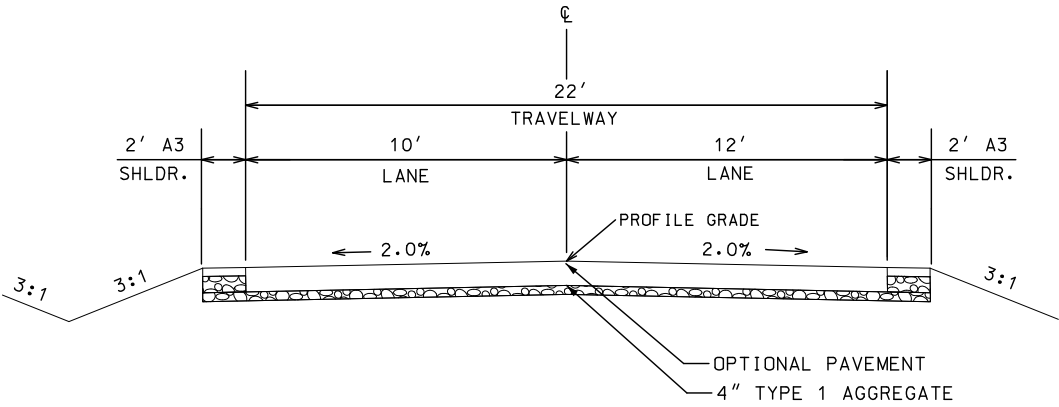
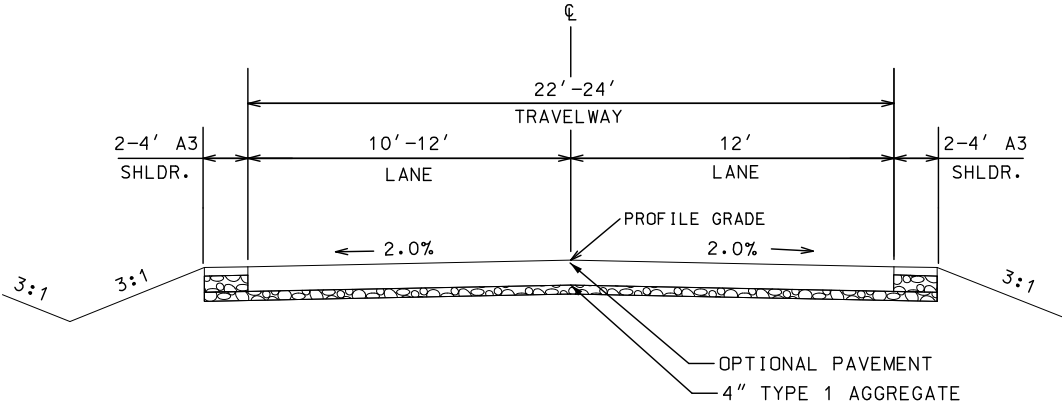


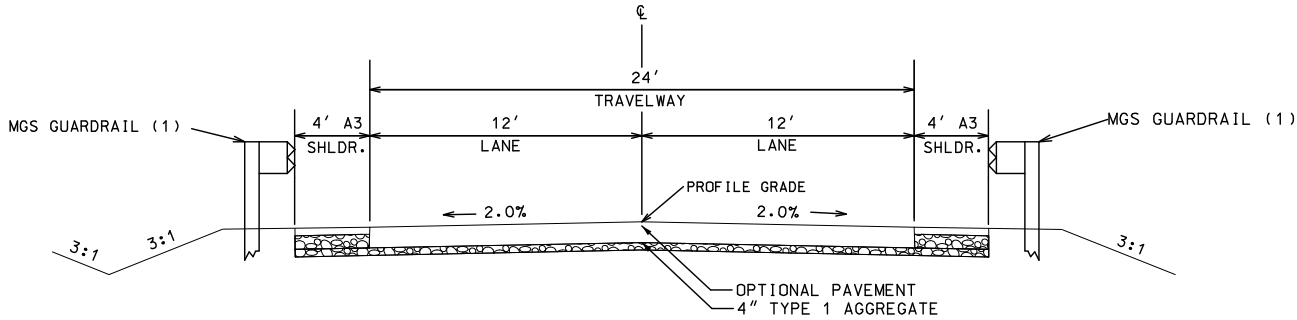
OPTIONAL PAVEMENT DESIGN FOR MINOR ROUTE		
ROADWAY	7" HMA/ 7" JPCP	4" TYPE 1 AGGREGATE BASE
BRIDGE APPROACH	10" HMA/8" JPCP	4" TYPE 1 AGGREGATE BASE



TYPICAL SECTION - ROUTE V
STA. 232+92.96 TO STA. 234+52.70

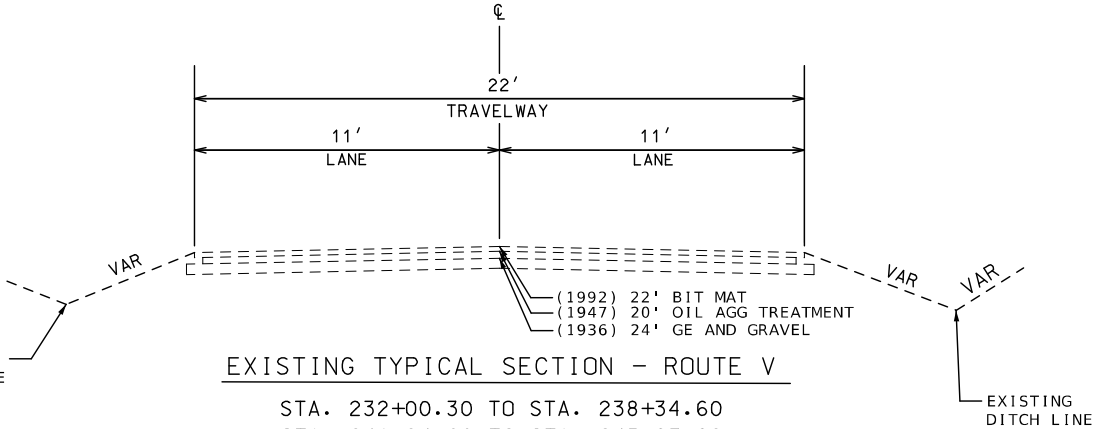


TYPICAL SECTION - ROUTE V
STA. 234+52.70 TO STA. 236+52.70



TYPICAL SECTION - ROUTE V
STA. 236+52.70 TO STA. 238+03.94
STA. 241+57.98 TO STA. 243+09.22

NOTE:
(1) SEE SPECIAL SHEET 1 OF 2 FOR EXACT LIMITS OF MGS GUARDRAIL.



EXISTING TYPICAL SECTION - ROUTE V
STA. 232+00.30 TO STA. 238+34.60
STA. 241+24.60 TO STA. 245+87.89



Warner D. Sherman
10/2/2023 2:36:53 PM
WARNER D. SHERMAN II - CIVIL
MO-PE-26645

DATE PREPARED
10/2/2023

ROUTE V STATE MO

DISTRICT SW SHEET NO. 2

COUNTY GREENE

JOB NO. J7S3486

CONTRACT ID.

PROJECT NO.

BRIDGE NO. A9119

DESCRIPTION

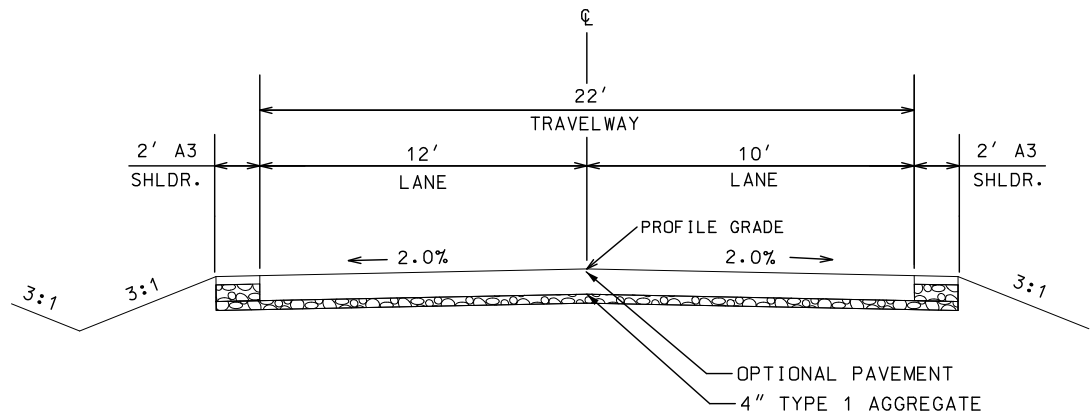
DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

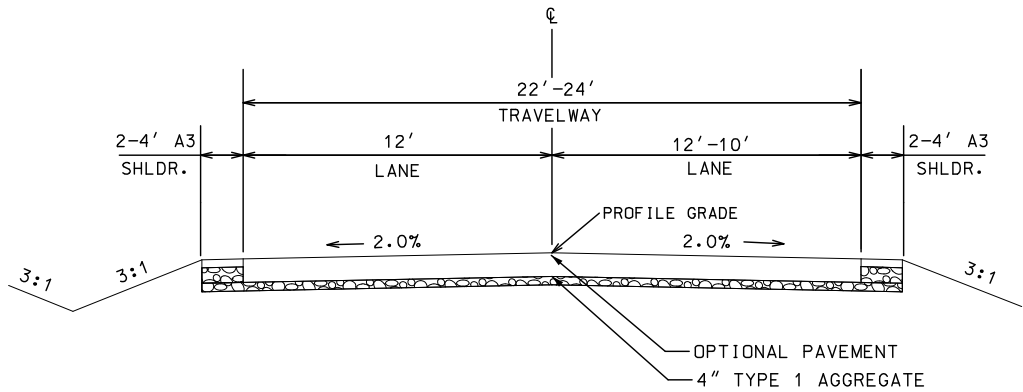
105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)

MoDOT

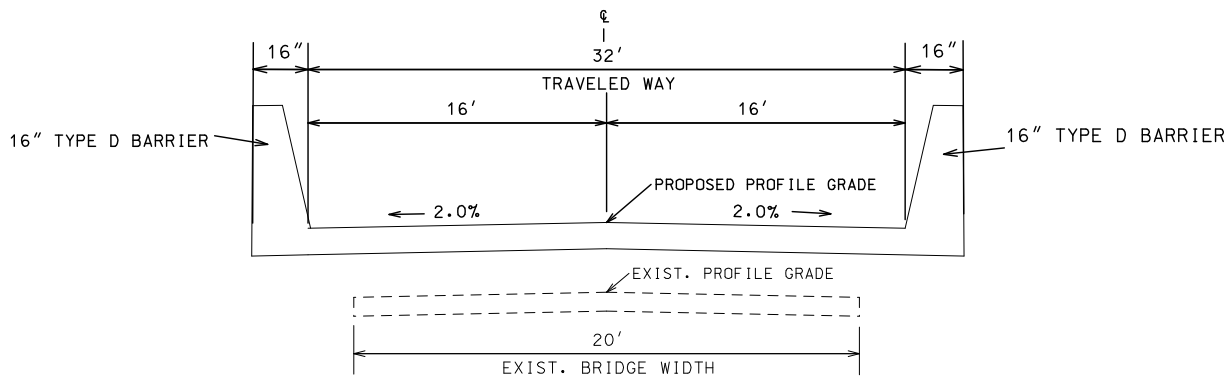
OPTIONAL PAVEMENT DESIGN FOR MINOR ROUTE		
ROADWAY	7" HMA/ 7" JPCP	4" TYPE 1 AGGREGATE BASE
BRIDGE APPROACH	10" HMA/8" JPCP	4" TYPE 1 AGGREGATE BASE



TYPICAL SECTION – ROUTE V
STA. 245+09.22 TO STA. 245+54.35



TYPICAL SECTION – ROUTE V
STA. 243+09.22 TO STA. 245+09.22



BRIDGE A9119
TYPICAL SECTION – ROUTE V
PROPOSED BRIDGE
STA. 238+23.94 TO STA. 241+37.98

NOT TO SCALE

TYPICAL SECTION SHEET
SHEET 2 OF 2

STATE OF MISSOURI

WARNER D. SHERMAN II

NUMBER PE-26645

PROFESSIONAL ENGINEER

Warner D. Sherman

10/02/2023 2:37:31 PM

WARNER D. SHERMAN II - CIVIL

MO-PE-26645

DATE PREPARED
10/2/2023

ROUTE V	STATE MO
DISTRICT SW	SHEET NO. 2

COUNTY
GREENE

JOB NO.
J7S3486

CONTRACT ID.

PROJECT NO.

BRIDGE NO.
A9119

DESCRIPTION	DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

MoDOT

105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)

OPTIONAL PAVEMENT, TYPE A3 SHOULDER & AGGREGATE BASE										
STA.	STA.	LOCATION	LENGTH	LT SHOULDER WIDTH	RT SHOULDER WIDTH	MAINLINE WIDTH	OPTIONAL PAVEMENT	TYPE A3 SHOULDER	4 INCH TYPE 1 AGGREGATE BASE	REMARKS
			(FT)	(FT)	(FT)	(FT)	(SY)	(SY)	(SY)	
232+92.96	234+52.70	LT / RT OF RTE V CL	159.74	2	2	22	390.48	71.00	461.5	10' NB LANE AND 12' SB LANE
234+52.70	236+52.70	LT / RT OF RTE V CL	200	2-4	2-4	22-24	511.11	133.33	644.4	WIDTH TRANSITION NORTH OF BRIDGE
236+52.70	238+03.94	LT / RT OF RTE V CL	151.24	4	4	24	403.31	134.44	537.7	12' LANES NORTH OF BRIDGE
241+57.98	243+09.22	LT / RT OF RTE V CL	151.24	4	4	24	403.31	134.44	537.7	12' LANES SOUTH OF BRIDGE
243+09.22	245+09.22	LT / RT OF RTE V CL	200	4-2	4-2	24-22	511.11	133.33	644.4	WIDTH TRANSITION SOUTH OF BRIDGE
245+09.22	245+54.35	LT / RT OF RTE V CL	45.13	2	2	22	110.32	20.06	130.4	12' NB LANE AND 10' SB LANE
						TOTAL	2329.64	626.60	2956.1	
						USE	2329.6	626.6	2956	

PERMANENT PAVEMENT MARKING							
STA	STA	LOCATION	LENGTH (FT)	4" SOLID WHITE WATERBORNE PAINT TYPE P BEADS (LF)	4" INTERMITTENT YELLOW WATERBORNE PAINT TYPE P BEADS (LF)	4" SOLID YELLOW WATERBORNE PAINT TYPE P BEADS (LF)	REMARKS
232+92.96	237+75.00	RT / LT / CL OF RTE V	482.04	964.1	120.5	482.0	CENTERLINE, SBL NO PASSING LINE, EDGELINES
237+75.00	242+25.00	RT / LT / CL OF RTE V	450	900.0	112.5		CENTERLINE & EDGELINES
242+25.00	243+25.00	RT / LT / CL OF RTE V	100	200.0	25.0	100.0	CENTERLINE, SBL NO PASSING LINE, EDGELINES
243+25.00	245+54.35	RT / LT / CL OF RTE V	229.35	458.7		458.7	DOUBLE NO PASSING LINE, EDGELINES
			TOTAL	2522.8	258.0	1040.7	
			USE	2523	258	1041	

GUARDRAIL							
STA	STA	LENGTH (LF)	LOCATION	GUARDRAIL (MGS) (LF)	TYPE A CRASHWORTHY END TERMINAL (MASH) (EA)	BRIDGE APPROACH TRANSITION SECTION (MGS) (EA)	REMARKS
236+52.70	238+02.70	150	RT OF RTE V EOS	62.5	1	1	NW CORNER OF BR A9119
236+96.18	238+21.18	125	LT OF RTE V EOS	37.5	1	1	NE CORNER OF BR A9119
241+40.74	242+65.74	125	RT OF RTE V EOS	37.5	1	1	SW CORNER OF BR A9119
241+59.22	243+09.22	150	LT OF RTE V EOS	62.5	1	1	SE CORNER OF BR A9119
			TOTAL	200.0	4	4	
			USE	200	4	4	

EARTHWORK										
STA	STA	LOCATION	APPROX EARTHWORK			UNCLASSIFIED EXCAVATION (CY)	COMPACTING EMBANKMENT (CY)	EMBANKMENT IN PLACE (CY)	COMPACTING IN CUT (STA)	REMARKS
			CUT (CY)	FILL (CY)	UNSUITABLE (CY)					
232+92.96	238+33.18	LT / RT OF RTE V CL	291.97	2246.02	232.46	292.0	546.4	1699.6	5.40	RTE V - NORTH OF BR A9119
238+14.70	238+96.10	LT / RT OF RTE V CL	637.58	26.62	3.77	637.6	26.6		0.81	SPILL FILL SLOPE - NORTH END OF BR A9119
240+79.73	241+47.22	LT / RT OF RTE V CL	502.92	15.30	4.94	502.9	15.3		0.67	SPILL FILL SLOPE - SOUTH END OF BR A9119
241+28.74	245+54.35	LT / RT OF RTE V CL	214.10	2710.90	181.05	214.1	423.6	2287.3	4.26	RTE V - SOUTH OF BR A9119
					TOTAL	1646.6	1011.9	3986.9	11.14	
					USE	1647	1012	3987	11.1	
UNSUITABLE MATERIAL CONSISTS OF REMOVED EXISTING PAVEMENT. THIS MATERIAL SHALL BE HAULED AWAY BY THE CONTRACTOR AT NO DIRECT PAY.										

ROCK BLANKET									
STA	STA	LOCATION	ROCK DEPTH (FT)	LENGTH (FT)	WIDTH (FT)	TYPE 2 ROCK BLANKET		GEOTEXTILE FABRIC (SY)	REMARKS
						FURNISH (CY)	PLACE (CY)		
237+94.70	238+96.10	LT / RT OF RTE V CL	2	101.4	VAR	458.1	458.1	793.2	NORTH END OF BRIDGE
240+79.73	241+67.22	LT / RT OF RTE V CL	2	87.5	VAR	389.8	389.8	678.3	SOUTH END OF BRIDGE
						TOTAL	847.9	847.9	1471.5
						USE	848	848	1472

STATE OF MISSOURI

WARNER D. SHERMAN

NUMBER PE-26645

PROFESSIONAL ENGINEER

10/02/2023 2:38:05 PM

WARNER D. SHERMAN II - CIVIL

MO-PE-26645

DATE PREPARED

10/2/2023

ROUTE

V

STATE

MO

DISTRICT

SW

SHEET NO.

3

COUNTY

GREENE

JOB NO.

J7S3486

CONTRACT ID.

PROJECT NO.

BRIDGE NO.

A9119

DESCRIPTION

DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

MoDOT

105 WEST CAPITOL

JEFFERSON CITY, MO 65102

1-888-ASK-MODOT (1-888-273-6636)

TEMPORARY EROSION CONTROL									
STA	STA	LOCATION	SILT FENCE (LF)	ROCK DITCH CHECK (LF)	TYPE C BERM (LF)	SEDIMENT REMOVAL (CY)	TEMPORARY SEEDING (ACRES)	MULCHING (ACRES)	REMARKS
232+92.90	238+96.10	LT / RT OF RTE V					0.62	0.62	RTE V - NORTH OF BRIDGE
232+92.90	237+94.70	RT OF RTE V	504.2			5.0			RTE V - NORTH OF BRIDGE
232+92.90	238+13.18	LT OF RTE V	521.6			5.2			RTE V - NORTH OF BRIDGE
233+09.29		LT OF RTE V		12		1.2			RTE V - NORTH OF BRIDGE
233+41.18		LT OF RTE V		12		1.2			RTE V - NORTH OF BRIDGE
233+70.14		LT OF RTE V		13.9		1.4			RTE V - NORTH OF BRIDGE
234+10.71		LT OF RTE V		13.3		1.3			RTE V - NORTH OF BRIDGE
234+56.19		LT OF RTE V		14.2		1.4			RTE V - NORTH OF BRIDGE
235+45.00		LT OF RTE V		13.7		1.4			RTE V - NORTH OF BRIDGE
237+94.70	239+02.98	LT / RT OF RTE V			244.4	2.4			RTE V - NORTH BRIDGE END
240+72.79	241+67.22	LT / RT OF RTE V			221.5	2.2			RTE V - SOUTH BRIDGE END
240+79.73	245+54.35	LT / RT OF RTE V					0.60	0.60	RTE V - SOUTH OF BRIDGE
241+48.74	245+54.35	RT OF RTE V	407.9			4.1			RTE V - SOUTH OF BRIDGE
241+67.22	245+54.35	LT OF RTE V	389.6			3.9			RTE V - SOUTH OF BRIDGE
		TOTAL	1823.3	79.1	465.9	30.7	1.22	1.22	
		USE	1823	79	466	31	1.2	1.2	

SEEDING							
STA	STA	LOCATION	LENGTH (FT)	WIDTH (FT)	SEEDING COOL SEASON MIX (ACRES)	MULCHING (ACRES)	REMARKS
232+92.90	238+96.10	LT / RT OF RTE V	603.2	VAR	0.62	0.62	RTE V - NORTH OF BRIDGE
240+79.73	245+54.35	LT / RT OF RTE V	474.62	VAR	0.60	0.60	RTE V - SOUTH OF BRIDGE
				TOTAL	1.22	1.22	
				USE	1.2	1.2	
PLAN QUANTITY WILL BE PAID UNLESS OVERUN IS AUTHORIZED.							

REMOVAL OF IMPROVEMENTS				
STA / LOG MILE	STA / LOG MILE	LOCATION	QTY	DESCRIPTION
4.158		RTE V RT	1	ONE LANE BRIDGE SIGN
232+55.19		RTE V RT	1	WEIGHT LIMIT SIGN
236+49.02		RTE V RT	1	YEILD TO ONCOMING TRAFFIC SIGN
237+42.00	242+17.00	RTE V RT	6	OBJECT MARKER
237+42.00	242+17.00	RTE V LT	6	OBJECT MARKER
243+10.40		RTE V LT	1	YEILD TO ONCOMING TRAFFIC SIGN
247+09.80		RTE V LT	1	WEIGHT LIMIT SIGN
4.738		RTE V LT	1	ONE LANE BRIDGE SIGN
1 LUMP SUM				

CLEARING AND GRUBBING
TOTAL 0.75 ACRES
USE 1.00 ACRES

MOBILIZATION
1 LUMP SUM
CONTRACTOR FURNISHED SURVEYING AND STAKING
1 LUMP SUM

STATE OF MISSOURI

WARNER D. SHERMAN II

NUMBER PE-26645

PROFESSIONAL ENGINEER

Warner D. Sherman

10/02/2023 2:39:16 PM

WARNER D. SHERMAN II - CIVIL

MO-PE-26645

DATE PREPARED

10/2/2023

ROUTE

V

STATE

MO

DISTRICT

SW

SHEET NO.

3

COUNTY

GREENE

JOB NO.

J7S3486

CONTRACT ID.

PROJECT NO.

BRIDGE NO.

A9119

DESCRIPTION

DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

105 WEST CAPITOL

JEFFERSON CITY, MO 65102

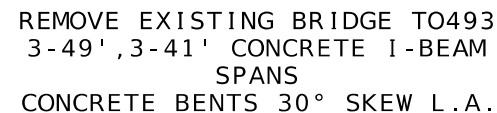
1-888-ASK-MODOT (1-888-275-6636)

MoDOT

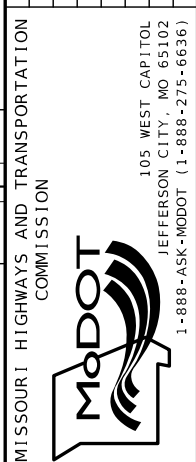
SUMMARY OF QUANTITIES
SHEET 2 OF 3

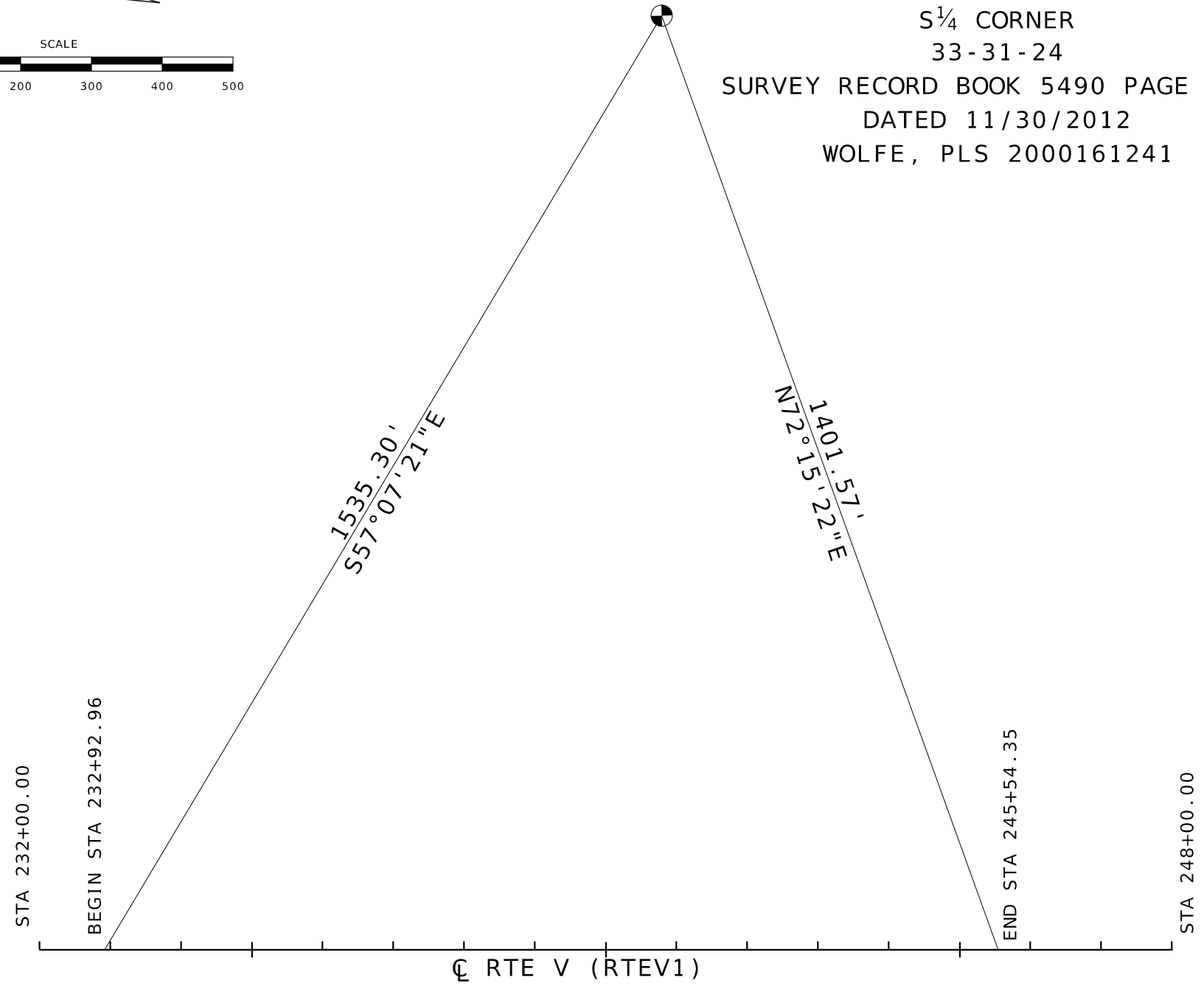
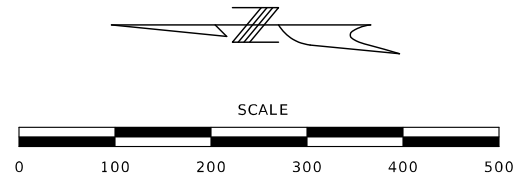
SEE SPECIAL SHEET 1 OF 2 FOR EXACT LIMITS OF MGS GUARDRAIL

NW¹/₄
4-30-24



PLAN-PROFILE SHEET
SHEET 1 OF 3





EXISTING IRON PIN
S¹/₄ CORNER
33-31-24
SURVEY RECORD BOOK 5490 PAGE 2329
DATED 11/30/2012
WOLFE, PLS 2000161241

STATE OF MISSOURI

WARNER D. SHERMAN II

NUMBER PE-26645

PROFESSIONAL ENGINEER

Warner D. Sherman II

10/02/2023 2:41:27 PM

WARNER D. SHERMAN II - CIVIL

MO-PE-26645

DATE PREPARED
10/2/2023

ROUTE
V

DISTRICT
SW

STATE
MO

SHEET NO.
5

COUNTY
GREENE

JOB NO.
J7S3486

CONTRACT ID.

PROJECT NO.

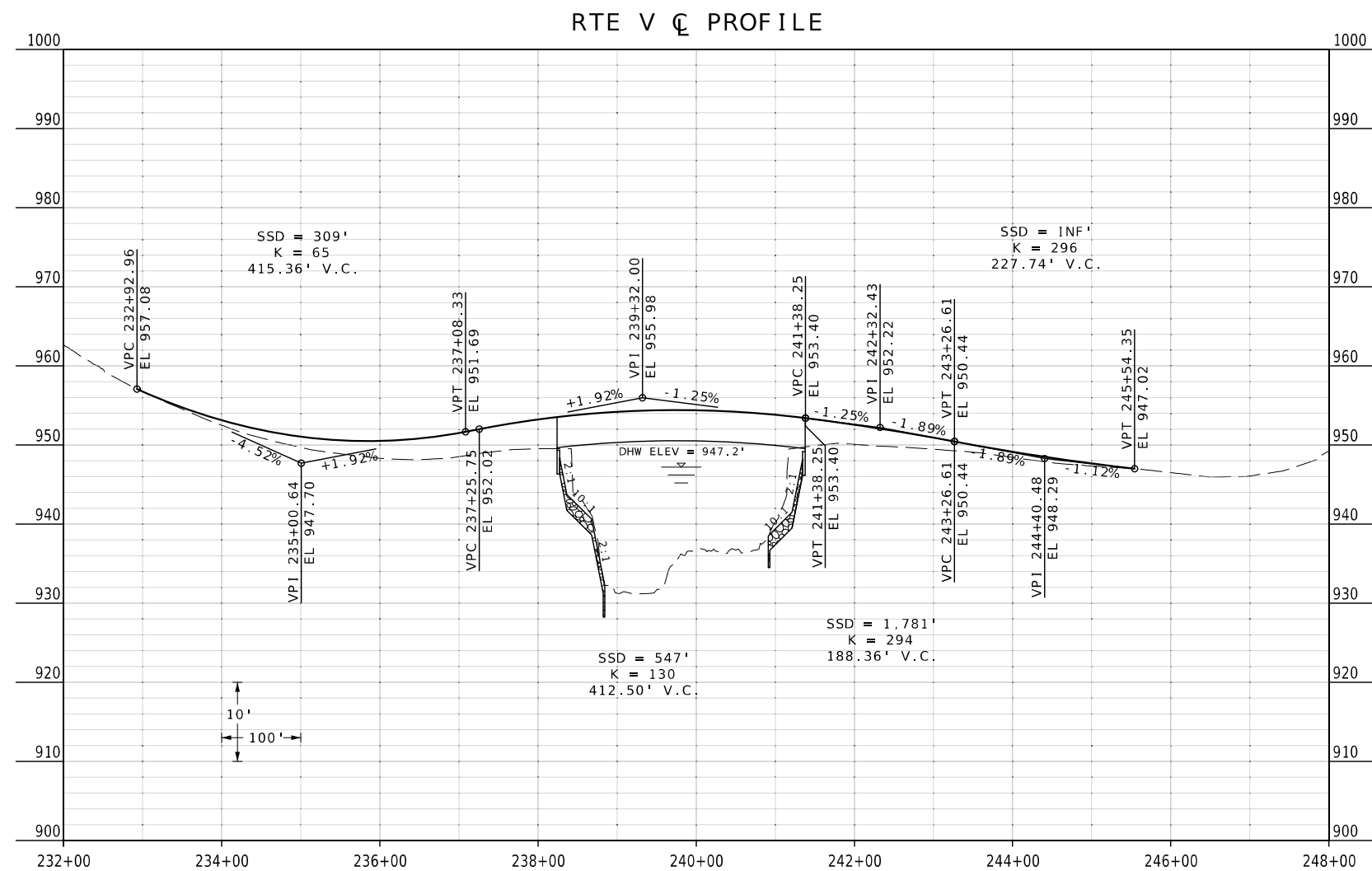
BRIDGE NO.
A9119

DATE	DESCRIPTION

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

MoDOT

105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)



NOTE:
NORTH K VALUE DOES NOT MEET MODOT STANDARDS. THE EXISTING K VALUE IS 65 AND THE PROPOSED K VALUE IS 65. A DESIGN EXCEPTION HAS BEEN CREATED FOR THIS VALUE.



Warner Sherman
10/02/2023 2:41:52 PM
WARNER D. SHERMAN II - CIVIL
MO-PE-26645

DATE PREPARED
10/2/2023

ROUTE V	STATE MO
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DISTRICT	SHEET NO.
SW	6

COUNTY
GREENE

J7S3486

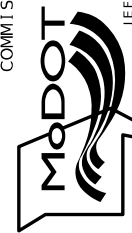
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CONTRACT ID.

PROJECT NO.

BRIDGE NO.
A9119

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MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION



105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)

ALL PROJECT COORDINATES HAVE BEEN PROJECTED FROM THE MISSOURI STATE PLANE COORDINATE (SPC) SYSTEM OF 1983 USING AN AVERAGE PROJECT PROJECTION (GRID TO GROUND) FACTOR. TO GET BACK TO STATE PLANE COORDINATES MULTIPLY THE PROJECT COORDINATES BY THE AVERAGE GRID FACTOR AS SHOWN IN THE "REFERENCE CONTROL INFORMATION" PORTION OF THIS TABLE.

[illegible]

COORDINATE SYSTEM	Modified State Plane	
HORIZONTAL DATUM	NAD83	
VERTICAL DATUM	NAVD88	
GEOID MODEL	Geoid 18	
ELEVATIONS	GPS	
DETERMINED BY		
PROJECT PROJECTION FACTOR	0.99999430	

REFERENCE	CONTROL	INFORMATION
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4	5	6
7	8	9
10	11	12
13	14	15
16	17	18
19	20	21
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364	365	366
367		

COORDINATE SYSTEM	Missouri State Plane
CONTROL STATION	MoDOT GNSS

DESIGNATION	MoDOT Springfield Cors ARP
CORS_ID	MOSF
PID	DL5388
LATITUDE	37 14 30.33454
LONGITUDE	093 13 47.54333
NORTHING (M)	156530.873
EASTING (M)	435242.882
ZONE	Central

PROJECT AVERAGE GRID FACTOR	1.00000570
EXAMPLE OF PROJECT COORDINATE TO S.P.C.	

```
PROJECT NORTHING X AVERAGE GRID FACTOR
= STATE PLANE NORTHING
PROJECT EASTING X AVERAGE GRID FACTOR
= STATE PLANE EASTING

EXAMPLE: CONTROL POINT # 1
N 560135.3905 X 1.0000057 = N 560138.5878
E 1325990.8618 X 1.0000057 = E 1325998.4198
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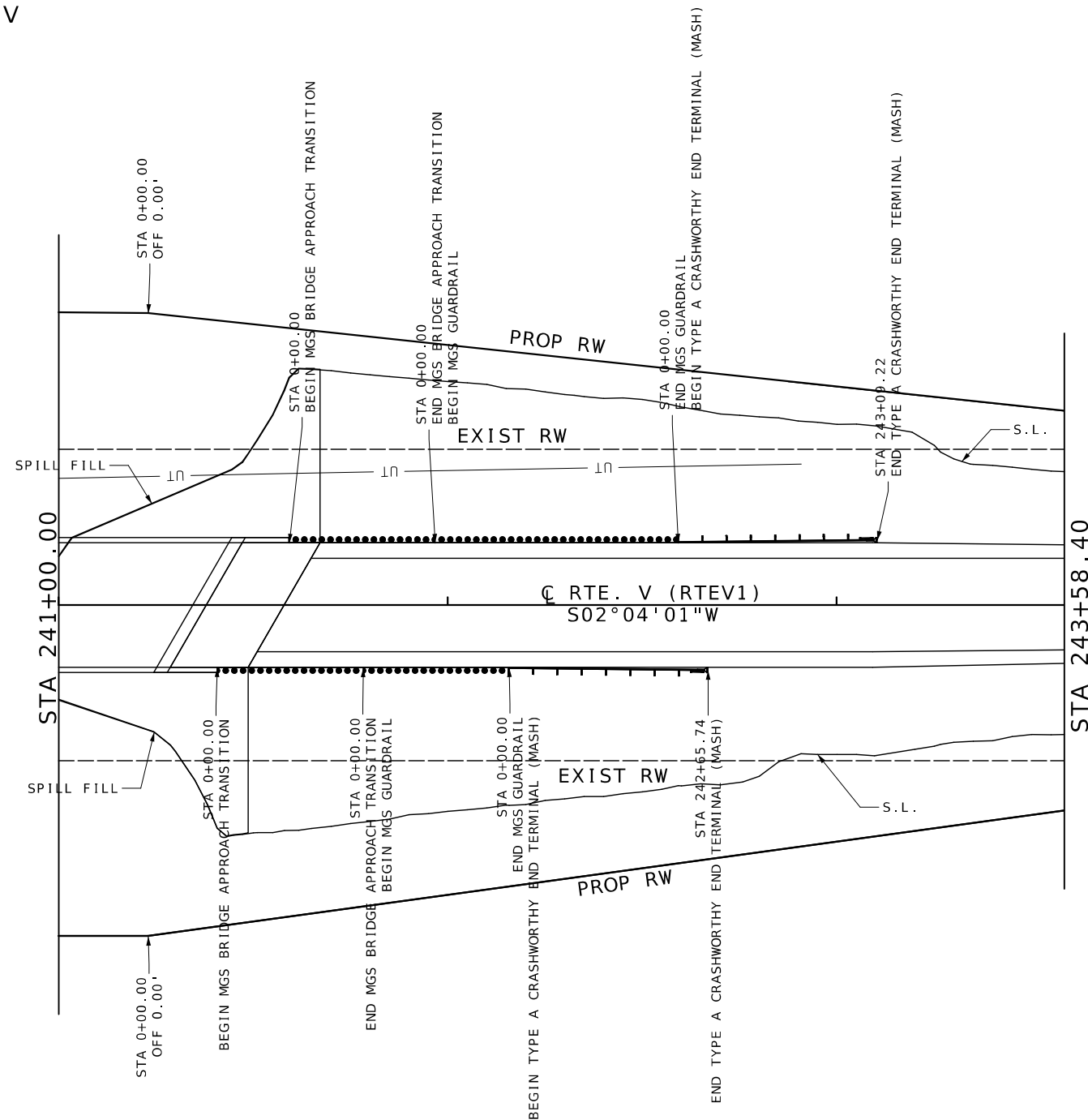
LINEAR UNIT CONVERSION

1 METER = 3.280833333 US SURVEY FEET (USFT)

COORDINATE POINT LISTING

[illegible]

COORDINATE POINT
SHEET 1 OF 1

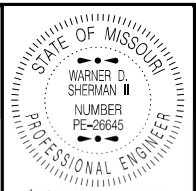
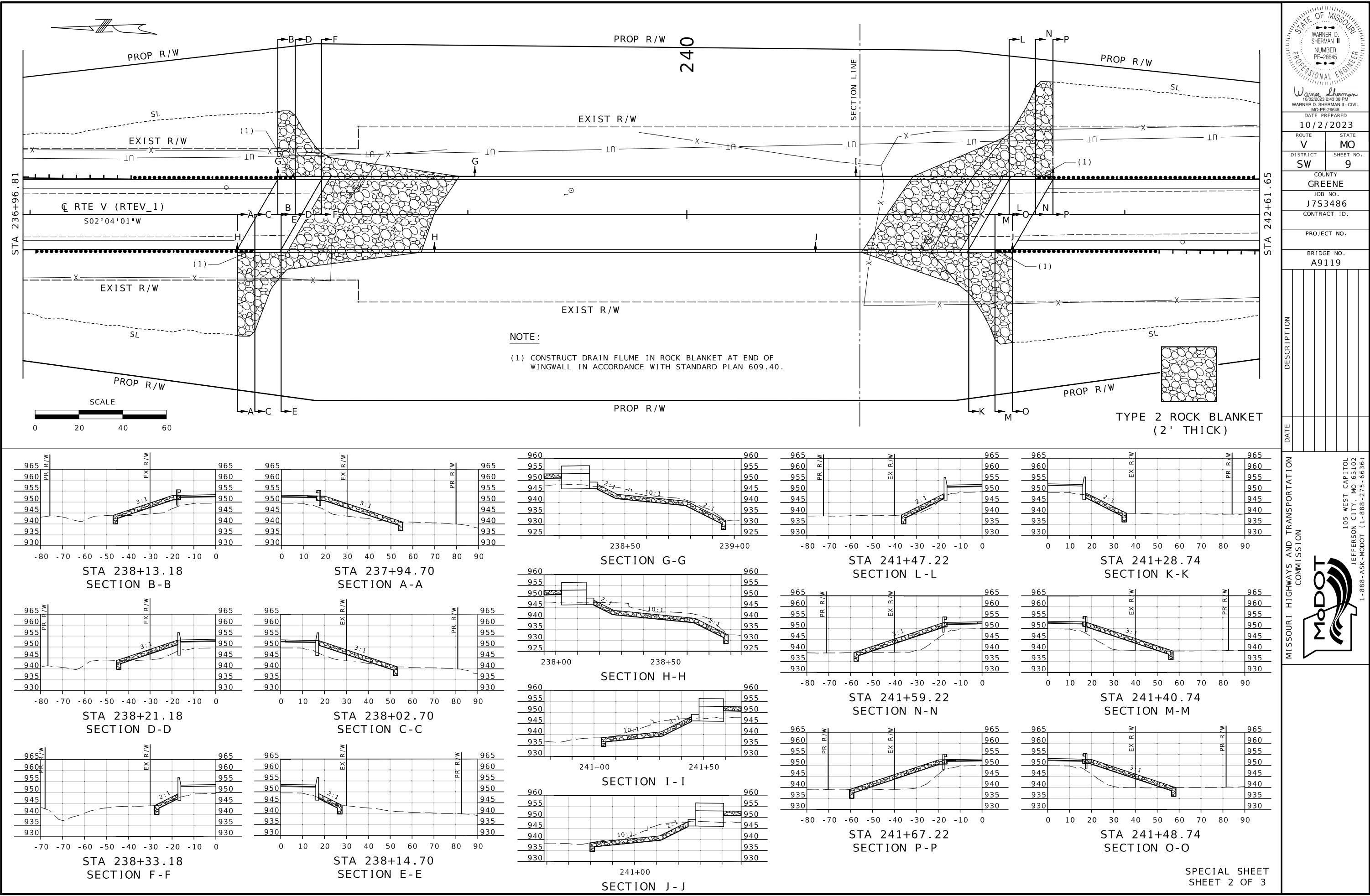


NOTE:
GUARDRAIL INSTALLATION AND GRADING SHALL BE IN ACCORDANCE WITH STANDARD PLAN 606.50 AND 606.81.
GUARDRAIL GRADING QUANTITY INCLUDED IN EARTHWORK PAY ITEMS.

SPECIAL SHEET
SHEET 1 OF 3

MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION

105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)



Warner D. Sherman
10/02/2023 2:43:08 PM
WARNER D. SHERMAN II - CIVIL
MO-PE-26645

DATE PREPARED
10/2/2023

ROUTE V STATE MO
DISTRICT SW SHEET NO. 9

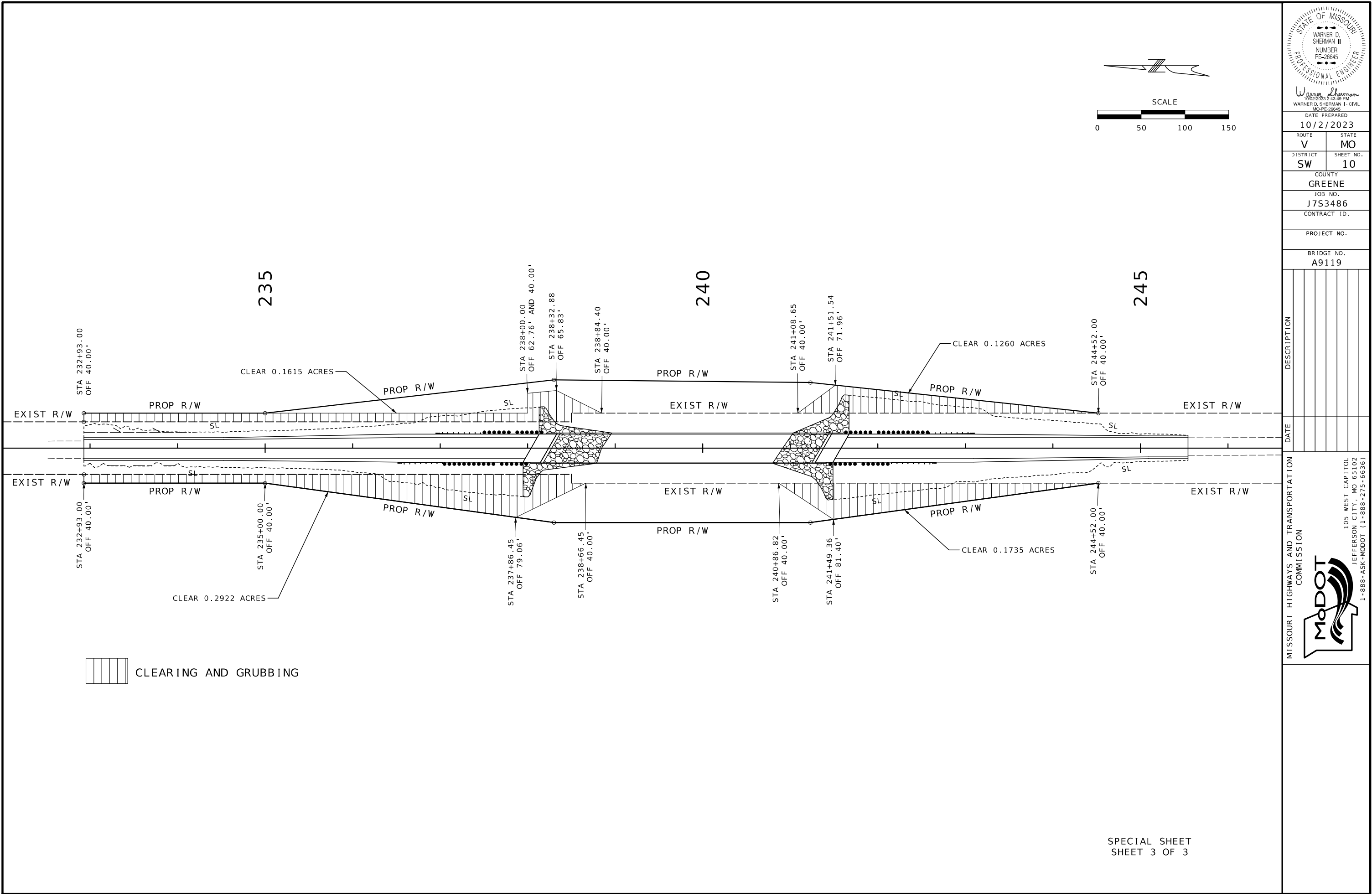
COUNTY GREENE
JOB NO. J7S3486
CONTRACT ID.

PROJECT NO.
BRIDGE NO. A9119

DESCRIPTION	DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION
105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)





SPECIAL SHEET
SHEET 3 OF 3

STATE OF MISSOURI

WARNER D. SHERMAN II

NUMBER PE-26645

PROFESSIONAL ENGINEER

Warner D. Sherman

10/02/2023 2:43:49 PM

WARNER D. SHERMAN II - CIVIL

MO-PE-26645

DATE PREPARED
10/2/2023

ROUTE V	STATE MO
DISTRICT SW	SHEET NO. 10

COUNTY
GREENE

JOB NO.
J7S3486

CONTRACT ID.

PROJECT NO.

BRIDGE NO.
A9119

DATE	DESCRIPTION

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)



WO20-3 WITH AWRS

(20)



WO20-3 WITH AWRS

(20A)



R11 - 2

(29)



R11 - 4

55

NOTE:

- (1) ROAD CLOSED SIGN MAY BE PLACED 7-10 FEET BEHIND THE BARRICADES AND AT A SIGN HEIGHT APPROPRIATE TO THE TYPE OF ROADWAY. ONE BARRICADE SHOULD BE REQUIRED TO COMPLETELY CLOSE EACH 8 FEET OF PAVEMENT. PAVED SHOULDERS SHALL BE INCLUDED IN THE AREA TO BE CLOSED.
- (2) IF USED, THE "ROAD CLOSED XX MILES AHEAD LOCAL TRAFFIC ONLY" OR "ROAD CLOSED TO THRU TRAFFIC" SIGN SHOULD BE LOCATED AT THE FIRST STATE ROUTE OR, AT THE DISCRETION OF THE ENGINEER, ANY OTHER INTERSECTION IN ADVANCE OF THE CLOSURE. ADDITIONAL BARRICADES MAY BE USED AND OFFSET TO FACILITATE ACCESS FOR WORK VEHICLES, LOCAL TRAFFIC, ETC.
- (3) LOCATION TO BE DETERMINED BY THE ENGINEER.

USE IN PLACE ALL SIGNS WHICH DO NOT CONFLICT WITH THIS PLAN. COVER AND/OR REMOVE CONFLICTING SIGNS AT NO DIRECT PAY.

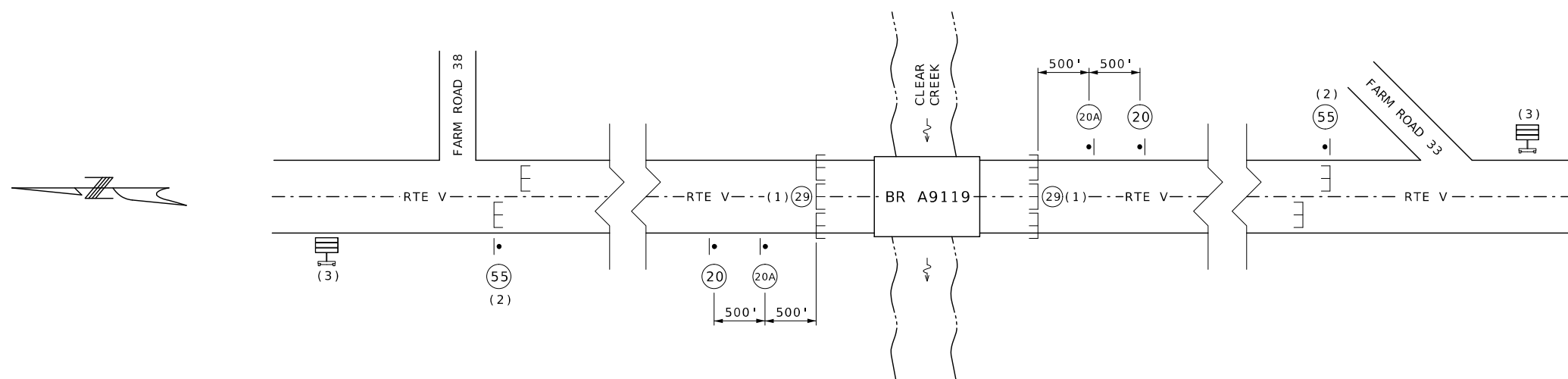
ALL R11-2 AND R11-3a SIGNS SHALL BE MOUNTED ON TYPE III BARRICADE.

TRAFFIC CONTROL SHOULD BE REMOVED AS SOON AS PRACTICAL AFTER CONDITION FOR CLOSURE NO LONGER EXISTS.

REFER TO STANDARD DRAWING 616.10, 619.10 AND 620.10 FOR ADDITIONAL DETAILS
AND 903.03 FOR SIGN AND SIGN MOUNTING REQUIREMENTS.

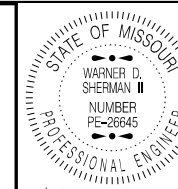
SPACING AND DISTANCES OF TRAFFIC CONTROL DEVICES ARE APPROXIMATE. THE EXACT LOCATIONS SHALL BE DETERMINED IN THE FIELD AND APPROVED BY THE ENGINEER.

TRAFFIC CONTROL SHALL CONFORM TO THE LATEST EDITION OF THE MANUAL ON UNIFORM
TRAFFIC CONTROL DEVICES (MUTCD).



TRAFFIC CONTROL LEGEND

- SIGN (SINGLE SIDED)
- E BARRICADE
-  CHANGEABLE MESSAGE BOARD (1)



Warner Sherman
10/02/2023 2:45:24 PM
WARNER D. SHERMAN II - CIV
MO-PE-26645

MO-PE-26645
DATE PREPARED
10/2/2023

ROUTE V	STATE MO
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DISTRICT	SHEET NO.
SW	11

COUNTY
GREENE

JOB NO.
J7S3486

CONTRACT ID.

PROJECT NO.

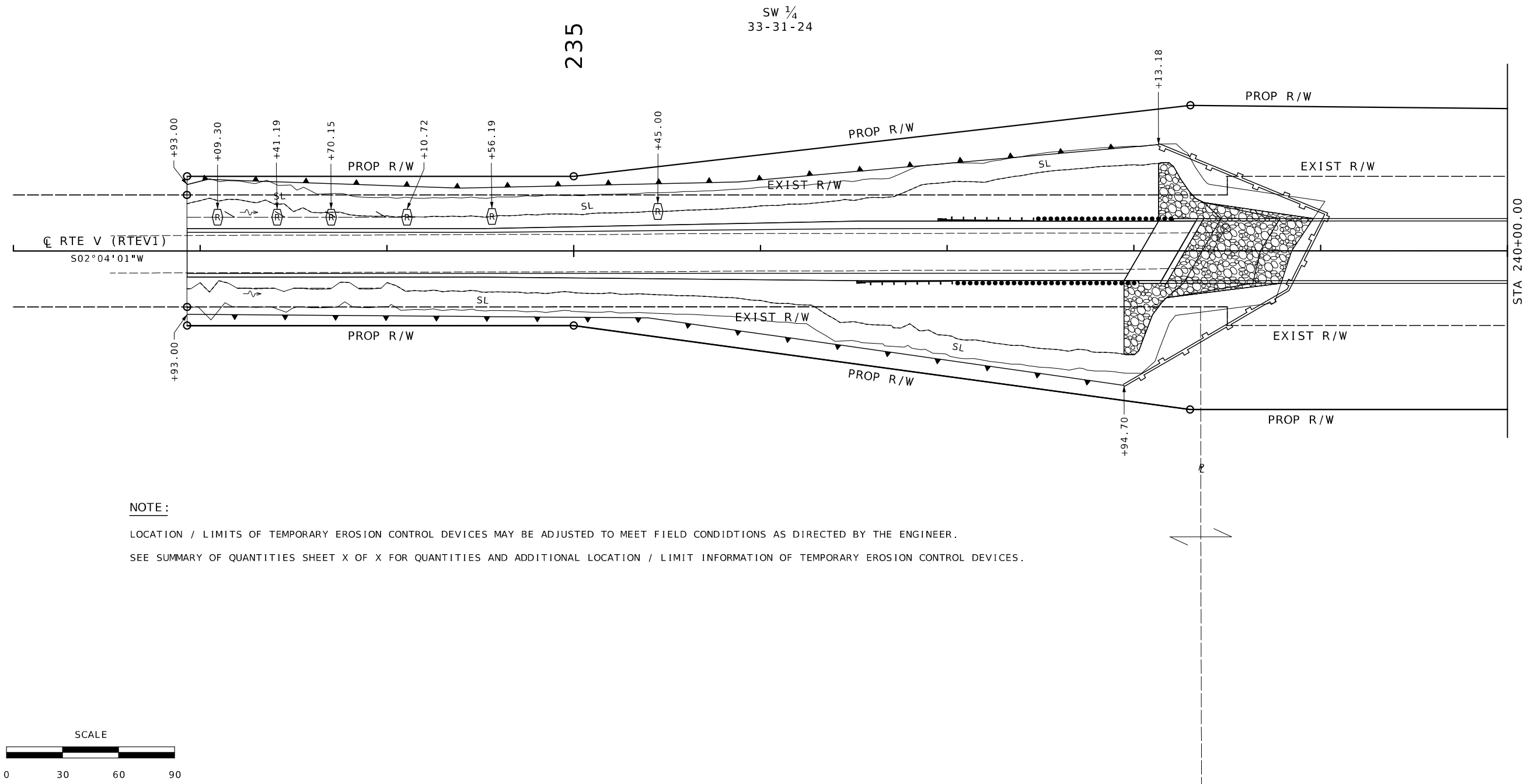
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COMMISSION

105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)



TEMPORARY EROSION CONTROL LEGEND

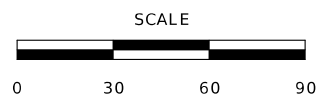
- (R) ROCK DITCH CHECK
- TEMPORARY BERM TYPE C
- SILT FENCE



NOTE:

LOCATION / LIMITS OF TEMPORARY EROSION CONTROL DEVICES MAY BE ADJUSTED TO MEET FIELD CONDIDTIONS AS DIRECTED BY THE ENGINEER.

SEE SUMMARY OF QUANTITIES SHEET X OF X FOR QUANTITIES AND ADDITIONAL LOCATION / LIMIT INFORMATION OF TEMPORARY EROSION CONTROL DEVICES.



DATE PREPARED
10/2/2023 2:47:11 PM
WARNER D. SHERMAN II - CIVIL
MO-PE-26645

ROUTE V STATE MO
DISTRICT SW SHEET NO. 12

COUNTY GREENE
JOB NO. J7S3486
CONTRACT ID.

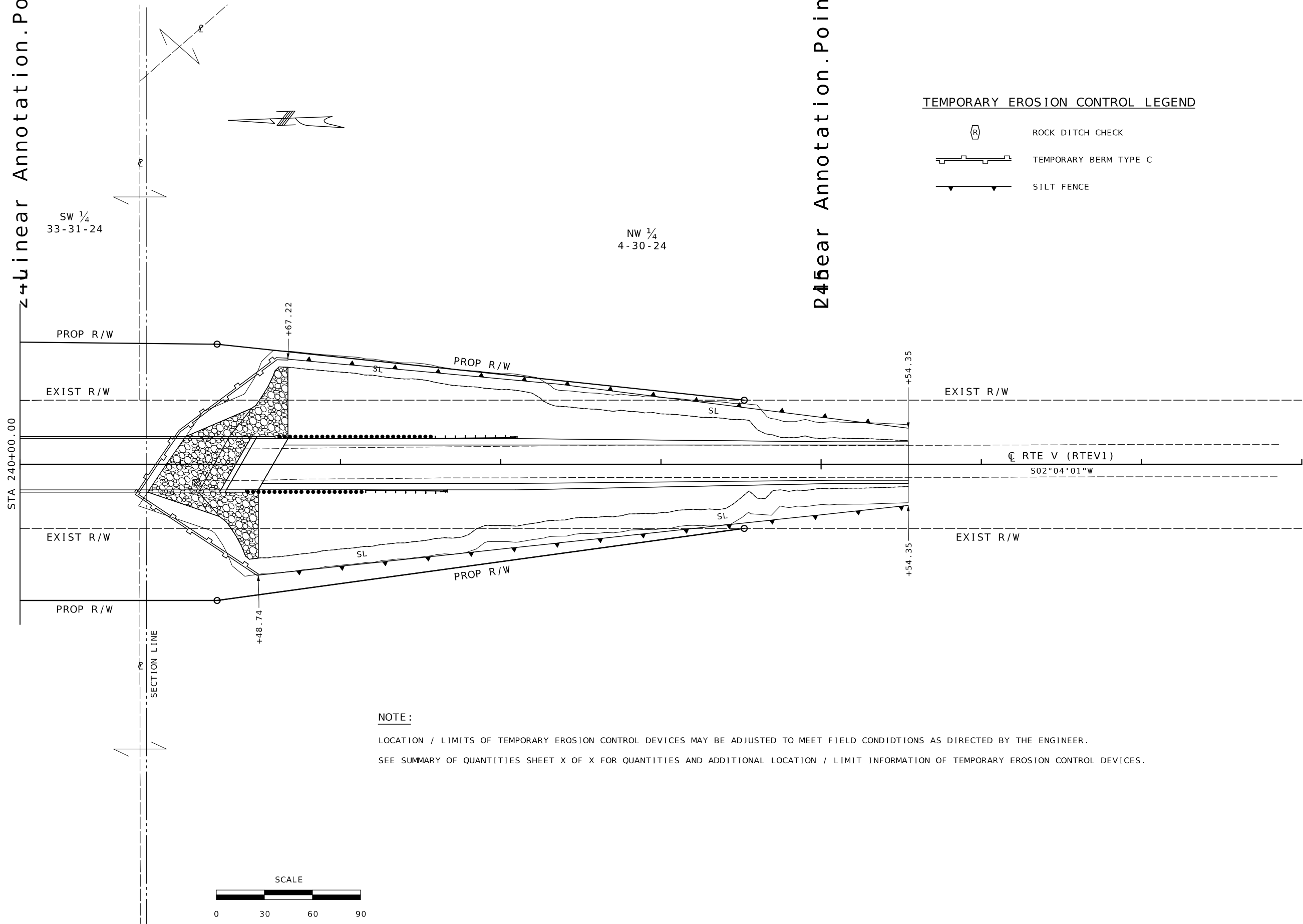
PROJECT NO.

BRIDGE NO. A9119

DATE	DESCRIPTION

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)



STATE OF MISSOURI

WARNER D. SHERMAN II

NUMBER PE-26645

PROFESSIONAL ENGINEER

Warner D. Sherman

10/02/2023 2:47:42 PM

WARNER D. SHERMAN II - CIVIL

MO-PE-26645

DATE PREPARED
10/2/2023

ROUTE V	STATE MO
DISTRICT SW	SHEET NO. 13

COUNTY
GREENE

JOB NO.
J7S3486

CONTRACT ID.

PROJECT NO.

BRIDGE NO.
A9119

DESCRIPTION	DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

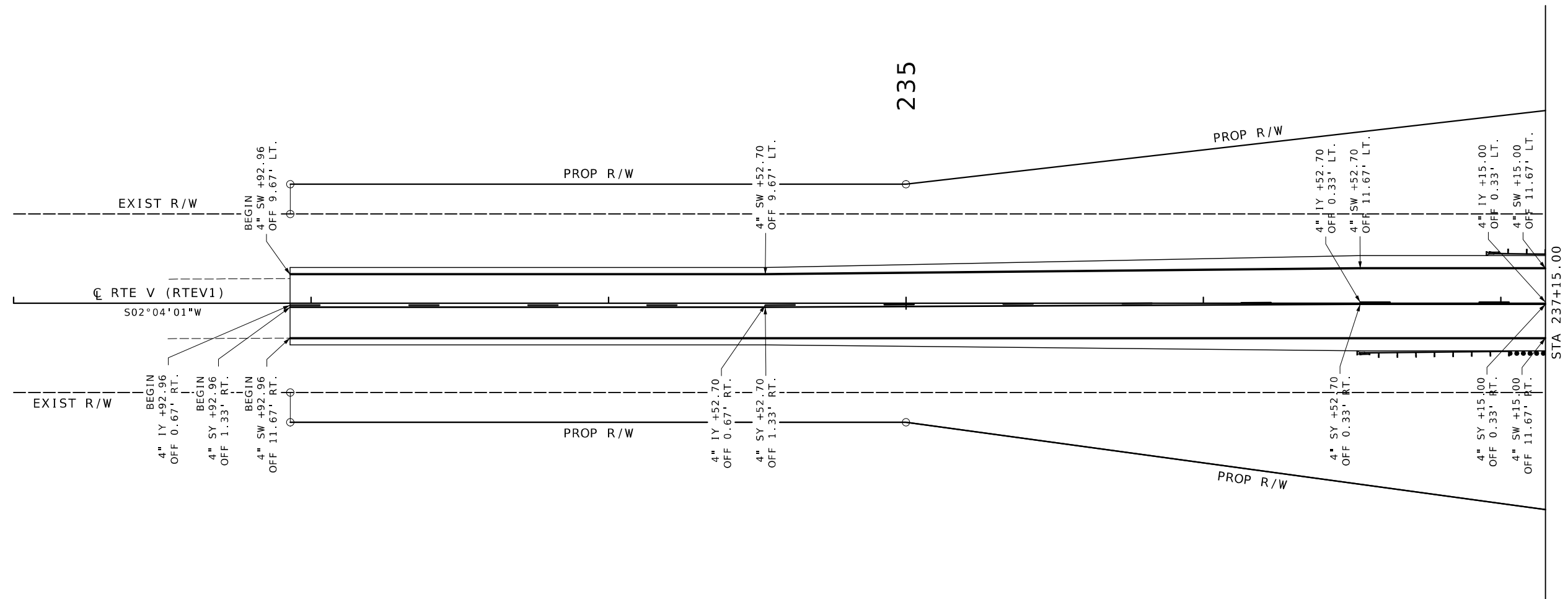
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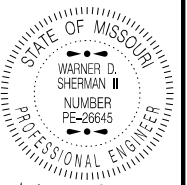
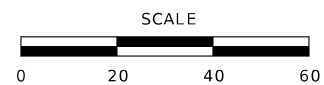
PAVEMENT MARKING LEGEND

SW - SOLID WHITE
SY - SOLID YELLOW
IY - INTERMITTENT YELLOW



NOTE:

THE PAVEMENT MARKING LAYOUT AND EXACT LOCATIONS OF STRIPING MAY BE ADJUSTED TO MEET FIELD CONDITIONS AS APPROVED BY THE ENGINEER.
PAVEMENT MARKING SHALL BE IN ACCORDANCE WITH STANDARD PLAN 620.00.



Warner D. Sherman
10/02/2023 2:48:27 PM
WARNER D. SHERMAN II - CIVIL
MO-PE-26645

DATE PREPARED
10/2/2023

ROUTE STATE
V MO

DISTRICT SHEET NO.
SW 14

COUNTY
GREENE

JOB NO.
J7S3486

CONTRACT ID.

PROJECT NO.

BRIDGE NO.
A9119

DATE	DESCRIPTION

MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION



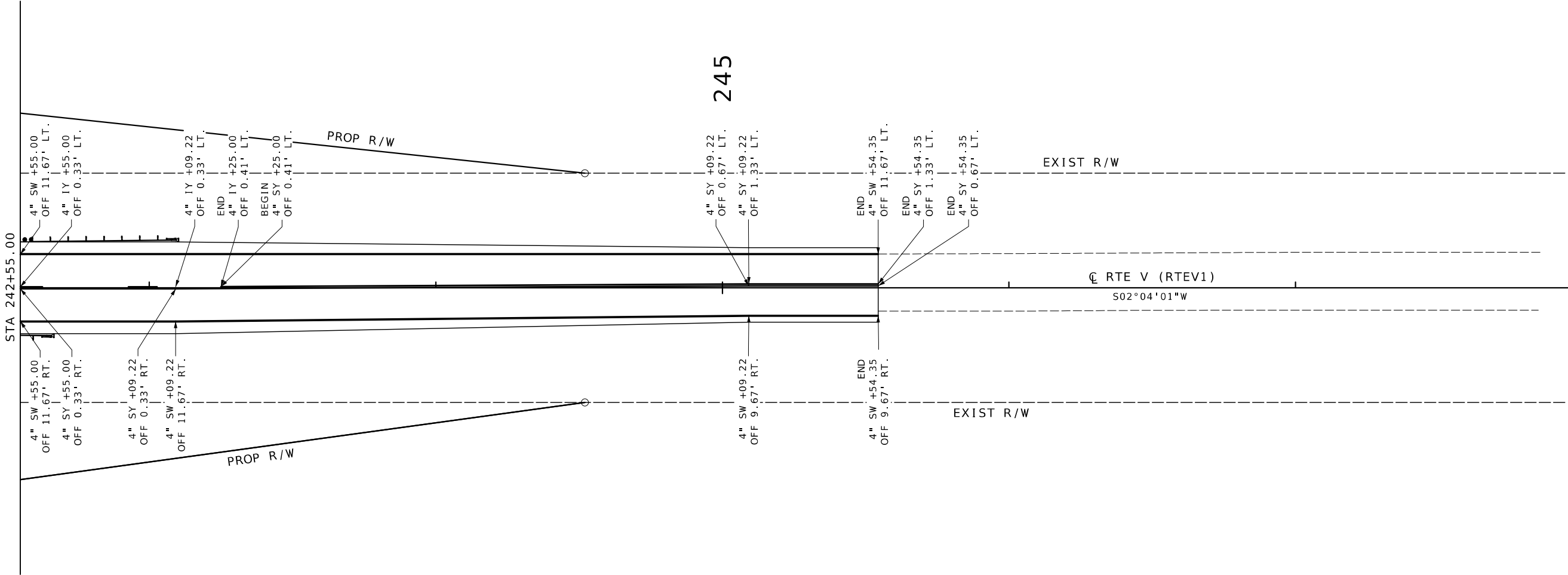
105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)





PAVEMENT MARKING LEGEND

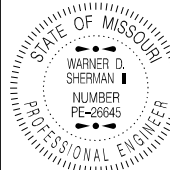
SW - SOLID WHITE
SY - SOLID YELLOW
IY - INTERMITTENT YELLOW



NOTE:

THE PAVEMENT MARKING LAYOUT AND EXACT LOCATIONS OF STRIPING MAY BE ADJUSTED TO MEET FEILD CONDITIONS AS APPROVED BY THE ENGINEER.
PAVEMENT MARKING SHALL BE IN ACCORDANCE WITH STANDARD PLAN 620.00.

SCALE



Warner D. Sherman
10/22/2023 2:49:45 PM
WARNER D. SHERMAN II - CIVIL
MO-PE-26645

DATE PREPARED
10/2/2023

ROUTE STATE
V MO

DISTRICT SHEET NO.
SW 16

COUNTY
GREENE

JOB NO.
J7S3486

CONTRACT ID.

PROJECT NO.

BRIDGE NO.
A9119

DATE	DESCRIPTION

MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION



105 WEST CAPITOL
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General Notes:

Design Specifications:

2020 AASHTO LRFD Bridge Design Specifications (9th Ed.)
2011 AASHTO Guide Specifications for LRFD Seismic Bridge Design (2nd Ed.) and 2014 Interim Revisions (Seismic Details)

Seismic Design Category = A

Design earthquake response special accelerated coefficient at 1.0 second period. SD1 = 0.123 g

Acceleration Coefficient (effective peak ground acceleration coefficient). As = 0.077 g

Design Loading:

Vehicular = HL-93
Future Wearing Surface = 35 lb/sf
Earth = 120 lb/cf
Equivalent Fluid Pressure = 45 lb/cf (Min.)
Superstructure: Simply-Suported, Non-Composite for dead load.
Continuous Composite for live load.

Design Unit Stresses:

Class B Concrete (Substructure) f'c = 3,000 psi
Class B-1 Concrete (Barrier) f'c = 4,000 psi
Class B-2 Concrete (Superstructure except Prestressed Girders and Barrier) f'c = 4,000 psi
Reinforcing Steel (Grade 60) fy = 60,000 psi
Structural Steel HP Pile (ASTM A709 Grade 50S) fy = 50,000 psi

For prestressed girder stresses, see Sheets No. 16 thru 21.
For precast prestressed panel stresses, see sheet No. 23.

Neoprene Pads:

Neoprene bearing pads shall be 60 durometer and shall be in accordance with Sec 716.

Joint Filler:

All joint filler shall be in accordance with Sec 1057 for preformed sponge rubber expansion and partition joint filler, except as noted.

Reinforcing Steel:

Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise shown.

Traffic Handling:

Structure to be closed during construction. Traffic to be maintained on other routes during construction. See Roadway plans for traffic control.

MoDOT Construction personnel will indicate the type of joint filler used under the precast panels for this structure:

- ☐ Constant Joint Filler
☐ Variable Joint Filler

Hydrologic Data
Drainage Area = 69.6 mi²
Design Flood Frequency = 50 years
Design Flood Discharge = 16,400 cfs
Design Flood (D.F.) Elevation = 947.2
Base Flood (100-year)
Base Flood Elevation = 948.0
Base Flood Discharge = 19,200 cfs
Estimated Backwater = 1.00 ft
Average Velocity thru Opening = 5.8 ft/s
Freeboard (50-year)
Freeboard = 2.0 ft
Roadway Overtopping
Overtopping Flood Discharge = 13,600 cfs
Overtopping Flood Frequency = 25 years
Overtopping Flood Elevation = 945.0

Roadway overtopping occurs near Sta. 246+50 (±).

Estimated Quantities for Slab on Concrete NU-Girder		
Item		Total
Class B-2 Concrete	cu. yard	354
Reinforcing Steel (Epoxy Coated)	pound	93,170

The table of Estimated Quantities for Slab on Concrete NU-Girder represents the quantities used by the State in preparing the cost estimate for concrete slabs. The area of the concrete slab will be measured to the nearest square yard longitudinally from end of slab to end of slab and transversely from out to out of bridge slab (or with the horizontal dimensions as shown on the plan of slab). Payment for prestressed panels, conventional forms, all concrete and epoxy coated reinforcing steel will be considered completely covered by the contract unit price for the slab. Variations may be encountered in the estimated quantities but the variations cannot be used for an adjustment in the contract unit price.

Method of forming the slab shall be as shown on the plans and in accordance with Sec 703. All hardware for forming the slab to be left in place as a permanent part of the structure shall be coated in accordance with ASTM A123 or ASTM B633 with a thickness class SC 4 and a finish type I, II or III.

The Estimated Quantities for Slab on Concrete NU-Girder are based on skewed precast prestressed end panels.

Class B-2 Concrete quantity is based on minimum top flange thickness and minimum joint material thickness.

The prestressed panel quantities are not included in the table of Estimated Quantities for Slab on Concrete NU-Girder.

Estimated Quantities				
Item		Substr.	Superstr.	Total
Class 1 Excavation	cu. yard	100		100
Removal of Bridges (T0493)	lump sum			1
Bridge Approach Slab (Minor)	sq. yard		144	144
Drilled Shafts (4 ft. 0 in. Dia.)	linear foot	133.5		133.5
Rock Sockets (3 ft. 6 in. Dia.)	linear foot	99.0		99.0
Video Camera Inspection	each	9		9
Foundation Inspection Holes	linear foot	189.0		189.0
Sonic Logging Testing	each	9		9
Galvanized Structural Steel Piles (12 in.)	linear foot	288		288
Pre-Bore For Piling	linear foot	252		252
Pile Point Reinforcement	each	12		12
Class B Concrete (Substructure)	cu. yard	157.1		157.1
Type D Barrier	linear foot		676	676
Slab on Concrete NU-Girder	sq. yard		1205	1205
NU 35, Prestressed Concrete NU-Girder	linear foot		1236	1236
Reinforcing Steel (Bridges)	pound	45,790		45,790
Vertical Drain at End Bents	each			2
Plain Neoprene Bearing Pad	each		8	8
Laminated Neoprene Bearing Pad	each		24	24

All concrete above the construction joint in the end bents is included in the Estimated Quantities for Slab on Concrete NU-Girder.

All reinforcement in the end bents is included in the Estimated Quantities for Slab on Concrete NU-Girder.

All reinforcement in the intermediate bent concrete diaphragms except reinforcement embedded in the beam cap is included in the Estimated Quantities for Slab on Concrete NU-Girder.

All concrete above the intermediate beam cap is included in the Estimated Quantities for Slab on Concrete NU-Girder.

Cost of L4x4 ASTM A709 Grade 36 HP pile anchors and 3/4-inch diameter ASTM F3125 Grade A325 Type 1 bolts, complete in place, will be considered completely covered by the contract unit price for Galvanized Structural Steel Piles (12 in.).

Foundation Data						
Type	Design Data	Bent Number				
		1	2	3	4	5
Load Bearing Pile	Pile Type and Size	HP 12x53	--	--	--	HP 12x53
	Number	ea 6	--	--	--	6
	Approximate Length Per Each	ft 24	--	--	--	24
	Pile Point Reinforcement	ea All	--	--	--	All
	Min. Galvanized Penetration (Elev.)	ft Full Length	--	--	--	Full Length
	Pile Driving Verification Method	DF	--	--	--	DF
	Resistance Factor	0.4	--	--	--	0.4
	Minimum Nominal Axial Compressive Resistance	kip 387	--	--	--	444
Rock Socket	Number	ea --	3	3	3	--
	Foundation Material	--	Rock	Rock	Rock	--
	Elevation Range	ft --	927.5-899.1	922.0-894.9	917.0-894.2	--
	Minimum Nominal Axial Compressive Resistance (Side Resistance)	ksf --	16.1	17.7	18.7	--
	Minimum Nominal Axial Compressive Resistance (Tip Resistance)	ksf --	0.0	0.0	0.0	--

Load Bearing Pile:

DF = FHWA-modified Gates Dynamic Pile Formula

Minimum Nominal Axial Compressive Resistance = $\frac{\text{Maximum Factored Loads}}{\text{Resistance Factor}}$

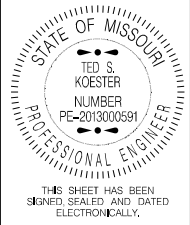
All piles shall be galvanized down to the minimum galvanized penetration (elevation).

Pile point reinforcement need not be galvanized. Shop drawings will not be required for pile point reinforcement.

The contractor shall make every effort to achieve the minimum galvanized penetration (elevation) shown on the plans for all piles. Deviations in penetration less than 5 feet of the minimum will be considered acceptable provided the contractor makes the necessary corrections to ensure the minimum penetration is achieved on subsequent piles.

Prebore for piles at Bents No. 1 & 5 to elevation 925.00.

Rock Socket (Drilled Shaft): Minimum Nominal Axial Compressive Resistance = $\frac{\text{Maximum Factored Loads}}{\text{Resistance Factors}}$ (Side Resistance + Tip Resistance)



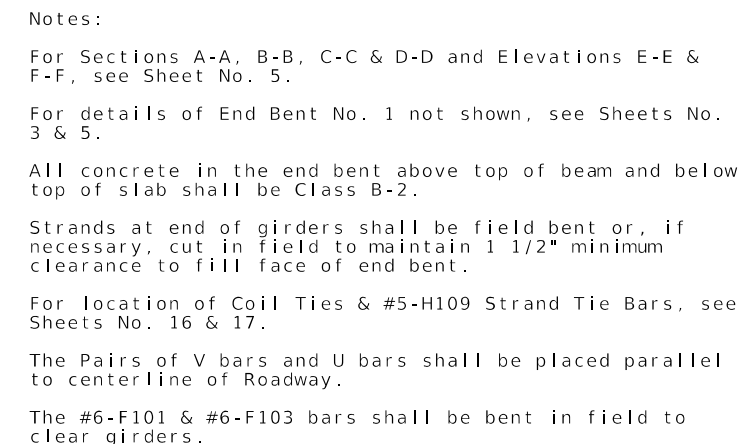
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10/30/2023	
ROUTE	STATE
V	MO
DISTRICT	SHEET NO.
BR	2
COUNTY	
GREENE	
JOB NO.	
J7S3486	
CONTRACT ID.	
PROJECT NO.	
BRIDGE NO.	
A9119	

DESCRIPTION	DATE					

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION



105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)

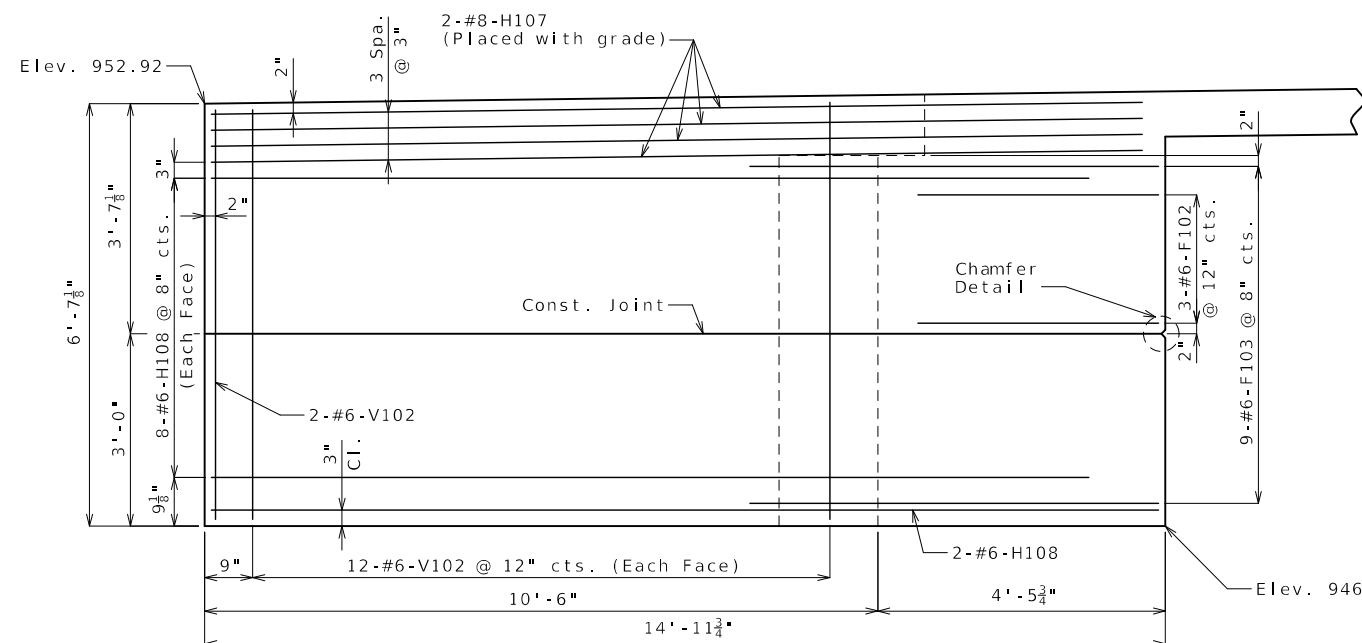


MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

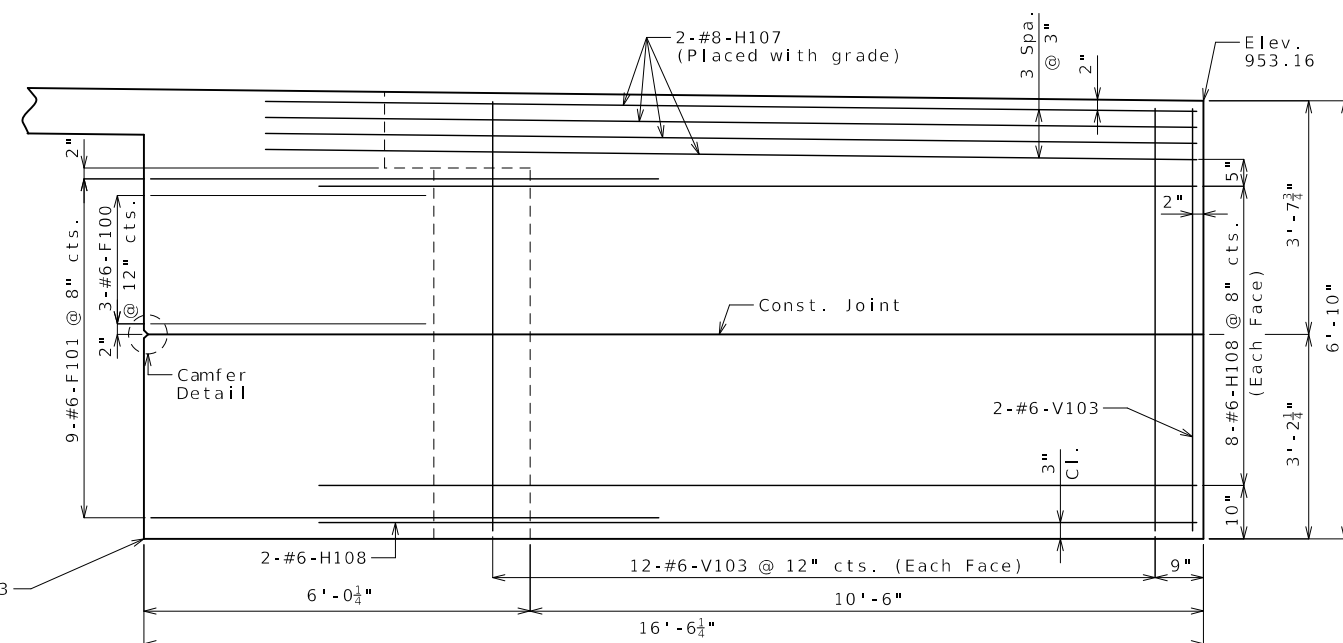
MoDOT

105 WEST CAPITOL
JEFFERSON CITY, MO 65102

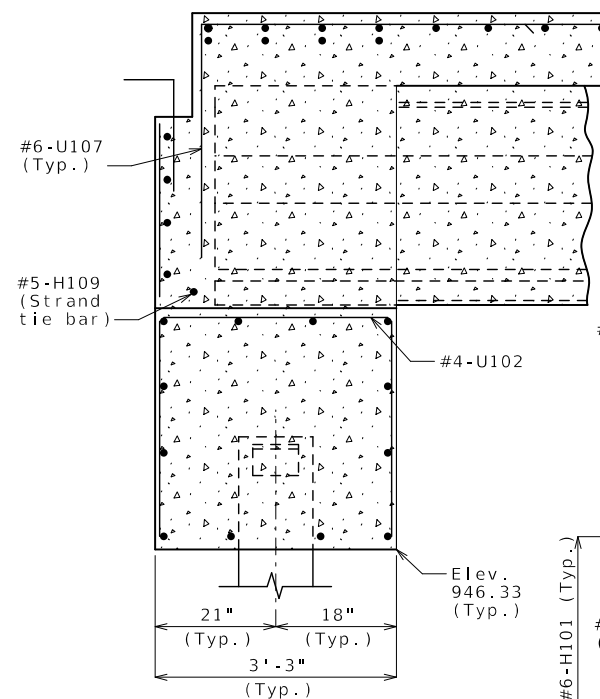




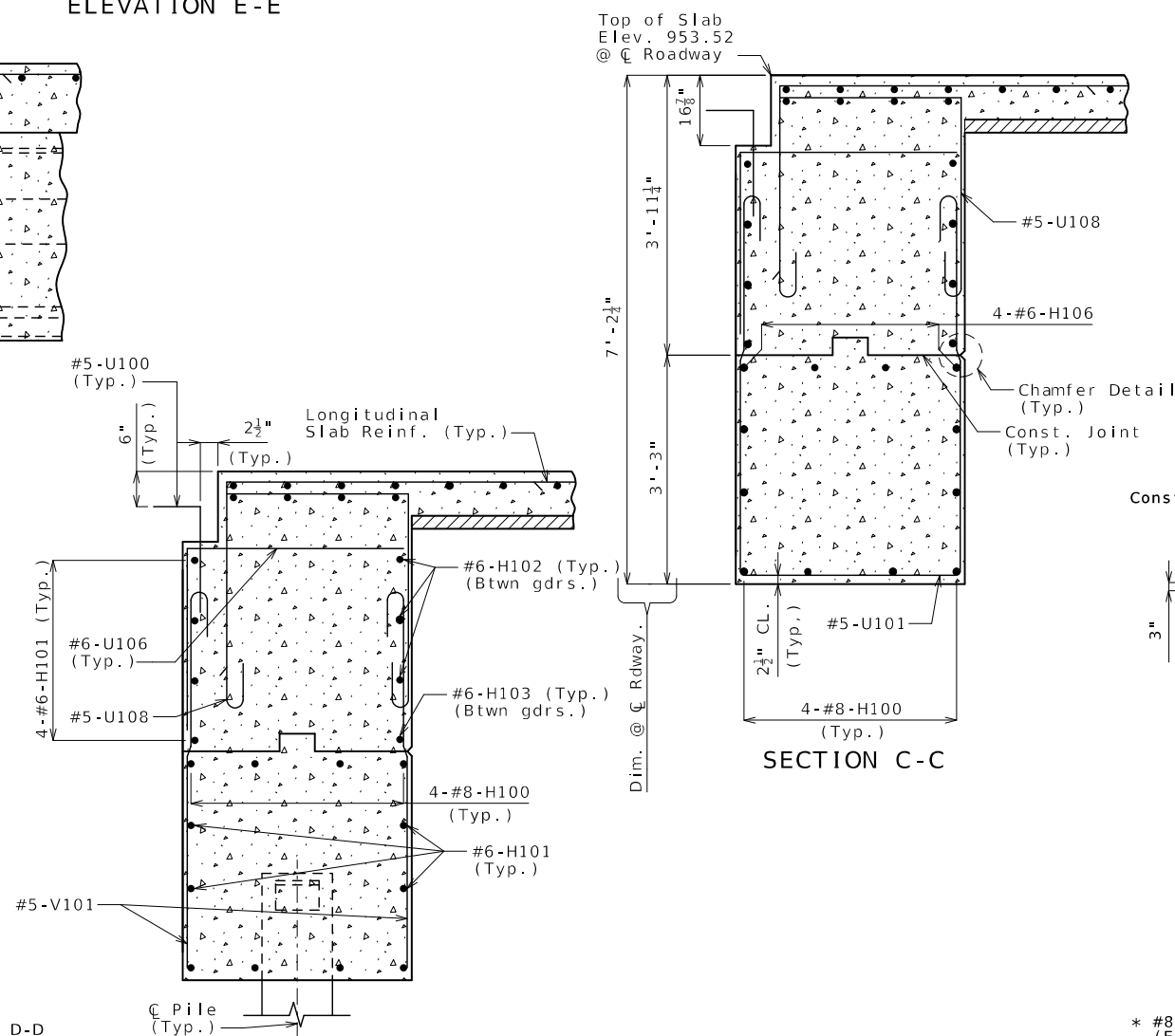
ELEVATION E-E



ELEVATION F-F

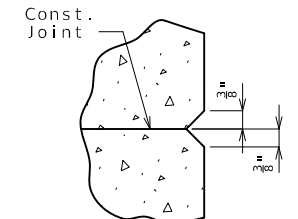


SECTION A-A

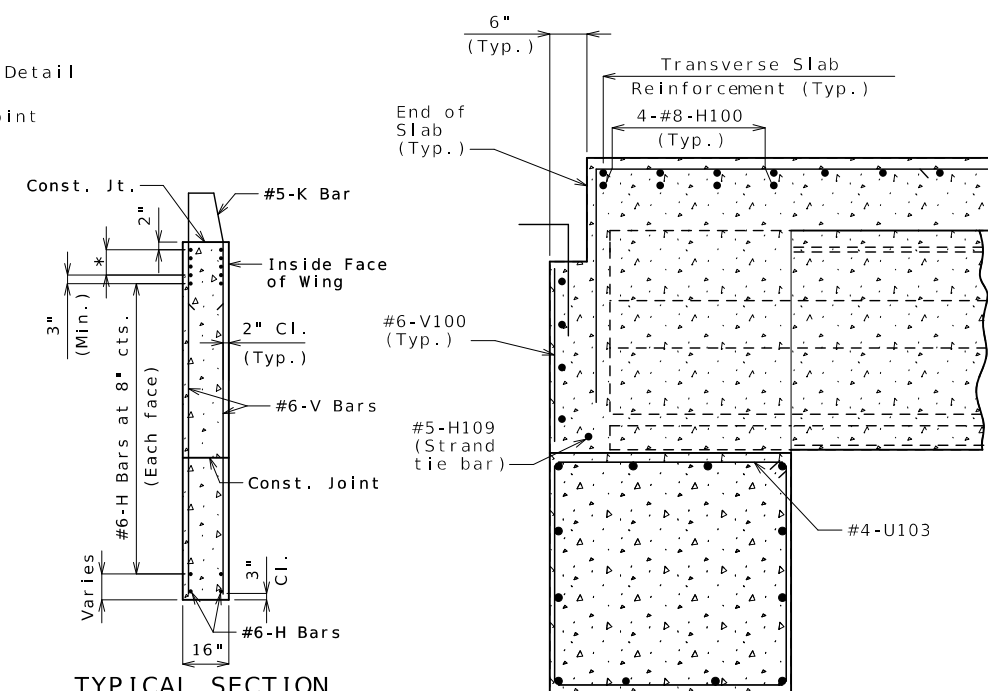


SECTION B-B

SECTION C-C



CHAMFER DETAIL



TYPICAL SECTION THRU WING

SECTION D-D

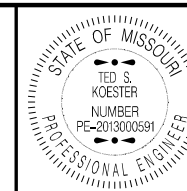
Notes:
Work this sheet with Sheets No. 3 & 4.
For Locations of Sections A-A, B-B, C-C, & D-D and Elevations E-E & F-F see Sheet No. 4.

DETAILS OF END BENT NO. 1

Detailed Sep. 2023
Checked Sep. 2023

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 5 of 35



THIS SHEET HAS BEEN
SIGNED, SEALED AND DATED
ELECTRONICALLY.

DATE PREPARED
10/30/2023

ROUTE V STATE MO

DISTRICT BR SHEET NO. 5

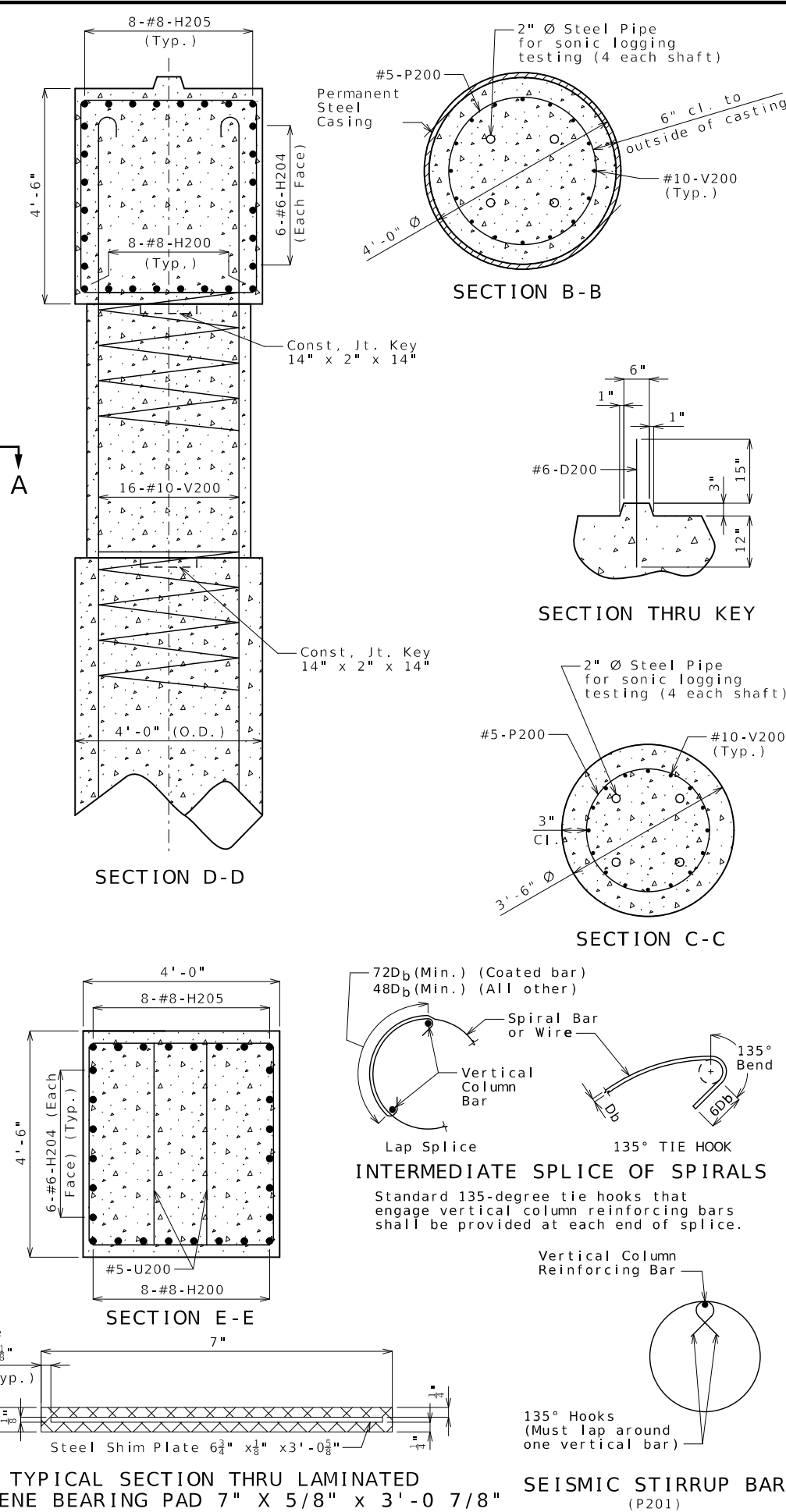
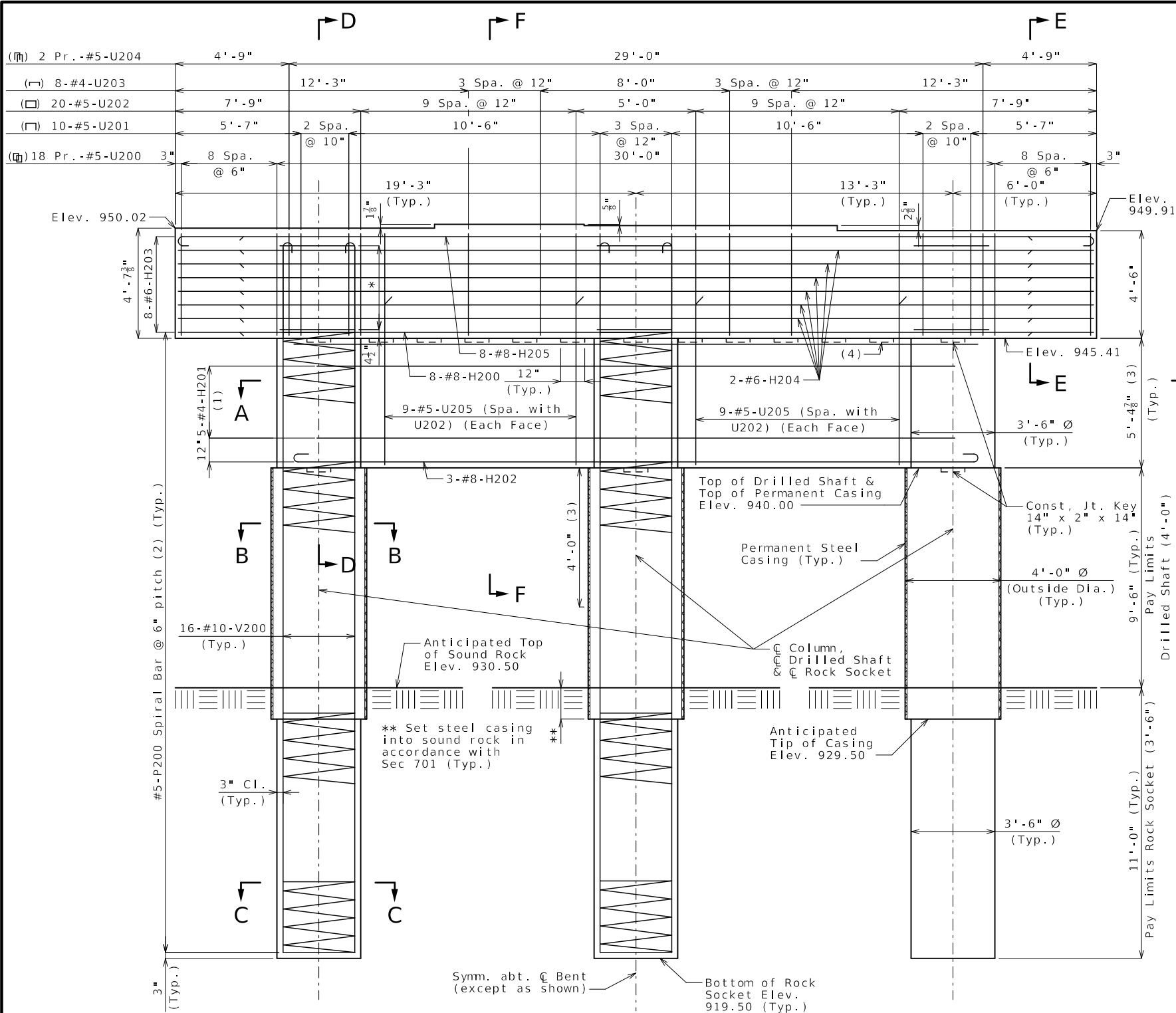
COUNTY GREENE

JOB NO. J7S3486

CONTRACT ID.

PROJECT NO.

BRIDGE NO. A9119



Notes:

Sonic logging shall be performed on all drilled shafts & rock sockets.

All reinforcement in drilled shafts & rock sockets is included in the Substructure Quantities.

At the contractor's option, the hooks of vertical bars embedded in the beam cap may be oriented inward or outward.

For steps 2 inches or more, use 2 1/4 x 1/2 inch joint filler up vertical face.

Work this sheet with Sheet No. 8.

DETAILS OF INTERMEDIATE BENT NO. 2

TYPICAL SECTION THRU LAMINATED NEOPRENE BEARING PAD 7" X 5/8" X 3'-0 7/8"

SEISMIC STIRRUP BAR (P201)

Notes:

* 15-#4-P201 @ 3" cts. (Typ.)

(1) Spa. @ abt. 12" (Each Face)

(2) Extend top of spiral bar 1 1/2" into beam.

(3) Lapping of spirals will not be permitted in these regions. If lapping is unavoidable, a mechanical bar splice will be required. No additional payment will be made for any mechanical connections used.

(4) Const. Jt. Key 12" x 2" x 7" @ abt. 2'-0" cts. (Between Columns) (Typ.)

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 7 of 35

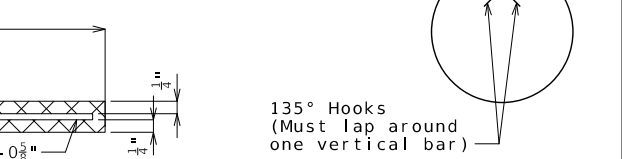
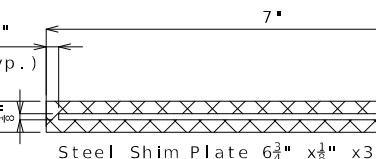
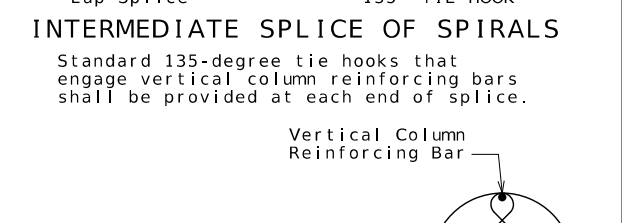
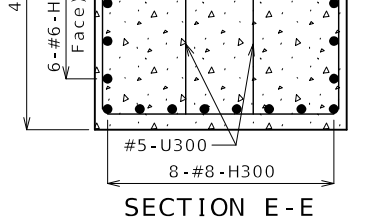
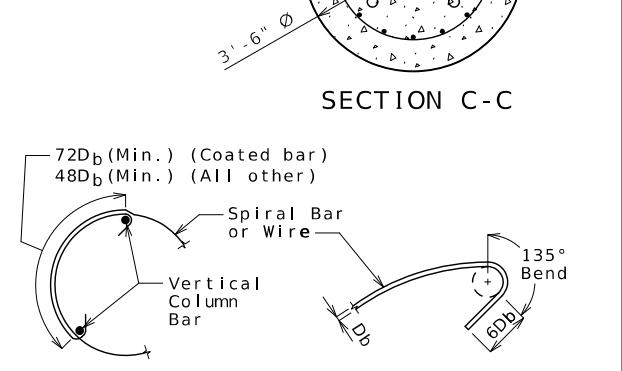
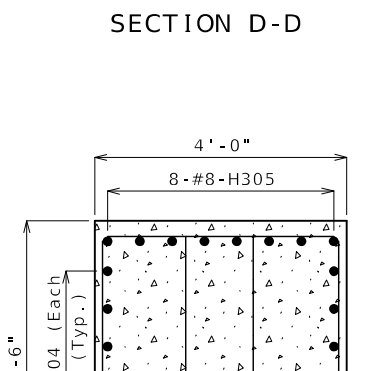
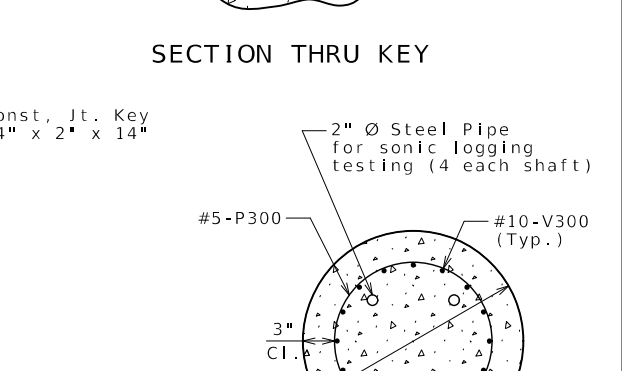
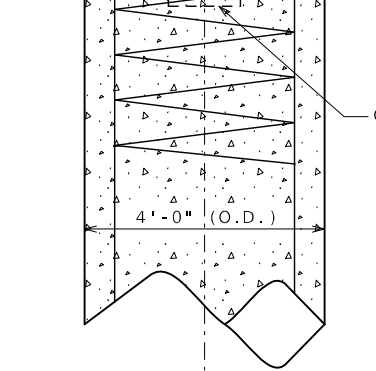
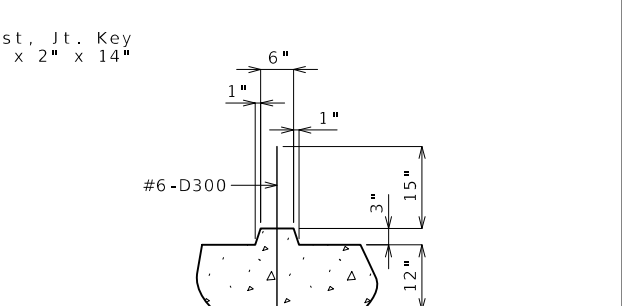
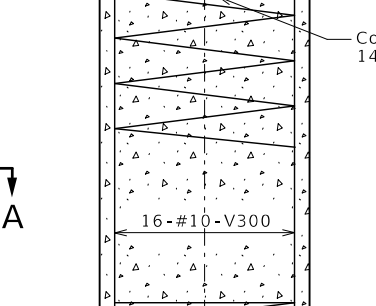
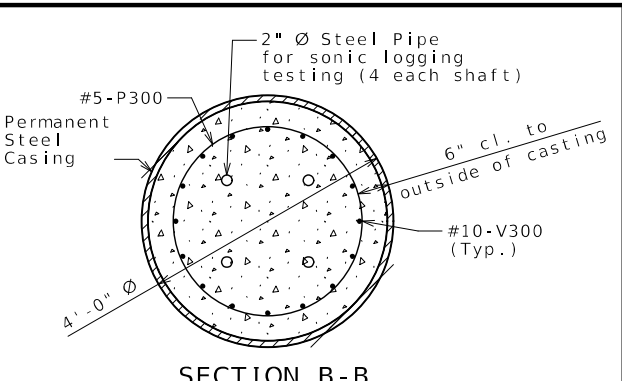
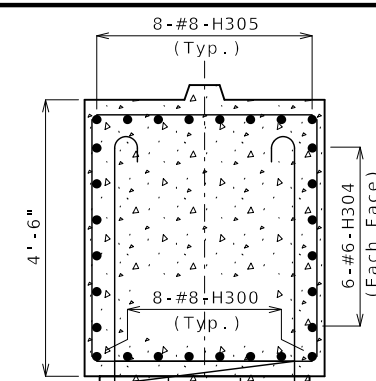
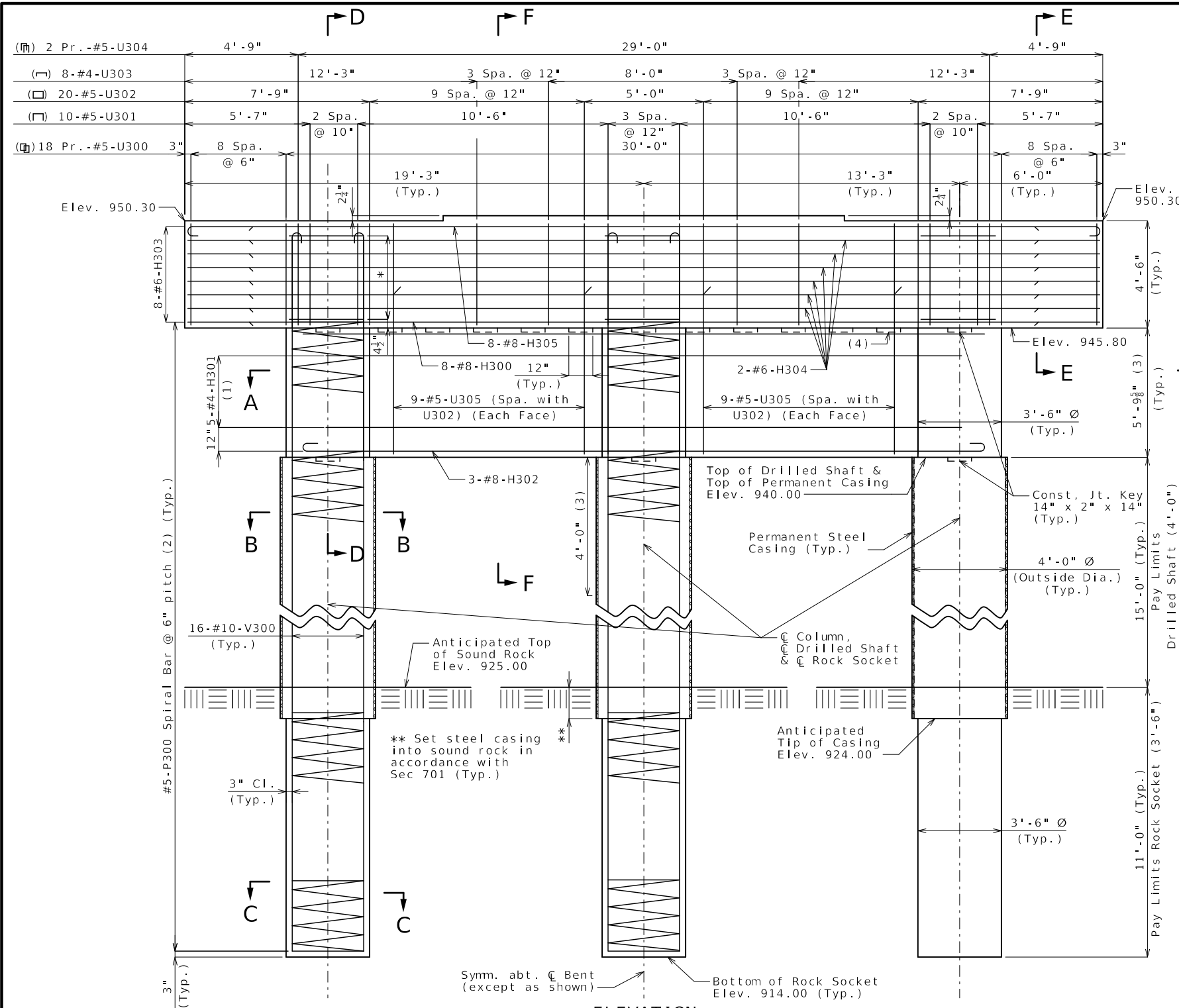
STATE OF MISSOURI
TED S. KOESTER
NUMBER
PE-2013000591
PROFESSIONAL ENGINEER
THIS SHEET HAS BEEN
SIGNED, SEALED AND DATED
ELECTRONICALLY.

DATE PREPARED
10/30/2023
ROUTE
V
STATE
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DISTRICT
BR
SHEET NO.
7
COUNTY
GREENE
JOB NO.
J753486
CONTRACT ID.
PROJECT NO.
BRIDGE NO.
A9119

DESCRIPTION

DATE

MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION
105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-273-6636)



Notes:

Sonic logging shall be performed on all drilled shafts & rock sockets.

All reinforcement in drilled shafts & rock sockets is included in the Substructure Quantities.

At the contractor's option, the hooks of vertical bars embedded in the beam cap may be oriented inward or outward.

For steps 2 inches or more, use 2 1/4 x 1/2 inch joint filler up vertical face.

Work this sheet with Sheet No. 10.

ELEVATION

* 15-#4-P301 @ 3" cts. (Typ.)

(1) Spa. @ abt. 12" (Each Face)

(2) Extend top of spiral bar 1 1/2" into beam.

(3) Lapping of spirals will not be permitted in these regions. If lapping is unavoidable, a mechanical bar splice will be required. No additional payment will be made for any mechanical connections used.

(4) Const. Jt. Key 12" x 2" x 7" @ abt. 2'-0" cts. (Between Columns) (Typ.)

DETAILS OF INTERMEDIATE BENT NO. 3

Note: This drawing is not to scale. Follow dimensions. Sheet No. 9 of 35

Detailed Sep. 2023
Checked Sep. 2023

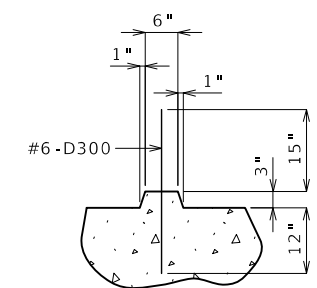
STATE OF MISSOURI
TED S. KOESTER
NUMBER
PE-2013000591
PROFESSIONAL ENGINEER

DATE PREPARED 10/30/2023	
ROUTE V	STATE MO
DISTRICT BR	SHEET NO. 9
COUNTY GREENE	
JOB NO. J7S3486	
CONTRACT ID.	
PROJECT NO.	
BRIDGE NO. A9119	
DESCRIPTION	
DATE	

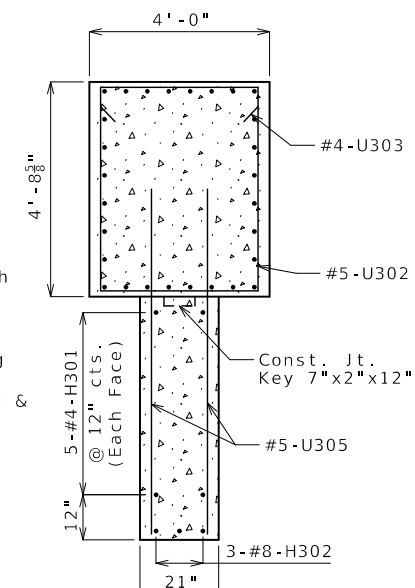
MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

MoDOT

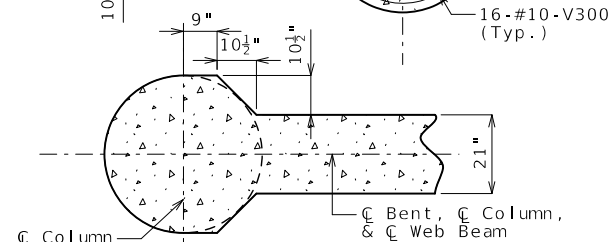
105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)



— ☐ Key,
☐ Column,
☐ Drilled Shaft
& ☐ Rock Socket



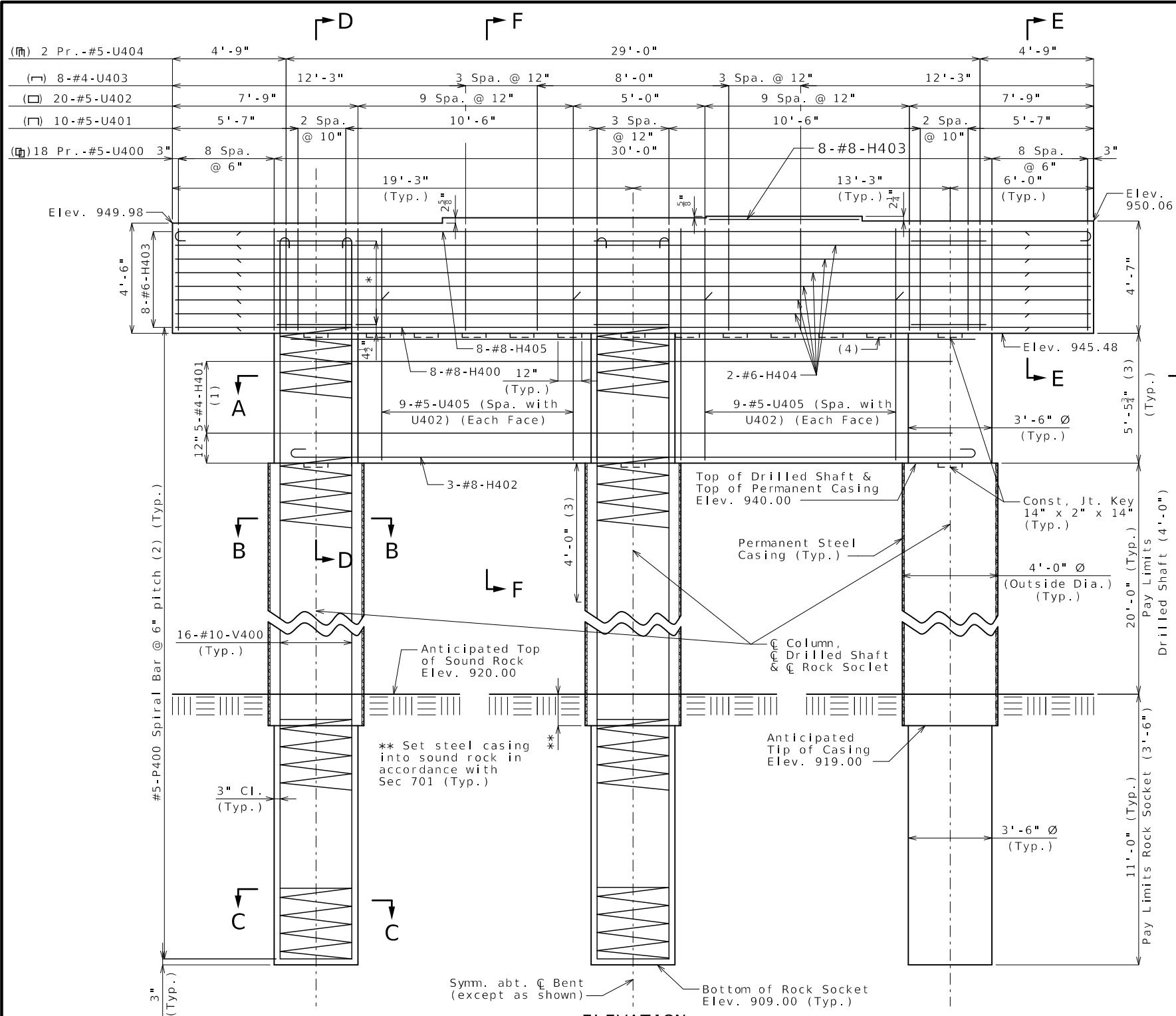
PLAN SHOWING REINFORCEMENT



OPTIONAL PART SECTION A-A

Substructure Quantity Table for Bent No. 3		
Item		Quantity
Drilled Shafts (4 ft. 0 in. Dia.)	linear foot	45.0
Rock Sockets (3 ft. 6 in. Dia.)	linear foot	33.0
Video Camera Inspection	each	3
Foundation Inspection Holes	linear foot	63.0
Sonic Logging Testing	each	3
Class B Concrete (Substructure)	cu. yard	39.9
Reinforcing Steel (Bridges)	pound	13,800

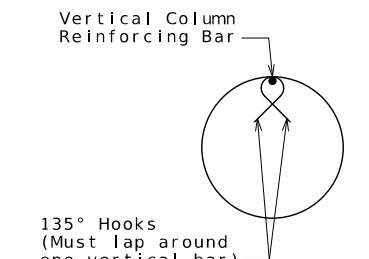
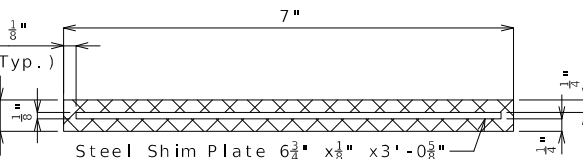
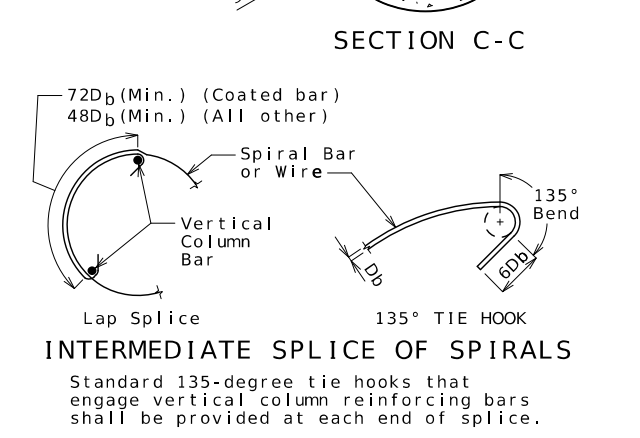
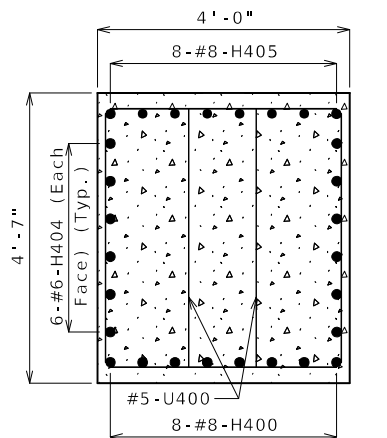
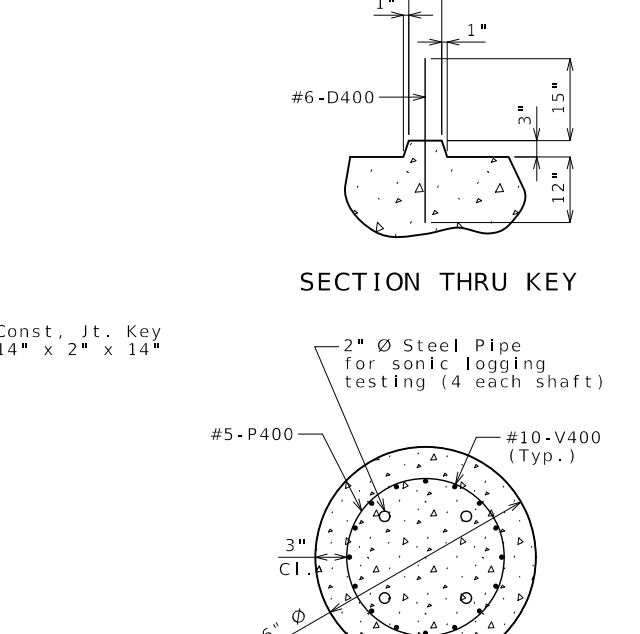
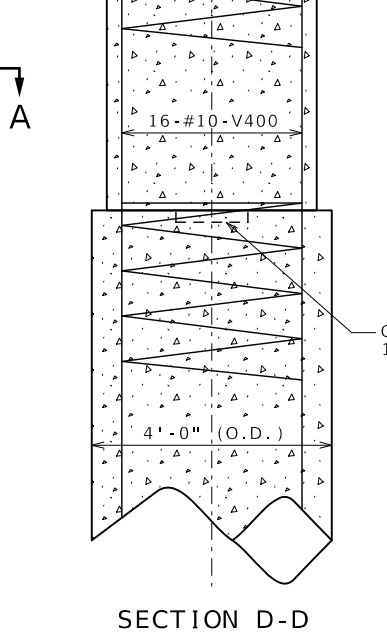
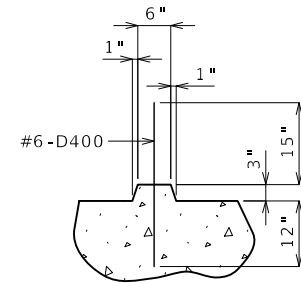
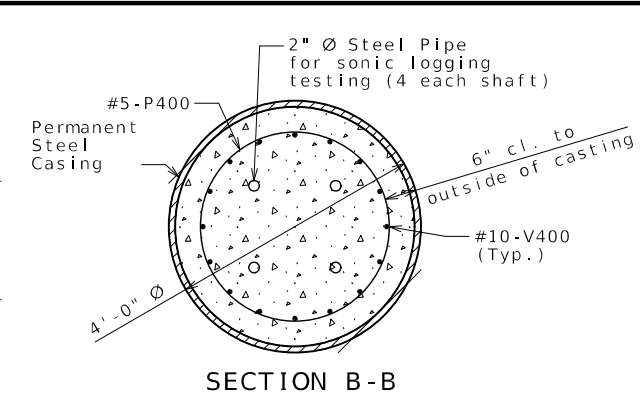
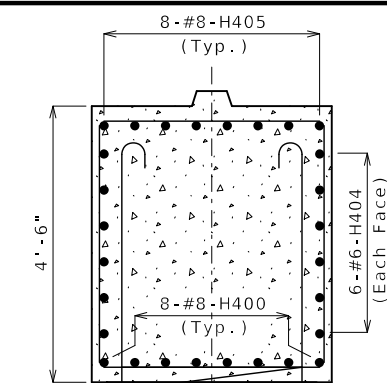
The tip of casing shall not extend into the rock socket elevation range reported in the Foundation Data table without approval by the engineer.



DETAILS OF INTERMEDIATE BENT NO. 4

Note: This drawing is not to scale. Follow dimensions.

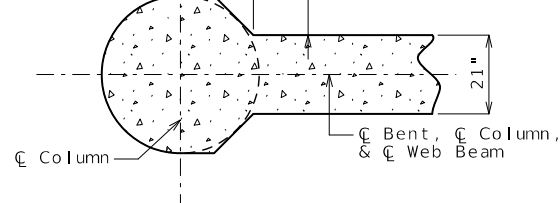
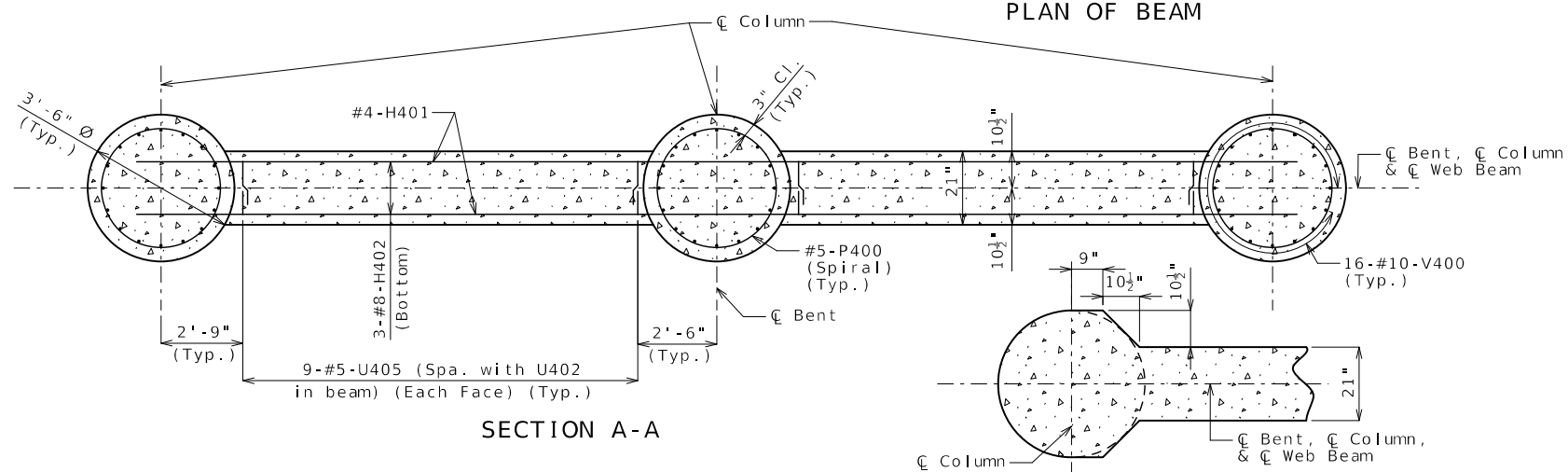
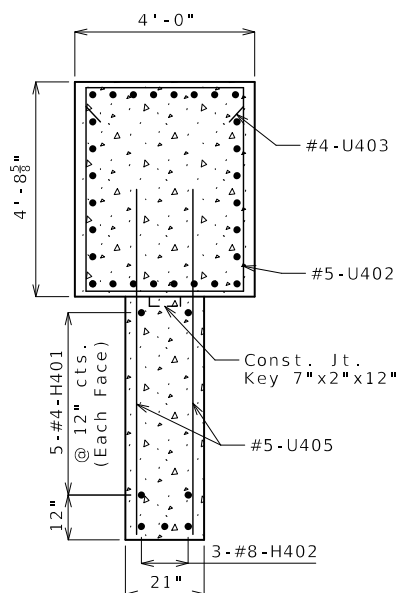
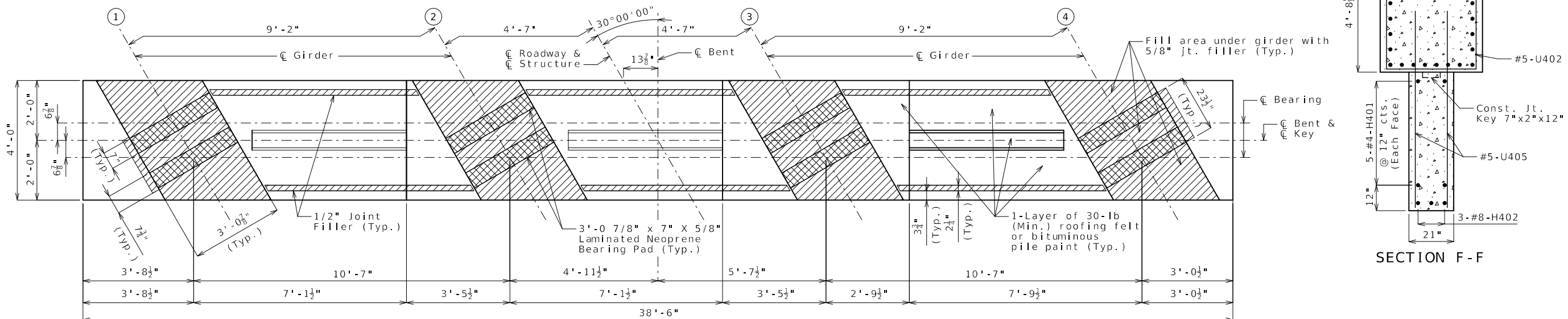
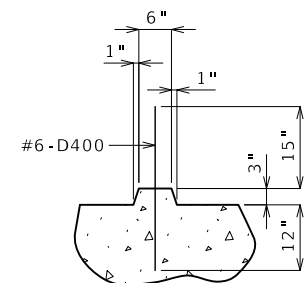
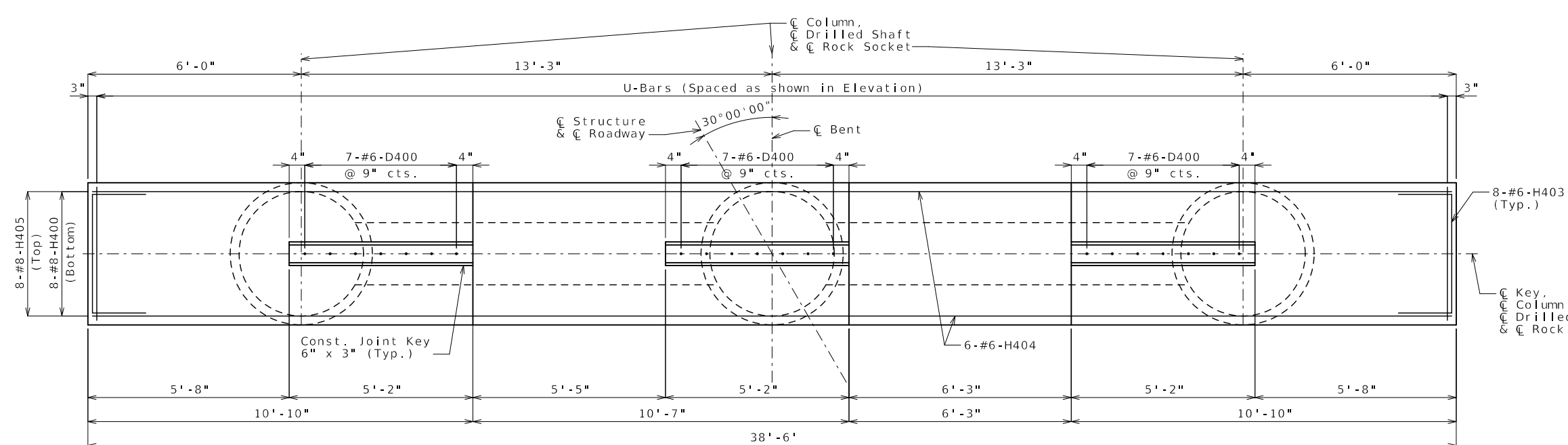
- * 15-#4-P401 @ 3" cts. (Typ.)
- (1) Spa. @ abt. 12" (Each Face)
- (2) Extend top of spiral bar 1 1/2" into beam.
- (3) Lapping of spirals will not be permitted in these regions. If lapping is unavoidable, a mechanical bar splice will be required. No additional payment will be made for any mechanical connections used.
- (4) Const. Jt. Key 12" x 2" x 7" @ abt. 2'-0" cts. (Between Columns) (Typ.)



Detailed Sep. 2023
Checked Sep. 2023

Sheet No. 11 of 35

STATE OF MISSOURI TED S. KOESTER PROFESSIONAL ENGINEER NUMBER PE-2013000591 THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY.	
DATE PREPARED 10/30/2023	
ROUTE V	STATE MO
DISTRICT BR	SHEET NO. 11
COUNTY GREENE	
JOB NO. J7S3486	
CONTRACT ID.	
PROJECT NO.	
BRIDGE NO. A9119	
DESCRIPTION	DATE
MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION	
105 WEST CAPITOL JEFFERSON CITY, MO 65102 1-888-ASK-MODOT (1-888-273-6636)	



Substructure Quantity Table for Bent No. 4		
Item		Quantity
Drilled Shafts (4 ft. 0 in. Dia.)	linear foot	60.0
Rock Sockets (3 ft. 6 in. Dia.)	linear foot	33.0
Video Camera Inspection	each	3
Foundation Inspection Holes	linear foot	63.0
Sonic Logging Testing	each	3
Class B Concrete (Substructure)	cu. yard	39.4
Reinforcing Steel (Bridges)	pound	16,610

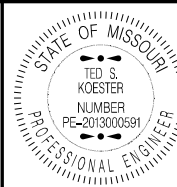
These quantities are included in the Estimated Quantities table on Sheet No. 2.

Notes:

Thickness of permanent steel casing shall be in accordance with Sec 701.

An additional 4 feet has been added to each #10-V400 bar length and height of each #5-P400 bar in the quantities, if required, for possible change in drilled shaft or rock socket length. If not required, the additional V-bar and P-bar length shall be cut off.

The tip of casing shall not extend into the rock socket elevation range reported in the Foundation Data table without approval by the engineer.



THIS SHEET HAS BEEN
SIGNED, SEALED AND DATED
ELECTRONICALLY.

DATE PREPARED
10/30/2023

ROUTE	STATE
-------	-------

V	MO
---	----

DISTRICT	SHEET NO
RD	12

BR	12
COUNTY	

GREENE

JOB NO.

J7S3486

[illegible]

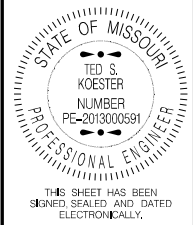
MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION

105 WEST CAPITAL
JEFFERSON CITY, MO 65102



1. *Journal of the American Medical Association*, 2000; 284: 2689-2695.

1. *Journal of the American Medical Association*, 2000; 284: 2689-2695.



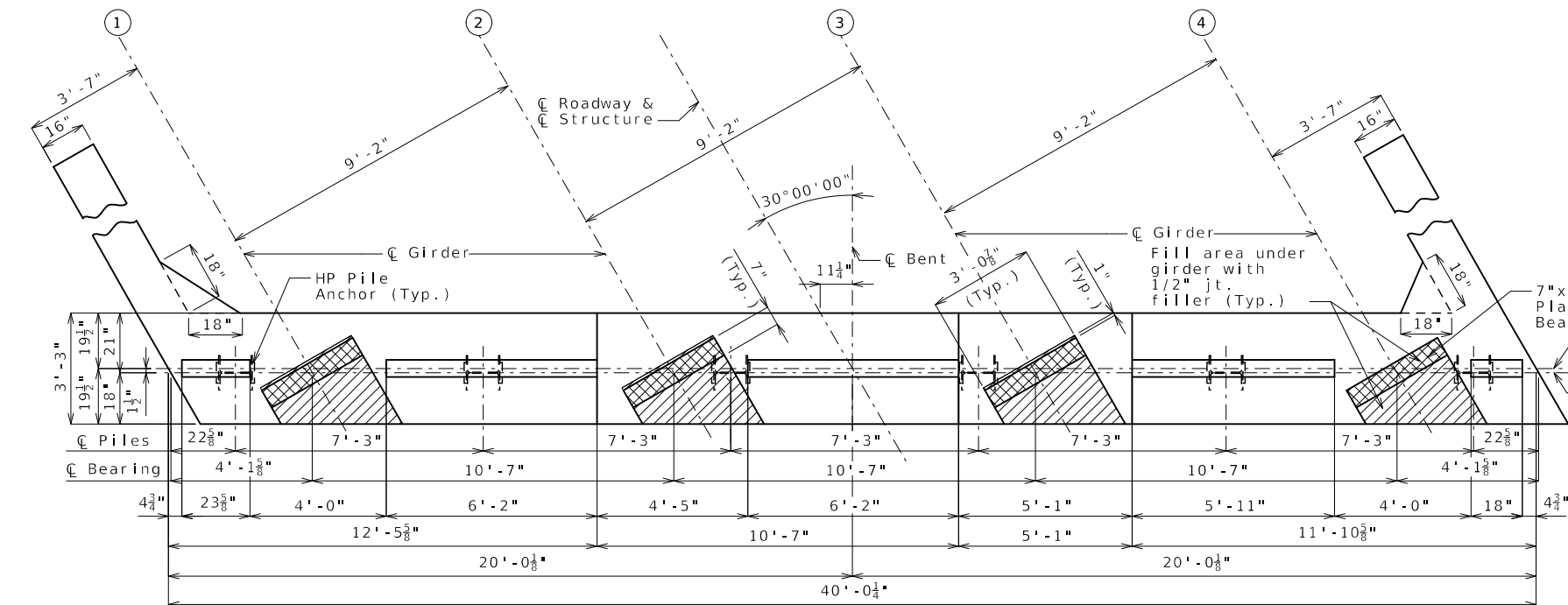
DATE PREPARED 10/30/2023	
ROUTE V	STATE MO
DISTRICT BR	SHEET NO. 13
COUNTY GREENE	
JOB NO. J7S3486	
CONTRACT ID.	

PROJECT NO.	
BRIDGE NO. A9119	

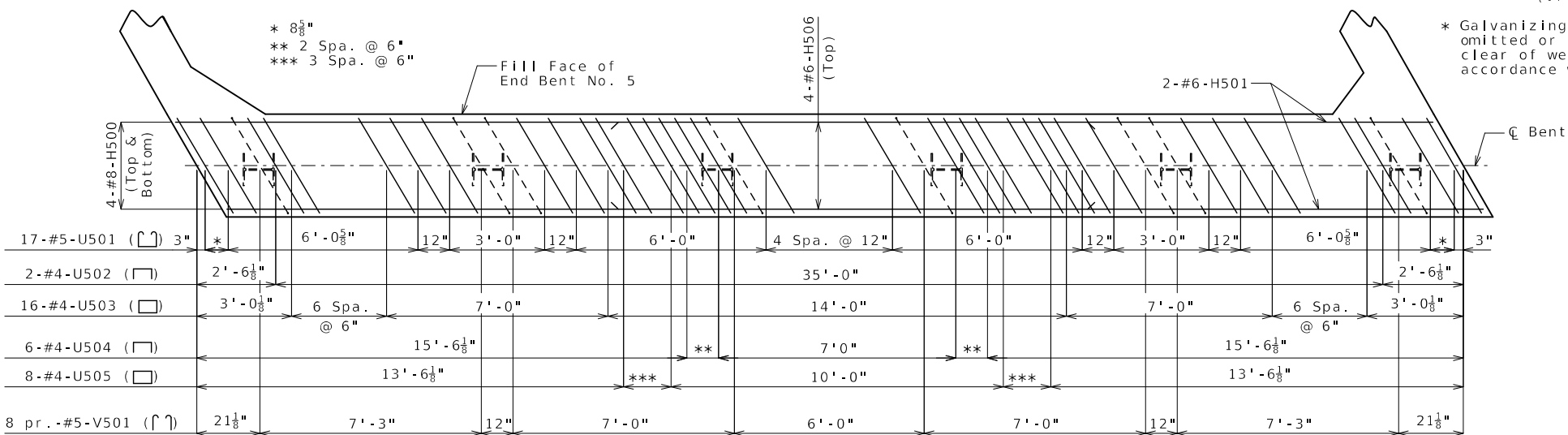
DESCRIPTION	DATE

DATE	DESCRIPTION

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION	105 WEST CAPITOL JEFFERSON CITY, MO 65102 1-888-ASK-MODOT (1-888-273-6636)
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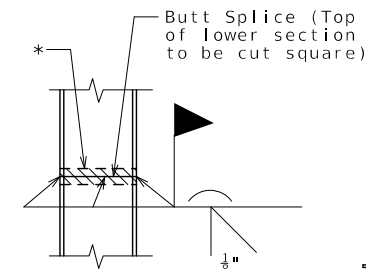
PLAN OF BEAM SHOWING DIMENSIONS



PLAN OF BEAM SHOWING REINFORCEMENT
Keys, Steps, and H-Pile Anchors not shown for clarity.

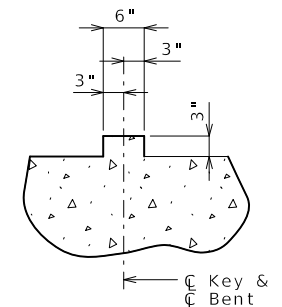
Substructure Quantity Table for Bent No. 5		
Item		Quantity
Class 1 Excavation	cu. yard	50
Galvanized Structural Steel Piles (12" in.)	linear foot	144
Pre-Bore for Piling	linear foot	126
Pile Point Reinforcement	each	6
Class B Concrete (Substructure)	cu. yard	19.3

These quantities are included in the Estimated Quantities table on Sheet No. 2.

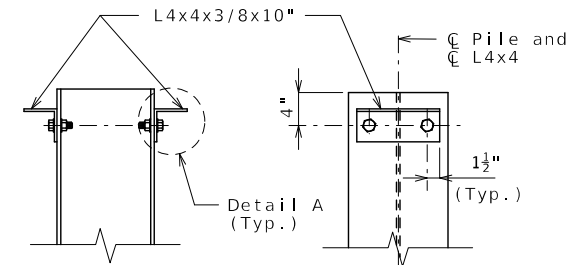


STEEL PILE SPLICE
(If required)

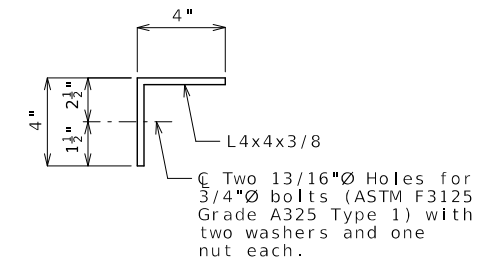
* Galvanizing material shall be omitted or removed one inch clear of weld locations in accordance with Sec 702.



SECTION THRU KEY



DETAILS OF HP PILE ANCHORS



DETAIL A

Angles shall be coated with a minimum of two coats of non-aluminum epoxy mastic primer to provide a dry film thickness of 4 mils minimum, 8 mils maximum, or galvanized in accordance with Sec 1081. Bolts, washers and nuts shall be galvanized in accordance with AASHTO M 232 (ASTM A153), Class C.

Notes:

For details of End Bent No. 5 not shown, see Sheets No. 14 & 15.

Reinforcing steel shall be shifted to clear piles. U bars shall clear piles by at least 1 1/2 inches.

The U bars and pairs of V bars shall be placed parallel to centerline of Roadway.

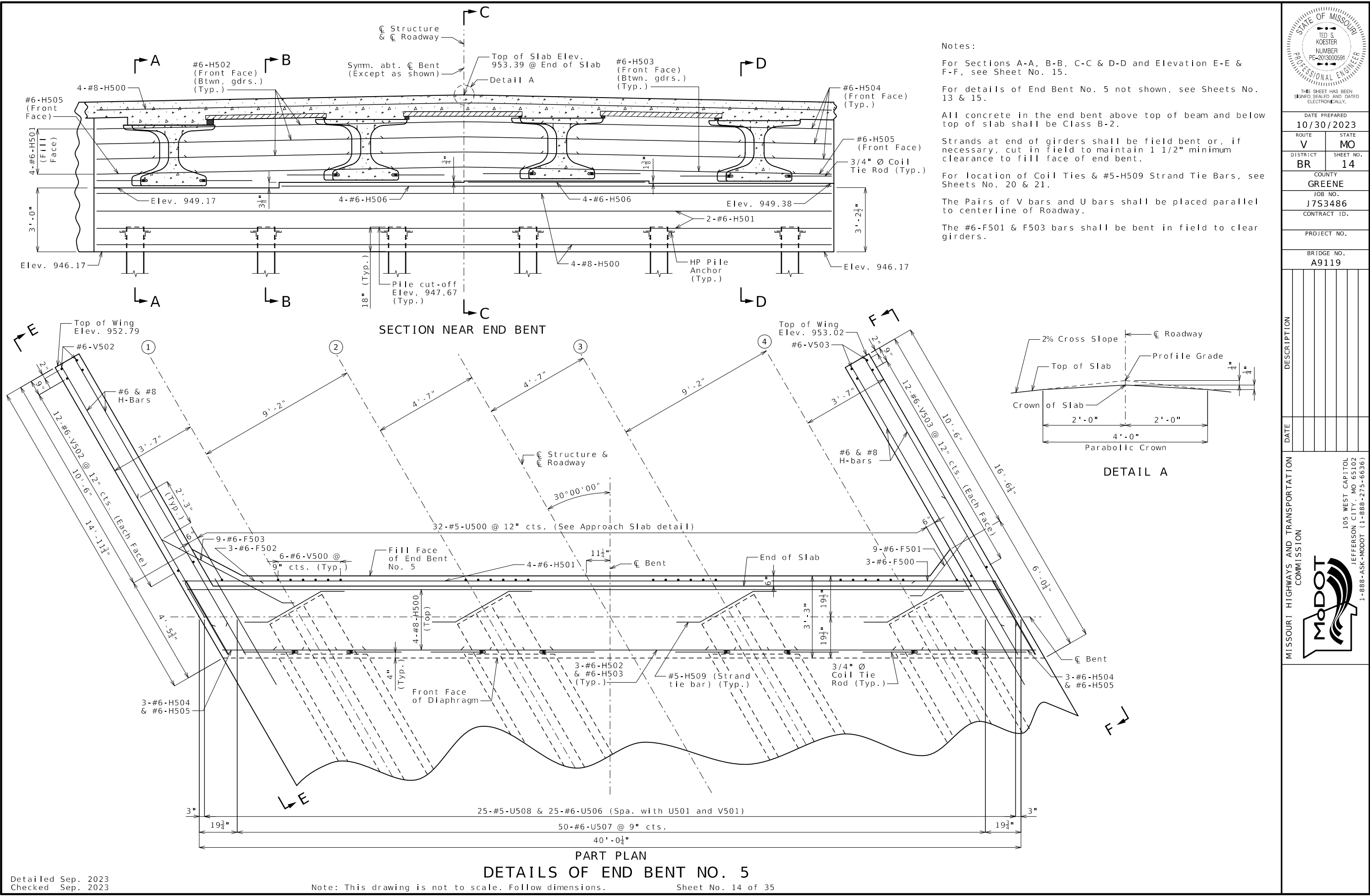
For details of Vertical Drain at End Bents, see Sheet No. 6.

DETAILS OF END BENT NO. 5

Detailed Sep. 2023
Checked Sep. 2023

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 13 of 35



Detailed Sep. 2023
Checked Sep. 2023

STATE OF MISSOURI

TED S. KOESTER

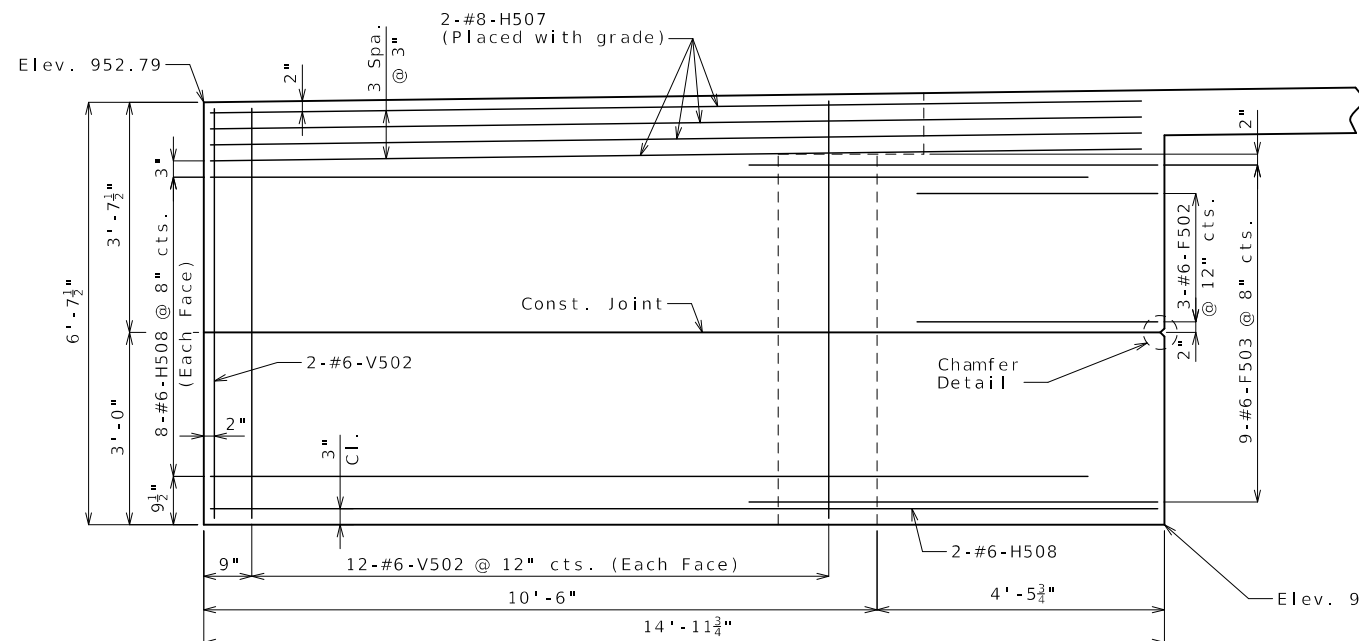
NUMBER

PE-2013000591

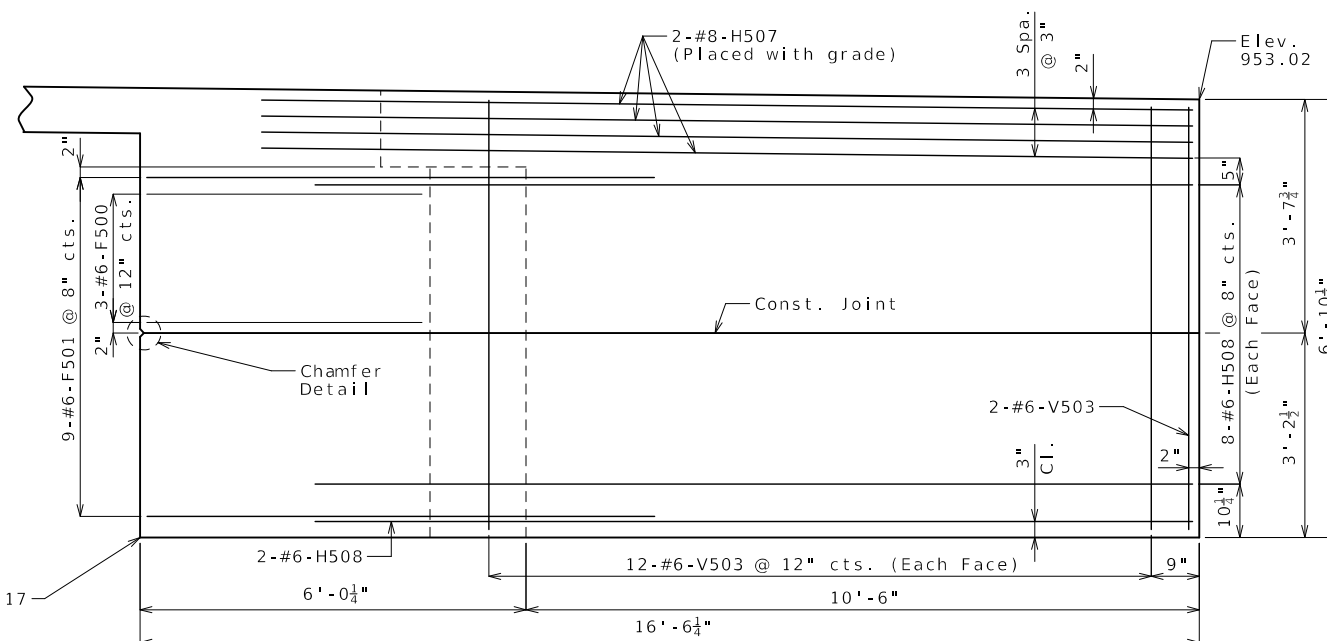
PROFESSIONAL ENGINEER

THIS SHEET HAS BEEN
SIGNED, SEALED AND DATED
ELECTRONICALLY.

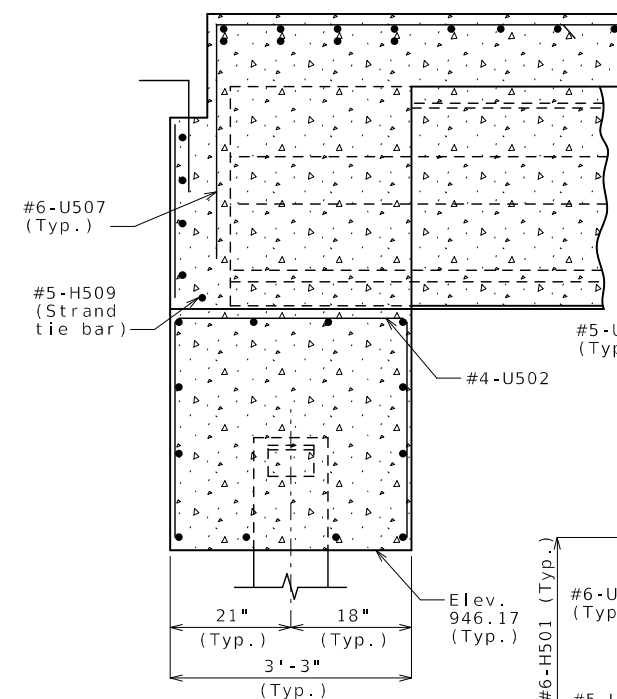
DATE PREPARED	
10/30/2023	
ROUTE	STATE
V	MO
DISTRICT	SHEET NO.
BR	14
COUNTY	
GREENE	
JOB NO.	
J7S3486	
CONTRACT ID.	
PROJECT NO.	
BRIDGE NO.	
A9119	
DESCRIPTION	
DATE	
MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION	
MoDOT 105 WEST CAPITOL JEFFERSON CITY, MO 65102 1-888-ASK-MODOT (1-888-273-6636)	



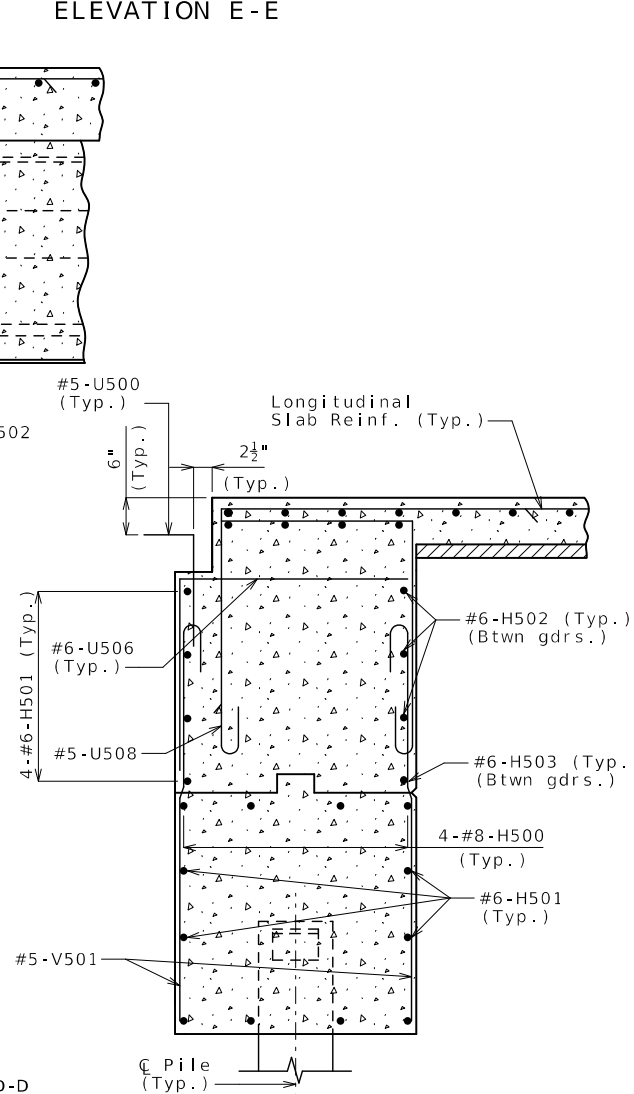
ELEVATION E-E



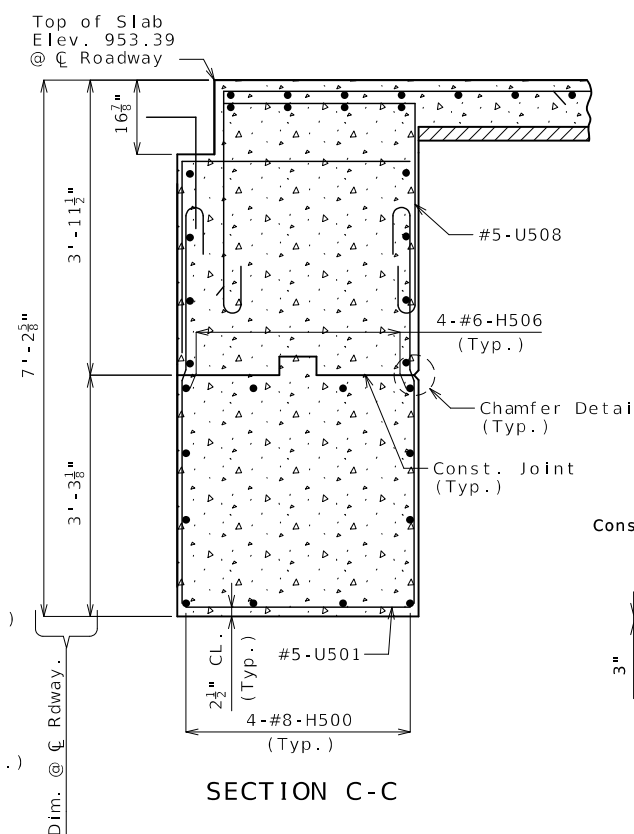
ELEVATION F-F



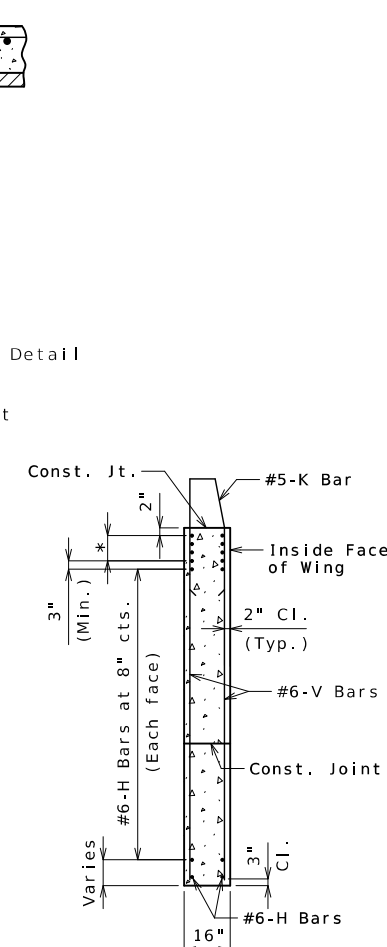
SECTION A-A



SECTION B-B

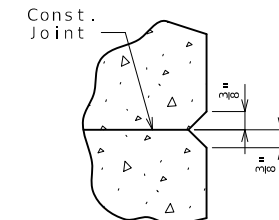


SECTION C-C

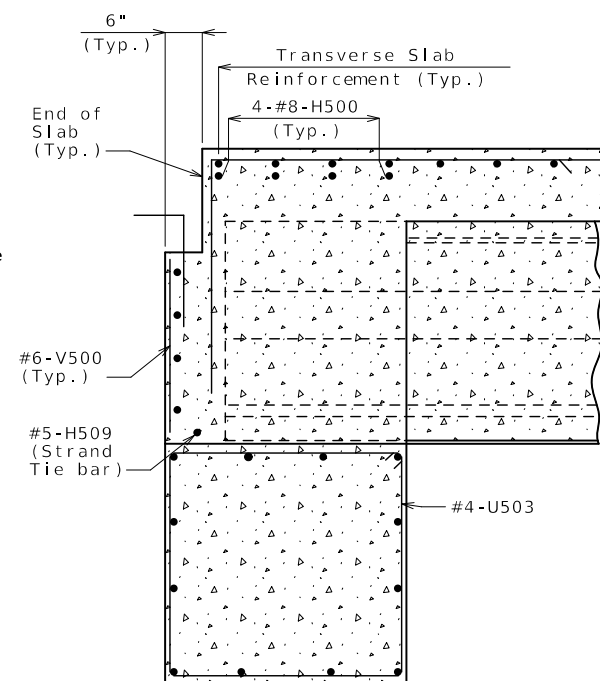


TYPICAL SECTION THRU WING

* #8-H Bars at 3\" cts. (Each face)(Place with grade)



CHAMFER DETAIL



SECTION D-D

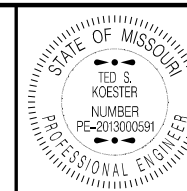
Notes:
Work this sheet with Sheets No. 13 & 14.
For Locations of Sections A-A, B-B, C-C, & D-D and Elevations E-E & F-F see Sheet No. 14.

DETAILS OF END BENT NO. 5

Detailed Sep. 2023
Checked Sep. 2023

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 15 of 35



THIS SHEET HAS BEEN
SIGNED, SEALED AND DATED
ELECTRONICALLY.

DATE PREPARED
10/30/2023

ROUTE V STATE MO

DISTRICT BR SHEET NO. 15

COUNTY GREENE

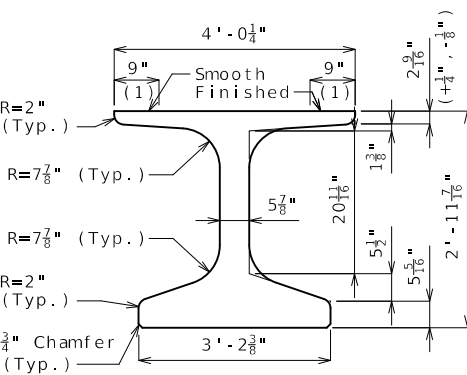
JOB NO. J7S3486

CONTRACT ID.

PROJECT NO.

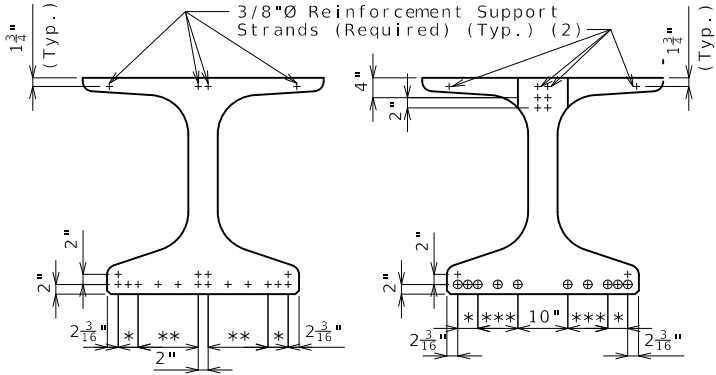
BRIDGE NO. A9119

(1) Fabricator shall apply a bond breaker to this region excluding where joint filler will be applied.



DIMENSIONS

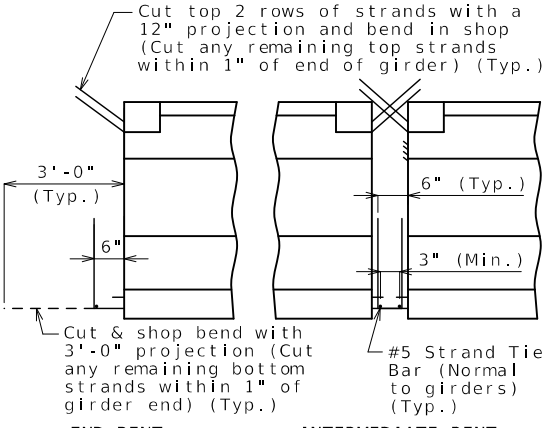
(2) Outer strands tensioned to 2.02 kips/strand and inner strands to 8 kips/strand. Placed symmetrical about C/Girder. May be moved laterally in pairs.



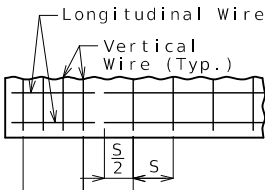
STRAND ARRANGEMENT

+ Indicates prestressing strand. o Indicates cut & shop bend with 3'-0" projection.

* 2 Spa. @ 2"
** 3 Spa. @ 4"
*** 2 Spa. @ 4"

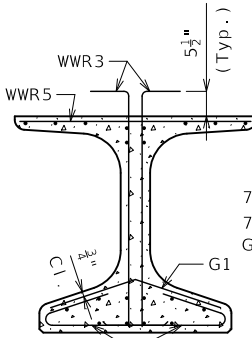
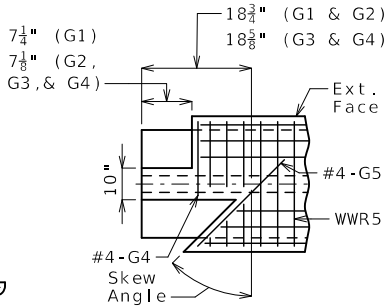


STRANDS AT GIRDER ENDS



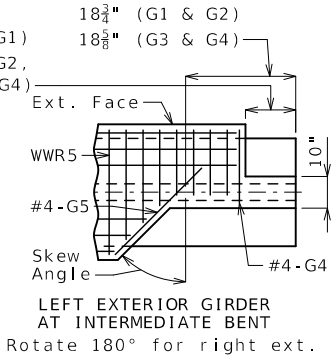
WELDED WIRE PLACEMENT

S = Vertical wire spacing
L = Length of WWR mats
J = Distance between WWR mats

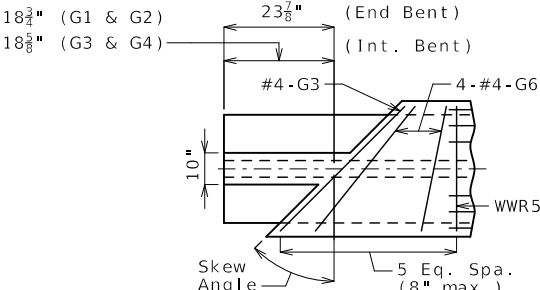


SECTION B-B

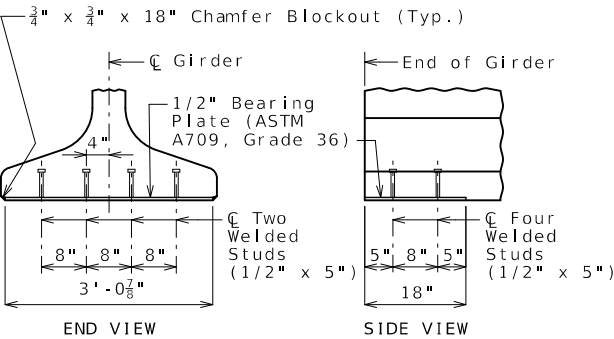
Strands not shown for clarity.



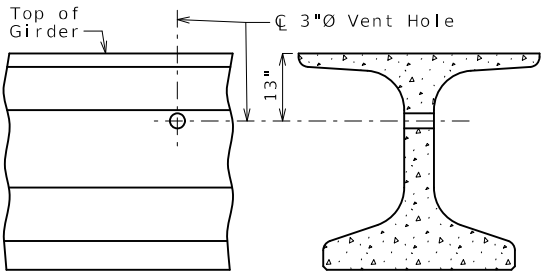
LEFT EXTERIOR GIRDER AT INTERMEDIATE BENT
Rotate 180° for right ext.



INTERIOR GIRDER AT ALL BENTS & EXTERIOR GIRDER AT END BENT
TOP FLANGE BLOCKOUT



BEARING PLATE

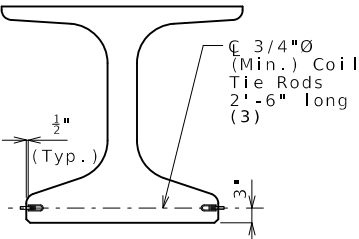


PART ELEVATION

PART SECTION

VENT HOLE

Place vent holes at or near upgrade 1/3 point of girders and clear reinforcing steel or strands by 1 1/2" minimum.



CLOSED DIAPHRAGMS AND INTEGRAL BENTS

COIL TIES

Exclude coil tie at exterior face of exterior girders except at integral end bents.

(3) 2'-1" at exterior face of exterior girders at end bents

Bill of Reinforcing Steel					
Bars Each Girder					Bending Diagrams
No.	Size	Mark	Length	Shape	
81	3	G1	2'-10"	8	Shape 8
2	4	G3	4'-5"	20	
2	4	G4	2'-3"	20	
2	4	G5	3'-1"	20	
8	4	G6	Varies	20	
Welded Wire Each Girder					Shape 20
Mark	Size	S	W	L	
WWR1	D20	4"	W8	7'-0"	6 3/4"
WWR2	D20	8"	W8	7'-4"	10"
WWR3	D20	12"	W8	12'-0"	6"
WWR6	D31	2"	W12	16"	4"

All dimensions are out to out.

Hooks and bends shall be in accordance with the CRSI Manual of Standard Practice for Detailing Reinforced Concrete Structures, Stirrup and Tie Dimensions.

Actual bar lengths are measured along centerline of bar to the nearest inch.

Minimum clearance to reinforcing shall be 1", unless otherwise shown.

All bar reinforcement shall be Grade 60.

WWR shall not be epoxy coated.

G4 and G5 not required for interior girders. Half no. of G3, G4, G5 and G6 not required for ext. girders of end spans.

General Notes:

Concrete for prestressed beams shall be Class A-1 with f'c = 8000 psi and f'ci = 6500 psi.

Use 16 strands, 0.6"Ø Grade 270, with an initial prestress force of 703 kips.

Pretensioned members shall be in accordance with Sec 1029.

Fabricator shall be responsible for location and design of lifting devices.

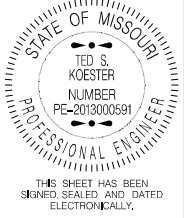
Exterior and interior girders are the same except: coil ties, top flange blockout, application of bond breaker.

The contractor shall provide bracing necessary for lateral and torsional stability of the girders during construction of the concrete slab and remove the bracing after the slab has attained 75% design strength. Contractor shall not drill holes in the girders.

For Girder Camber Diagram, see Sheet No. 24.

For location of coil ties at concrete diaphragms and integral bents, see Sheets No. 22 and 4.

Alternate bar reinforcing steel details are provided and may be used. The same type of reinforcing steel shall be used for all girders in all spans.



DATE PREPARED
10/30/2023

ROUTE
V

STATE
MO

DISTRICT
BR

SHEET NO.
16

COUNTY
GREENE

JOB NO.
J7S3486

CONTRACT ID.

PROJECT NO.

BRIDGE NO.
A9119

DESCRIPTION

DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

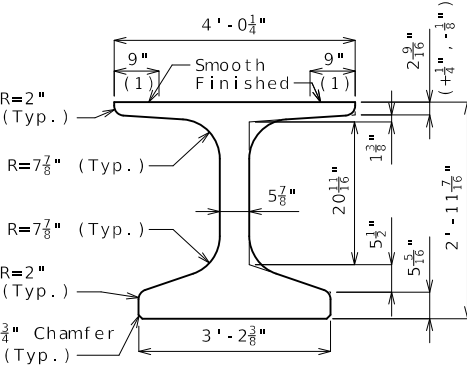
105 WEST CAPITOL

JEFFERSON CITY, MO 65102

1-888-ASK-MODOT (1-888-273-6636)

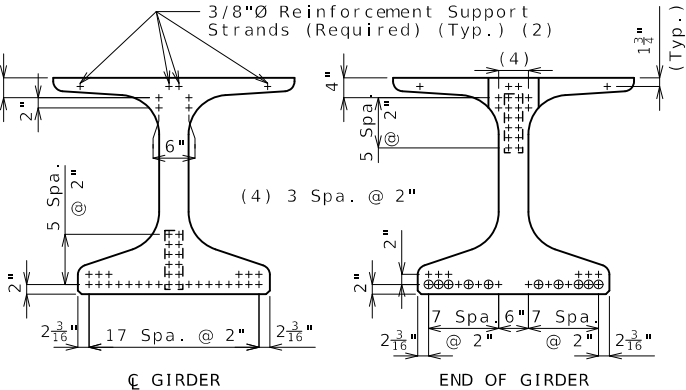
MoDOT

(1) Fabricator shall apply a bond breaker to this region excluding where joint filler will be applied.



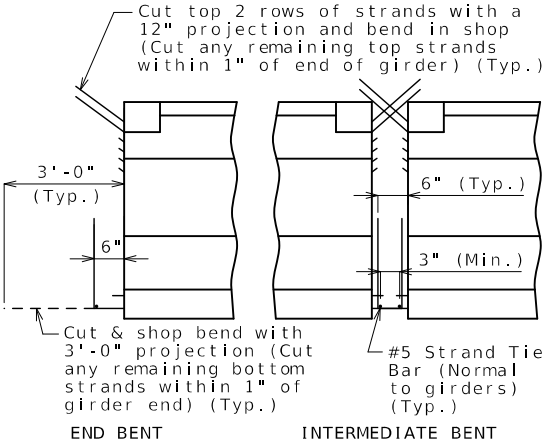
DIMENSIONS

(2) Outer strands tensioned to 2.02 kips/strand and inner strands to 8 kips/strand. Placed symmetrical about $\bar{C}$ Girder. May be moved laterally in pairs.

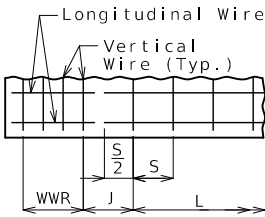


STRAND ARRANGEMENT

+ Indicates prestressing strand. o Indicates cut & shop bend with 3'-0" projection.



STRANDS AT GIRDER ENDS



WELDED WIRE PLACEMENT

S = Vertical wire spacing
L = Length of WWR mats
J = Distance between WWR mats

Bill of Reinforcing Steel					
Bars Each Girder					Bending Diagrams
No.	Size/Mark	Length	Shape		
101	3 G1	2'-10"	8		
2	4 G3	4'-5"	20		
2	4 G4	2'-3"	20		
2	4 G5	3'-1"	20		
8	4 G6	Varies	20		
Welded Wire Each Girder					
Mark	Size	S	W	L	J
WWR1	D20	4"	W8	10'-4"	4 1/4"
WWR2	D20	8"	W8	10'-0"	12"
WWR3	D20	12"	W8	16'-0"	6"
WWR6	D31	2"	W12	16"	4"

Shape 8

8 1/2"
2"
5"
23 3/4"
5"
d = 2 1/2"
Wire Size @ 5"
D11 @ 6"

7"
16 3/4"
6 3/4"
3'-3 3/4"
d = 2 1/2"
W2 (Typ.)
W8 (Typ.)

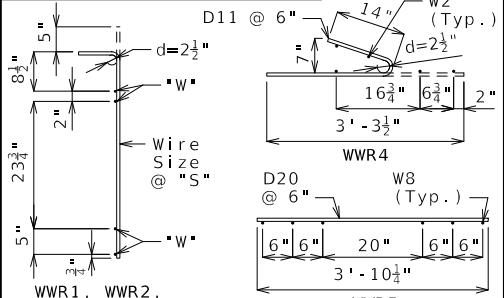
6"
6"
20"
6"
6"
3'-10 1/4"
D20 @ 6"
W8 (Typ.)

WWR1, WWR2,
WWR3 & WWR6

WWR4

WWR5

Shape 20



All dimensions are out to out.
Hooks and bends shall be in accordance with the CRSI Manual of Standard Practice for Detailing Reinforced Concrete Structures, Stirrup and Tie Dimensions.

Actual bar lengths are measured along centerline of bar to the nearest inch.
Minimum clearance to reinforcing shall be 1", unless otherwise shown.

All bar reinforcement shall be Grade 60.
WWR shall not be epoxy coated.

G4 and G5 not required for interior girders. Half no. of G3, G4, G5 and G6 not required for ext. girders of end spans.

General Notes:
Concrete for prestressed beams shall be Class A-1 with $f'c = 8000$ psi and $f'ci = 6500$ psi.

Use 38 strands, 0.6"Ø Grade 270, with an initial prestress force of 1670 kips.
Pretensioned members shall be in accordance with Sec 1029.

Fabricator shall be responsible for location and design of lifting devices.

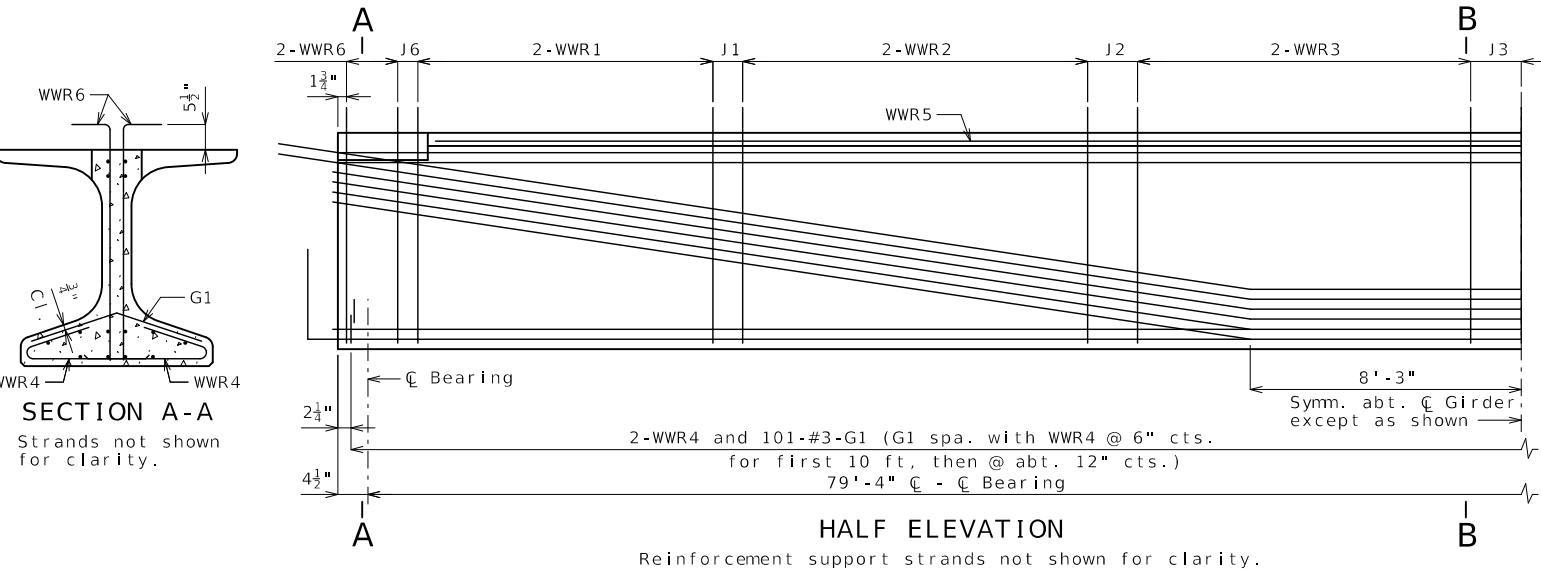
Exterior and interior girders are the same except: coil ties, top flange breakout, application of bond breaker.

The contractor shall provide bracing necessary for lateral and torsional stability of the girders during construction of the concrete slab and remove the bracing after the slab has attained 75% design strength. Contractor shall not drill holes in the girders.

For Girder Camber Diagram, see Sheet No. 24.

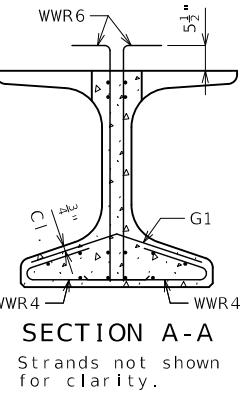
For location of coil ties at concrete diaphragms and integral bents, see Sheets No. 22 and 14.

Alternate bar reinforcing steel details are provided and may be used. The same type of reinforcing steel shall be used for all girders in all spans.

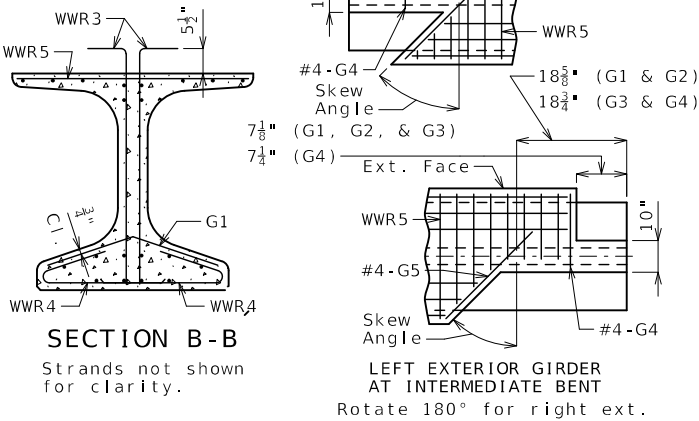


HALF ELEVATION

Reinforcement support strands not shown for clarity.

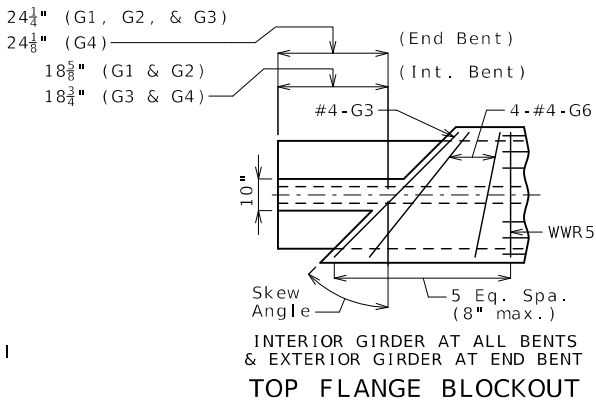


SECTION A-A
Strands not shown for clarity.

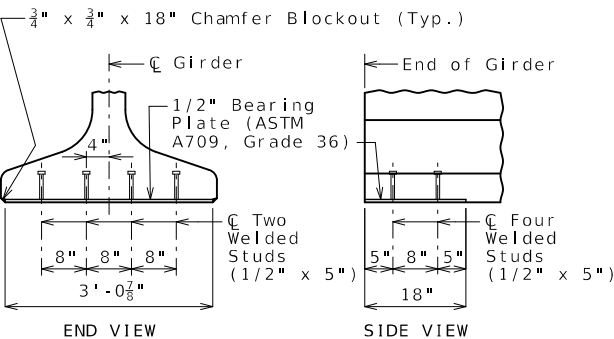


SECTION B-B

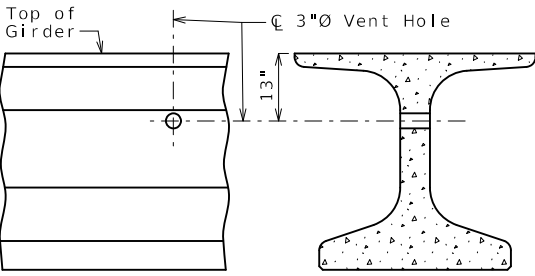
Strands not shown for clarity.



TOP FLANGE BLOCKOUT



BEARING PLATE

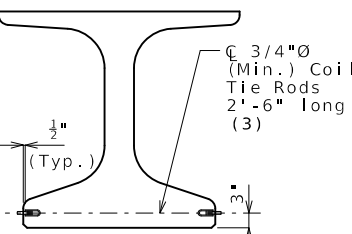


PART ELEVATION

PART SECTION

VENT HOLE

Place vent holes at or near upgrade 1/3 point of girders and clear reinforcing steel or strands by 1 1/2" minimum.



COIL TIES

Exclude coil tie at exterior face of exterior girders except at integral end bents.
(3) 2'-1" at exterior face of exterior girders at end bents

NU-GIRDERS - SPAN (4-5)

STATE OF MISSOURI
TED S. KOESTER
PROFESSIONAL ENGINEER
NUMBER
PE-2013000591
THIS SHEET HAS BEEN
SIGNED, SEALED AND DATED
ELECTRONICALLY.

DATE PREPARED
10/30/2023
ROUTE
V
DISTRICT
BR
COUNTY
GREENE
JOB NO.
J7S3486
CONTRACT ID.
PROJECT NO.
BRIDGE NO.
A9119

DESCRIPTION

DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-273-6636)

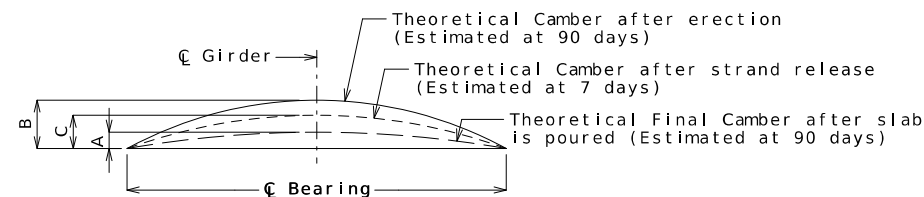
MoDOT

Girder No.	1	2	3	4
Girder No. 1	17"	17"	17"	17"
Girder No. 2	17"	17"	17"	17"
Girder No. 3	17"	17"	17"	17"
Girder No. 4	17"	17"	17"	17"
Bottom of Slab	17"	17"	17"	17"
Top of Girder	17"	17"	17"	17"
Span (1-2)	4 Equal Spaces	10 Equal Spaces	10 Equal Spaces	10 Equal Spaces
Span (2-3)	59'-4"	83'-8"	83'-8"	79'-4"
Span (3-4)	59'-4"	83'-8"	83'-8"	79'-4"
Span (4-5)	59'-4"	83'-8"	83'-8"	79'-4"

If girder beam camber is different from that shown in the camber diagram, in order to maintain minimum slab thickness, an adjustment of the slab haunches, an increase in slab thickness or a raise in grade uniformly throughout the structure shall be necessary. The haunch shall be limited to ensure the projecting girder reinforcement is embedded into slab at least 2 inches. No payment will be made for additional labor or materials required for variation in haunching, slab thickness or grade adjustment.

Concrete in the slab haunches is included in the Estimated Quantities for Slab on Concrete NU-Girder.

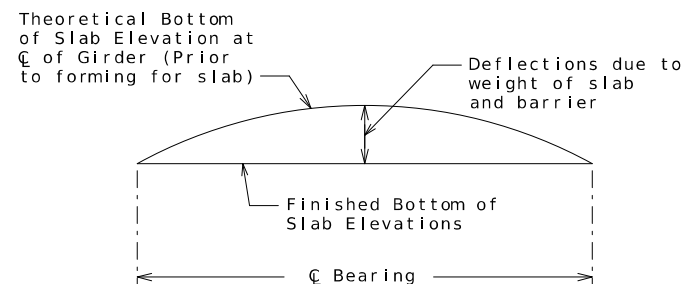
Longitudinal dimensions are horizontal.



Girder	Span (1-2)			Span (2-3)			Span (3-4)			Span (4-5)		
	A	B	C	A	B	C	A	B	C	A	B	C
Exterior	$\frac{1}{2}$ "	1"	$\frac{5}{8}$ "	$\frac{1}{4}$ "	2"	$1\frac{1}{2}$ "	$\frac{1}{4}$ "	2"	$1\frac{1}{2}$ "	$\frac{5}{8}$ "	$2\frac{1}{8}$ "	$1\frac{1}{2}$ "
Interior	$\frac{1}{4}$ "			$\frac{1}{8}$ "			$\frac{1}{8}$ "			$\frac{1}{4}$ "		

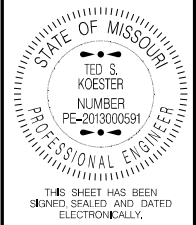
Conversion Factors for Girder Camber (Estimated at 90 days):

0.1 pt. = 0.314 x 0.5 pt.
0.2 pt. = 0.593 x 0.5 pt.
0.3 pt. = 0.813 x 0.5 pt.
0.4 pt. = 0.952 x 0.5 pt.

$$0.25 \text{ pt.} = 0.7125 \times 0.5 \text{ pt.}$$


Theoretical Bottom of Slab Elevations at Centerline of Girder (Prior to forming for slab) (Estimated at 90 days)											
Girder Number	Span (1-2) (59'-4" C Brg. - C Brg.)										
	C Brg.	.25	.50	.75	C Brg.						
1	952.66	952.84	952.99	953.10	953.17						
2	952.79	952.98	953.13	953.24	953.32						
3	952.73	952.92	953.08	953.20	953.29						
4	952.48	952.68	952.84	952.97	953.06						
Girder Number	Span (2-3) (83'-8" C Brg. - C Brg.)										
	C Brg.	.10	.20	.30	.40	.50	.60	.70	.80	.90	C Brg.
1	953.18	953.28	953.37	953.45	953.50	953.54	953.55	953.55	953.52	953.48	953.44
2	953.33	953.43	953.52	953.59	953.65	953.69	953.71	953.71	953.69	953.66	953.64
3	953.30	953.40	953.49	953.57	953.63	953.67	953.69	953.69	953.68	953.65	953.63
4	953.07	953.18	953.28	953.37	953.43	953.48	953.50	953.51	953.50	953.47	953.44
Girder Number	Span (3-4) (83'-8" C Brg. - C Brg.)										
	C Brg.	.10	.20	.30	.40	.50	.60	.70	.80	.90	C Brg.
1	953.43	953.48	953.51	953.52	953.52	953.50	953.46	953.40	953.33	953.24	953.14
2	953.62	953.67	953.70	953.73	953.73	953.72	953.68	953.62	953.55	953.46	953.36
3	953.62	953.67	953.71	953.74	953.74	953.73	953.70	953.65	953.57	953.49	953.39
4	953.43	953.48	953.52	953.55	953.56	953.55	953.52	953.47	953.41	953.33	953.24
Girder Number	Span (4-5) (79'-4" C Brg. - C Brg.)										
	C Brg.	.10	.20	.30	.40	.50	.60	.70	.80	.90	C Brg.
1	953.13	953.11	953.08	953.03	952.97	952.90	952.80	952.69	952.57	952.44	952.33
2	953.35	953.33	953.31	953.27	953.22	953.14	953.05	952.95	952.82	952.69	952.60
3	953.38	953.37	953.35	953.31	953.26	953.19	953.11	953.00	952.88	952.75	952.66
4	953.23	953.22	953.19	953.16	953.11	953.05	952.96	952.86	952.75	952.63	952.54

Elevations are based on a constant slab thickness of 8 1/2" and include allowance for theoretical dead load deflections due to weight of slab (including precast panel) and barrier.



DATE PREPARED
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ROUTE V	STATE MO
DISTRICT BR	SHEET NO. 24

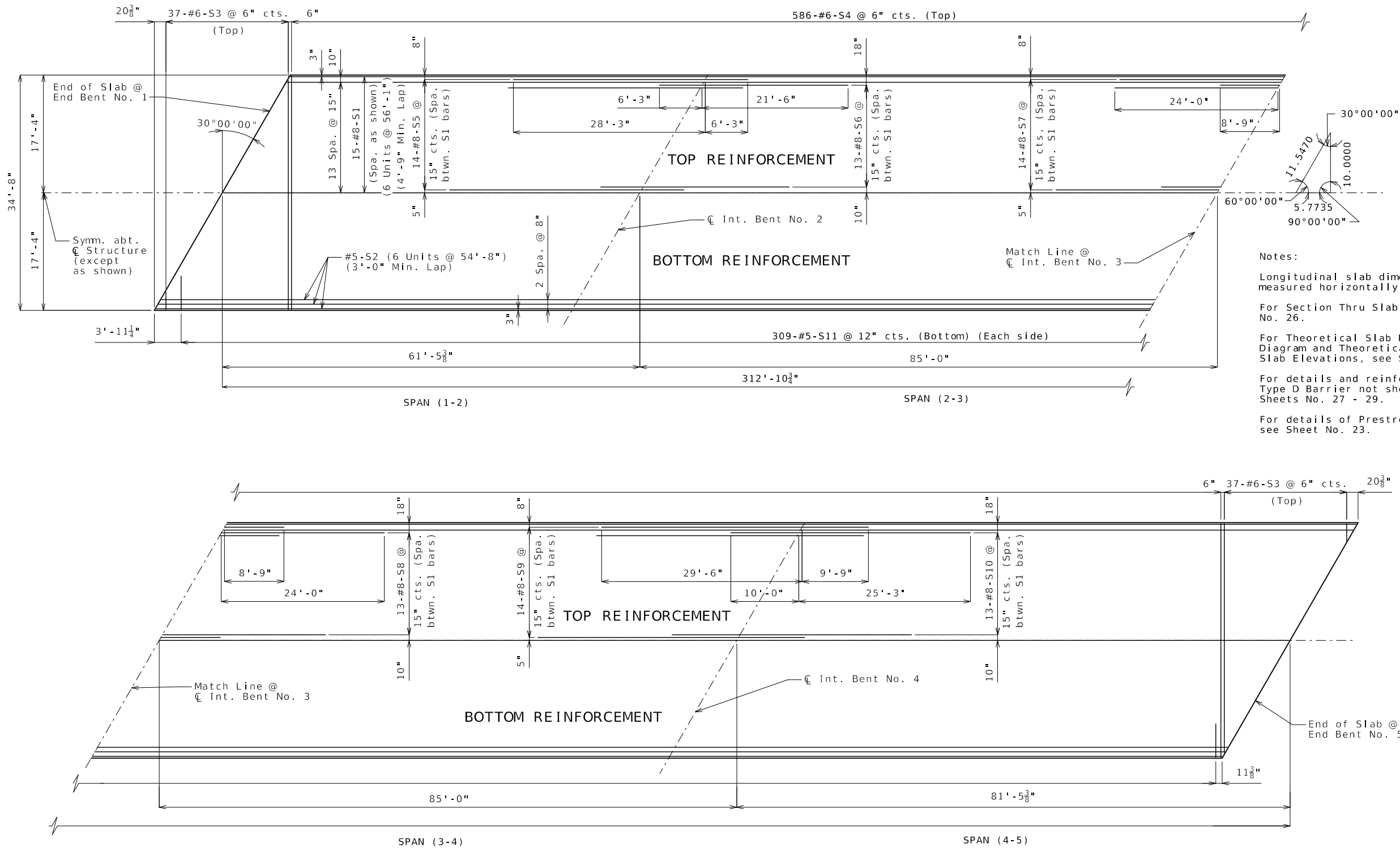
COUNTY
GREENE

JOB NO.
J7S3486
CONTRACT ID.

PROJECT NO.

BRIDGE NO.	A9119
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COMMISSION

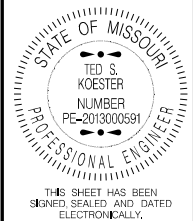


PLAN OF SLAB SHOWING REINFORCEMENT

Detailed Sept. 2023
Checked Sept. 2023

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 25 of 35

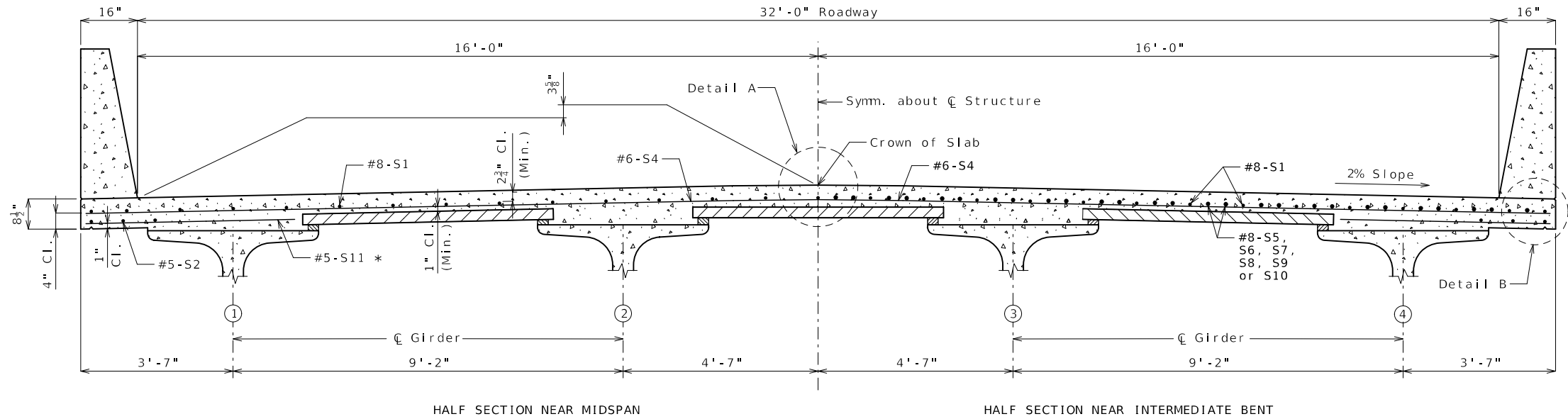


DATE PREPARED 10/30/2023	
ROUTE V	STATE MO
DISTRICT BR	SHEET NO. 25
COUNTY GREENE	
JOB NO. J7S3486	
CONTRACT ID.	
PROJECT NO.	
BRIDGE NO. A9119	

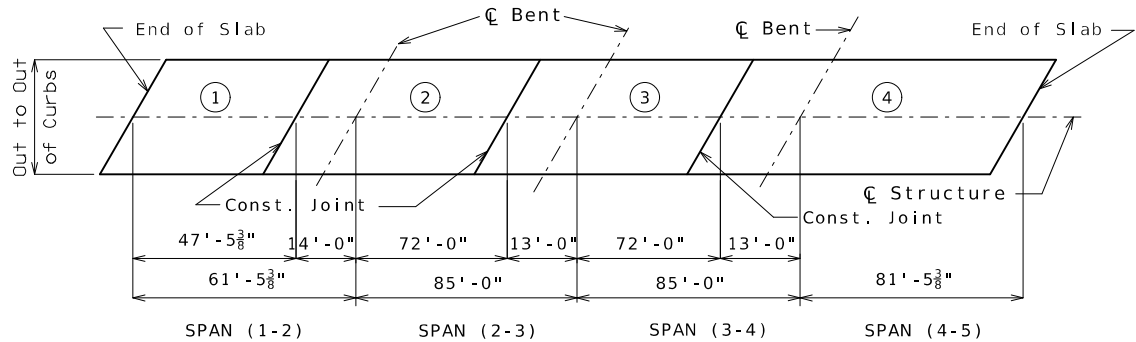
DESCRIPTION	DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

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SECTION THRU SLAB
* Alternate bar shape available, see barrier sheet.



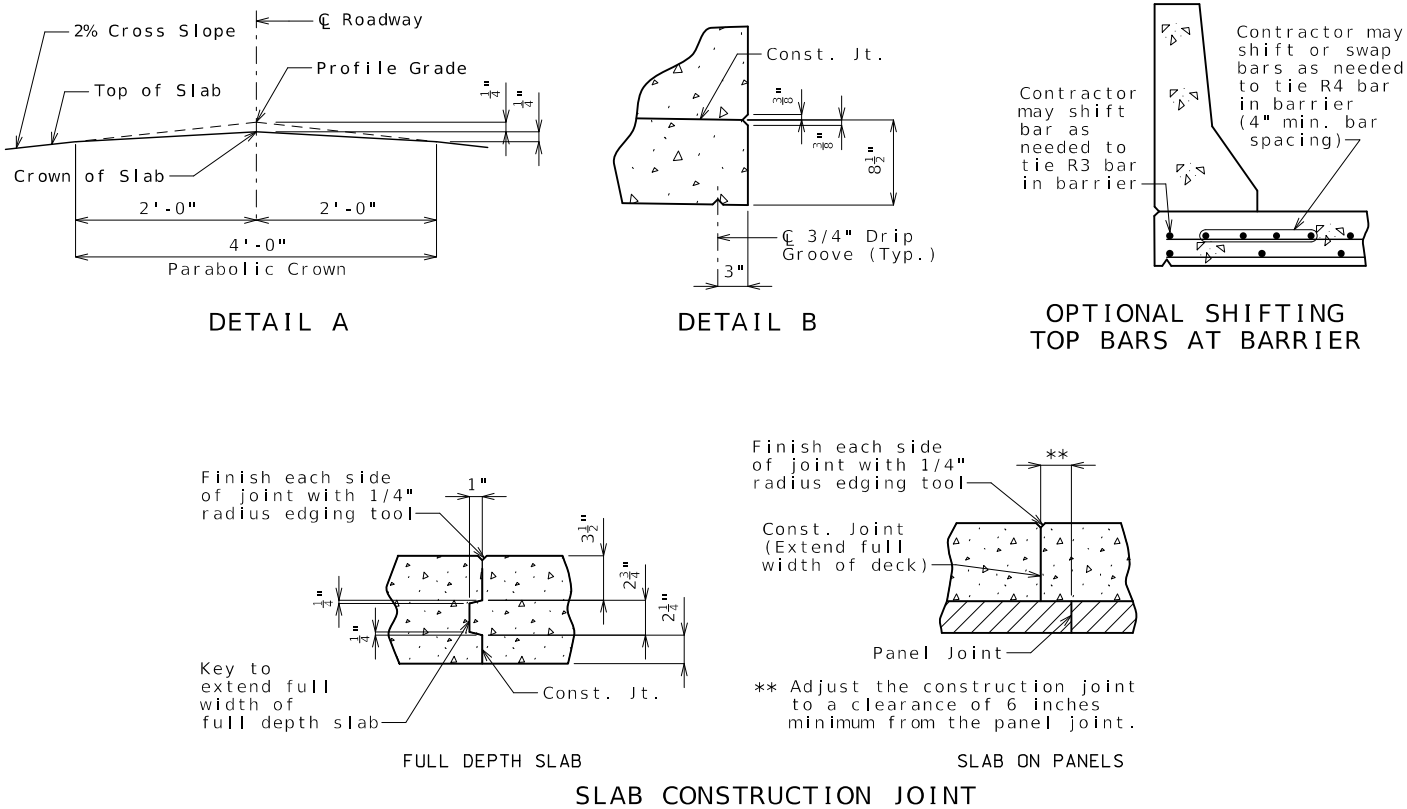
	Sequence of Pours				Min. Rate of Pour Cu. Yds./Hr.
	Direction				With Retarder
Basic Sequence	1	2	3	4	25
	End to 2	1 to 3	2 to 4	3 to End	
Alternate pours to the basic sequence are subject to the approval of the engineer in accordance with Sec 703.					
Alternate A Pours	1 + 2		3	4	25
	End to 3		2 to 4	3 to End	
Alternate B Pours	1 + 2		3 + 4		25
	End to 3		2 to End		
Alternate C Pours	1 + 2 + 3 + 4				25
	End to End				

The contractor shall furnish an approved retarder to retard the set of the concrete to 2.5 hours, and shall pour and satisfactorily finish the slab pours at the rate given.

The concrete diaphragm at the intermediate bents and integral end bents shall be poured a minimum of 30 minutes and a maximum of 2 hours before the slab is poured.

SLAB POURING SEQUENCE

SLAB DETAILS



Notes:

For details of Prestressed Panels, see Sheet No. 23.

For details and reinforcement of barrier not shown, see Sheets No. 27 thru 29.

For Theoretical Bottom of Slab Elevations, Girder Camber Diagram and Theoretical Slab Haunching Diagram, see Sheet No. 24.

For Plan of Slab Showing Reinforcement, see Sheet No. 25.

STATE OF MISSOURI
TED S. KOESTER
NUMBER
PE-2013000591
PROFESSIONAL ENGINEER

THIS SHEET HAS BEEN
SIGNED, SEALED AND DATED
ELECTRONICALLY.

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10/30/2023

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STATE
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DISTRICT
BR

SHEET NO.
26

COUNTY
GREENE

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PROJECT NO.

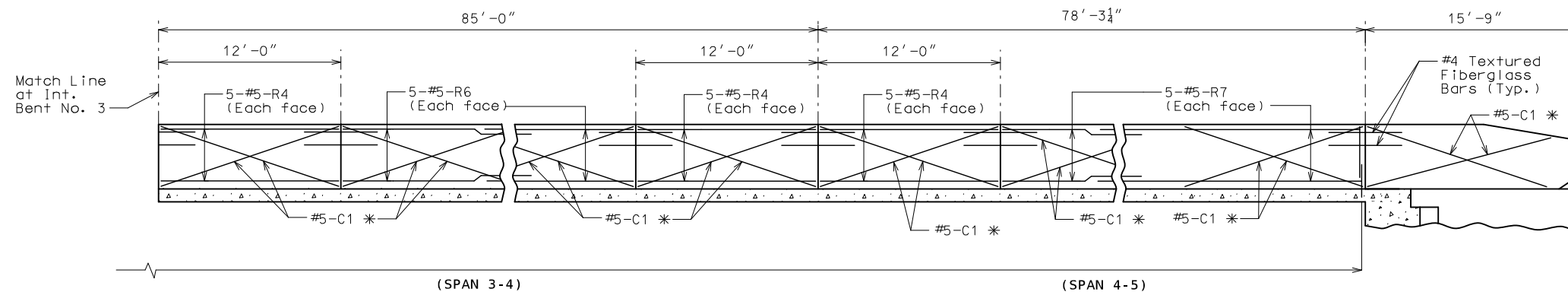
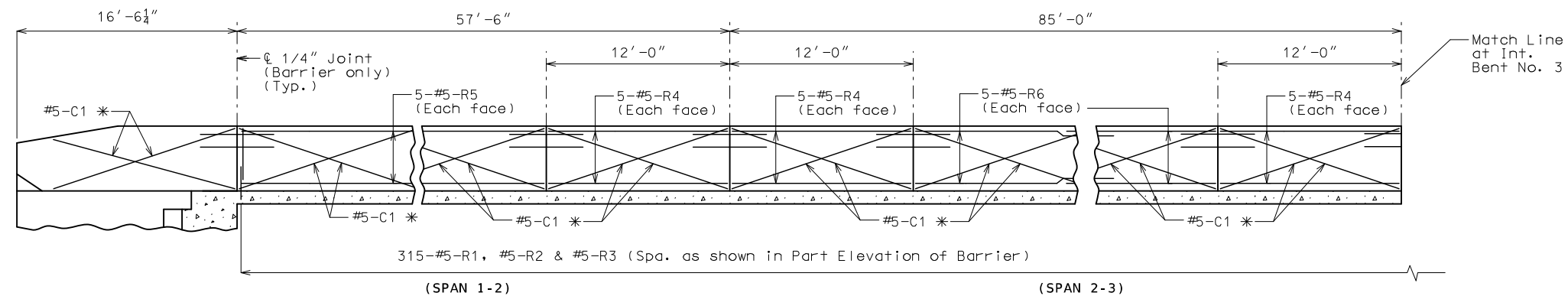
BRIDGE NO.
A9119

DESCRIPTION

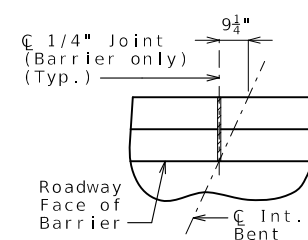
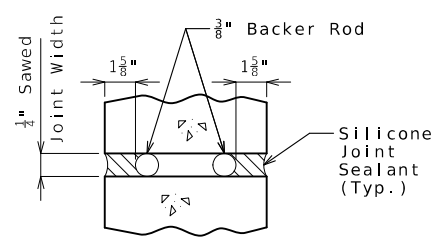
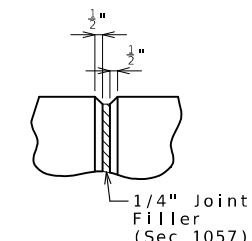
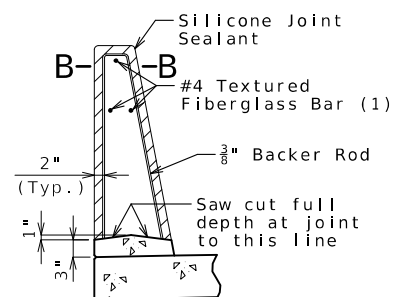
DATE

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ELEVATION OF LEFT BARRIER
Longitudinal dimensions are horizontal.



General Notes:

* Slip-formed option only.

Conventional forming or slip forming may be used. Saw cut joints may be used with conventional forming.

Top of barrier shall be built parallel to grade and barrier joints (except at end bents) normal to grade.

All exposed edges of barrier shall have either a 1/2-inch radius or a 3/8-inch bevel, unless otherwise noted.

Payment for all concrete and reinforcement, complete in place, will be considered completely covered by the contract unit price for Type D Barrier per linear foot.

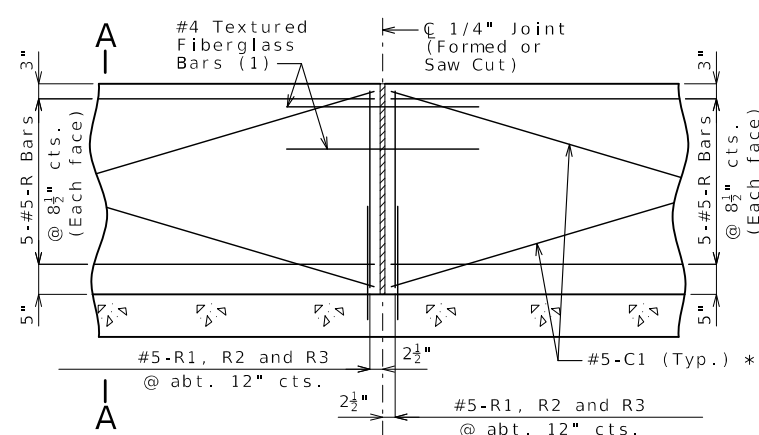
Concrete in barrier shall be Class B-1.

Measurement of barrier is to the nearest linear foot for each structure, measured along the outside top of slab from end of wing to end of wing.

Concrete traffic barrier delineators shall be placed on top of the barrier as shown on Missouri Standard Plan 617.10 and in accordance with Sec 617. Delineators on bridges with two-lane, two-way traffic shall have retroreflective sheeting on both sides. Concrete traffic barrier delineators will be considered completely covered by the contract unit price for Type D Barrier.

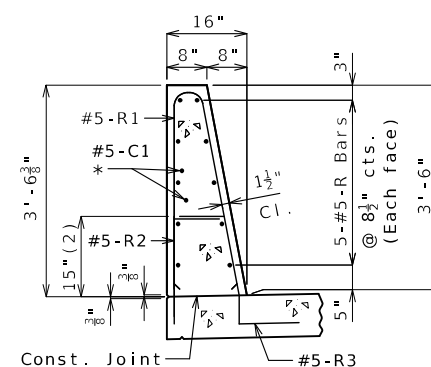
Joint sealant and backer rods shall be in accordance with Sec 717 for silicone joint sealant for saw cut and formed joints.

For slip-formed option, both sides of barrier shall have a vertically broomed finish and the top shall have a transversely broomed finish.



PART ELEVATION OF BARRIER

(1) Four feet long, centered on joint,
slip-formed option only

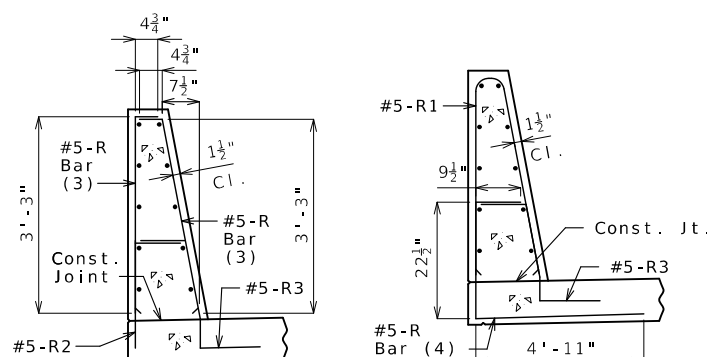


SECTION A-A

Use a minimum lap of 3'-1" for
#5 horizontal barrier bars.

The cross-sectional area above the slab is 3.52 square feet.

(2) To top of bar

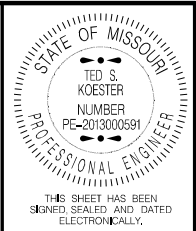


R-BAR PERMISSIBLE ALTERNATE SHAPE

(3) The R1 bar may be separated into two bars as shown, at the contractor's option, only when slip forming is not used. (All dimensions are out to out.)

(4) The R2 bar and #5 bottom transverse slab bar in cantilever (prestressed panels only) combination may be furnished as one bar as shown, at the contractor's option.

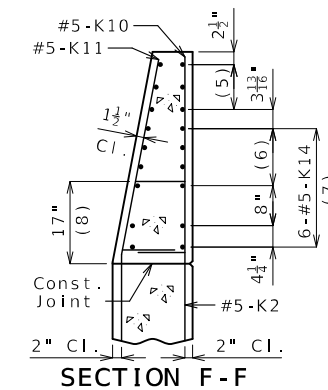
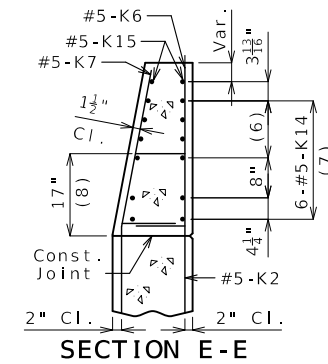
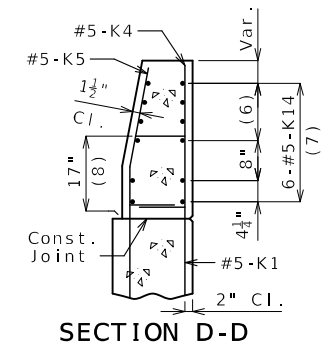
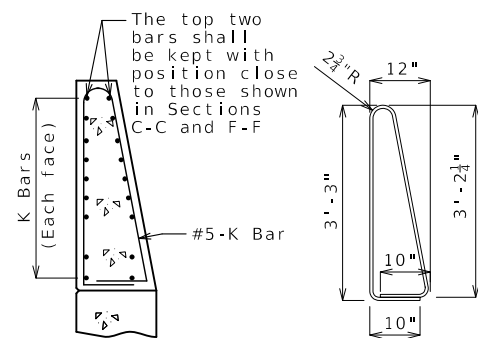
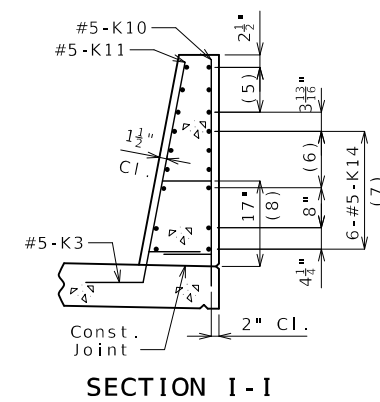
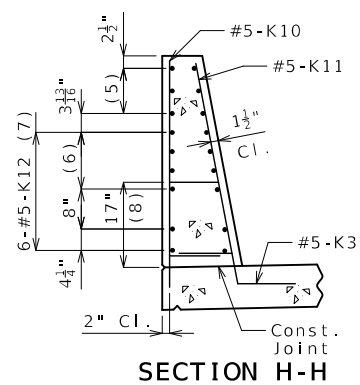
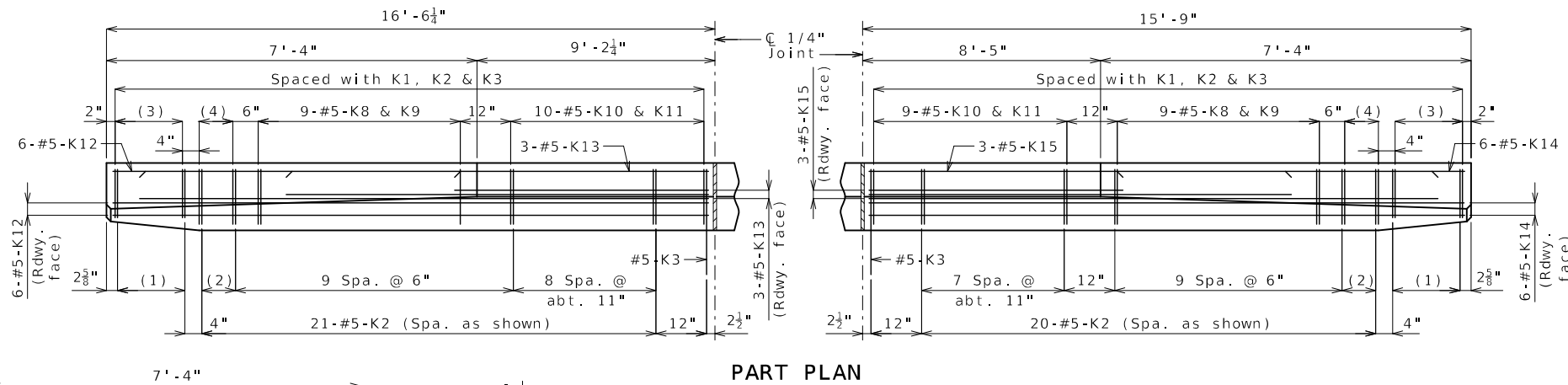
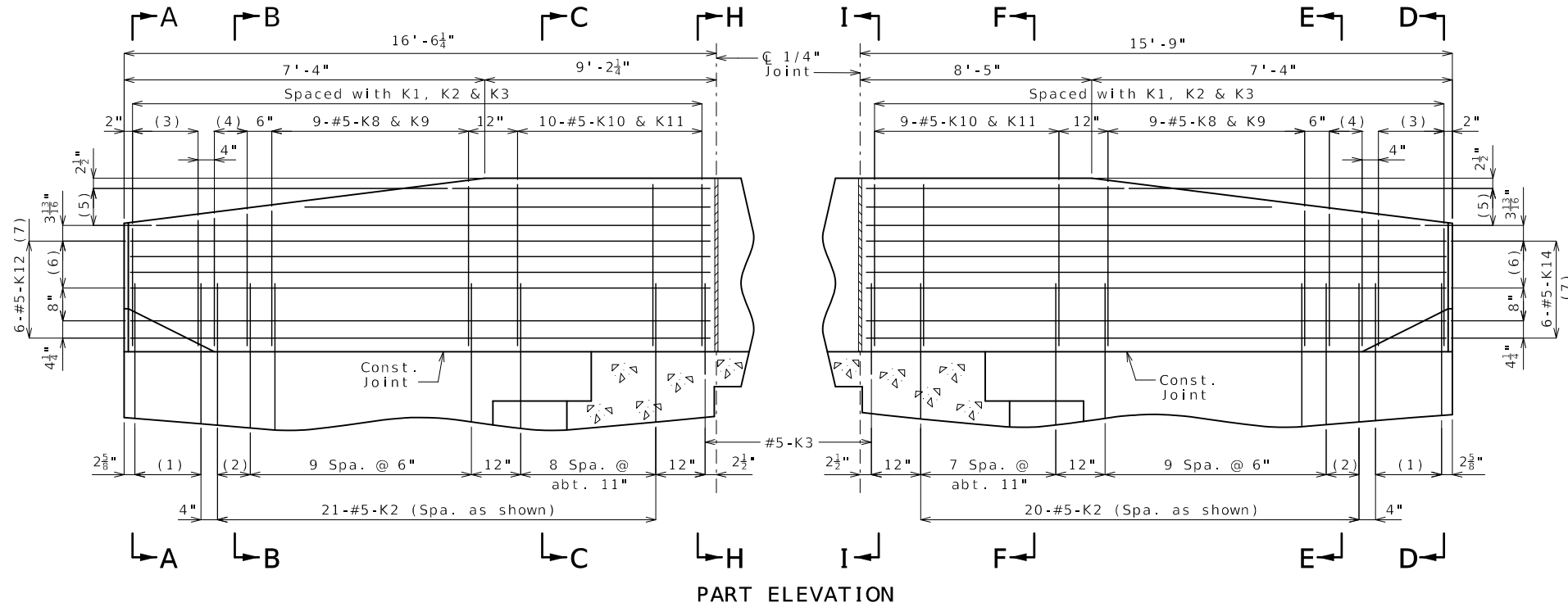
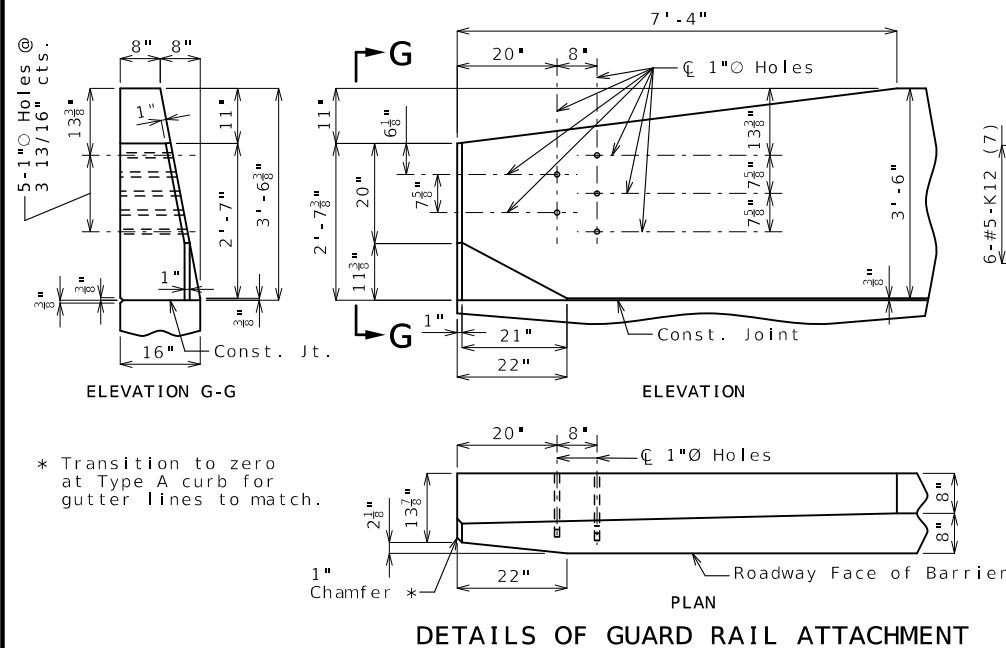
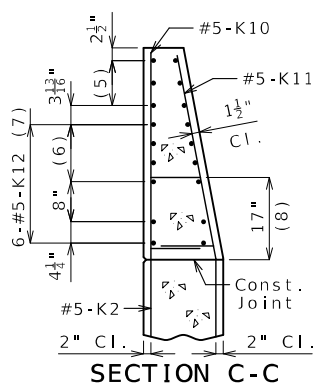
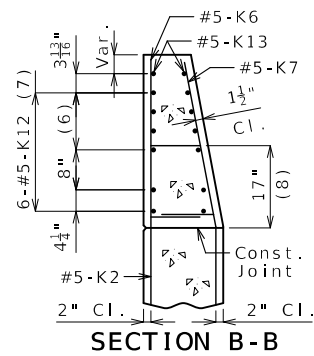
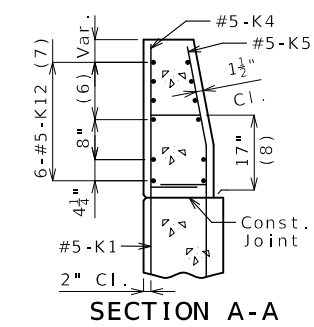
TYPE D BARRIER (LEFT SIDE)



DATE PREPARED					
10/30/2023					
ROUTE		STATE			
V		MO			
DISTRICT		SHEET NO.			
BR		27			
COUNTY					
GREENE					
JOB NO.					
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CONTRACT ID.					
PROJECT NO.					
BRIDGE NO.					
A9119					
DESCRIPTION					
DATE					

MISSOURI HIGHWAYS AND TRANSPORTATION
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JEFFERSON CITY, MO 65102
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General Notes:
Concrete traffic barrier delineators shall be placed on top of the barrier as shown on Missouri Standard Plan 617.10 and in accordance with Sec 617. Delineators on bridges with two-lane, two-way traffic shall have retroreflective sheeting on both sides. Concrete traffic barrier delineators will be considered completely covered by the contract unit price for Type D Barrier.

Reinforcing Steel:
Minimum clearance to reinforcing steel shall be 1 1/2" except as shown for bars embedded into end bent.

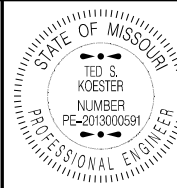
TYPE D BARRIER AT END BENTS

(Left barrier shown, right barrier similar)

**K10-K11 BAR PERMISSIBLE
ALTERNATE SHAPE**
(Other K bars not shown for clarity)

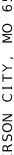
The K10-K11 bar combination may be furnished as one bar as shown, at the contractor's option.

All dimensions are out to out.



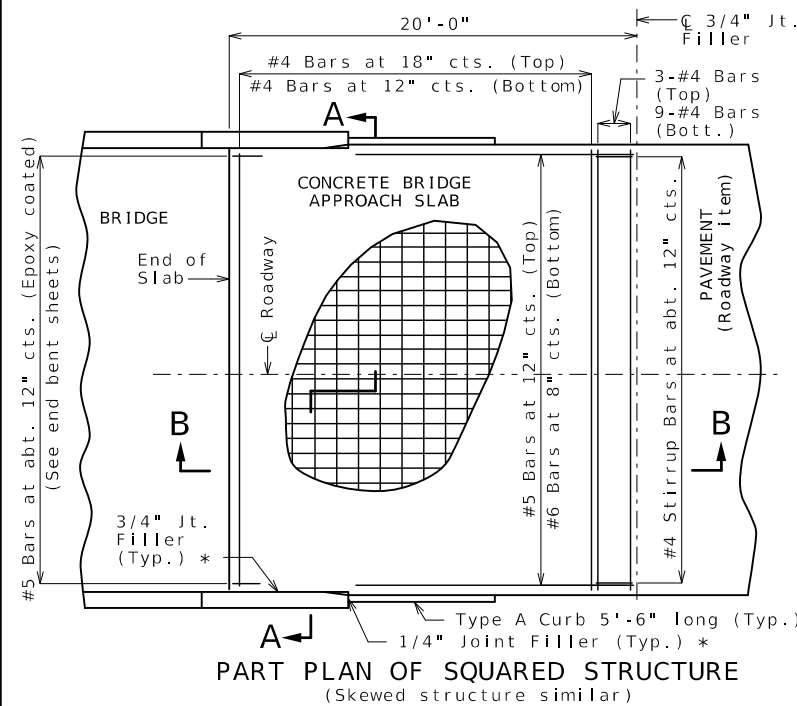
DATE PREPARED	
10/30/2023	
ROUTE	STATE
V	MO
DISTRICT	SHEET NO
BR	29
COUNTY	
GREENE	
JOB NO.	
J753486	
CONTRACT ID.	

PROJECT NO.
BRIDGE NO. A9119

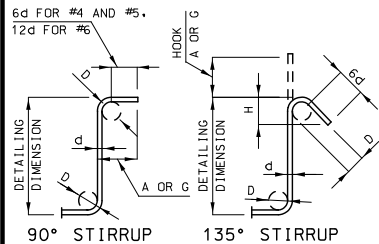
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MISSOURI HIGHWAYS AND TRANSPORTATION
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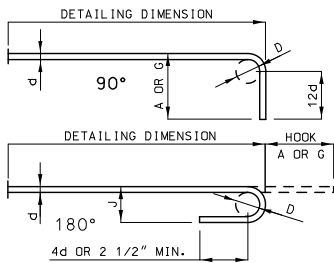


BILL OF REINFORCING STEEL																										
NO.	REQ'D.	MARK NO.	LOCATION	EPOXY (E)	SHAPE NO.	STIRRUP (S)	SUBSTR. (X)	VARIES (V)	NO. EACH	DIMENSIONS												NOMINAL LENGTH	ACTUAL LENGTH	WEIGHT		
										B		C		D		E		F		H					K	
										FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.				FT.	IN.
			SUBSTRUCTURE																							
			INT. BENT 2																							
21		6 D200	BEAM		20	X				2	6.000							2	6	2	6	79				
8		8 H200	BEAM		20	X				38	3.000							38	3	38	3	817				
10		4 H201	WEB		20	X				27	2.000							27	2	27	2	181				
3		8 H202	WEB		17	X				28	0.000							28	11	28	11	232				
16		6 H203	BEAM		10	S	X					22.000	3	7.750				7	4	7	0	168				
12		6 H204	BEAM		20	X				38	3.000							38	3	38	3	689				
8		8 H205	BEAM		17	X				38	3.000							39	2	39	2	837				
3		5 P200	SOCKET		35	X				3	0.000	6.000	29	10.000				581	2581	2		1818				
45		4 P201	BEAM		34	S	X			3	0.000							10	4	10	4	311				
36		5 U200	BEAM		13	S	X			2	6.000	4	3.000	2	6.000	4	3.000		14	5	14	1	529			
10		5 U201	BEAM		10	S	X					4	3.000	3	9.000			12	3	12	1	126				
20		5 U202	BEAM		13	S	X			3	9.000	4	3.000	3	9.000	4	3.000	16	11	16	7	346				
8		4 U203	BEAM		10	S	X					6.000	3	9.000				4	9	4	7	24				
4		5 U204	BEAM		10	S	X					4	3.000	2	6.000			11	0	10	10	45				
36		5 U205	WEB		19	S	X			7	1.000	18.000						8	7	8	6	319				
48		10 V200	SOCKET		17	X				33	10.000							35	3	35	3	7281				
			INT. BENT 3																							
21		6 D300	BEAM		20	X				2	6.000							2	6	2	6	79				
8		8 H300	BEAM		20	X				38	3.000							38	3	38	3	817				
10		4 H301	WEB		20	X				27	2.000							27	2	27	2	181				
3		8 H302	WEB		17	X				28	0.000							28	11	28	11	232				
16		6 H303	BEAM		10	S	X					22.000	3	7.750				7	4	7	0	168				
12		6 H304	BEAM		20	X				38	3.000							38	3	38	3	689				
8		8 H305	BEAM		17	X				38	3.000							39	2	39	2	837				
3		5 P300	SOCKET		35	X				3	0.000	6.000	35	9.000				690	1	690	1	2162				
45		4 P301	BEAM		34	S	X			3	0.000							10	4	10	4	311				
36		5 U300	BEAM		13	S	X			2	6.000	4	3.000	2	6.000	4	3.000		14	5	14	1	529			
10		5 U301	BEAM		10	S	X					4	3.000	3	9.000			12	3	12	1	126				
20		5 U302	BEAM		13	S	X			3	9.000	4	3.000	3	9.000	4	3.000	16	11	16	7	346				
8		4 U303	BEAM		10	S	X					6.000	3	9.000				4	9	4	7	24				
4		5 U304	BEAM		10	S	X					4	3.000	2	6.000			11	0	10	10	45				
36		5 U305	WEB		19	S	X			7	6.000	18.000						9	0	8	11	335				
48		10 V300	SOCKET		17	X				39	9.000							41	2	41	2	8503				
			INT. BENT 4																							
21		6 D400	BEAM		20	X				2	6.000							2	6	2	6	79				
8		8 H400	BEAM		20	X				38	3.000							38	3	38	3	817				
10		4 H401	WEB		20	X				27	2.000							27	2	27	2	181				
3		8 H402	WEB		17	X				28	0.000							28	11	28	11	232				



STIRRUP HOOK DIMENSIONS				
GRADES 40 - 50 - 60 KSI				
BAR SIZE	D (IN.)	90° HOOK A OR G	135° HOOK A OR G	APPROX. H
#4	2"	4 1/2"	4 1/2"	3"
#5	2 1/2"	6"	5 1/2"	3 3/4"
#6	4 1/2"	12"	8"	4 1/2"

NOTE: UNLESS OTHERWISE NOTED, DIAMETER "D" IS THE SAME FOR ALL BENDS AND HOOKS ON A BAR.



END HOOK DIMENSIONS				
ALL GRADES				
BAR SIZE	D (IN.)	180° HOOKS A OR G	90° HOOKS A OR G	
#3	2 1/4"	5"	3"	6"
#4	3"	6"	4"	8"
#5	3 3/4"	7"	5"	10"
#6	4 1/2"	8"	6"	12"
#7	5 1/4"	10"	7"	14"
#8	6"	11"	8"	16"
#9	9 1/2"	15"	11 3/4"	19"
#10	10 3/4"	17"	13 1/4"	22"
#11	12"	19"	14 3/4"	2'-0"
#14	18 1/4"	2'-3"	21 3/4"	2'-7"

NOTE:
ALL STANDARD HOOKS AND BENDS OTHER THAN 180 DEGREE ARE TO BE BENT WITH SAME PROCEDURE AS FOR 90 DEGREE STANDARD HOOKS.
HOOKS AND BENDS SHALL BE IN ACCORDANCE WITH THE PROCEDURES AS SHOWN ON THIS SHEET.
E = EPOXY COATED REINFORCEMENT.
S = STIRRUP.
X = BAR IS INCLUDED IN SUBSTRUCTURE QUANTITIES.
V = BAR DIMENSIONS VARY IN EQUAL INCREMENTS BETWEEN DIMENSIONS SHOWN ON THIS LINE AND THE FOLLOWING LINE.
NO. EA. = NUMBER OF BARS OF EACH LENGTH.
NOMINAL LENGTHS ARE BASED ON OUT TO OUT DIMENSIONS SHOWN IN BENDING DIAGRAMS AND ARE LISTED FOR FABRICATORS USE. (NEAREST INCH)
ACTUAL LENGTHS ARE MEASURED ALONG CENTERLINE BAR TO THE NEAREST INCH.
PAYWEIGHTS ARE BASED ON ACTUAL LENGTHS.
FOUR ANGLE OR CHANNEL SPACERS ARE REQUIRED FOR EACH COLUMN SPIRAL. SPACERS ARE TO BE PLACED ON INSIDE OF SPIRALS. LENGTH AND WEIGHT OF COLUMN SPIRALS DO NOT INCLUDE SPLICES OR SPACERS.
REINFORCING STEEL (GRADE 60) FY = 60,000 PSI.

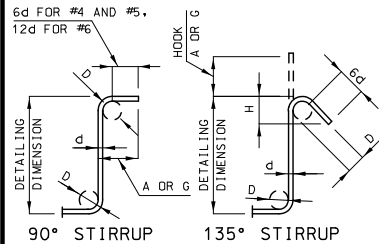
Detailed Sep. 2023
Checked Sep. 2023

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 31 of 35

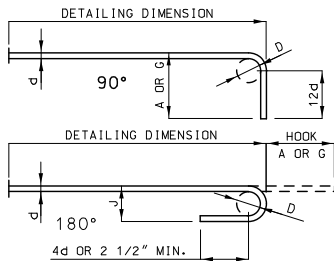
BILL OF REINFORCING STEEL																										
NO.	REQ'D.	MARK NO.	LOCATION	EPOXY (E)	SHAPE NO.	STIRRUP (S)	SUBSTR. (X)	VARIES (V)	NO. EACH	DIMENSIONS								NOMINAL LENGTH	ACTUAL LENGTH	WEIGHT						
										B		C		D		E					F		H		K	
										FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.				FT.	IN.	FT.	IN.	FT.	IN.
16		6 H403	BEAM		10	S	X					22.000	3	7.750				7	4	7	0	168				
12		6 H404	BEAM		20		X			38	3.000							38	3	38	3	689				
8		8 H405	BEAM		17		X			38	3.000							39	2	39	2	837				
3		5 P400	SOCKET		35	X				3	0.000	6.000	40	5.000				777	6777	6	2433					
45		4 P401	BEAM		34	S	X			3	0.000							10	4	10	4	311				
36		5 U400	BEAM		13	S	X			2	6.000	4	3.000	2	6.000	4	3.000	14	5	14	1	529				
10		5 U401	BEAM		10	S	X					4	3.000	3	9.000			12	3	12	1	126				
20		5 U402	BEAM		13	S	X			3	9.000	4	3.000	3	9.000	4	3.000	16	11	16	7	346				
8		4 U403	BEAM		10	S	X					6.000	3	9.000				4	9	4	7	24				
4		5 U404	BEAM		10	S	X					4	3.000	2	6.000			11	0	10	10	45				
36		5 U405	WEB		19	S	X			7	2.000	18.000						8	8	8	7	322				
48		10 V400	SOCKET		17		X			44	5.000							45	10	45	10	9467				
			SUPERSTRUCT.																							
			END BENT 1																							
3		6 F100	DIAPHRAGM	E	23	S				3	4.625	5	0.000				2	11.250	20.250	8	5	8	4	38		
9		6 F101	WING BRACE	E	15	S					13.875	3	7.000	2	3.125	23.500	13.500	12.000	7	0	6	11	93			
3		6 F102	DIAPHRAGM	E	21	S				3	4.625	6	6.000					2	11.250	20.250	9	11	9	6	43	
9		6 F103	WING BRACE	E	15	S				2	3.125	6	0.000		13.875	7.000	12.000	13.500	23.500	9	5	9	5	127		
12		8 H100	BEAM&DIAPH.	E	20					39	9.000							39	9	39	9	1274				
8		6 H101	BEAM&DIAPH.	E	20					39	9.000							39	9	39	9	478				
9		6 H102	DIAPHRAGM	E	20					9	4.000							9	4	9	4	126				
3		6 H103	DIAPHRAGM	E	20					6	7.000							6	7	6	7	30				
6		6 H104	DIAPHRAGM	E	20					3	4.000							3	4	3	4	30				
2		6 H105	DIAPHRAGM	E	20					2	0.000							2	0	2	0	6				
4		6 H106	BEAM	E	20					15	5.000							15	5	15	5	93				
16		8 H107	WING	E	20					14	6.000							14	6	14	6	619				
36		6 H108	WING	E	20					13	8.000							13	8	13	8	739				
4		5 H109	STRD. TIE	E	23						15.000	3	3.000		15.000	7.500	13.000	7.500	13.000	5	9	5	9	24		
32		5 U100	APP. TIE	E	19	S				2	0.000		15.000							3	3	3	2	106		
17		5 U101	BEAM	E	31	S				4	9.000	3	5.500	4	9.000			13	11	13	8	242				
2		4 U102	BEAM	E	10	S						2	8.000	3	5.500					8	10	8	8	12		
16		4 U103	BEAM	E	13	S				3	5.500	2	8.000	3	5.500	2	8.000			13	0	12	9	136		
6		4 U104	BEAM	E	10	S						2	11.000	3	5.500					9	4	9	2	37		
8		4 U105	BEAM	E	13	S				3	5.500	2	11.000	3	5.500	2	11.000			13	6	13	3	71		
25		6 U106	DIAPHRAGM	E	19	S				2	3.000	3	5.500							5	9	5	7	210		
50		6 U107	DIAPHRAGM	E	19	S				3	4.000	5	3.000							8	7	8	5	632		
25		5 U108	DIAPHRAGM	E	31	S				3	4.000	2	10.500	3	4.000					10	6	10	3	267		
24		6 V100	DIAPHRAGM	E	20					2	3.000									2	3	2	3	81		
16		5 V101	BEAM	E	17					4	9.000									5	4	5	4	89		
26		6 V102	WING	E	20					6	4.000									6	4	6	4	247		
26		6 V103	WING	E	20					6	7.000									6	7	6	7	257		
			END BENT 5																							
3		6 F500	DIAPHRAGM	E	23	S				3	4.625	5	0.000					2	11.250	20.250	8	5	8	4	38	
9		6 F501	WING BRACE	E	15	S					13.875	3	7.000	2	3.125	23.500	13.500	12.000	7.000	7	0	6	11	93		
3		6 F502	DIAPHRAGM	E	21	S				3	4.625	6	6.000					2	11.250	20.250	9	11	9	6	43	
9		6 F503	WING BRACE	E	15	S				2	3.125	6	0.000		13.875	7.000	12.000	13.500	23.500	9	5	9	5	127		

BILL OF REINFORCING STEEL																							
NO.	REQ'D.	MARK NO.	LOCATION	EPOXY (E)	SHAPE NO.	STIRRUP (S)	SUBSTR. (X)	VARIES (V)	NO. EACH	DIMENSIONS								NOMINAL LENGTH	ACTUAL LENGTH	WEIGHT			
										B	C	D	E	F	H	K							
										FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.
12	8	H500	BEAM&DIAPH.	E	20					39	9.000								39	9	39	9	1274
8	6	H501	BEAM&DIAPH.	E	20					39	9.000								39	9	39	9	478
9	6	H502	DIAPHRAGM	E	20					9	4.000								9	4	9	4	126
3	6	H503	DIAPHRAGM	E	20					6	7.000								6	7	6	7	30
6	6	H504	DIAPHRAGM	E	20					3	4.000								3	4	3	4	30
2	6	H505	DIAPHRAGM	E	20					2	0.000								2	0	2	0	6
4	6	H506	BEAM	E	20					15	5.000								15	5	15	5	93
16	8	H507	WING	E	20					14	6.000								14	6	14	6	619
36	6	H508	WING	E	20					13	8.000								13	8	13	8	739
4	5	H509	STRD. TIE	E	23					15.000	3	3.000	15.000	7.500	13.000	7.500	13.000	5	9	5	9	24	
32	5	U500	APP. TIE	E	19	S				2	0.000	15.000						3	3	3	2	106	
17	5	U501	BEAM	E	31	S				4	9.000	3	5.500	4	9.000			13	11	13	8	242	
2	4	U502	BEAM	E	10	S					2	8.000	3	5.500				8	10	8	8	12	
16	4	U503	BEAM	E	13	S				3	5.500	2	8.000	3	5.500	2	8.000	13	0	12	9	136	
6	4	U504	BEAM	E	10	S					2	11.000	3	5.500				9	4	9	2	37	
8	4	U505	BEAM	E	13	S				3	5.500	2	11.000	3	5.500	2	11.000	13	6	13	3	71	
25	6	U506	DIAPHRAGM	E	19	S				2	3.000	3	5.500					5	9	5	7	210	
50	6	U507	DIAPHRAGM	E	19	S				3	4.000	5	3.000					8	7	8	5	632	
25	5	U508	DIAPHRAGM	E	31	S				3	4.000	2	10.500	3	4.000			10	6	10	3	267	
24	6	V500	DIAPHRAGM	E	20					2	3.000							2	3	2	3	81	
16	5	V501	BEAM	E	17					4	9.000							5	4	5	4	89	
26	6	V502	WING	E	20					6	4.000							6	4	6	4	247	
26	6	V503	WING	E	20					6	7.000							6	7	6	7	257	
			DIAPHRAGMS																				
			AT INT BENT																				
			NO 2,3,4																				
18	6	H600	DIAPHRAGM	E	20					9	4.000							9	4	9	4	252	
18	6	H601	DIAPHRAGM	E	20					6	7.000							6	7	6	7	178	
36	4	H602	DIAPHRAGM	E	20					9	8.000							9	8	9	8	232	
4	5	H603	STRD. TIE	E	23					15.000	3	3.000				7.500	13.000	4	6	4	6	19	
4	5	H604	STRD. TIE	E	23					15.000	3	3.000	15.000	7.500	13.000	7.500	13.000	5	9	5	9	24	
90	4	U600	DIAPHRAGM	E	28	S					2	11.000	3	2.000	18.000			7	7	7	5	446	
36	6	U601	DIAPHRAGM	E	28	S					3	9.000	3	2.000	2	2.000		9	1	8	9	473	
36	6	U602	DIAPHRAGM	E	28	S					3	9.000	3	2.000	2	2.000		9	1	8	9	473	
24	6	U603	DIAPHRAGM	E	28	S					22.000	3	2.000	2	2.000			7	2	6	10	246	
36	5	U604	DIAPHRAGM	E	11	S	V	12		4	7.000	12.000	19.000					7	2	7	0		
			INCREMENT =								4	10.000	12.000	19.000				7	5	7	3	268	
			1.500 INCH																				
12	5	U605	DIAPHRAGM	E	19	S				4	6.000	12.000						5	6	5	5	68	
24	5	V600	DIAPHRAGM	E	20					3	5.000							3	5	3	5	86	
			SLAB																				
174	8	S1	SLAB	E	20					56	1.000							56	1	56	1	26055	
36	5	S2	SLAB	E	20					54	8.000							54	8	54	8	2053	
74	6	S3	SLAB	E	20		V	2		2	6.000							2	6	2	6		
			INCREMENT =							33	8.000							33	8	33	8	2010	
			10.375 INCH																				
586	6	S4	SLAB	E	20					34	5.000							34	5	34	5	30293	
28	8	S5	SLAB	E	20					34	6.000							34	6	34	6	2579	
26	8	S6	SLAB	E	20					27	9.000							27	9	27	9	1926	
54	8	S7	SLAB	E	20					32	9.000							32	9	32	9	4722	
618	5	S8	SLAB	E	20					4	11.000							4	11	4	11	3169	



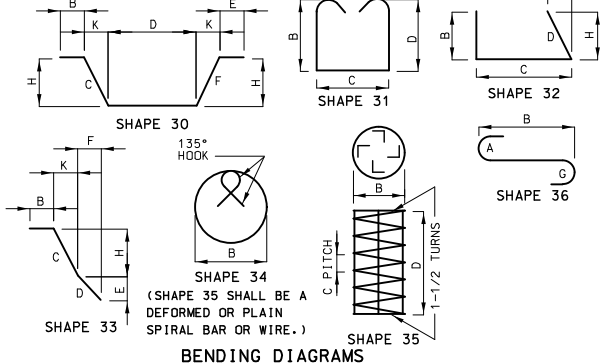
STIRRUP HOOK DIMENSIONS				
GRADES 40 - 50 - 60 KSI				
BAR SIZE	D (IN.)	90° HOOK A OR G	135° HOOK A OR G	APPROX. H
#4	2"	4 1/2"	4 1/2"	3"
#5	2 1/2"	6"	5 1/2"	3 3/4"
#6	4 1/2"	12"	8"	4 1/2"

NOTE: UNLESS OTHERWISE NOTED, DIAMETER "D" IS THE SAME FOR ALL BENDS AND HOOKS ON A BAR.



END HOOK DIMENSIONS				
ALL GRADES				
BAR SIZE	D (IN.)	180° HOOKS A OR G	J	90° HOOKS A OR G
#3	2 1/4"	5"	3"	6"
#4	3"	6"	4"	8"
#5	3 3/4"	7"	5"	10"
#6	4 1/2"	8"	6"	12"
#7	5 1/4"	10"	7"	14"
#8	6"	11"	8"	16"
#9	9 1/2"	15"	11 3/4"	19"
#10	10 3/4"	17"	13 1/4"	22"
#11	12"	19"	14 3/4"	2'-0"
#14	18 1/4"	2'-3"	21 3/4"	2'-7"

NOTE:
ALL STANDARD HOOKS AND BENDS OTHER THAN 180 DEGREE ARE TO BE BENT WITH SAME PROCEDURE AS FOR 90 DEGREE STANDARD HOOKS.
HOOKS AND BENDS SHALL BE IN ACCORDANCE WITH THE PROCEDURES AS SHOWN ON THIS SHEET.
E = EPOXY COATED REINFORCEMENT.
S = STIRRUP.
X = BAR IS INCLUDED IN SUBSTRUCTURE QUANTITIES.
V = BAR DIMENSIONS VARY IN EQUAL INCREMENTS BETWEEN DIMENSIONS SHOWN ON THIS LINE AND THE FOLLOWING LINE.
NO. EA. = NUMBER OF BARS OF EACH LENGTH.
NOMINAL LENGTHS ARE BASED ON OUT TO OUT DIMENSIONS SHOWN IN BENDING DIAGRAMS AND ARE LISTED FOR FABRICATORS USE. (NEAREST INCH)
ACTUAL LENGTHS ARE MEASURED ALONG CENTERLINE BAR TO THE NEAREST INCH.
PAYWEIGHTS ARE BASED ON ACTUAL LENGTHS.
FOUR ANGLE OR CHANNEL SPACERS ARE REQUIRED FOR EACH COLUMN SPIRAL. SPACERS ARE TO BE PLACED ON INSIDE OF SPIRALS. LENGTH AND WEIGHT OF COLUMN SPIRALS DO NOT INCLUDE SPLICES OR SPACERS.
REINFORCING STEEL (GRADE 60) FY = 60,000 PSI.

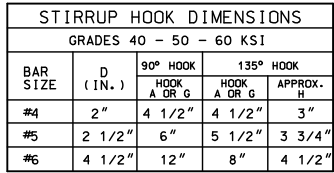


Detailed Sep. 2023
Checked Sep. 2023

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 32 of 35

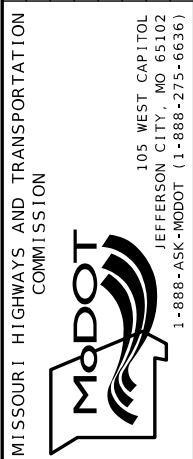
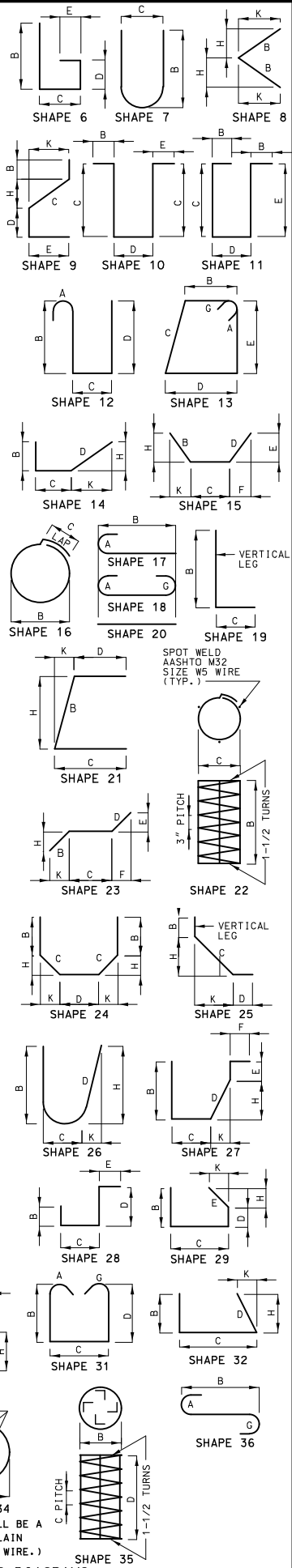
BILL OF REINFORCING STEEL																							
NO.	REQ'D.	MARK NO.	LOCATION	EPOXY (E)	SHAPE NO.	STIRRUP (S)	SUBSTR. (X)	VARIES (V)	NO. EACH	DIMENSIONS								NOMINAL LENGTH	ACTUAL LENGTH	WEIGHT			
										B	C	D	E	F	H	K							
										FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.
28	8	S9	SLAB	E	20					39	3.000							39	3	2934			
26	8	S10	SLAB	E	20					35	3.000							35	3	2447			
			TYPE D																				
			BARRIER																				
20	5	K1	BARRIER	E	27	S				3	8.000	9.250	5.375	3	2.750		5.250	1.000	8	1	165		
82	5	K2	BARRIER	E	27	S				3	8.000	9.250	14.500	2	5.750		14.250	2.750	8	2	677		
4	5	K3	BARRIER	E	27	S				22.500	9.250	14.750	7.750	12.000	14.500	2.750	5	6	5	3	22		
20	5	K4	BARRIER	E	19	S	V	4		2	4.250	10.000						3	2	3	1		
			INCREMENT =							2	6.250	10.000						3	4	3	66		
			0.500 INCH																				
20	5	K5	BARRIER	E	14	S	V	4		8.250	9.500	18.500			4.000	18.000	3	0	2	11			
			INCREMENT =							8.250	9.500	20.500			4.500	20.000	3	2	3	1	63		
			0.500 INCH																				
12	5	K6	BARRIER	E	19	S	V	4		2	6.750	10.000						3	5	3	4		
			INCREMENT =							2	7.750	10.000						3	6	3	5	42	
			0.500 INCH																				
12	5	K7	BARRIER	E	21	S	V	4		2	6.625	10.000			2	6.000	6.250	3	5	3	3		
			INCREMENT =							2	7.625	10.000			2	7.000	6.500	3	6	3	4	41	
			0.500 INCH																				
36	5	K8	BARRIER	E	19	S	V	4		2	8.500	10.000						3	7	3	5		
			INCREMENT =							3	2.500	10.000						4	1	3	11	138	
			0.750 INCH																				
36	5	K9	BARRIER	E	21	S	V	4		2	8.500	10.000			2	7.750	6.750	3	7	3	5		
			INCREMENT =							3	2.500	10.000			3	1.750	7.750	4	1	3	11	138	
			0.750 INCH																				
38	5	K10	BARRIER	E	19	S				3	3.000	10.000						4	1	4	0	159	
38	5	K11	BARRIER	E	21	S				3	3.000	10.000			3	2.250	7.750	4	1	3	11	155	
24	5	K12	BARRIER	E	20					16	3.000							16	3	16	3	407	
12	5	K13	BARRIER	E	20		V	4		9	6.000							9	6	9	6		
			INCREMENT =							15	6.000							15	6	15	6	156	
			36.000 INCH																				
24	5	K14	BARRIER	E	20					15	6.000							15	6	15	6	388	
12	5	K15	BARRIER	E	20		V	4		8	9.000							8	9	8	9		
			INCREMENT =							14	9.000							14	9	14	9	147	
			36.000 INCH																				
630	5	R1	BARRIER	E	26					3	3.000	5.500	3	3.625		3	3.000	6.750	6	9	6	9	4435
630	5	R2	BARRIER	E	19	S				20.500	9.500							2	6	2	5	1588	
630	5	R3	BARRIER	E	27	S				9.500	15.250	5.000	12.000	15.000	3.000			3	6	3	4	2190	
120	5	R4	BARRIER	E	20					11	9.000							11	9	11	9	1471	
10	5	R5	BARRIER	E	20					45	3.000							45	3	45	3	472	
80	5	R6	BARRIER	E	20					31	11.000							31	11	31	11	2663	
20	5	R7	BARRIER	E	20					34	7.000							34	7	34	7	721	
10	5	R8	BARRIER	E	20					46	0.000							46	0	46	0	480	
20	5	R9	BARRIER	E	20					34	2.000							34	2	34	2	713	
			SLIP FORM OPTION																				
64	5	C1	SLIP FORM	E	20					12	0.000							12	0	12	0	801	
			TOTALS																				

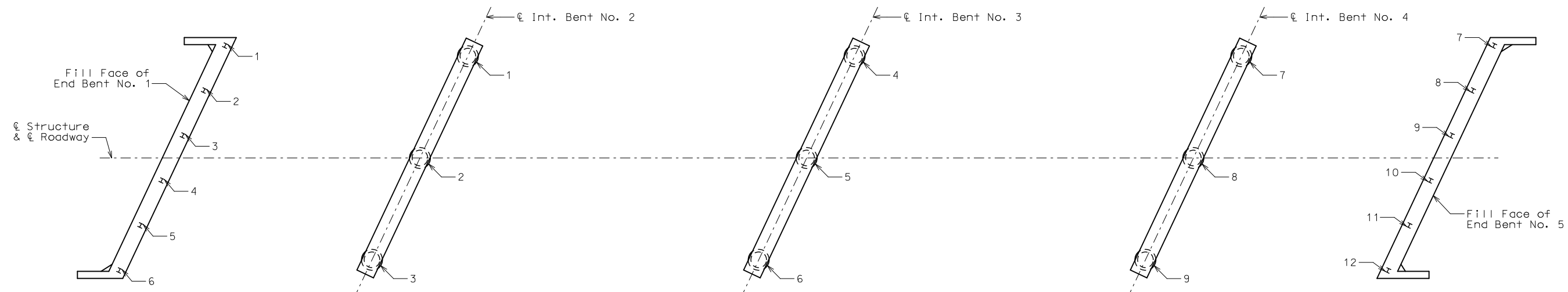
[illegible]

END HOOK DIMENSIONS					
BAR SIZE	D. (IN.)	ALL GRADES			
		180° HOOKS		90° HOOKS	
		A	R	A	R
#3	2 1/4"	5"	3"	6"	
#4	3"	6"	4"	8"	
#5	3 3/4"	7"	5"	10"	
#6	4 1/2"	8"	6"	12"	
#7	5 1/4"	10"	7"	14"	
#8	6"	11"	8"	16"	
#9	9 1/2"	15"	11 3/4"	19"	
#10	10 3/4"	17"	13 1/4"	22"	
#11	12"	19"	14 3/4"	2'-0"	
#14	18 1/4"	2'-3"	21 3/4"	2'-7"	

FOUR ANGLE OR CHANNEL SPACERS ARE REQUIRED FOR EACH COLUMN SPIRAL. SPACERS ARE TO BE PLACED ON INSIDE OF SPIRALS. LENGTH AND WEIGHT OF COLUMN SPIRALS DO NOT INCLUDE SPLICES OR SPACERS.

REINFORCING STEEL (GRADE 60) $F_y = 60,000$ PSI.

[illegible]



PART PLAN SHOWING PILE & DRILLED SHAFT NUMBERING FOR RECORDING AS-BUILT PILE DATA
& AS-BUILT DRILLED SHAFT DATA

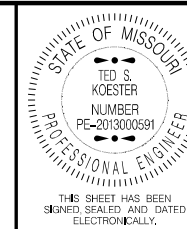
As-Built Pile Data			
Pile No.	Length in Place (ft)	Computed Nominal Axial Compressive Resistance (kips)	Remarks
			End Bent No. 1
1			
2			
3			
4			
5			
6			
			End Bent No. 5
7			
8			
9			
10			
11			
12			

As-Built Drilled Shaft Data				
Shaft No.	Top of Sound Rock (Elev.)	Tip of Casing (Elev.)	Bottom of Rock Socket (Elev.)	Remarks
				Int. Bent No. 2
1				
2				
3				
				Int. Bent No. 3
4				
5				
6				
				Int. Bent No. 4
7				
8				
9				

Note:
Indicate in remarks column:
A. Pile type and grade
B. Batter
C. Driven to practical refusal

This sheet to be completed by MoDOT construction personnel.

AS-BUILT PILE DATA & AS-BUILT DRILLED SHAFT DATA



DATE PREPARED
10/30/2023

ROUTE	STATE
11	MO

V	MO
CO = 1,410,000,000, 1,410,000	CO = 1,410,000,000, 1,410,000

DISTRICT	SHEET NO.
RR	34

BR	54
COUNTY	

GREENE

JOB NO.
1762486

J7S3486

CONTRACT ID.

CONCLUSIONS

PROJECT NO.

1990-1991, 1991-1992, 1992-1993, 1993-1994, 1994-1995, 1995-1996, 1996-1997, 1997-1998, 1998-1999, 1999-2000, 2000-2001, 2001-2002, 2002-2003, 2003-2004, 2004-2005, 2005-2006, 2006-2007, 2007-2008, 2008-2009, 2009-2010, 2010-2011, 2011-2012, 2012-2013, 2013-2014, 2014-2015, 2015-2016, 2016-2017, 2017-2018, 2018-2019, 2019-2020, 2020-2021, 2021-2022, 2022-2023, 2023-2024, 2024-2025, 2025-2026, 2026-2027, 2027-2028, 2028-2029, 2029-2030, 2030-2031, 2031-2032, 2032-2033, 2033-2034, 2034-2035, 2035-2036, 2036-2037, 2037-2038, 2038-2039, 2039-2040, 2040-2041, 2041-2042, 2042-2043, 2043-2044, 2044-2045, 2045-2046, 2046-2047, 2047-2048, 2048-2049, 2049-2050, 2050-2051, 2051-2052, 2052-2053, 2053-2054, 2054-2055, 2055-2056, 2056-2057, 2057-2058, 2058-2059, 2059-2060, 2060-2061, 2061-2062, 2062-2063, 2063-2064, 2064-2065, 2065-2066, 2066-2067, 2067-2068, 2068-2069, 2069-2070, 2070-2071, 2071-2072, 2072-2073, 2073-2074, 2074-2075, 2075-2076, 2076-2077, 2077-2078, 2078-2079, 2079-2080, 2080-2081, 2081-2082, 2082-2083, 2083-2084, 2084-2085, 2085-2086, 2086-2087, 2087-2088, 2088-2089, 2089-2090, 2090-2091, 2091-2092, 2092-2093, 2093-2094, 2094-2095, 2095-2096, 2096-2097, 2097-2098, 2098-2099, 2099-2100, 2100-2101, 2101-2102, 2102-2103, 2103-2104, 2104-2105, 2105-2106, 2106-2107, 2107-2108, 2108-2109, 2109-2110, 2110-2111, 2111-2112, 2112-2113, 2113-2114, 2114-2115, 2115-2116, 2116-2117, 2117-2118, 2118-2119, 2119-2120, 2120-2121, 2121-2122, 2122-2123, 2123-2124, 2124-2125, 2125-2126, 2126-2127, 2127-2128, 2128-2129, 2129-2130, 2130-2131, 2131-2132, 2132-2133, 2133-2134, 2134-2135, 2135-2136, 2136-2137, 2137-2138, 2138-2139, 2139-2140, 2140-2141, 2141-2142, 2142-2143, 2143-2144, 2144-2145, 2145-2146, 2146-2147, 2147-2148, 2148-2149, 2149-2150, 2150-2151, 2151-2152, 2152-2153, 2153-2154, 2154-2155, 2155-2156, 2156-2157, 2157-2158, 2158-2159, 2159-2160, 2160-2161, 2161-2162, 2162-2163, 2163-2164, 2164-2165, 2165-2166, 2166-2167, 2167-2168, 2168-2169, 2169-2170, 2170-2171, 2171-2172, 2172-2173, 2173-2174, 2174-2175, 2175-2176, 2176-2177, 2177-2178, 2178-2179, 2179-2180, 2180-2181, 2181-2182, 2182-2183, 2183-2184, 2184-2185, 2185-2186, 2186-2187, 2187-2188, 2188-2189, 2189-2190, 2190-2191, 2191-2192, 2192-2193, 2193-2194, 2194-2195, 2195-2196, 2196-2197, 2197-2198, 2198-2199, 2199-2200, 2200-2201, 2201-2202, 2202-2203, 2203-2204, 2204-2205, 2205-2206, 2206-2207, 2207-2208, 2208-2209, 2209-2210, 2210-2211, 2211-2212, 2212-2213, 2213-2214, 2214-2215, 2215-2216, 2216-2217, 2217-2218, 2218-2219, 2219-2220, 2220-2221, 2221-2222, 2222-2223, 2223-2224, 2224-2225, 2225-2226, 2226-2227, 2227-2228, 2228-2229, 2229-2230, 2230-2231, 2231-2232, 2232-2233, 2233-2234, 2234-2235, 2235-2236, 2236-2237, 2237-2238, 2238-2239, 2239-2240, 2240-2241, 2241-2242, 2242-2243, 2243-2244, 2244-2245, 2245-2246, 2246-2247, 2247-2248, 2248-2249, 2249-2250, 2250-2251, 2251-2252, 2252-2253, 2253-2254, 2254-2255, 2255-2256, 2256-2257, 2257-2258, 2258-2259, 2259-2260, 2260-2261, 2261-2262, 2262-2263, 2263-2264, 2264-2265, 2265-2266, 2266-2267, 2267-2268, 2268-2269, 2269-2270, 2270-2271, 2271-2272, 2272-2273, 2273-2274, 2274-2275, 2275-2276, 2276-2277, 2277-2278, 2278-2279, 2279-2280, 2280-2281, 2281-2282, 2282-2283, 2283-2284, 2284-2285, 2285-2286, 2286-2287, 2287-2288, 2288-2289, 2289-2290, 2290-2291, 2291-2292, 2292-2293, 2293-2294, 2294-2295, 2295-2296, 2296-2297, 2297-2298, 2298-2299, 2299-2300, 2300-2301, 2301-2302, 2302-2303, 2303-2304, 2304-2305, 2305-2306, 2306-2307, 2307-2308, 2308-2309, 2309-2310, 2310-2311, 2311-2312, 2312-2313, 2313-2314, 2314-2315, 2315-2316, 2316-2317, 2317-2318, 2318-2319, 2319-2320, 2320-2321, 2321-2322, 2322-2323, 2323-2324, 2324-2325, 2325-2326, 2326-2327, 2327-2328, 2328-2329, 2329-2330, 2330-2331, 2331-2332, 2332-2333, 2333-2334, 2334-2335, 2335-2336, 2336-2337, 2337-2338, 2338-2339, 2339-2340, 2340-2341, 2341-2342, 2342-2343, 2343-2344, 2344-2345, 2345-2346, 2346-2347, 2347-2348, 2348-2349, 2349-2350, 2350-2351, 2351-2352, 2352-2353, 2353-2354, 2354-2355, 2355-2356, 2356-2357, 2357-2358, 2358-2359, 2359-2360, 2360-2361, 2361-2362, 23

BRIDGE NO.
A9119

[illegible]MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION

105 WEST CAPITOL
JEFFERSON CITY, MO 65102

