be cleanly stripped and embedded into new concrete where possible. If length is available, existing bars shall extend into new concrete at least 40 diameters for plain bars and 30 diameters for deformed bars, unless otherwise noted

Roadway surfacing adjacent to bridge ends shall match new bridge slab súrface. (Roadway item)

Outline of existing work is indicated by light dashed lines. Heavy

Contractor shall verify all dimensions in field before ordering

The existing bridge rails shall be stored at a location as designated by the engineer on the MoDOT Maintenance Lot at $9101\ E\ 40th\ Ter.$, Kánsas City, MO 64133

Field welded fillet welds shall be NDT by the magnetic particle process as required by AASHTO/AWS D 1.5 2002, Bridge Welding Code clause 6.7.2.

Structure to be closed during construction. Traffic to be maintained on other routes during construction. See roadway plans for traffic control.

Structural Steel Protective Coating:

Protective Coating: System G in accordance with Sec 1081.

Surface Preparation: Surface preparation of the existing steel shall be in accordance with Sec 1081 for Recoating of Structural Steel. The cost of surface preparation will be considered completely covered by the contract unit price per sq. foot for Surface Preparation for Recoating Structural Steel (System G).

Prime Coat (New Steel): The cost of the prime coat will be considered completely covered by the contract unit price for the Fabricated Structural Low Alloy Steel (Plate Girder) A709 Grade 50. Tint of the prime coat for System G shall be similar to the color of the field coat to be used.

Prime Coat (Recoating): The cost of the prime coat will be considered completely covered by the contract unit price per sq. foot for Field Application of Inorganic Zinc Primer.

Field Coat: The color of the finish field coat shall be Gray (Federal Standard #26373). The cost of the intermediate field coat will be considered completely covered by the contract unit or the finish field coat will be considered completely covered by the contact unit price per sq. foot for Intermediate Field Coat (System G). The cost of the finish field coat will be considered completely covered by the contact unit price per sq. foot for Finish Field Coat (System

At the option of the contractor, the intermediate field coat and finish field coat may be applied in the shop for the replacement section of the girder. The contractor shall exercise extreme care during all phases of loading, hauling, handling, erection and pouring of the slab to minimize damage and shall be fully reposited for all repairs and cleaning of the coating systems as responsible for all repairs and cleaning of the coating systems as required by the engineer.

Coating Limits: All new modification steel near Int. Bents No. 3 & 6 shall be coated in accordance with Sec 1081. The surface of all existing steel shall be recoated within a distance not less than 10 feet from beam ends at End Bents No. 1 & 8. Complete System G includes field application of inorganic zinc primer, intermediate field coat and finish field coat. Items to be coated shall be beams, stiffeners, bearings and miscellaneous structural steel items.

Limits of paint overlap: System G shall overlap the existing coating between 6 inches and 12 inches in order to achieve maximum coverage at the paint limit of each complete system near the end bents. The final field coating shall be masked to provide crisp, straight lines and to prevent overspray beyond the overlap required

At the contractors option, System I may be used in place of System G in accordance with Sec 1081. This two coat system consists of inorganic zinc primer and finish field coat. Only one paint system shall be used for the entirety of this bridge.

S3 * Bar continues entire length of span

Span 3 | Span 4 | Span 4 | Span 5 | Span 5 | Span 6 | Span 6 | Span 7

13'-0"

*

S2

9 0

Table Showing S2 & S3 Bar Lengths

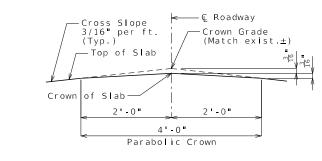
Int. Bent No. 2 Int. Bent No. 3 Int. Bent No. 4 Int. Bent No. 5 Int. Bent No. 6 Int. Bent No. 7

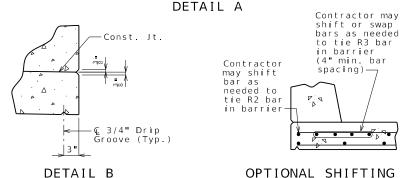
25'-0" | **23'-0"** | **23'-0"** | **25'-0"** |

Span 2 Span 2 Span 3

52

13 0





OPTIONAL SHIFTING TOP BARS AT BARRIER

REPAIRS TO BRIDGE: ROUTE 65 NB OVER MUDDY CREEK

ROUTE 65 FROM ROUTE HH TO ROUTE J ABOUT 3.9 MILES SOUTH OF ROUTE J BEGINNING STATION 514+74.00± (MATCH EXISTING)



NUMBER

SIGNED, SEALED, AND DATED ELECTRONICALLY

DATE PREPARED 10/2/2023

PETTIS J3P3203

CONTRACT ID PROJECT NO

BRIDGE NO A30401

MO

65

BR

9801 Renner Blvd., Ste. 30 Lenexa KS 66219 913.492.0400 gbateam.com GEORGE BUTLER

ASSOCIATES, INC.
PRO. ENGINEER 000133
ARCHITECT 000212
D. LAND SURVEYOR 0000

RYAN G. HAGERTY PROFESIONAL PE-2016034015

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Detailed Sept 2023 Checked Sept 2023

Note: This drawing is not to scale. Follow dimensions

Sheet No. 1 of 18

I t em		Total
Removal of Miscellaneous ACM (Non-Friable)	sq. foot	35
Class 1 Excavation	cu. yard	25
Removal and Storage of Existing Bridge Rail	linear foot	750
Removal of Existing Bridge Deck	sq. foot	15,851
Removal of Existing Bearings	each	30
Bridge Approach Slab (Major)	sq. yard	180
Slab on Steel	sq. yard	1,758
Type D Barrier	linear foot	774
Fiber Reinforced Polymer Wrap	sq. foot	304
Protective Coating – Concrete Bents and Pier (Epoxy)	lump sum	1
Fabricated Structure Low Alloy (Plate Girder) A709, Grade 50	pound	28,180
Slab Drain	each	64
Surface Preparation for Recoating Structural Steel	sq. foot	1,100
Field Application of Inorganic Zinc Primer	sq. foot	1,100
Intermediate Field Coat (System G)	sq. foot	3,300
Finish Field Coat (System G)	sq. foot	3,300
Non-Destructive Testing	linear foot	41
Reconfigure Existing Structural Steel	lump sum	1
Vertical Drain at End Bents	each	2
Laminated Neoprene Bearing Pad Assembly	each	10
Type N PTFE Bearing	each	20
Strip Seal Expansion Joint System	linear foot	79

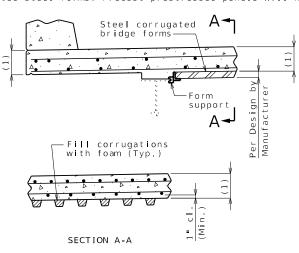
of all concrete and reinforcement in the end bent backwall and wings shall be considered completely covered by the contract unit price for Slab on

Estimated Quantities	for Slab on St	ee I
I t em		Total
Class B-2 Concrete	cu. yard	378
Reinforcing Steel (Epoxy Coated)	pound	154,570

The table of Estimated Quantities for Slab on Steel represents the quantities used by the State in preparing the cost estimate for concrete slabs. The area of the concrete slab will be measured to the nearest square yard longitudinally from end of slab to end of slab and transversely from out to out of bridge slab (or with the horizontal dimensions as shown on the plan of slab). Payment for stay-in-place corrugated steel forms, conventional forms, all concrete and epoxy coated reinforcing steel will be considered completely covered by the contract unit price for the slab. Variations may be encountered in the estimated quantities but the variations cannot be used for an adjustment in the contract unit price

Method of forming the slab shall be in accordance with Sec 703. All hardware for forming the slab to be left in place as a permanent part of the structure shall be coated in accordance with ASTM A123 or ASTM B633 with a thickness Class SC 4 and a finish Type I, II or III.

Slab shall be cast-in-place with conventional forming or stay-in-place corrugated steel forms. Precast prestressed panels will not be permitted.



OPTIONAL STAY-IN-PLACE FORM DETAILS

General Notes:

Stay-In-Place Forms:

Corrugated steel forms, supports, closure elements and accessories shall be in accordance with grade requirement and coating designation G165 of ASTM A653. Complete shop drawings of the permanent steel deck forms shall be required in accordance with Sec 1080.

Corrugations of stay-in-place forms shall be filled with an expanded polystyrene material. The polystyrene material shall be placed in the forms with an adhesive in accordance with the manufacturer's recommendations.

Form sheets shall not rest directly on the top of girder/beam flanges. Sheets shall be securely fastened to form supports with a minimum bearing length of one inch on each end. Form supports shall be placed in direct contact with the flange. Welding on or drilling holes in the girder/beam flanges will not be permitted. All steel fabrication and construction shall be in accordance with Sec 1080 and 712. Certified field welders will not be required for welding of the form supports.

The design of stay-in-place corrugated steel forms is per manufacturer which shall be in accordance with Sec 703 for false work and forms. Maximum actual weight of corrugated teel forms allowed shall be 4 psf assumed for girder/beam Loading.

The contractor shall provide a method of preventing the direct contact of the stay-in-place forms and connection components with uncoated weathering steel members that is approved by the engineer.

Pouring and Finishing Slab:

The contractor shall provide bracing necessary for lateral and torsional stability of the girders/beams during construction of the concrete slab and remove the bracing after the slab has attained 75% design strength. Contractor shall not weld on or drill holes in the girders/beams. The cost for furnishing, installing, and removing bracing will be considered completely covered by the contract unit price for Slab on Steel

The contractor shall pour slab up grade from end to end at a minimum rate of 25 cubic yards per hour.

Alternate pour sequences may be submitted to the engineer for approval. Keyed construction joints shall be provided between pours.

Haunching

19

19

4 Equal Spaces

31'-0"

– Ç Brg. Stiff. —

SPAN (1-2)(7-8)

(1) Slab is to be considered a uniform thickness as shown on the plans. Haunching will vary. See front sheet for slab thickness.

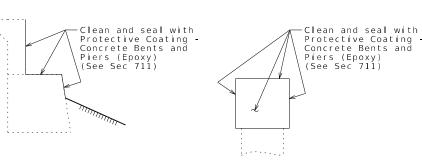
In order to properly form the haunches for the new deck, the contractor shall shoot deck elevations along each girder line prior to deck removal followed by shooting elevations of the tops of the girders after deck removal.

16

6" to 12" 10' recoating area from beam ends @ End Bents No. 1 & 8 System G Surface Preparation Inorganic Zinc Primer Limit Existing Coating Limit Intermediate Field Coat Limit and Final Field Coat Limit Mechanical cleaning in accordance with Sec 1081.10.5.4.2.1

PART ELEVATION SHOWING LIMITS OF PAINT OVERLAP

(Vertical or horizontal paint limit. Horizontal limit shown)



TYPICAL SECTION THRU END BENTS NO. 1 & 8 SHOWING PROTECTIVE COATING

Symm. abt. C

3 =

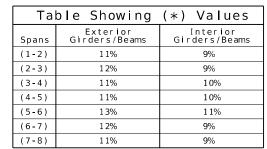
 $\frac{7}{16}$

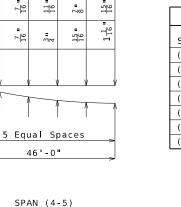
46'-0"

TYPICAL SECTION THRU INT. BENTS NO. 3 & 6 SHOWING PROTECTIVE COATING

The protective coating shall not be applied beneath the FRP System at Int. Bent No. 6.

For location of FRP wrap, see Sheet No. 6.







DEAD LOAD DEFLECTION

SPAN (2-3)(6-7) SPAN (3-4)(5-6)

(*) % of dead load deflection is due to the weight of structural steel.

4 Equal Spaces

35'-0"

Dead load deflection includes weight of structural steel, concrete slab, and barrier.

16 =

1₆ "

16

4 Equal Spaces

74'-0"

-Ç Brg. Stiff. →

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NUMBER

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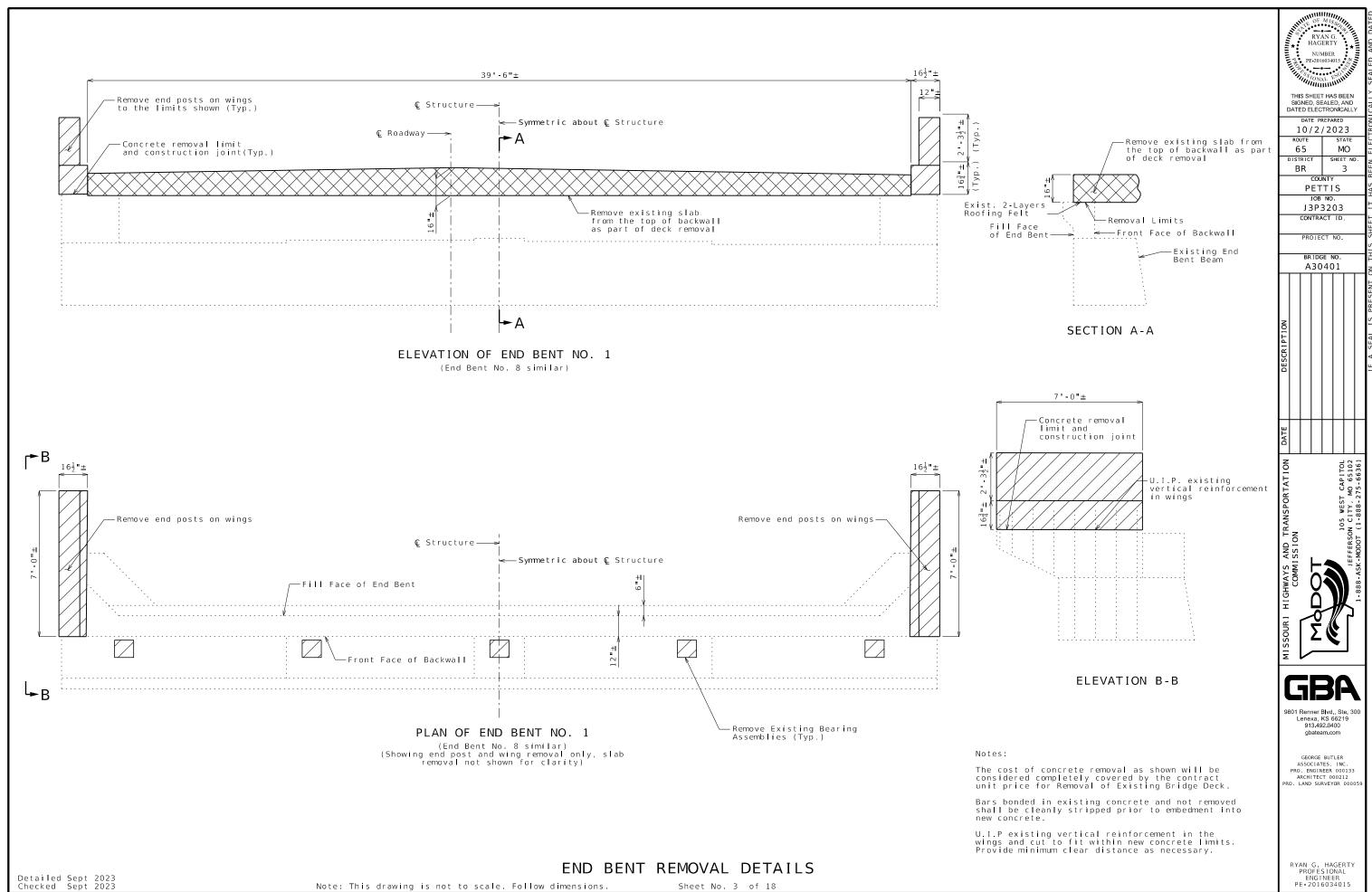
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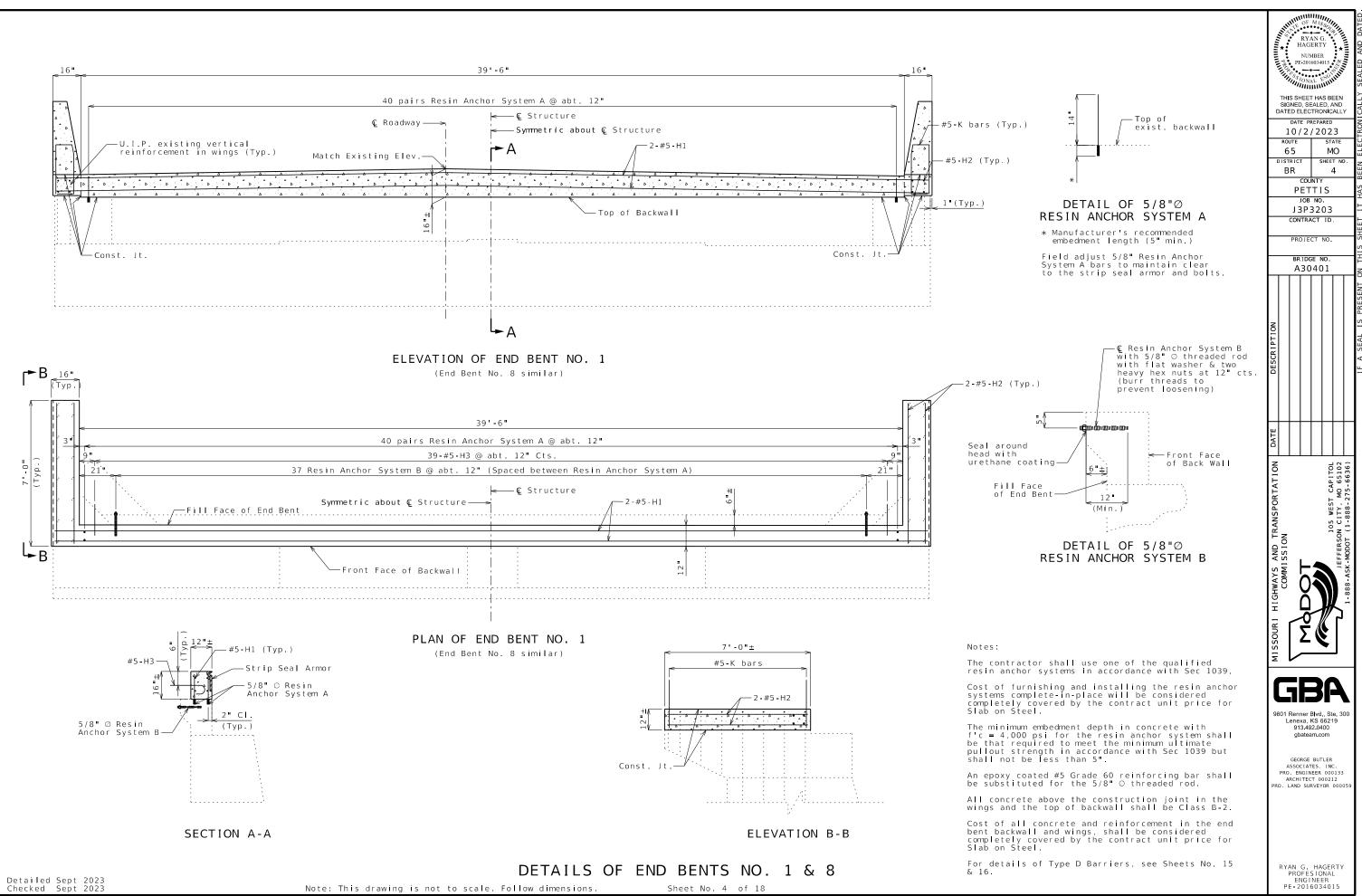
Exterior Girders/Beams

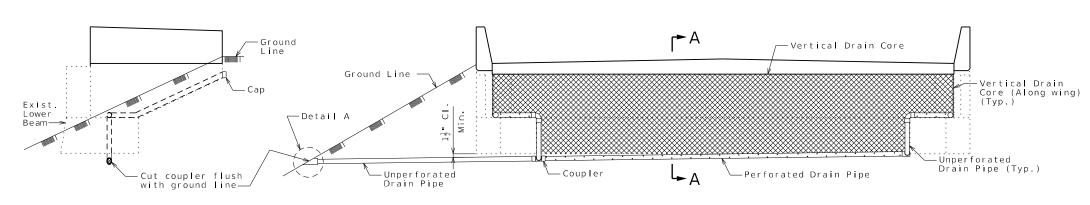
Interior Girders/Beams

Chord Between Bents

Bottom of Top Flange







ELEVATION OF WING

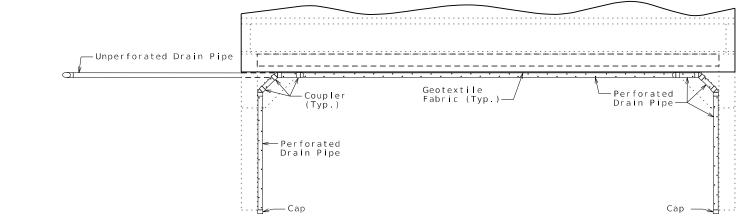
Unperforated Drain Pipe—

DETAIL A

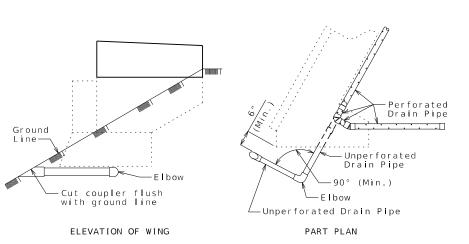
to slope of ground line

Rodent Screen

ELEVATION OF END BENT

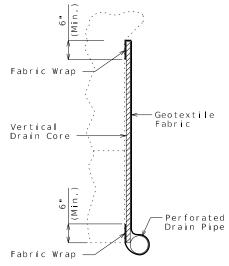


PLAN OF END BENT



OPTIONAL TURNED DRAIN

(Use only when straight drain is not practical.)



PART SECTION A-A (Section thru wing similar)

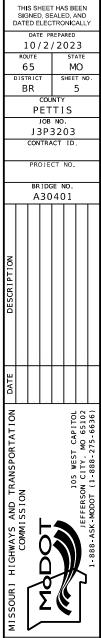
General Notes:

All drain pipe shall be sloped 1 to 2

Drain pipe may be either 6-inch diameter corrugated metallic-coated steel pipe underdrain, 4-inch diameter corrugated polyvinyl chloride (PVC) drain pipe, or 4-inch diameter corrugated polyethylene (PE) drain pipe.

Drain pipe shall be placed at fill face of end bent and inside face of wings. The pipe shall slope to lowest grade of ground line, also missing the lower beam of end bent by a minimum of 1 1/2 inches.

Perforated pipe shall be placed at fill face side and inside face of wings at the bottom of end bent and plain pipe shall be used where the vertical drain ends to the exit at ground line.



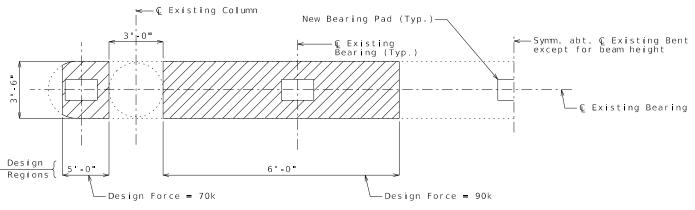
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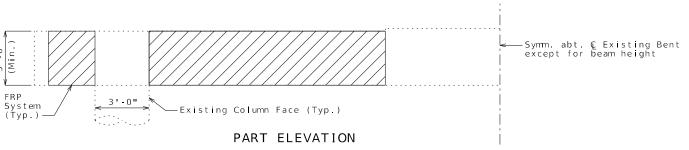
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VERTICAL DRAIN AT END BENTS

Sheet No. 5 of 18



PART PLAN



FRP WRAP AT INT. BENT NO. 6

Notes:

Design Force is the factored shear force at any cross section in each design region that shall be resisted entirely by the FRP reinforcement.

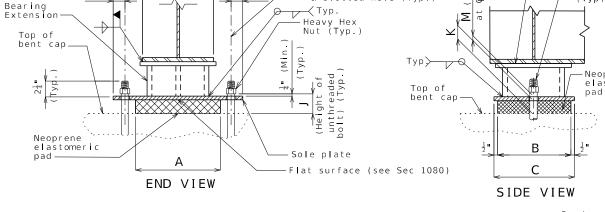
See special provisions.

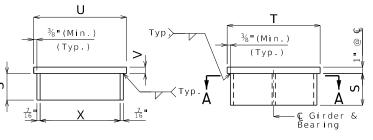
Protective Coating - Concrete Bents and Piers (Epoxy) shall not be applied on the FRP system.

BENT CAP SHEAR STRENGTHENING USING FRP WRAP

Detailed Sept 2023 Checked Sept 2023 Plate

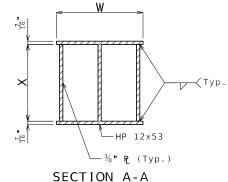
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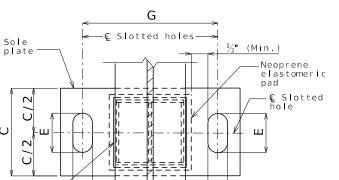




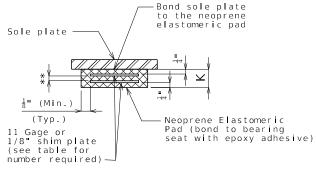
SIDE ELEVATION OF

BEARING EXTENSION





PART PLAN



** Layers of 1/2 elastomer alternating

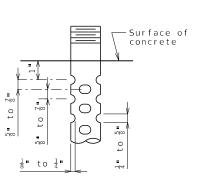
with	1 1	gage	or	1/8-	snim	prate	
NEC	PF	RENE	Ε	LAS	ГОМЕ	RIC	PAD

	EXPANSION BEARINGS																						
BENT NO.	Α	В	С	Д	Е	F	G	J	K	L	М	N	Q	R	S	\dashv	U	٧	W	Χ	Υ	NUMBER OF SHIM PLATES *	NUMBER REQUIRED
3	12"	12"	13"	21½"	6"	1 5 "	17"	5½ •	3 ³ / ₄	12"	1½"	2 1 "	2 1 "	1 m	5½ "	14"	14"	1"	12"	107 "	8 <u>1</u> •	6	2 (Ext.)
3	12"	12"	13"	21 1 "	6"	1 5 "	17"	5 <u>1</u> •	3 3 •	13"	1½"	2"	2 1 "	18	5 1 "	14"	14"	1"	12"	10 ⁷ / ₈ "	8½ •	6	3 (Int.)
6	12"	12"	13"	21½"	4"	15"	17"	4 ¹ / ₄	2½ •	12"	1½"	2 1 "	21 "	1 16	6∛"	14"	14"	1"	12"	107 "	8½ •	4	2 (Ext.)
6	12"	12"	13"	21½"	4"	15"	17"	4 ¹ / ₄ •	2½ •	13"	1 ½ "	2"	21 "	16	6 [%] "	14"	14"	1"	12"	107 "	8½ •	4	3 (Int.)
*	* The required shim plate shall be placed between													TOTAL BEARINGS	10								

layers of elastomer and molded together to form an integral unit.

Surface of

concrete



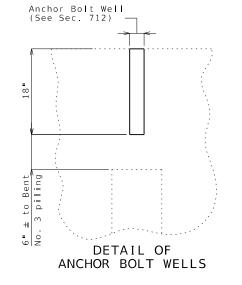
DETAIL OF 3/4"Ø THRU 2 1/2"Ø ANCHOR BOLTS

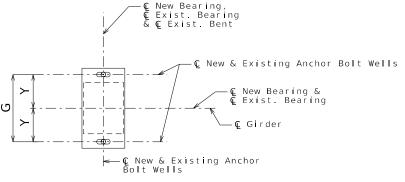
OPTIONAL DETAIL OF 1 3/8"Ø THRU 2 1/2"Ø ANCHOR BOLTS

SWEDGE ANCHOR BOLT DETAILS

(1) $\frac{1}{8}$ " for $\frac{3}{4}$ "Ø thru $1\frac{1}{4}$ "Ø anchor bolts

 $\frac{1}{8}$ " to $\frac{1}{4}$ " for $1\frac{3}{8}$ "Ø thru $2\frac{1}{2}$ "Ø anchor bolts





PART PLAN OF ANCHOR BOLTS AT INT. BENTS NO. 3 & 6

END ELEVATION OF

BEARING EXTENSION

GENERAL NOTES:

Existing anchor bolts shall be cored, removed and replaced with the new

Anchor bolts shall be 1 1/2"Ø ASTM F1554 Grade 55 swedged bolts and shall extend 15" into the concrete with ASTM A563 Grade A Heavy Hex nuts. Actual manufacturer's certified mill test reports(chemical and mechanical) shall be provided. Swedging shall be 1" less than extension into the concrete.

Anchor bolt shall be at the centerline of slotted hole at 60°F. Bearing position shall be adjusted R for each 10° fall or rise in temperature at

Anchor bolts and heavy hex nuts shall be coated with a minimum of two coats of inorganic zinc primer to provide a total dry film thickness of 4 mils minimum, 6 mils maximum, or galvanized in accordance with Sec 1081.

Neoprene Elastomeric Pads shall be 60 Durometer.

Structural steel for sole plate shall be ASTM A709 Grade 50 and shall be coated with a minimum of two coats of inorganic zinc primer to provide a total dry film thickness of 4 mils minimum, 6 mils maximum.

Laminated Neoprene Bearing Pad Assembly shall be in accordance with Sec

All protective coating and FRP Wrap shall be completed before the new bearings are installed.

Cost of removing the existing bearings and anchor bolts will be considered completely covered by the contract unit price per each for Removal of Existing Bearings (See Special Provisions).

Cost of anchor bolts, drilling, grouting and bearing extensions, complete in place, will be considered completely covered by the contract unit price for Laminated Neoprene Bearing Pad Assembly.

Sheet No. 7 of 18

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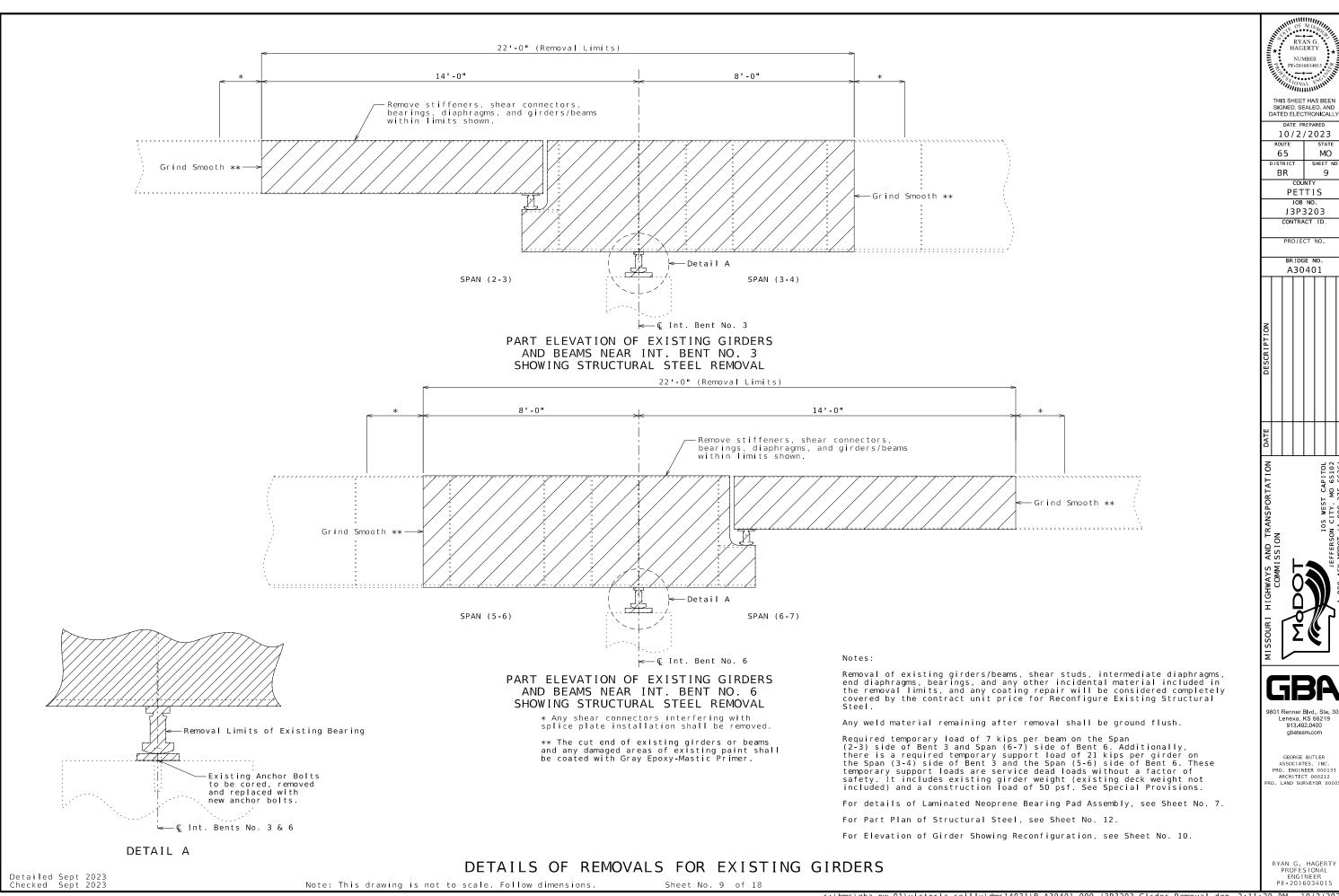
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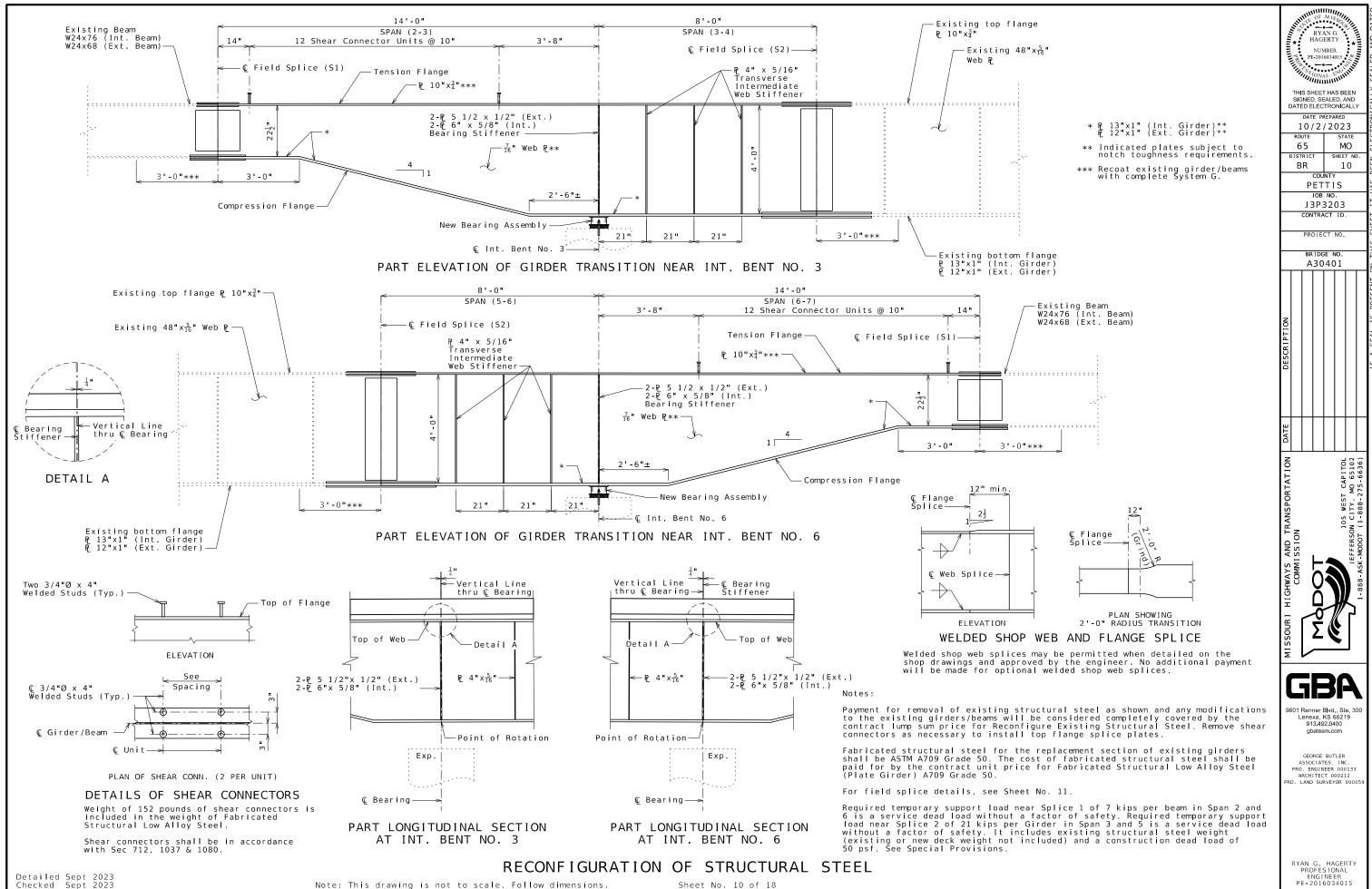
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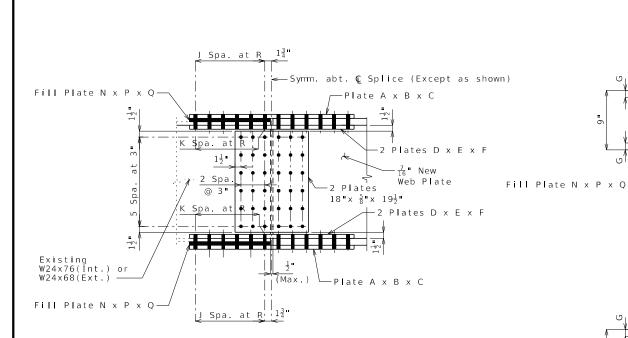
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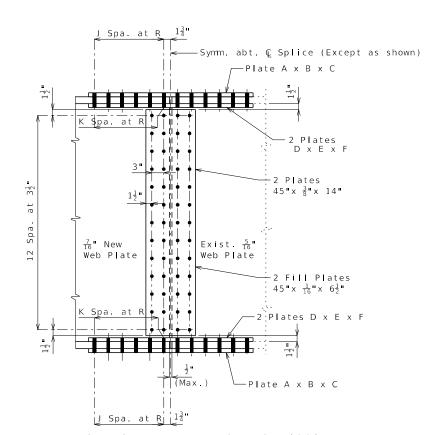
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DETAIL OF BOLTED FIELD SPLICE (S1)



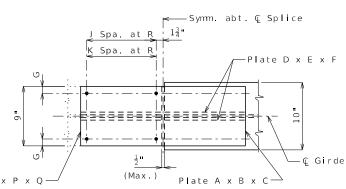
DETAIL OF BOLTED FIELD SPLICE (S2)

Bolts shall be 7/8-inch diameter ASTM F3125 Grade A325 Type 1 in 15/16-inch diameter holes.

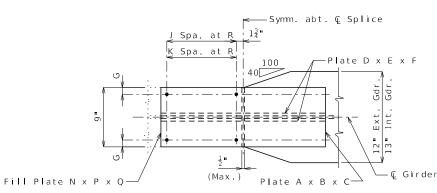
Note: This drawing is not to scale. Follow dimensions.

Contact surfaces shall be in accordance with Sec 1081 for surface preparation.

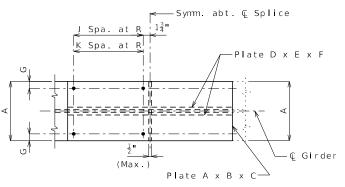
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PLAN OF FLANGE S1 (TOP)



PLAN OF FLANGE S1 (BOTTOM)



PLAN OF FLANGE S2 (TOP AND BOTTOM)

	T.	ABL	E OF	DIM	IENS	SIONS	- F	IEL	_D	SPL	I CE	-	
LOCATION	Α	В	С	D	Е	F	G	J	K	N	Р	Q	R
S1 (TOP, EXT.)	9"	<u>1</u> ≡	18½"	3½"	<u>5</u> 	18½"	2"	2	2	9"	3 m	9"	3"
S1 (TOP, INT.)	9"	<u>1</u> ≡	18½"	3½"	5 <u>8</u>	18½"	2 "	2	2	9"	1 m	9"	3"
S1 (BOTTOM, EXT.)	9"	1/2 "	24½"	3½"	5 =	24 1 "	2 "	3	3	9"	7 16	12"	3"
S1 (BOTTOM, INT.)	9"	<u>1</u> ≡	24 1 "	3½"	<u>5</u> 11	24 1 "	2"	3	3	9"	5 H	12"	3"
S2 (TOP INT. & EXT.)	10"	1/2 m	30½"	4 "	<u>5</u> 11	30½"	2 "	4	4	-	-	-	3"
S2 (BOTTOM, EXT.)	12"	<u>5</u> =	48½"	5 "	3 u	48½"	2"	7	7	-	-	-	3"
S2 (BOTTOM, INT.)	13"	<u>5</u> =	48½"	5½"	<u>3</u> II	48½"	2"	7	7	-	-	-	3"

The flange and web splice plates shall be subject to notch toughness requirements, when notch toughness is required for flanges on both sides of splice.

FIELD SPLICE DETAILS

Sheet No. 11 of 18

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MISSOURI HIGHWAYS AND TRANSPORTA
COMMISSION

105 WEST CAL

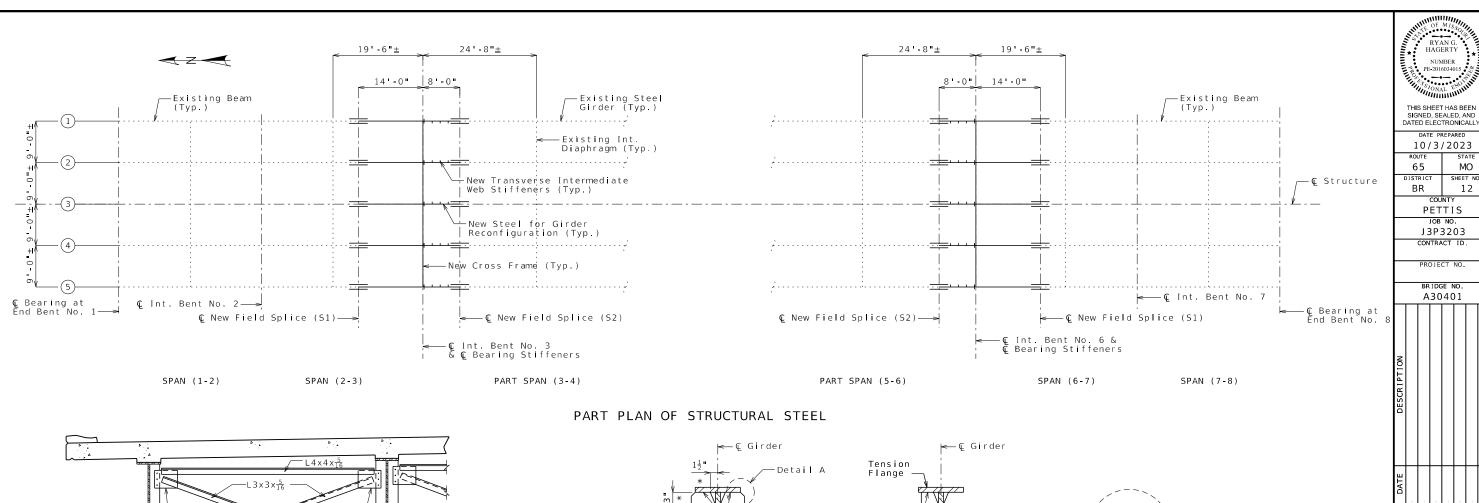
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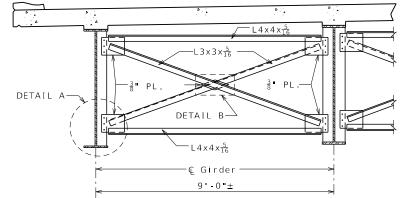
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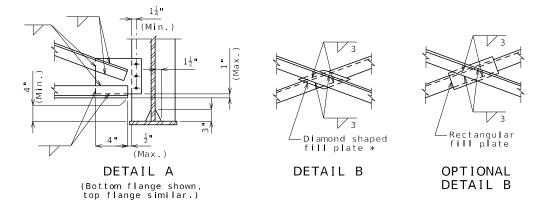
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ARCHITECT 000212
PRO. LAND SURVEYOR 00005:

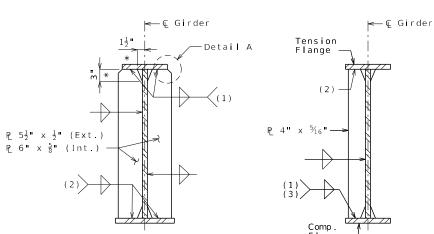
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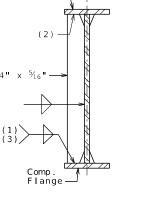


TYPICAL PART SECTION SHOWING CROSS FRAMES AT INT. BENTS NO. 3 & 6









INTERMEDIATE (One side only)

WELDING DETAILS

- (1) Tight fit
- (2) Grind or mill to bear
- (3) Weld to compression flange as located on Elevation of Girder.
- * Typical for all intermediate web stiffeners, intermediate diaphragm connection plates and bearing stiffeners.

All fabricated structural steel shall be ASTM A709 Grade 50. For details of structural steel removal, see Sheet No. 9. For additional details of new structural steel and shear connector details, see Sheet No. 10.

DETAIL A

For details of bolted field splices, see Sheet No. 11.

PLAN OF STRUCTURAL STEEL AND DIAPHRAGM DETAILS

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 \ast At the contractor's option, rectangular fill plates may be used in lieu of diamond fill plates as shown in Optional Detail B.

Tack Weld

Detailed Sept 2023 Checked Sept 2023

1/2"Ø machine bolt

armor after concrete on each side has taken initial set.)

@ abt. 18" cts. (Cut machine bolt

flush with steel

DETAIL A

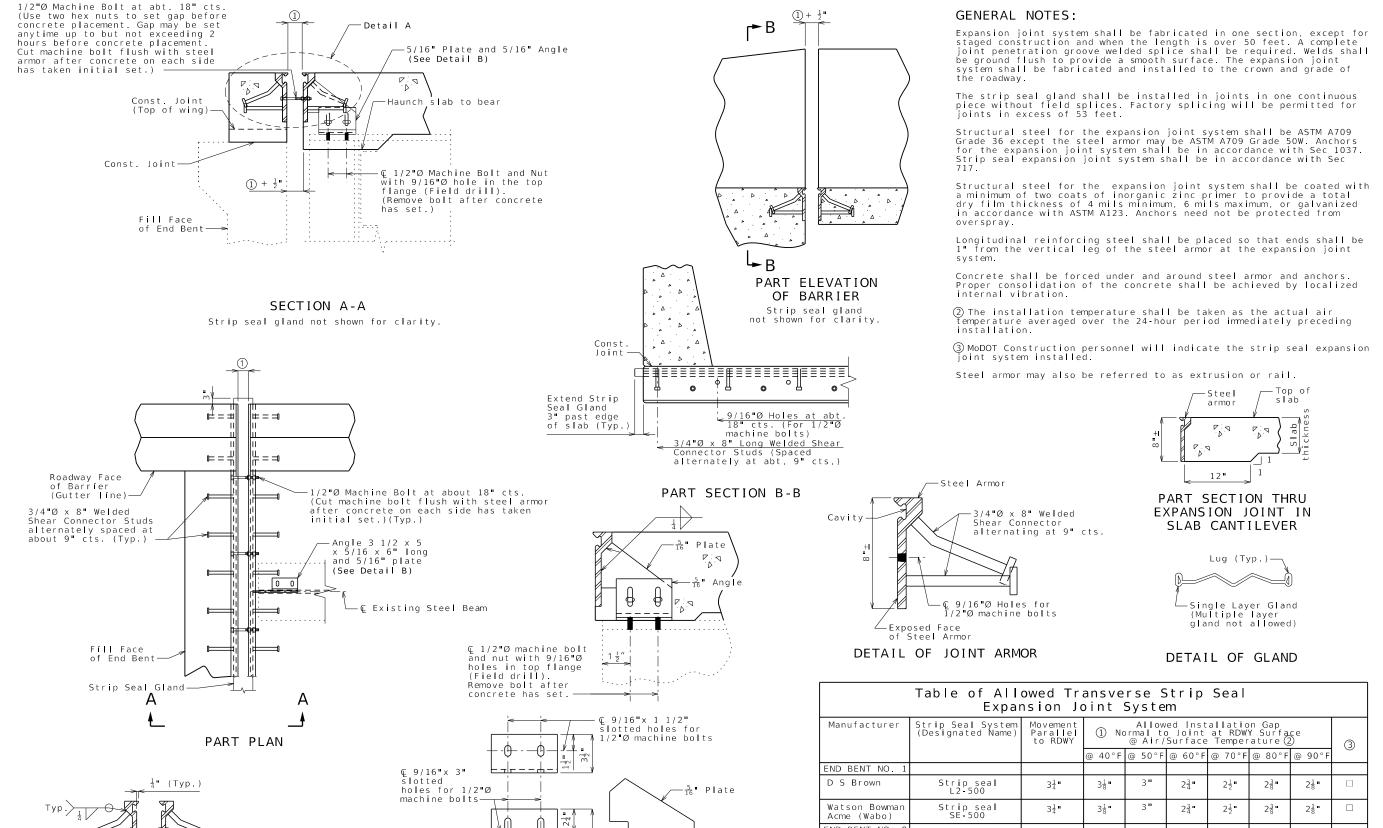


	Table of Allowed Transverse Strip Seal Expansion Joint System													
	Manufacturer	Strip Seal System (Designated Name)	Movement Parallel to RDWY	Allowed Installation Gap ① Normal to Joint at RDWY Surface @ Air/Surface Temperature ②										
				@ 40°F	@ 50°F	@ 60°F	@ 70°F	@ 80°F	@ 90°F	3				
	END BENT NO. 1													
te	D S Brown	Strip seal L2-500	3 ¹ / ₄ "	3 1 "	3"	2 3 "	2 1 "	23"	2 1 8"					
	Watson Bowman Acme (Wabo)	Strip seal SE-500	3 <u>1</u> "	3 1 "	3"	2 3 "	2 1 "	23"	2 1 8"					
	END BENT NO. 8													
	D S Brown	Strip seal L2-400	2"	2¾"	2 5 "	2½"	28"	2 1 "	2 1 8"					
© 9/16"x 1 1/2" slotted holes for 1/2"Ø	D S Brown	Strip seal L2-500	2"	23"	25"	2 1 "	28"	2 1 "	2 1 "					
machine bolts	Watson Bowman Acme (Wabo)	Strip seal SE-400	2"	2¾"	25 "	2 1 "	2 3 "	2 1 "	2 1 8"					
	Watson Bowman Acme (Wabo)	Strip seal SE-500	2"	2 3 "	25 "	2 1 "	2 3 "	2 1 "	2 1 8"					

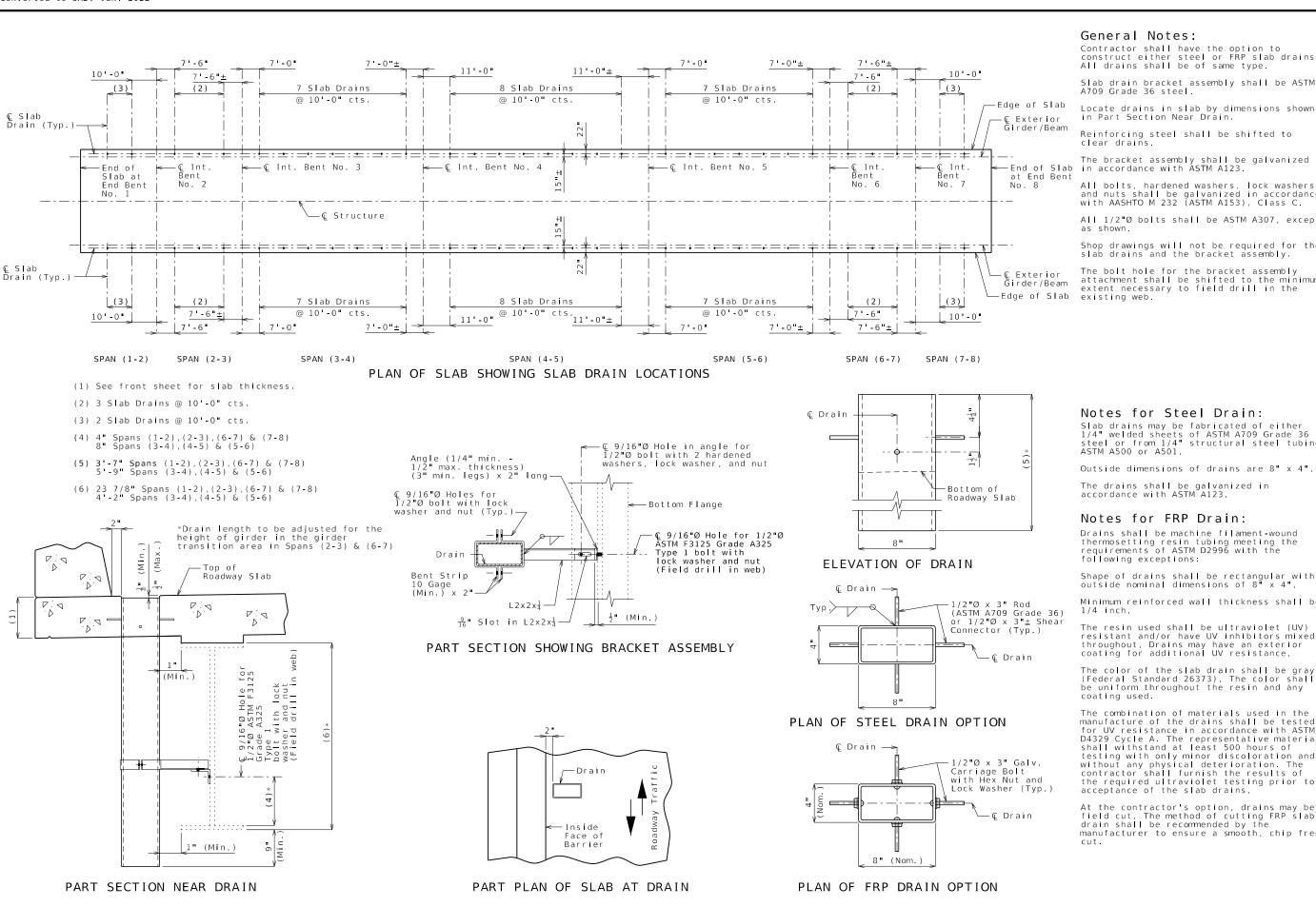
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DETAIL B

Angle 5 x

3 1/2 x 5/16 x 6" | Jona -----

Detailed Sept 2023 Checked Sept 2023



Contractor shall have the option to construct either steel or FRP slab drains.

Slab drain bracket assembly shall be ASTM

Locate drains in slab by dimensions shown in Part Section Near Drain.

The bracket assembly shall be galvanized

All bolts, hardened washers, lock washers and nuts shall be galvanized in accordance with AASHTO M 232 (ASTM A153), Class C.

All 1/2"Ø bolts shall be ASTM A307, except

Shop drawings will not be required for the slab drains and the bracket assembly.

The bolt hole for the bracket assembly attachment shall be shifted to the minimum extent necessary to field drill in the

Slab drains may be fabricated of either 1/4" welded sheets of ASTM A709 Grade 36 steel or from 1/4" structural steel tubing ASTM A500 or A501.

Outside dimensions of drains are 8" x 4"

thermosetting resin tubing meeting the requirements of ASTM D2996 with the following exceptions:

Shape of drains shall be rectangular with outside nominal dimensions of 8" x 4"

Minimum reinforced wall thickness shall be

The resin used shall be ultraviolet (UV) resistant and/or have UV inhibitors mixed throughout. Drains may have an exterior coating for additional UV resistance.

The color of the slab drain shall be gray (Federal Standard 26373). The color shall be uniform throughout the resin and any

The combination of materials used in the manufacture of the drains shall be tested for UV resistance in accordance with ASTM D4329 Cycle A. The representative material shall withstand at least 500 hours of testing with only minor discoloration and without any physical deterioration. The contractor shall furnish the results of the required ultraviolet testing prior to acceptance of the slab drains.

field cut. The method of cutting FRP slab drain shall be recommended by the manufacturer to ensure a smooth, chip free

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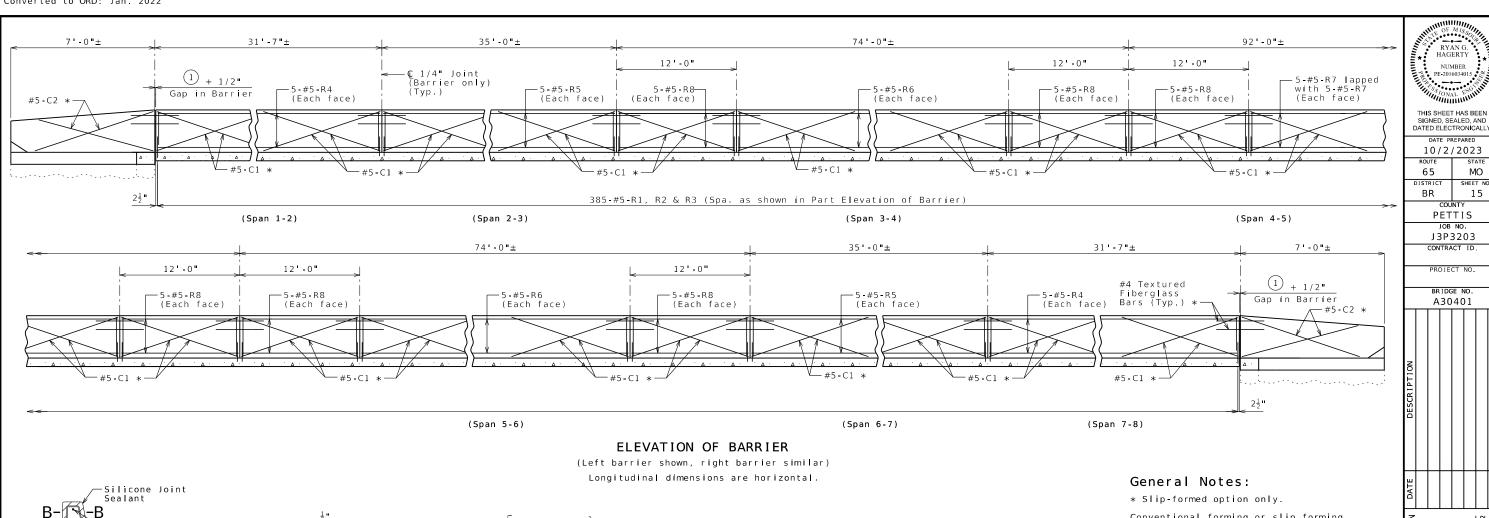
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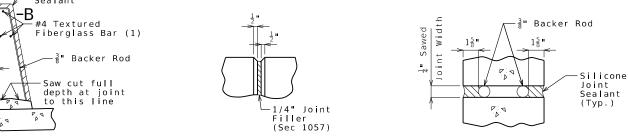
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GEORGE BUTLER ASSOCIATES, INC. PRO. ENGINEER 000133 ARCHITECT 000212 O. LAND SURVEYOR 0000

RYAN G. HAGERTY PROFESIONAL PE-2016034015

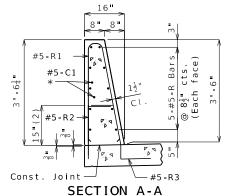
SLAB DRAINS





PART ELEVATION

AT FORMED JOINT



SECTION B-B

Use a minimum lap of 3'-1" for #5 horizontal barrier bars.

The cross-sectional area above the slab is 3.52 square feet.

(2) To top of bar

Note: This drawing is not to scale. Follow dimensions

#5-R Bar (3) Bar (3) Const Joint - #5 - R3

R-BAR PERMISSIBLE ALTERNATE SHAPE

(3) The R1 bar may be separated into two bars as shown, at the contractor's option, only when slip forming is not used. (All dimensions are out to out.)

Conventional forming or slip forming may be used. Saw cut joints may be used with conventional forming.

Top of barrier shall be built parallel to grade and barrier joints (except at end bents) normal to grade.

All exposed edges of barrier shall have either a 1/2-inch radius or a 3/8-inch bevel. unless otherwise noted.

Payment for all concrete and reinforcement, complete in place, will be considered completely covered by the contract unit price for Type D Barrier per linear foot.

Concrete in barrier shall be Class B-1.

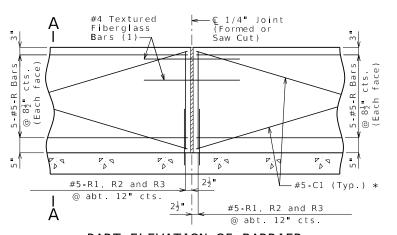
Measurement of barrier is to the nearest linear foot for each structure, measured along the outside top of slab from end of wing to end of wing.

Concrete traffic barrier delineators shall be placed on top of the barrier as shown on Missouri Standard Plan 617.10 and in accordance with Sec 617. Delineators on bridges with two-lane, two-way traffic shall have retroreflective sheeting on both sides. Concrete traffic barrier delineators will be considered completely covered by the contract unit price for Type D Barrier

Joint sealant and backer rods shall be in accordance with Sec 717 for silicone joint sealant for saw cut and formed joints.

For slip-formed option, both sides of barrier shall have a vertically broomed finish and the top shall have a transversely broomed finish.

(1) See Sheet No. 13 for dimension of gap in the barrier.



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SECTION THRU

SAW CUT JOINT

Detailed Sept 2023 Checked Sept 2023

PART ELEVATION OF BARRIER

(1) Four feet long, centered on joint, slip-formed option only

TYPE D BARRIER

Sheet No. 15 of 18

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9801 Renner Blvd., Ste. 30 Lenexa, KS 66219 913.492.0400 gbateam.com

HAGERTY

NUMBER PE-201603401

DATE PREPARED 10/2/2023

PETTIS

J3P3203

CONTRACT ID

PROJECT NO

BRIDGE NO

A30401

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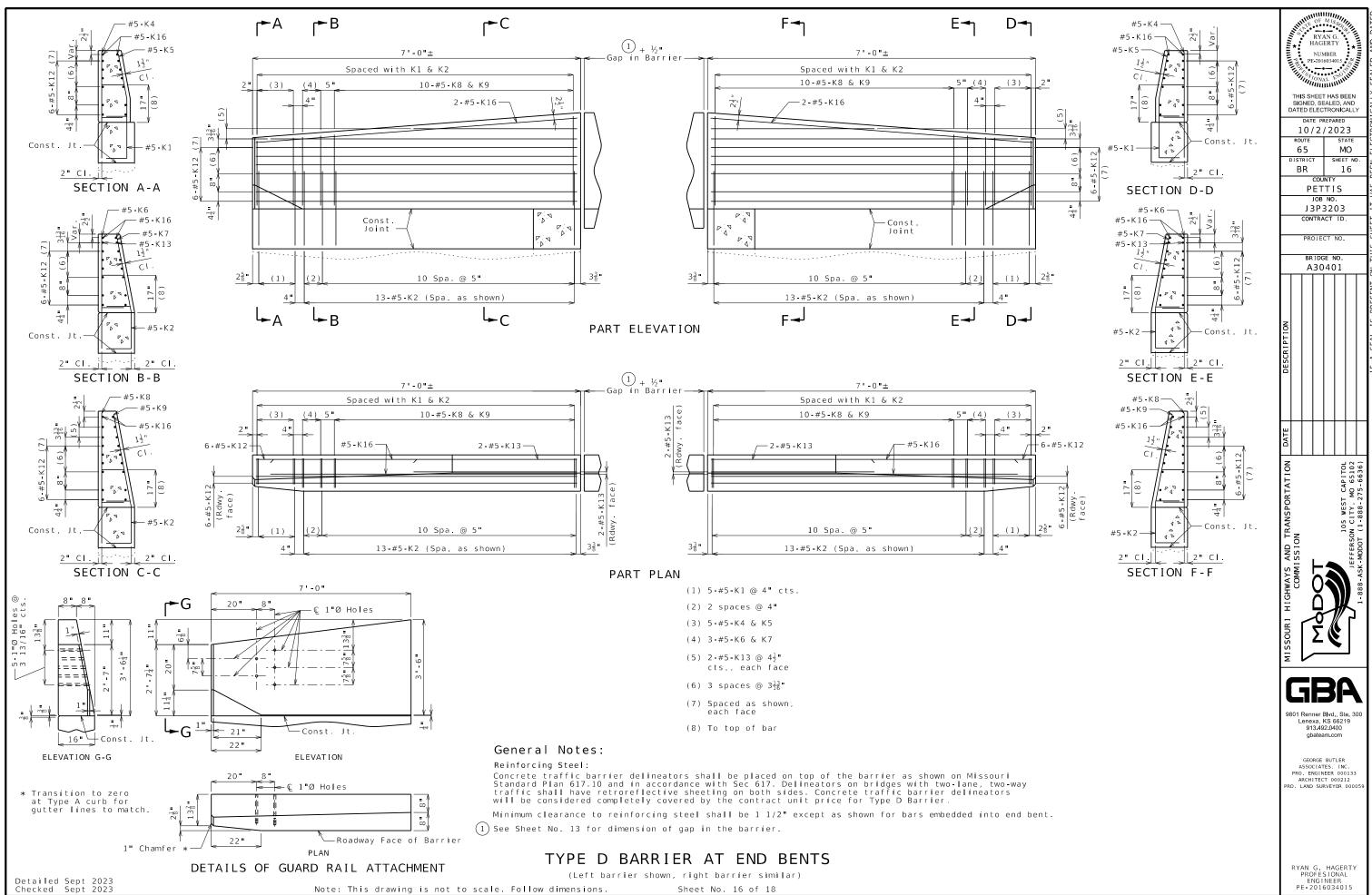
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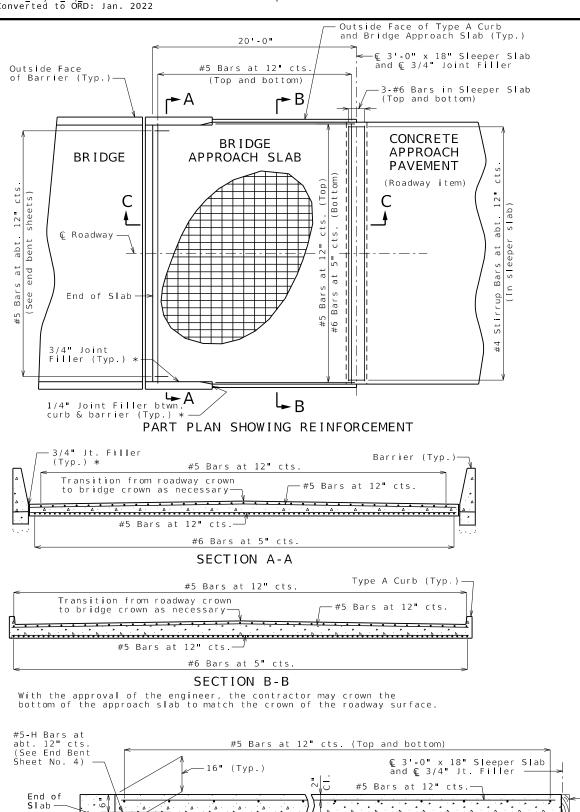
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#6 Bars at 5" cts.-

Performance Class A

Detailed Sept 2023 Checked Sept 2023

-2 Layers of 4 Mil Polyethylene Sheeting (Placed between bridge

approach slab and granular base) in accordance with ASTM E 1745

SECTION C-C

Type 5

Aggregate

Perforated

Drain Pipe

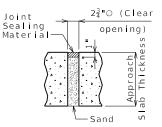
(Slope to drain)

18"

Note: This drawing is not to scale. Follow dimensions

3-#6 Bars

(Top and bottom 3'-0"



UNDERSEAL ACCESS HOLE DETAIL

(If required)

Timber Header

2 Layers of 30-1b (Min.)

Roofing Felt (Placed between bridge approach slab,

roadway concrete approach

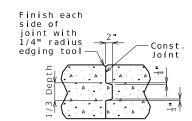
pavement and sleeper slab)

Actual length = 8'-3"

CRSI 90° stirrup hook

— Bottom of Sleeper Slab

-#4 Stirrup Bars at abt. 12" cts.; 2'-9"x 13 1/2" out to out;



CONSTRUCTION JOINT DETAIL

General Notes:

All concrete for the bridge approach slab and sleeper slab shall be in accordance with Sec 503 (f c = 4,000

The reinforcing steel in the bridge approach slab and the sleeper slab shall be epoxy coated Grade 60 with fy = 60,000 psi.

Drain pipe may be either 6" diameter corrugated metallic-coated pipe underdrain, 4" diameter corrugated polyvinyl chloride (PVC) drain pipe, or 4" diameter corrugated polyethylene (PE) drain pipe.

Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise shown.

The reinforcing steel in the bridge approach slab and the sleeper slab shall be continuous. The transverse reinforcing steel may be made continuous by providing a minimum lap splice of 29 inches for #5 bars and 44 inches for #6 bars, or by mechanical bar splice.

All joint filler shall be in accordance with Sec 1057 for preformed fiber expansion joint filler except as noted.

The contractor shall pour and satisfactorily finish the bridge slab before placing the bridge approach slab.

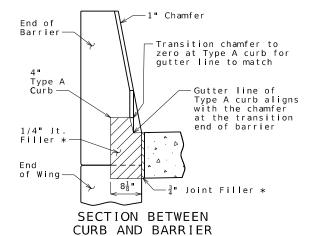
Longitudinal construction joints in approach slab and sleeper slab shall be aligned with longitudinal construction joints in bridge slab.

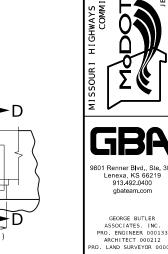
For concrete approach pavement details, see roadway plans

See Missouri Standard Plan 609.00 for details of Type A

Payment for furnishing all materials, labor and excavation necessary to construct the approach slab, including the timber header, sleeper slab, underdrain, Type 5 aggregate base, joint filler and all other appurtenances and incidental work as shown on this sheet, complete in place, will be considered completely covered by the contract unit price for Bridge Approach Slab (Major) per square vard (Major) per square yard

* Seal joint between vertical face of approach slab and wing with sealant in accordance with Sec 717 for silicone joint sealant for saw cut and formed joints.





3Bf

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RYAN G. HAGERTY PROFESIONAL

PF-2016034015

HAGERTY

NUMBER PE-201603401

SIGNED, SEALED, AND DATED ELECTRONICALLY

DATE PREPARED

10/2/2023

PETTIS

J3P3203

CONTRACT ID

PROJECT NO.

BRIDGE NO

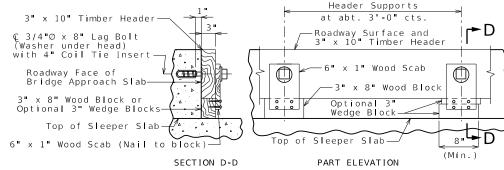
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DETAILS OF TIMBER HEADER

Remove timber header when concrete pavement is placed.

BRIDGE APPROACH SLAB (MAJOR)

