DESIGN DESIGNATION

ROUTE 6

A.A.D.T. - 2022 = 3,788

A.A.D.T. - 2042 = 4,185

T = 7.3%

 $V \ = \ 45 \ \text{M.P.H.}$ FUNCTIONAL CLASSIFICATION - PRINCIPAL ARTERIAL

US-61

A.A.D.T. - 2022 = 14,697 A.A.D.T. - 2042 = 17,933 T = 24.2%

FUNCTIONAL CLASSIFICATION - FREEWAY AND EXPRESSWAY

V = 65 M,P,H,

NO RIGHT-OF-WAY ACQUISITION

CONVENTIONAL SYMBOLS

| (USED IN PLANS) | | |
|--|-------------|--|
| | EXISTING | NEW |
| BUILDINGS AND STRUCTURES GUARD RAIL GUARD CABLE CONCRETE RIGHT-OF-WAY MARKER STEEL RIGHT-OF-WAY MARKER LOCATION SURVEY MARKER UTILITIES | 00000 | •••• |
| FIBER OPTICS OVERHEAD CABLE TV UNDERGROUND CABLE TV OVERHEAD TELEPHONE UNDERGROUND TELEPHONE OVERHEAD POWER UNDERGROUND POWER SANITARY SEWER STORM SEWER GAS WATER | - F0 | -F0- -OTV- -UTV- -OE- -UE- -S- -SS- -G- -W |
| MANHOLE | SAN |) |
| FIRE HYDRANT | HYD | } |
| WATER VALVE | ₩V WM |) |
| WATER METER | " "⊕ |) |
| DROP INLET | " | |
| DITCH BLOCK | = | = |
| GROUND MOUNTED SIGN | SIGN | - |
| LIGHT POLE | | |
| H-FRAME POWER POLE | | |
| TELEPHONE PEDESTAL FENCE CHAIN LINK WOVEN WIRE GATE POST BENCHMARK | PED | |
| DEITOTIMETAL | × |) |

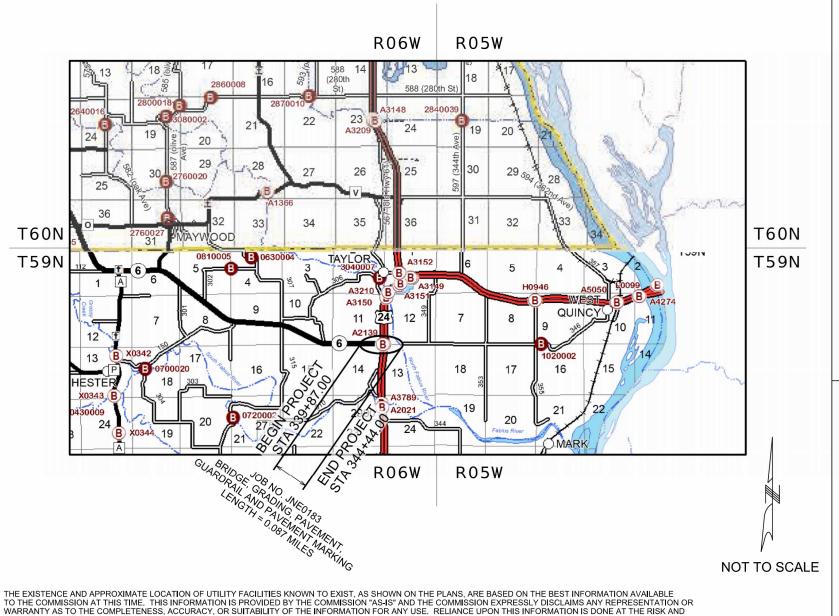
NOTE: DASHED OR OPEN SYMBOLS INDICATE EXISTING FEATURES

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION CONSTRUCTION PLANS FOR PROPOSED ROUTE 6



WITH THE LISTED UTILITIES.

MARION COUNTY



PERIL OF THE USER, AND THE COMMISSION SHALL NOT BE LIABLE FOR ANY DAMAGES THAT MAY ARISE FROM ANY ERROR IN THE INFORMATION. IT IS, THEREFORE, THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY THE EXISTENCE, LOCATION AND STATUS OF ANY FACILITY. SUCH VERIFICATION INCLUDES DIRECT CONTACT

INDEX OF SHEETS

| DESCRIPTION | SHEET NUMBER |
|----------------------------------|-----------------|
| TITLE SHEET | 1 |
| QUANTITIES (QU) (2 SHEETS) | 3 |
| PLAN SHEET (PL) (1 SHEET) | 4 |
| PROFILE SHEET (PF) (1 SHEET) | 5 |
| TRAFFIC CONTROL (TC) (14 SHEETS) | 6 - 19 |
| BRIDGE PLANS (B) (27 SHEETS) | 1 - 27 |
| | |

| THIS SHEET SIGNED, SE | MISSOLUTION AL BIOCHARD AND CITRONICALLY. | ELECTRONICALLY SEALED AND DATED. |
|-----------------------|---|----------------------------------|
| 4/2/ ROUTE | REPARED 2024 STATE | ECTRON I |
| 6 DISTRICT | MO SHEET NO. | 1 |
| NE | 1 | BEEN |
| MAR | ION | HAS |
| J NE (| NO. 0183 | L |
| | | |
| | CT NO | |
| PROJE | CT NO. | |
| | CT NO. | THIS SHEET |
| PROJE | CT NO. | |



LENGTH OF PROJECT

ROUTE 6

BEGINNING OF PROJECT STA 339+87.00 END OF PROJECT STA 344+44.00

APPARENT LENGTH 457.0 FEET

EQUATIONS AND EXCEPTIONS: NONE

0.00 FEET

 TOTAL CORRECTIONS
 NONE

 TOTAL BRIDGE EXCEPTIONS
 0.0 FEET

 NET LENGTH OF PROJECT
 0.087 MILES

 STATE LENGTH
 0.087 MILES

 FOR INFORMATION ONLY
 0.11 ACRES

 ESTIMATED DISTURBED ACRES
 0.11 ACRES

HDR Engineering, Inc.
S. 18th Street

HDR Eng 401 S. 18th Street Sulte 300 St. Louis, MO 63103-2267 314-425-8300

BRIDGE EXCEPTIONS

| | REMOVAL OF IMPROVEMENTS | | | | | | | | | |
|-------|--|--------|---------|----------|---------------------------|---------|--|--|--|--|
| | FROM | TO | | | | | | | | |
| S NO. | STA | STA | ROADWAY | LOCATION | DESCRIPTION | REMARKS | | | | |
| 1 | 339+99 | 344+22 | ROUTE 6 | RT | GUARDRA I L | 423 LF | | | | |
| 2 | 340+03 | 344+23 | ROUTE 6 | LT | GUARDRA I L | 420 LF | | | | |
| 3 | 340+58 | 340+73 | ROUTE 6 | RT/LT | APPROACH SLAB | 66.7 SY | | | | |
| 4 | 343+58 | 343+73 | ROUTE 6 | RT/LT | APPROACH SLAB | 66.7 SY | | | | |
| 5 | 340+86 | 341+34 | ROUTE 6 | RT/LT | CONCRETE SLOPE PROTECTION | 219 SY | | | | |
| 6 | 343+05 | 343+53 | ROUTE 6 | RT/LT | CONCRETE SLOPE PROTECTION | 180 SY | | | | |
| 7 | 340+15 | 340+58 | ROUTE 6 | RT/LT | RT/LT PAVEMENT | | | | | |
| 8 | 8 343+73 344+00 ROUTE 6 RT/LT PAVEMENT | | | | 119 SY | | | | | |
| | | | L | .UMP SUM | = 1 | | | | | |

| MOB | ILIZ | ATION |
|------|-------|-------|
| LUMP | SUM = | 1 |

| CONTRACTOR | FURNISHED | SURVEYING | & | STAKING | | | | |
|--------------|-----------|-----------|---|---------|--|--|--|--|
| LUMP SUM = 1 | | | | | | | | |

| CONCRETE APPROACH PAVEMENT | | | | | | | | | |
|----------------------------|--------|--------|---------|-----------|----------------|--|--|--|--|
| | | | | | CONC. APPROACH | | | | |
| SHEET | FROM | TO | | | PAVMENT | | | | |
| NO. | STA | STA | ROADWAY | LOCATION | SY | | | | |
| 4 | 340+58 | 340+73 | ROUTE 6 | RT/LT | 66.7 | | | | |
| 4 | 343+58 | 343+73 | ROUTE 6 | RT/LT | 66.7 | | | | |
| | | | | | | | | | |
| | | | | SUBTOTAL | 133.4 | | | | |
| | | | | PAY TOTAL | 133.4 | | | | |

| | PERMANENT EROSION CONTROL | | | | | | | | | | |
|-------|---------------------------|--------|---------|-----------|-------------|---------|-----------------|--|--|--|--|
| | | | | | TYPE 2 ROCK | BLANKET | PERMANENT | | | | |
| PLAN | | | | | | | EROSION CONTROL | | | | |
| SHEET | FROM | ТО | | | FURNISHING | PLACING | GEOTEXTILE | | | | |
| NO. | STA | STA | ROADWAY | LOCATION | CY | CY | SY | | | | |
| 4 | 340+86 | 341+34 | ROUTE 6 | RT/LT | 153 | 153 | 259 | | | | |
| 4 | 343+05 | 343+53 | ROUTE 6 | RT/LT | 127 | 127 | 216 | | | | |
| | | | | | | | | | | | |
| | | | | SUBTOTAL | 280 | 280 | 475 | | | | |
| | | | | PAY TOTAL | 280 | 280 | 475 | | | | |

| CONCRETE PAVEMENT | | | | | | | | | |
|-------------------|--------|--------|---------|----------|-----------|------------|---------------|--|--|
| | | | | | | 8" NON- | TYPE 1 AGG | | |
| | | | | | | REINFORCED | FOR BASE | | |
| SHEET | FROM | TO | | | PVMT | JPCP | (4 IN. THICK) | | |
| NO. | STA | STA | ROADWAY | LOCATION | WIDTH | SY | SY | | |
| 4 | 340+15 | 340+58 | ROUTE 6 | RT/LT | 40' | 204.6 | 205 | | |
| 4 | 343+73 | 344+00 | ROUTE 6 | RT/LT | 40' | 118.7 | 119 | | |
| | | | | | | | | | |
| | | | | | SUBTOTAL | 323.3 | 324 | | |
| | | | | | PAY TOTAL | 323.3 | 324 | | |

| | GUARDRA I L | | | | | | | | | | |
|-----------|-------------|--------|----------|-----------|-----------|------------|--------------|--------|-----------|--|--|
| | | | | | | | TYPE A | | | | |
| | | | | | MGS | MGS BRIDGE | CRASHWORTHY | MGS | SHAPING | | |
| | | | | | GUARDRAIL | APPROACH | END TERMINAL | END | SLOPES, | | |
| | FROM | TO | | | 6' POSTS | TRANSITION | (MASH) | ANCHOR | CLASS III | | |
| SHEET NO. | STA | STA | LOCATION | ROADWAY | LF | EA | EA | EA | 100F | | |
| 4 | 339+87 | 340+75 | RT | ROUTE 6 | 75 | 1 | | 1 | 1.2 | | |
| 4 | 339+87 | 340+75 | LT | ROUTE 6 | 87.5 | 1 | 1 | | 1.7 | | |
| 4 | 343+56 | 344+44 | RT | ROUTE 6 | 125 | 1 | 1 | | 2.1 | | |
| 4 | 343+56 | 344+39 | LT | ROUTE 6 | 87.5 | 1 | | 1 | 1.3 | | |
| | | | | | | | | | | | |
| | | | | SUBTOTAL | 375 | 4 | 2 | 2 | 6.3 | | |
| | | | | PAY TOTAL | 375 | 4 | 2 | 2 | 6 | | |

| SEEDING AND MULCHING | | | | | | | | |
|----------------------|---------|-----------|----------------|----------|--|--|--|--|
| | | | SEEDING - COOL | | | | | |
| SHEET | | | SEASON MIXTURE | MULCHING | | | | |
| NO. | ROADWAY | LOCATION | ACRE | ACRE | | | | |
| 4 | ROUTE 6 | RT | 0.2 | 0.2 | | | | |
| | | | | | | | | |
| | | SUBTOTAL | 0.2 | 0.2 | | | | |
| | | PAY TOTAL | 0.2 | 0.2 | | | | |

| TEMPORARY EROSION CONTROL | | | | | | | | | |
|---------------------------|--------|--------|---------|-----------|-------|----------|--|--|--|
| | | | | | SILT | SEDIMENT | | | |
| | FROM | TO | | | FENCE | REMOVAL | | | |
| SHEET NO. | STA | STA | ROADWAY | LOCATION | LF | CY | | | |
| 4 | 339+97 | 341+44 | ROUTE 6 | RT/LT | 456 | 5 | | | |
| 4 | 342+95 | 344+33 | ROUTE 6 | RT/LT | 490 | 5 | | | |
| | | | | | | | | | |
| | | | | SUBTOTAL | 946 | 10 | | | |
| | | | | PAY TOTAL | 946 | 10 | | | |

| | PERMANENT STRIPING | | | | | |
|-------|--------------------|--------------|--------------|--|--|--|
| | | 6" WHITE | 4" YELLOW | | | |
| | | HIGH BUILD | HIGH BUILD | | | |
| | | WATERBORNE | WATERBORNE | | | |
| | | TYPE L BEADS | TYPE L BEADS | | | |
| SHEET | | SOLID | SOLID | | | |
| NO. | ROADWAY | LF | LF | | | |
| 4 | ROUTE 6 | 770 | 770 | | | |
| | | | | | | |
| | TOTAL LENGTH | 770 | 770 | | | |
| | ADJUSTMENT FACTOR | 1 | 1 | | | |
| | SUBTOTAL | 770 | 770 | | | |
| | PAY TOTAL | 770 | 770 | | | |

ELEME COLD PROPERTY OF THE SHEET HAS BEEN

THIS SHEET HAS BEEN SIGNED, SEALED, AND DATED ELECTRONICALLY.

DATE PREPARED
4/2/2024
ROUTE STATE
6 MO
DISTRICT SHEET NO.
NE 3
COUNTY
MAR I ON

COUNTY
MAR I ON
JOB NO.
JNE 0 1 8 3
CONTRACT ID.

PROJECT NO

BRIDGE NO

DATE DESCRIPTION

SOURT HIGHWAYS AND TRANSPORTATION
COMMISSION

MODOT

105 WEST CAPITO
JEFFERSON CITY, MO 6510

HDR Engineering, Inc.

SUMMARY OF QUANTITIES
SHEET 1 OF 2

| | | | | TOTAL | . QTY | TOTAL | SIGN | | П | | | 1 | | QTY | TOTAL S I | GN | | П |
|----------------------|----------------|-------|------|--------|-------|--------|--|--|----------------------|----------------|--------------|------|----------|---------|-----------|---------|---|------------------------|
| | SIZE | AREA | QTY | AREA | RELOC | RELOC | NUM. | | | SIZE | ARE | A Q | Υ ΤΟΤΑΙ | RELOC | RELOC NU | JM . | | Ш |
| SIGN | IN. | SQ.FT | EACH | SQ.FT. | EACH | SQ.FT. | | | SIGN | IN. | SQ.F | T EA | CH SQ.FT | . EACH | SQ.FT. | | | ll I |
| | • | | WAR | NING S | GIGNS | | • | DESCRIPTION | | • | | GL | JIDE SIG | GNS | • | | DESCRIPTION | _{NU} |
| WO1 - 1L | 48X48 | 16.00 |) | | | | | TURN (SYMBOL LEFT) | E05-1 | 36X48 | 12.0 | 00 | 1 12 | | 4 | 19 0 | GORE EXIT | 61 |
| WO1 - 1R | _ | _ | | | | | | TURN (SYMBOL RIGHT) | E05-2 | 48X36 | | _ | | | | | EXIT OPEN | 61 |
| WO1 - 2L | | 16.00 | | | | | | CURVE (SYMBOL LEFT) | E05-2a | 48X36 | _ | _ | | | | _ | EXIT CLOSED | 61 |
| WO1 - 2R | | | | | | | | CURVE (SYMBOL RIGHT) | GO20-1 | 60X24 48X24 | | | | | | _ | ROAD WORK NEXT XX MILES | 61 |
| WO1 - 3L WO1 - 3R | | 16.00 | | | | | | REVERSE TURN (SYMBOL LEFT) REVERSE TURN (SYMBOL RIGHT) | GO20-2 GO20-4 | 36X18 | | | | | | _ | END ROAD WORK PILOT CAR FOLLOW ME | 61 |
| WO1 - 4L | | 16.00 | | | | | | REVERSE CURVE (SYMBOL LEFT) | GO20-4a | 42X30 | | | | | | | PILOT CAR IN USE WAIT & FOLLOW | 61 |
| WO1 - 4R | | | | | | | | REVERSE CURVE (SYMBOL RIGHT) | GO20-4a | 18X12 | | | | | | _ | PILOT CAR IN USE WAIT & FOLLOW | 61 |
| WO1-4bL | | | | | | | | DOUBLE ARROW REVERSE CURVE (SYMBOL LEFT) | GO20-5aP | | | | _ | 4 | | | VORK ZONE (PLAQUE) | 61 |
| WO1 - 4bF | | | | | | | | DOUBLE ARROW REVERSE CURVE (SYMBOL RIGHT) | MO4 - 8 a | 24X18 | | | 2 6 | | 5 | - | ND DETOUR | 61 |
| WO1 - 4 c L | | | | | | | | TRIPLE ARROW REVERSE CURVE (SYMBOL LEFT) | MO4 - 9L MO4 - 9R | 48X36 | | | | | | | DETOUR (LEFT) | 61 |
| WO1 - 4 cF | | 12.50 | | | | | | TRIPLE ARROW REVERSE CURVE (SYMBOL RIGHT) HORIZONTAL ARROW (SYMBOL) | MO4 - 9R MO4 - 9P | 48X36 48X12 | | | | | | | DETOUR (RIGHT) STREET NAME (PLAQUE) | 61 |
| WO1 - 6a | | | | | | | | HORIZ, ARROW (SYMBOL ON PERMANENT BARRICADE) | MO4 - 10L | 48X18 | | | | | | _ | DETOUR ARROW (LEFT) | 61 |
| WO1 - 7 | | 12.50 | | | | | | DOUBLE HEAD HORIZONTAL ARROW (SYMBOL) | MO4 - 10R | 48X18 | | | | | | _ | DETOUR ARROW (RIGHT) | 61 |
| WO1-7a | | |) | | | | | DOUBLE HEAD HORIZ. ARROW (SYMBOL ON PERM. BARR. | 1 | | | _ | GULATO | RY SIG | NS | | | 61 |
| WO1 - 8 | | 3.00 | | | | | | CHEVRON (SYMBOL) | R1-1 | 48X48 | | | | | | _ | STOP | 61 |
| WO1 - 8a | | 7.50 | | | - | | - | CHEVRON (SYMBOL FOR DIVIDED HIGHWAYS) | R1-2 | 48TR I | | _ | | | | _ | /IELD FO ONCOMING TRAFFIC (PLAQUE) | 61 |
| WO3 - 1 WO3 - 2 | | 16.00 | | | | | | STOP AHEAD (SYMBOL) YIELD AHEAD (SYMBOL) | R1-2a R1-3P | 36X36 30X12 | | | | | | | ALL WAY (PLAQUE) | 61 |
| WO3 - 2 | | 16.00 | | | | | | SIGNAL AHEAD (SYMBOL) | R2-1 | 36X48 | | _ | 48 | | 1 1 | _ | SPEED LIMIT 45 | 61 |
| WO3 - 4 | | 16.00 | | | | | | BE PREPARED TO STOP | R2-1 | 36X48 | | _ | | | | | SPEED LIMIT 55 | 61 |
| WO3 - 5 | | 16.00 | | 64 | | | 3 | SPEED LIMIT AHEAD | R2-1 | 36X48 | | _ | | 4 | | | SPEED LIMIT 65 | ثاك |
| WO4 - 1L | 48X48 | | | | | | | MERGE (SYMBOL FROM LEFT) | R3-1 | 48X48 | | | | | | | NO RIGHT TURN (SYMBOL) | 61 |
| WO4 - 1R | _ | _ | | | | | | MERGE (SYMBOL FROM RIGHT) | R3-2 | 48X48 | | | | | | _ | NO LEFT TURN (SYMBOL) | - . |
| WO4 - 1aL | | | | 32 | - | | | MERGE (LEFT) | R3-3 R3-4 | 36X36 | _ | _ | 1.0 | | . | _ | NO TURNS | 616 |
| WO4 - 1 ar | 48X48 48X48 | _ | | 32 | | | 6A | MERGE (RIGHT) ROAD/BRIDGE/RAMP NARROWS | R3-4 R3-7L | 48X48 30X30 | | | 16 | | 9 | _ | NO U-TURN (SYMBOL) LEFT LANE MUST TURN LEFT | $ \parallel_{61}$ |
| WO5 - 3 | | 16.00 | | | | | | ONE LANE BRIDGE | R3-7E | 30X30 | | | | | | _ | RIGHT LANE MUST TURN RIGHT | 616 |
| WO5 - 5 | | 16.00 | | | | | | NARROW LANES | R4-1 | 36X48 | | | | | | _ | OO NOT PASS | 61 |
| WO6 - 1 | | 16.00 | | | | | | DIVIDED HIGHWAY (SYMBOL) | R4-2 | 36X48 | | _ | | | | _ | PASS WITH CARE | 11 |
| WO6 - 2 | | 16.00 | | | | | | DIVIDED HIGHWAY END (SYMBOL) | R4-7a | 36X48 | | _ | | | | | (EEP RIGHT (HORIZONTAL ARROW) | 617 |
| WO6 - 3 | 48X48 | |) | | | | | TWO WAY TRAFFIC (SYMBOL) | R4-8a | 36X48 | | | | | | | (EEP LEFT (HORIZONTAL ARROW) | - ∐ |
| WO7 - 3a | | 5.00 | | | | | | NEXT XX MILES (PLAQUE) | R5-1 | 30X30 | | - | | | | | OO NOT ENTER | 617 |
| WO8 - 1 WO8 - 2 | | 16.00 | | | - | | | BUMP DIP | R5-1a R6-1L | 36X24 54X18 | | | | | | _ | VRONG WAY | 617 |
| WO8 - 2 | | 16.00 | | | | | | PAVEMENT ENDS | R6-1L | 54X18 | | | | | | | DNE WAY ARROW (LEFT) DNE WAY ARROW (RIGHT) | -1161/ |
| WO8 - 4 | | 16.00 | | | | | | SOFT SHOULDER | R6-2L | 24X30 | | | | | | | ONE WAY (LEFT) | $ _{617}$ |
| WO8 - 5 | | 16.00 | | | | | | SLIPPERY WHEN WET (SYMBOL) | R6-2R | 24X30 | _ | | | | | _ | ONE WAY (RIGHT) | 11 |
| WO8 - 6 | | 16.00 | | | | | | TRUCK CROSSING | R9-9 | 24X12 | _ | _ | | | | S | SIDEWALK CLOSED | 617 |
| WO8 - 6 c | | 16.00 |) | | | | | TRUCK ENTRANCE | | | | | | | | | SIDEWALK CLOSED AHEAD, | 620 |
| WO8 - 7 | | 9.00 | | | | | | LOOSE GRAVEL | R9-11L | 24X18 | 3.0 | 0 | | | | | (ARROW LEFT) CROSS HERE | 90 |
| WO8 - 7a WO8 - 9 | | 16.00 | | | - | | | FRESH OIL / LOOSE GRAVEL LOW SHOULDER | R9-11R | 24X18 | 2 0 | ٨ | | | | | SIDEWALK CLOSED AHEAD, (ARROW RIGHT) CROSS HERE | 90 |
| WO8 - 11 | 48X48 | | | | | | | UNEVEN LANES | R10-6 | 24X16 | | _ | | | | | STOP HERE ON RED (45^ ARROW) | $\dashv \vdash \vdash$ |
| WO8 - 12 | | | | | | | | NO CENTER LINE | R11-2 | 48X30 | | | 60 | 1 | 10 2 | | ROAD CLOSED | ┨ |
| WO8 - 15 | 48X48 | 16.00 |) | | | | | GROOVED PAVEMENT | | | | | | | | R | ROAD CLOSED XX MILES AHEAD | |
| WO8 - 15F | | | | | | | | MOTORCYCLE (PLAQUE) | R11-3a | 60X30 | | _ | | | | | OCAL TRAFFIC ONLY | \bot |
| WO8 - 17L | | | | | | | | SHOULDER DROP-OFF (SYMBOL LEFT) | R11-4 | 60X30 | | | | | | _ | ROAD CLOSED TO THRU TRAFFIC | Щ_ |
| WO8 - 17F | | | | | - | | | SHOULDER DROP-OFF (SYMBOL RIGHT) | CONST 3X | | | _ | | | | | FINE SIGN | - |
| WO8 - 17F W10 - 1 | 42RND. | _ | | | | | | SHOULDER DROP-OFF (PLAQUE) RAILROAD CROSSING | CONST-3X | 56X12 | 4.6 | | SCELLA | VIENIIS | SIGNS | 5 | SPEEDING/PASSING (PLATE) | - |
| W012-1 | | | | | | | | DOUBLE DOWN ARROW (SYMBOL) | CONST-5 | 48X36 | 12.0 | | JULLA | 1003 | O I OINO | P | POINT OF PRESENCE | 1 |
| WO12-1 | | | | | | | | LOW CLEARANCE (SYMBOL) | CONST-5 | 96X48 | _ | _ | | | | | POINT OF PRESENCE | * |
| W012-2X | | | | | | | | LOW CLEARANCE (PLAQUE) | CONST - 7 | | | | | | | _ | RATE OUR WORK ZONE | _ |
| WO12-2a | | | | | | | | OVERHEAD LOW CLEARANCE (FEET AND INCHES) | CONST-7 | 72X36 | | _ | | | | | RATE OUR WORK ZONE |] * > |
| WO12-4 | | | | | | | | LOW CLEARANCE XX FT XX IN XX MILES AHEAD | CONST-8 | 48X36 | | _ | | | | _ | VORK ZONE NO PHONE ZONE | _ |
| WO12-5 | | | | | | | | WIDTH RESTRICTION XX FT XX IN XX MILES AHEAD | MISC - 1 | | | | | | | | DETOUR US-61 N STRAIGHT ARROW | 4 |
| WO13-1 | | | | | - | | - | ADVISORY SPEED (PLAQUE) XXX FEET (PLAQUE) | MISC - 2 | | | | _ | | | | DETOUR US-61 N RT. ARROW DETOUR US-61 N LT. ARROW | \dashv |
| WO16-2 WO16-3 | | | | | + | | | X MILE (PLAQUE) | MISC - 4 | | | _ | _ | | | | DETOUR US-61 N RT. AHEAD ARROW | \dashv |
| WO20-1 | | | | 80 | | | | ROAD/BRIDGE/RAMP WORK AHEAD | MISC - 5 | | | | | | | _ | DETOUR US-61 N LT. AHEAD ARROW | 1 |
| WO20 - 2 | | | | 32 | | | | DETOUR AHEAD | MISC - 6 | | | | | | | | DETOUR RTE 6 W STRAIGHT ARROW | |
| WO20-3 | 48X48 | 16.00 | 5 | 80 | | | 20/90 | ROAD CLOSED AHEAD | MISC - 7 | 36X78 | 19.5 | 50 : | | | | 78 D | DETOUR RTE 6 W RT. ARROW | |
| WO20-4 | | | | | | | | ONE LANE ROAD AHEAD | MISC - 8 | _ | _ | _ | | | | _ | DETOUR RTE 6 W LT. ARROW | 4 |
| WO20-5 | | | | 64 | 4 | 64 | 5L/5R | RIGHT/CENTER/LEFT LANE CLOSED AHEAD | MISC - 9 | | _ | _ | | | | | DETOUR RTE 6 W MERGE RT. ARROW | ⊣,— |
| WO20 - 5a | | | | 32 | - | 32 | 61 /60 | 2 RIGHT/CENTER/LEFT LANES CLOSED AHEAD RIGHT/CENTER/LEFT LANE CLOSED | MISC - 1 | | | _ | 19.5 | | | _ | DETOUR RTE 6 W LT. AHEAD ARROW DETOUR RTE 6 W RT. AHEAD ARROW | - |
| WO20 - 6a | | | | 32 | 2 | 32 | OL/6K | FLAGGER (SYMBOL) | HM13C - 1 | 1 207/8 | 19.5 | | 19.5 | | |) Z L | DETOUR RIE O W RI. AREAD ARROW | ⊣⊢ |
| WO20-78 | | | | | _ | | | FRESH OIL | 616-10 | .05 | 1 | | TOTAL | | | | | ٦ |
| WO21-5 | | | | | | | | SHOULDER WORK / SHOULDER WORK AHEAD | CONSTR | | N SI | GNS | 1421 | ⊣ | | | | |
| WO21-5a | | | | 64 | | | 21A | RIGHT/LEFT SHOULDER CLOSED | 616-10 | | | | | - | TOTAL | | | |
| WO21-5b | | | | 64 | | | 21B | RIGHT/LEFT SHOULDER CLOSED AHEAD | RELOCA | TED S | <u>I GNS</u> | | | | ** | | | |
| WO22-1 | | | | | | | | BLASTING ZONE AHEAD | | | | | | | | | | |
| IWO22-2 | 42X36 | | | | | | | TURN OFF 2-WAY RADIO AND PHONE | - | | | | | | | | | |
| | | | | | 1 | 1 | 1 | END BLASTING ZONE | | | | | | | | | | |
| WO22-3 GO22-1 | | | | | | | | WET PAINT (ARROW PIVOTS) | 1 | | | | | | | | | |

| ITEM | TOTAL | |
|---------------|--------------|---|
| NUMBER | R QTY | DESCRIPTION |
| 6122008 | 3 | IMPACT ATTENUATOR 40 MPH (SAND BARRELS) |
| 6122009 | 9 | IMPACT ATTENUATOR 45 MPH (SAND BARRELS) |
| 6122010 |) | IMPACT ATTENUATOR 50 MPH (SAND BARRELS) |
| 6122012 | 2 | IMPACT ATTENUATOR 55 MPH (SAND BARRELS) |
| 6122014 | 4 | IMPACT ATTENUATOR 60 MPH (SAND BARRELS) |
| 612201 | 7 * 2 | IMPACT ATTENUATOR 65 MPH (SAND BARRELS) |
| 6122019 | 9 | IMPACT ATTENUATOR 70 MPH (SAND BARRELS) |
| 6122020 |) * 2 | REPLACEMENT SAND BARREL |
| 6122030 |) * 2 | IMPACT ATTENUATOR (RELOCATION) |
| 612300 | 1 | TRUCK MOUNTED ATTENUATOR (TMA) |
| 6161008 | 3 | ADVANCED WARNING RAIL SYSTEM |
| 6161012 | 2 | BUOYS (BOATS KEEP OUT) |
| 6161013 | 3 | BUOYS (NO WAKE) |
| 6161014 | | SPECIAL SIGN ASSEMBLY (BOATS KEEP OUT) |
| 616102 | | CHANNELIZER (TRIM LINE) |
| 6161030 | | TYPE III MOVEABLE BARRICADE |
| 6161033 | _ | DIRECTION INDICATOR BARRICADE |
| 6161040 | _ | FLASHING ARROW PANEL |
| 616104 | _ | TYPE III OBJECT MARKER |
| 616105 | · _ | SEQUENTIAL FLASHING WARNING LIGHT |
| 6161070 | _ | TUBULAR MARKER |
| 616109 | | RADAR SPEED ADVISORY SYSTEM |
| 010103 | , | CHANGEABLE MESSAGE SIGN. |
| 6161096 | 5 | COMMISSION FURNISHED/RETAINED |
| 0101030 | * | CHANGEABLE MESSAGE SIGN W/O COMM. |
| 6161098 | | INTERFACE - CONTRACTOR FURNISHED/RETAINED |
| 0101030 | A 0 | CHANGEABLE MESSAGE SIGN WITH COMM. |
| 6161099 | a | INTERFACE - CONTRACTOR FURNISHED/RETAINED |
| 6162000 | | WORK ZONE TRAFFIC SIGNAL SYSTEM |
| 616200 | | TEMPORARY LONG-TERM RUMBLE STRIPS |
| 1 3 3 2 3 0 7 | * | TEMPORARY TRAFFIC BARRIER |
| 6173600 | | CONTRACTOR FURNISHED/RETAINED |
| 151,3000 | 5 504 | TEMPORARY TRAFFIC BARRIER |
| 6173602 | В | CONTRACTOR FURNISHED/COMMISSION RETAINED |
| 6174000 | _ | TEMP. TRAFFIC BARRIER HEIGHT TRANSITION |
| 6175010 | | RELOCATING TEMPORARY TRAFFIC BARRIER |
| 151,3010 | | TEMPORARY TRAFFIC BARRIER |
| 6176000 | В | COMMISSION FURNISHED/RETAINED |
| 51,0000 | 5 | TEMP, TRAFFIC BARRIER HEIGHT TRANSITION |
| 6177000 | B | COMMISSION FURNISHED/RETAINED |
| 6208064 | | TEMPORARY RAISED PAVEMENT MARKER |
| 9029400 | | TEMPORARY TRAFFIC SIGNALS |
| 902940 | | TEMPORARY TRAFFIC SIGNALS AND LIGHTING |
| 302340 | 1 | TENIONANT TRAITIC SIGNALS AND EIGHIING |
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| | | |

EFFECTIVE: 04-01-2024

* ITEMS NOT INCLUDED IN LUMP SUM TEMPORARY TRAFFIC CONTROL

** NO DIRECT PAY FOR RELOCATING TEMPORARY TRAFFIC CONTROL DEVICES

TEMPORARY TRAFFIC CONTROL
PAY TOTAL: 1 LUMP SUM

SUMMARY OF QUANTITIES
SHEET 2 OF 2



DATE PREPARED
4/2/2024

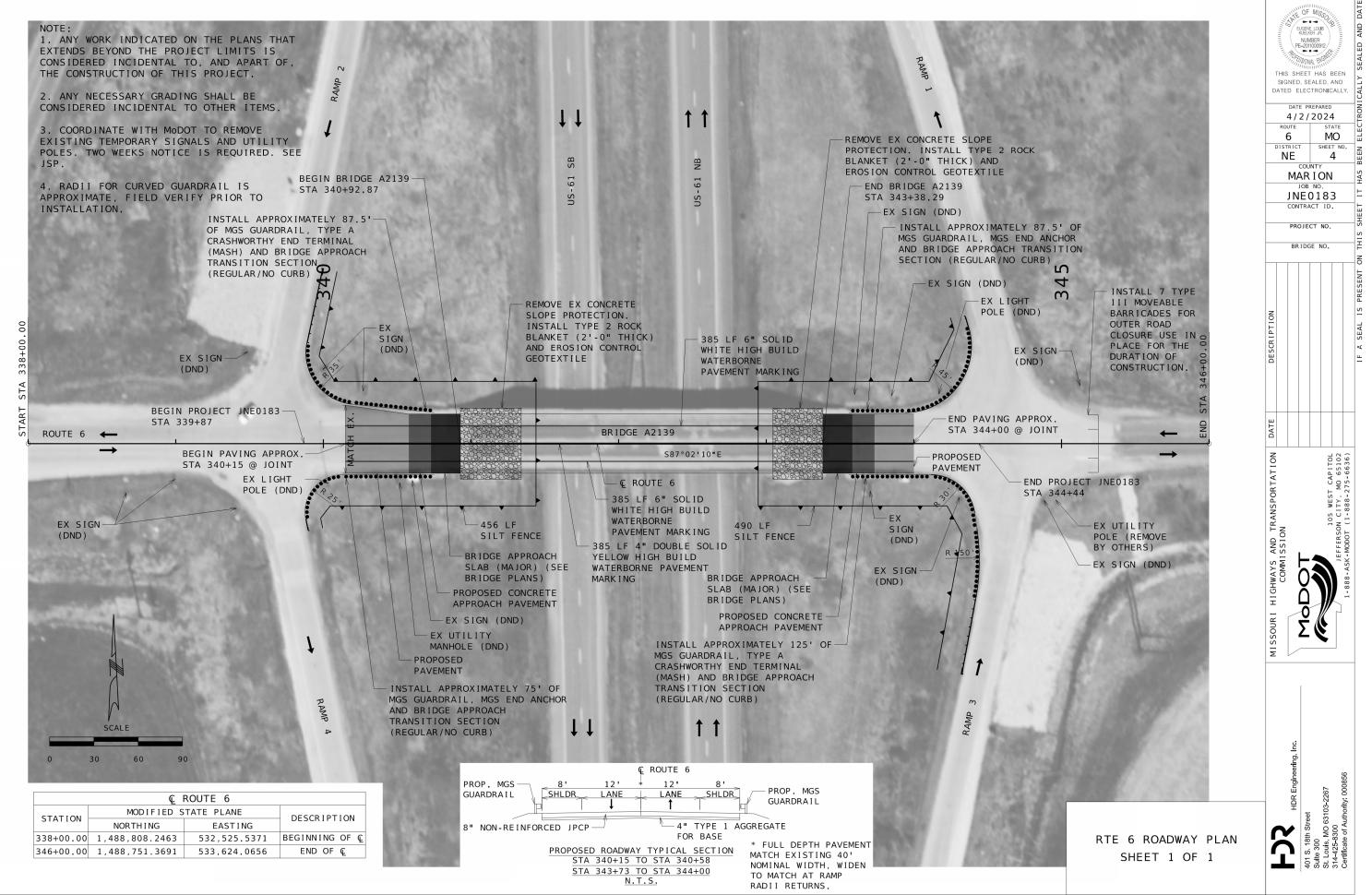
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3 STATE
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JOB NO.
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CONTRACT ID.
PROJECT NO.

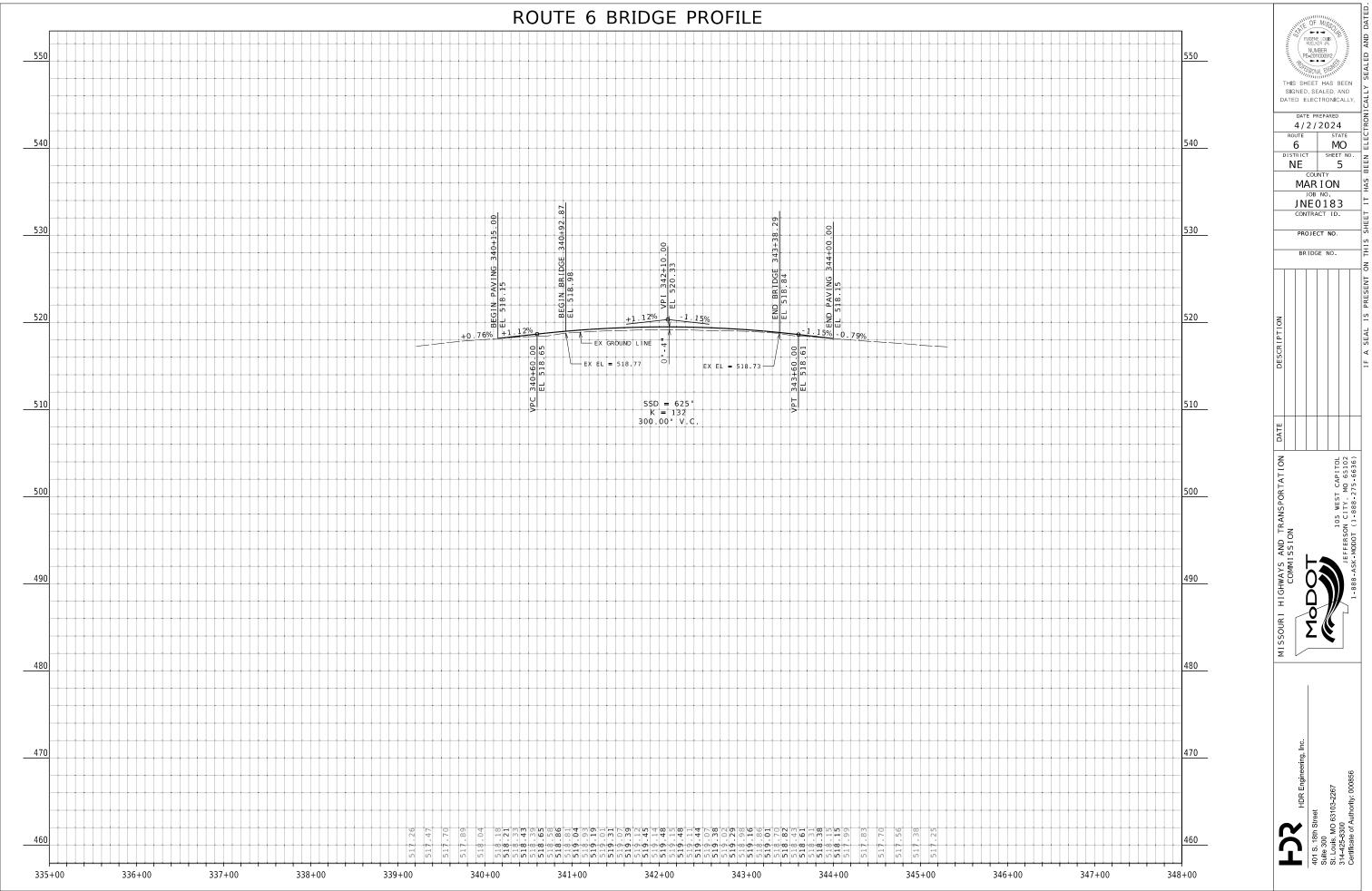
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JEFFERSON CITY, MO

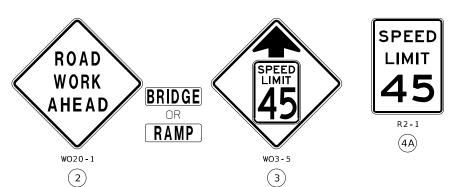
HDR Engineering, Inc.





NOTES:

- EXISTING GUIDE, WARNING AND REGULATORY SIGNS WITHIN THE WORK ZONE SHALL NOT CONFLICT WITH THE CONSTRUCTION SIGNING (SUCH AS SPEED LIMIT, BRIDGE CLEARANCE, ETC.) AND SHALL BE COVERED, REMOVED OR RELOCATED WITH NO DIRECT PAYMENT. CONSTRUCTION SIGNS SHALL NOT OBSTRUCT EXISTING SIGNS REMAINING IN PLACE.
- 2. ALL SPACING AND DISTANCES ARE APPROXIMATE. THEY MAY BE ADJUSTED BY THE ENGINEER TO FIT FIELD CONDITIONS.
- 3. SEE STANDARD PLAN 616.10BC FOR TEMPORARY TRAFFIC CONTROL DEVICES DETAILS.
- 4. SEE STANDARD PLAN 620.10G FOR TEMPORARY PAVEMENT MARKING DETAILS.
- 5. CONSTRUCTION SIGNS NOTED AS POST-MOUNTED (POST) SHALL BE MOUNTED PER STANDARD PLAN 616 10RB
- THE SPEED LIMIT DURING ACTIVE CONSTRUCTION SHALL REMAIN 65 M.P.H. UNLESS OTHERWISE NOTED IN THE TRAFFIC CONTROL PLAN.
- 7. CONFLICTING STRIPING SHALL BE REMOVED PRIOR TO GUIDING TRAFFIC THROUGH WORK ZONES. ANY STRIPING WHICH IS DAMAGED DURING A DAYS WORK SHALL BE RE-STRIPED NO LATER THAN THE END OF THE SAME DAY.
- 8. ALL WARNING SIGNS SHALL HAVE A FLUORESCENT SHEETING.
- WORDING FOR CHANGEABLE MESSAGE SIGNS SHALL BE AS APPROVED OR DIRECTED BY THE ENGINEER.
- 10. EXACT LOCATIONS FOR CHANGEABLE MESSAGE SIGNS SHALL BE AS DIRECTED BY THE
- 11. MISSOURI ONE CALL (800 DIG RITE) SHALL BE CONTACTED FOR LOCATES (800-344-7483 OR 811) BEFORE BEGINNING ANY EXCAVATION OR BORING ACTIVITIES
- 12. UNDERGROUND FACILITIES, STRUCTURES AND UTILITIES SHOWN ON PLANS HAVE BEEN DRAFTED FROM FIELD NOTES AND AERIAL PHOTOGRAPHY; THE COMMISSION DOES NOT WARRANT THE LOCATIONS OF THESE FACILITIES AS PRECISE. IT IS POSSIBLE THERE MAY BE OTHERS, THE EXISTENCE OF WHICH IS PRESENTLY NOT KNOWN OR SHOWN. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE THE EXISTENCE AND PRECISE LOCATION OF ALL FACILITIES AND TO AVOID DAMAGE TO THEM. SEE THE JOB SPECIAL PROVISIONS FOR A LIST OF UTILITY COMPANIES ON OR WITHIN THE VICINITY OF THE PROJECT LIMITS.
- 13. FOR REMOVALS SEE PLANS AND REMOVAL OF IMPROVEMENTS TABLE.



TEMPORARY TRAFFIC CONTROL TYPICAL APPLICATIONS NOTES:

- 1. REMOVE AND/OR MODIFY ANY EXISTING PAVEMENT MARKINGS AS NEEDED.
- 2. WHEN WORK IS BEING PREFORMED WITHIN THE MEDIAN, TA-5B "LONG-TERM SHOULDER CLOSURE WITH TEMPORARY TRAFFIC BARRIER" SHALL BE UTILIZED. SEE SHEET 17 FOR FURTHER DETAILS.
- 3. A FLASHING ARROW BOARD SHALL BE USED WHEN A FREEWAY LANE IS CLOSED. WHEN MORE THAN ONE FREEWAY LANE IS CLOSED, A SEPARATE ARROW BOARD SHALL BE USED FOR EACH LANE.
- 4. ALTERNATIVE TEMPORARY TRAFFIC CONTROL SET-UPS SHOULD BE CONSIDERED WHEN REDUCTION IN CAPACITY CANNOT BE TOLERATED. SET-UPS MUST BE SUBMITTED TO ENGINEER FOR APPROVAL PRIOR TO IMPLEMENTATION.
- 5. UPON APPROVAL OF THE ENGINEER, THE CONTRACTOR MAY PROVIDE ADDITIONAL PROTECTIVE TRUCKS EQUIPPED WITH PROPER WARNING DEVICES.
- TRUCK MOUNTED ATTENUATOR WITH FLASHING ARROW PANEL SHALL BE USED IN PROTECTED WORK ZONES WHEN WORKERS ARE PRESENT.
- PROTECTIVE TRUCK AND WORK VEHICLES SHALL DISPLAY HIGH-INTENSITY ROTATING, FLASHING, OSCILLATING, OR STROBE LIGHTS.
- 8. VEHICLE HAZARD WARNING SIGNALS SHALL NOT BE USED INSTEAD OF THE VEHICLE'S HIGH-INTENSITY ROTATING, FLASHING, OSCILLATING OR STROBE LIGHTS.
- 9. FLASHING ARROW PANELS SHALL, AS A MINIMUM, BE TYPE B, WITH A SIZE OF 60 X 30 INCHES.

SUGGESTED SEQUENCE OF CONSTRUCTION:

- 1. INSTALL DETOUR SIGNING AND CLOSE ROUTE 6 OVERPASS AT US-61.
- 2. USE SIGNING PLAN SHOWN IN "US-61 CLOSURE ROUTE 6 BRIDGE DEMO" FOR WEEKEND CLOSURE
- ONCE BRIDGE IS CLOSED AND DEMOLITION IS COMPLETE, USE TA-43S ON TRAFFIC CONTROL PLAN TYPICAL APPLICATIONS SHEET 4 OF 4" TO CLOSE RAMP PERMANENTLY THROUGH THE DURATION OF CONSTRUCTION
- I. USE ROLLING CLOSURE DESCRIBED IN WORK ZONE TRAFFIC MANAGEMENT SPECIAL PROVISION TO PLACE PROPOSED GIRDERS.
- 5. TYPICAL APPLICATIONS SHOWN IN PLANS TO BE USED AS NEEDED AS PROPOSED CONSTRUCTION REQUIRES.











(6B)

TRAFFIC CONTROL LEGEND

CHANNELIZER

WORK AREA

TRAFFIC USAGE

SIGN (SINGLE SIDED)

FLASHING ARROW PANEL

DIRECTIONAL INDICATOR BARRICADE

TYPE III MOVEABLE BARRICADE

CHANGEABLE MESSAGE BOARD (CONTRACTOR FURNISHED



(6L)

EUGENE LOUIS EUGENE LOUIS NUMBER PE-2011000912

THIS SHEET HAS BEEN SIGNED, SEALED, AND DATED ELECTRONICALLY

A / 2 / 2024

ROUTE STATE

6 MO

SHEET NO

NE 6 COUNTY MAR I ON

JNE0183

PROJECT NO

BRIDGE NO.

COMMISSION

COMMISSION

105 WEST CAPITOL

JEFFERSON CITY, MO 65102

1-888-ASK-MODOT (1-888-275-6636)

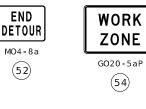








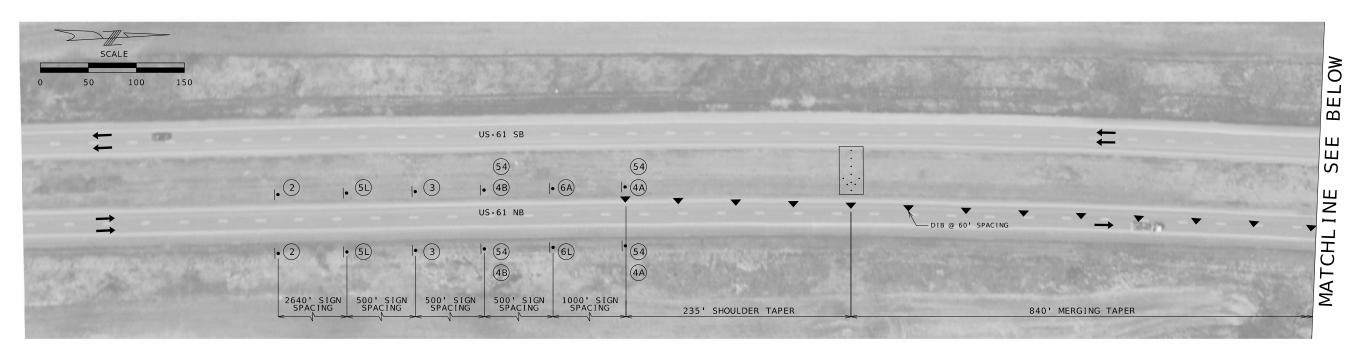






TRAFFIC CONTROL PLAN
GENERAL NOTES
AND LEGEND
SHEET 1 OF 1

US-61 NB AND SB CLOSURE FOR ROUTE 6 BRIDGE DEMO

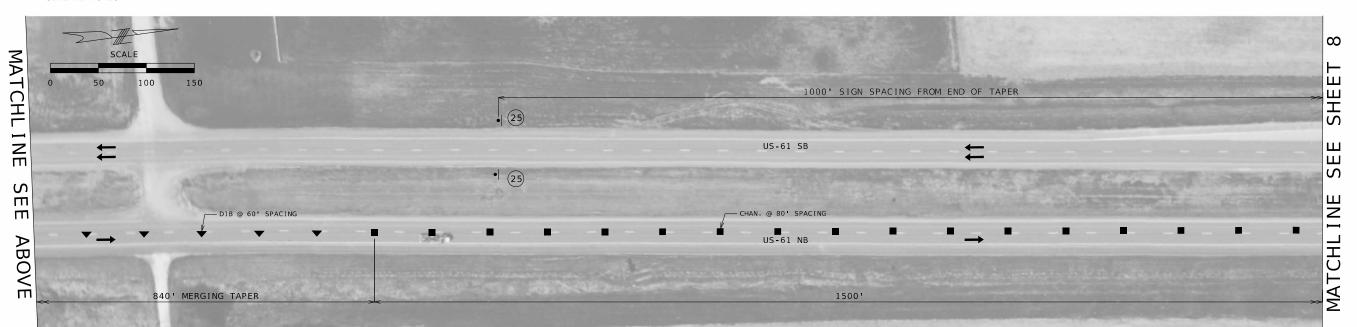


NOTES:

USE TYPICAL APPLICATION ON SHEET 16 FOR ADVANCED SIGNING FOR SPEED REDUCTION GREATER THAN 20 MPH.

SEE SHEET 18 FOR BEGIN/END OF PROJECT SIGNING. TO BE USED IN PLACE FOR DURATION OF THE PROJECT.

SEE JOB SPECIAL PROVISIONS FOR RESTRICTIONS. SEE SHEET 6 FOR LEGEND, SIGN DETAILS, AND GENERAL NOTES.

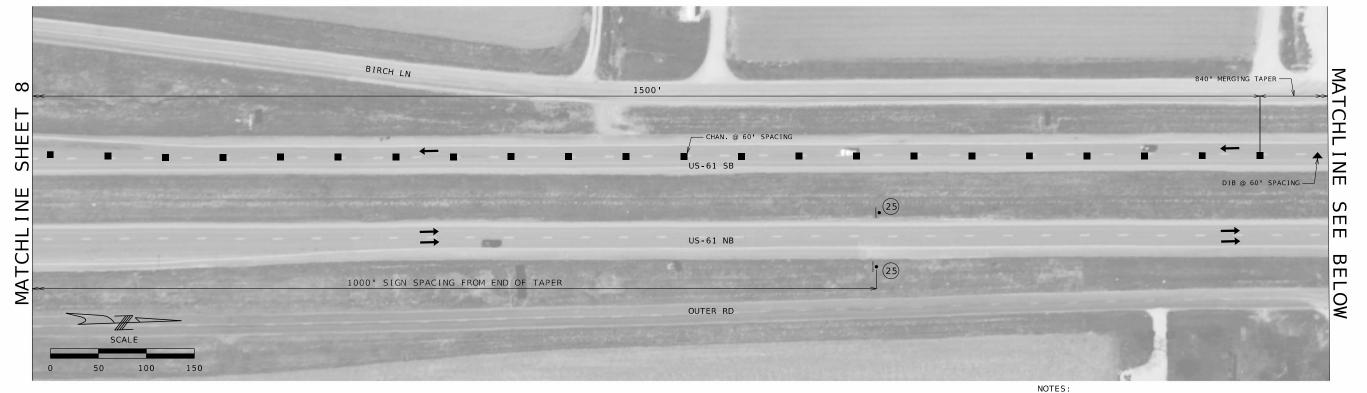


TRAFFIC CONTROL PLAN US-61 CLOSURE ROUTE 6 BRIDGE DEMO SHEET 1 OF 3

SIGNED, SEALED, AND DATED ELECTRONICALLY 4/2/2024 6 MO NE 7 MAR I ON JNE0183 PROJECT NO.



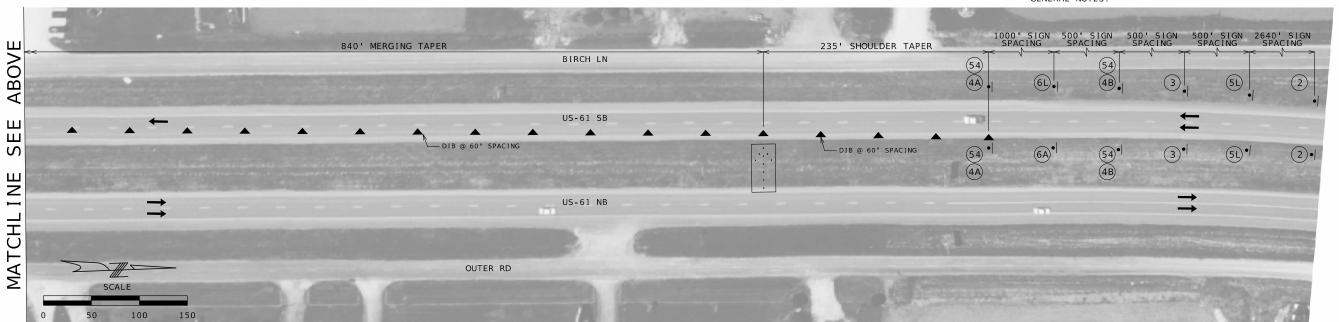
US-61 NB AND SB CLOSURE FOR ROUTE 6 BRIDGE DEMO



USE TYPICAL APPLICATION ON SHEET 16 FOR ADVANCED

SIGNING FOR SPEED REDUCTION GREATER THAN 20 MPH. SEE SHEET 18 FOR BEGIN/END OF PROJECT SIGNING. TO BE USED IN PLACE FOR DURATION OF THE PROJECT.

SEE JOB SPECIAL PROVISIONS FOR RESTRICTIONS. SEE SHEET 6 FOR LEGEND, SIGN DETAILS, AND GENERAL NOTES.



TRAFFIC CONTROL PLAN US-61 CLOSURE ROUTE 6 BRIDGE DEMO SHEET 3 OF 3

SIGNED, SEALED, AND DATED ELECTRONICALLY

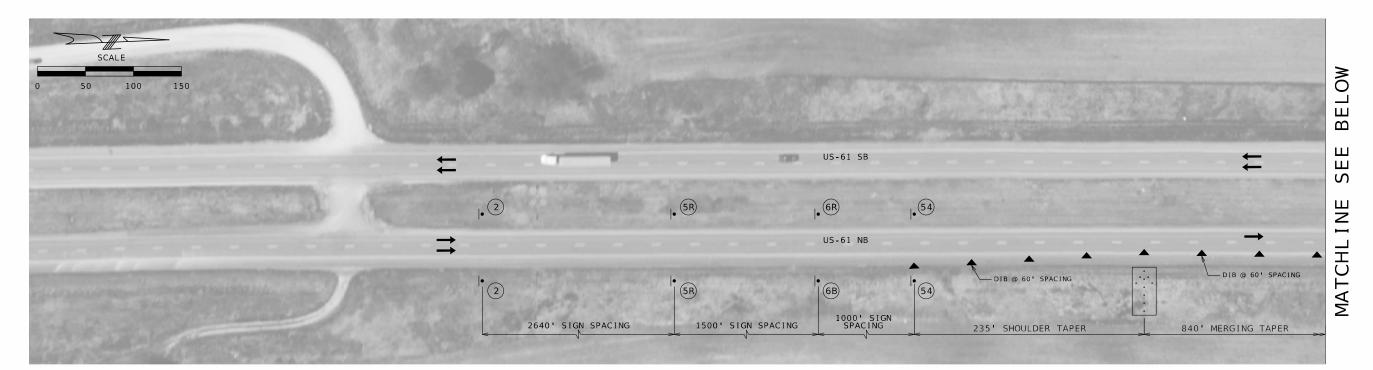
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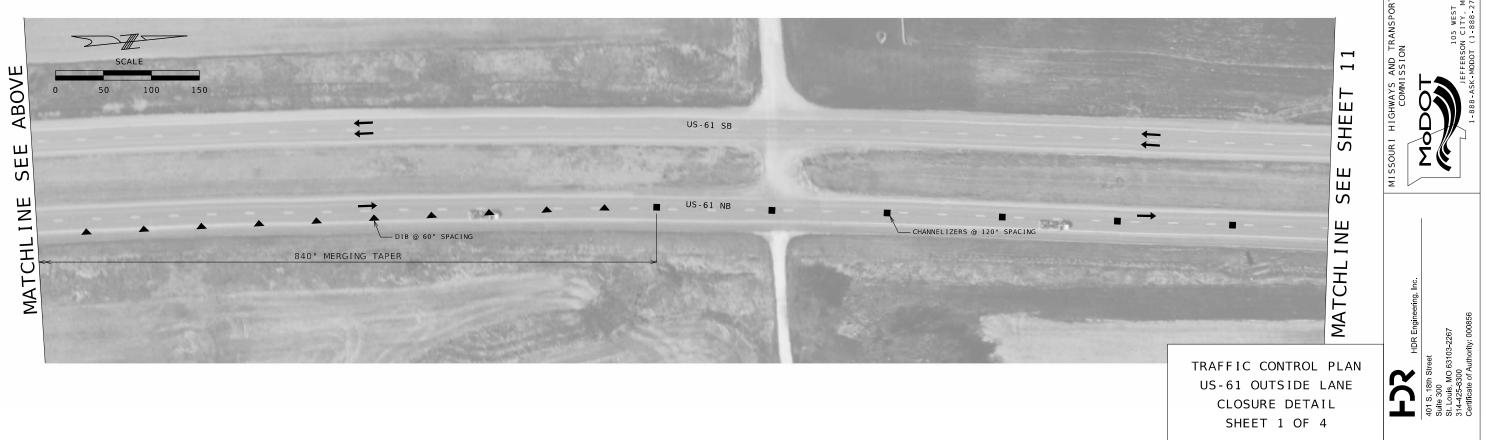
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PROJECT NO.

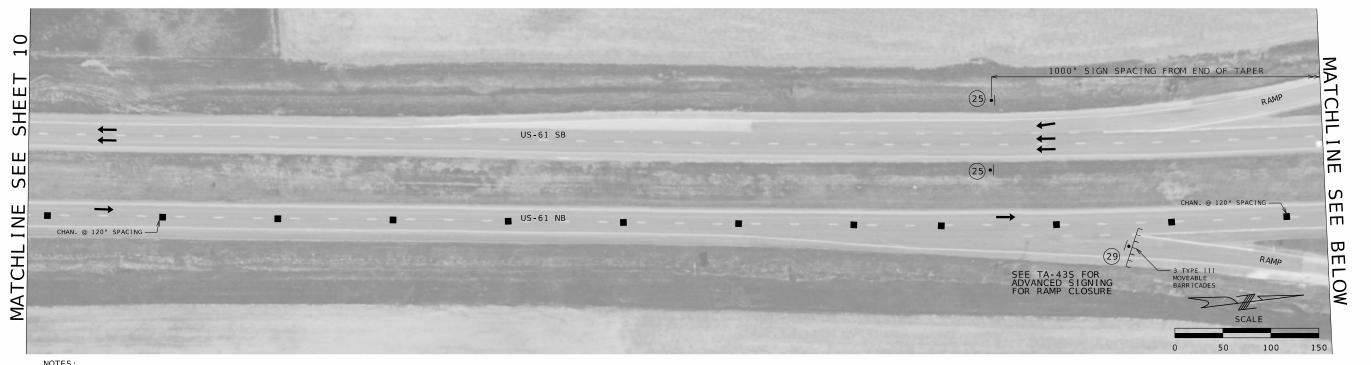
US-61 OUTSIDE LANE CLOSURE

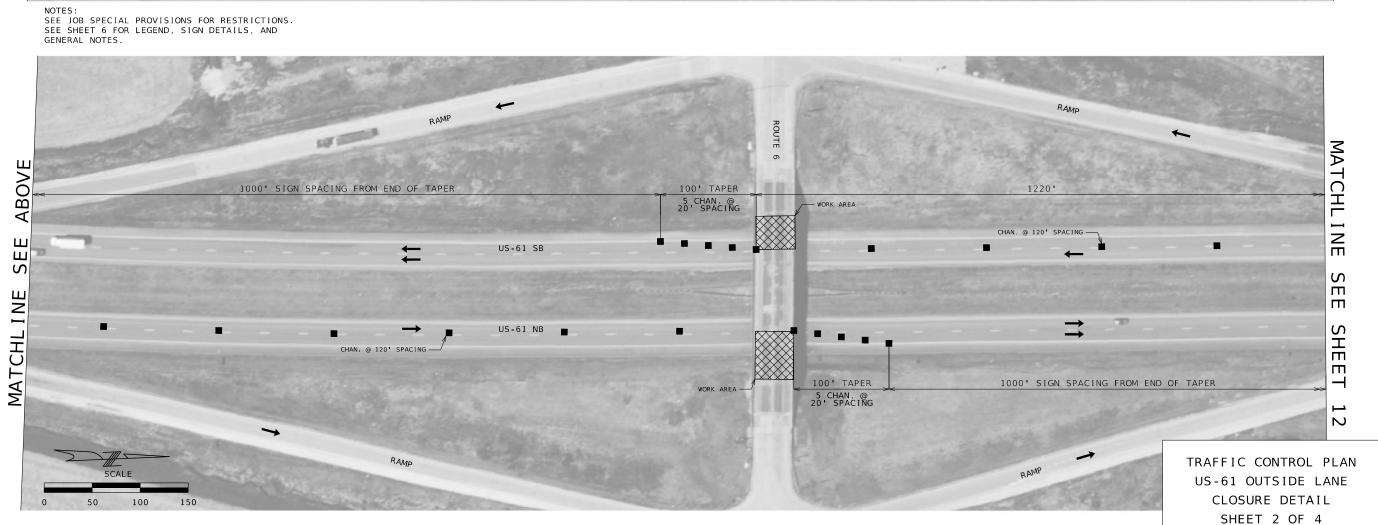


NOTES: SEE JOB SPECIAL PROVISIONS FOR RESTRICTIONS. SEE SHEET 6 FOR LEGEND, SIGN DETAILS, AND GENERAL NOTES.



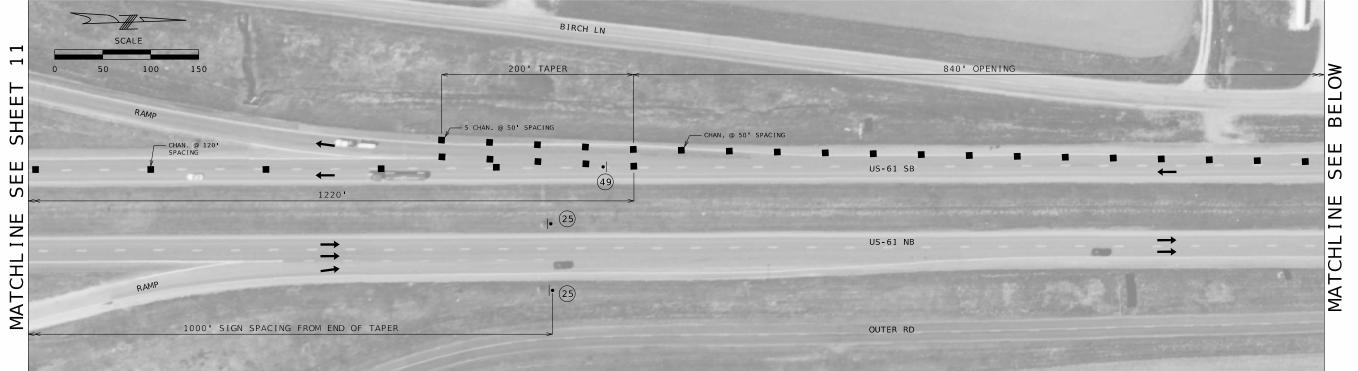
US-61 OUTSIDE LANE CLOSURE



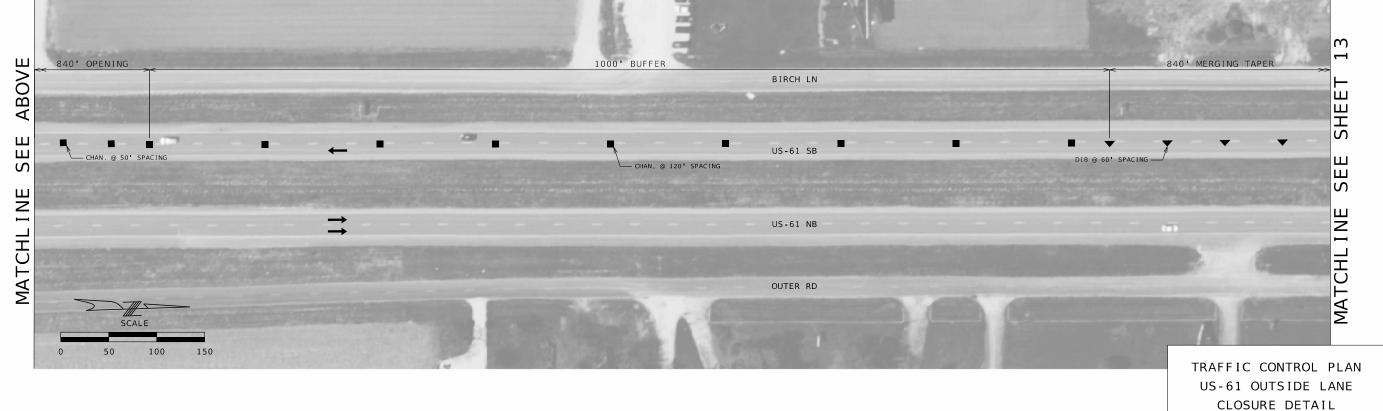


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US-61 OUTSIDE LANE CLOSURE

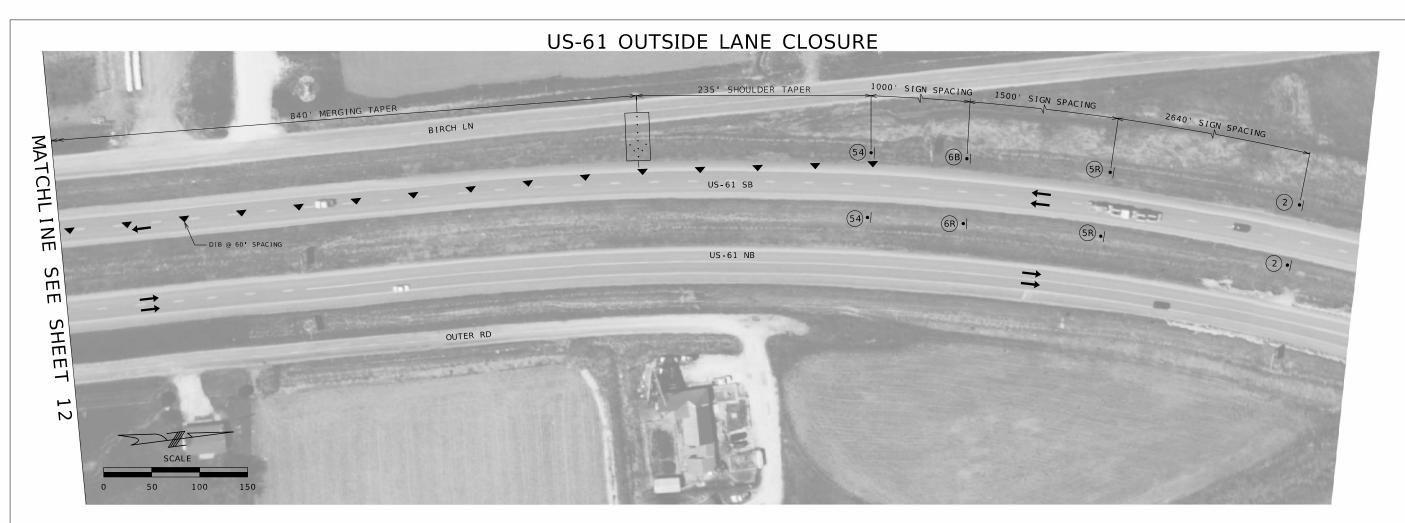


NOTES: SEE JOB SPECIAL PROVISIONS FOR RESTRICTIONS. SEE SHEET 6 FOR LEGEND, SIGN DETAILS, AND GENERAL NOTES.



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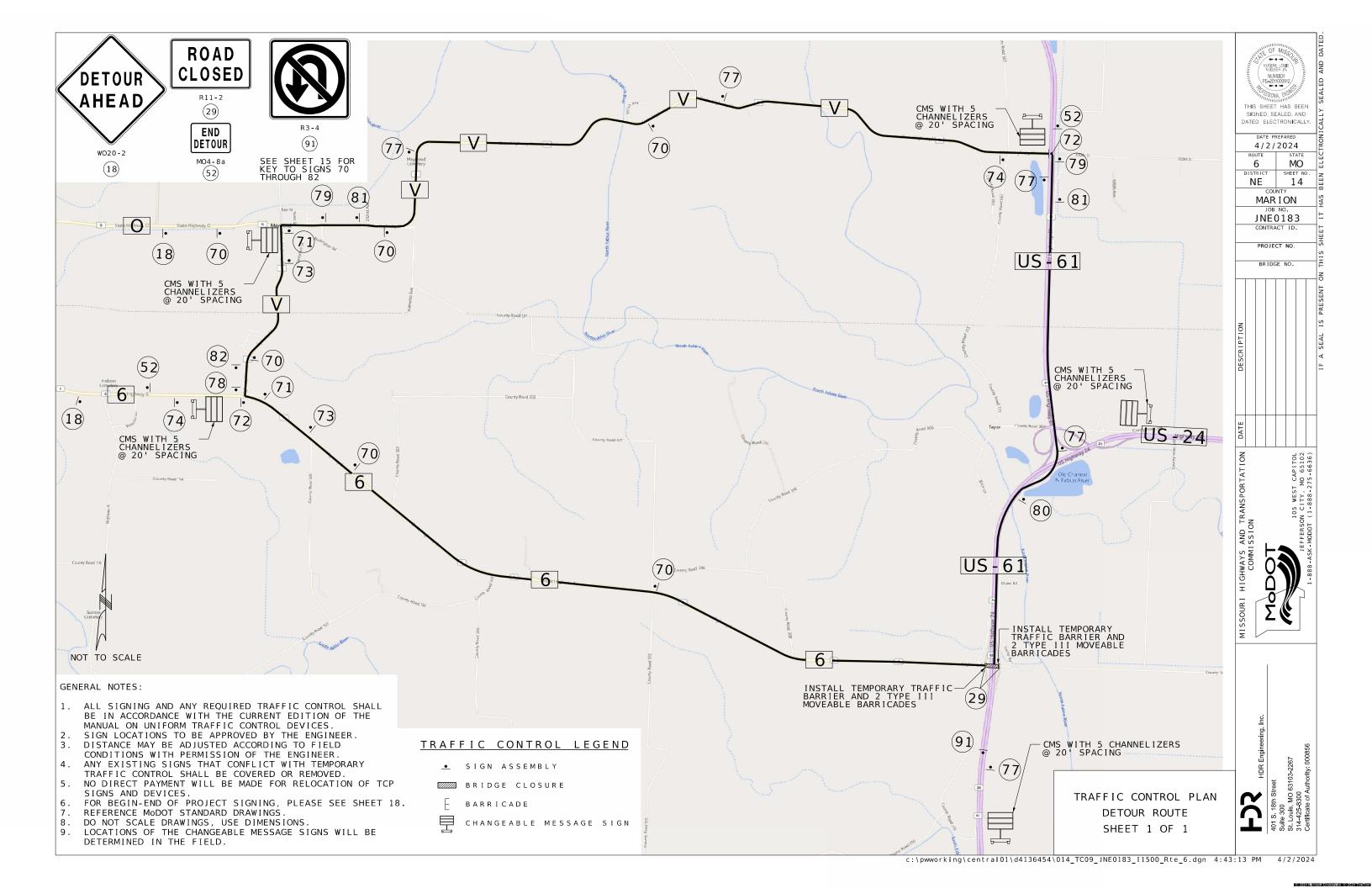
SHEET 3 OF 4

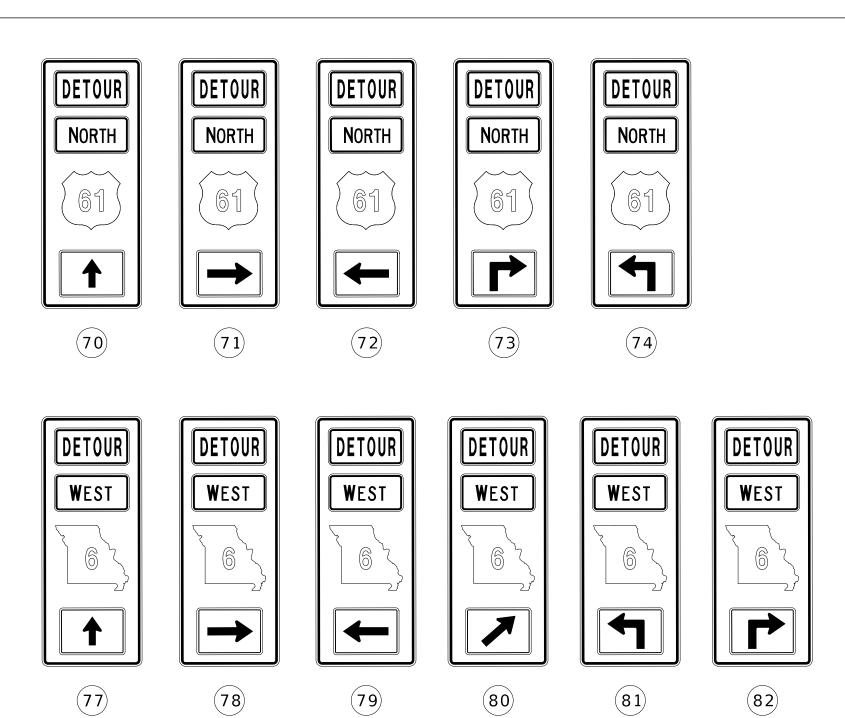


NOTES: SEE JOB SPECIAL PROVISIONS FOR RESTRICTIONS. SEE SHEET 6 FOR LEGEND, SIGN DETAILS, AND GENERAL NOTES.

TRAFFIC CONTROL PLAN US-61 OUTSIDE LANE CLOSURE DETAIL SHEET 4 OF 4



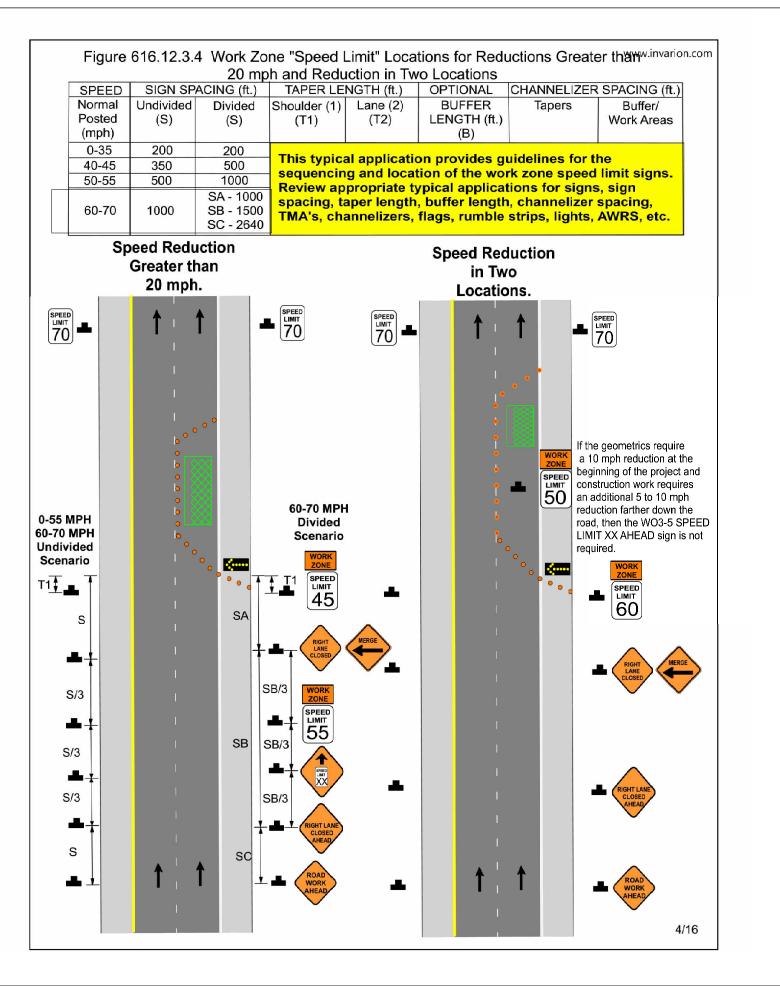






TRAFFIC CONTROL PLAN
DETOUR SIGN DETAIL
SHEET 1 OF 1







TRAFFIC CONTROL PLAN
TYPICAL APPLICATIONS
SHEET 1 OF 4



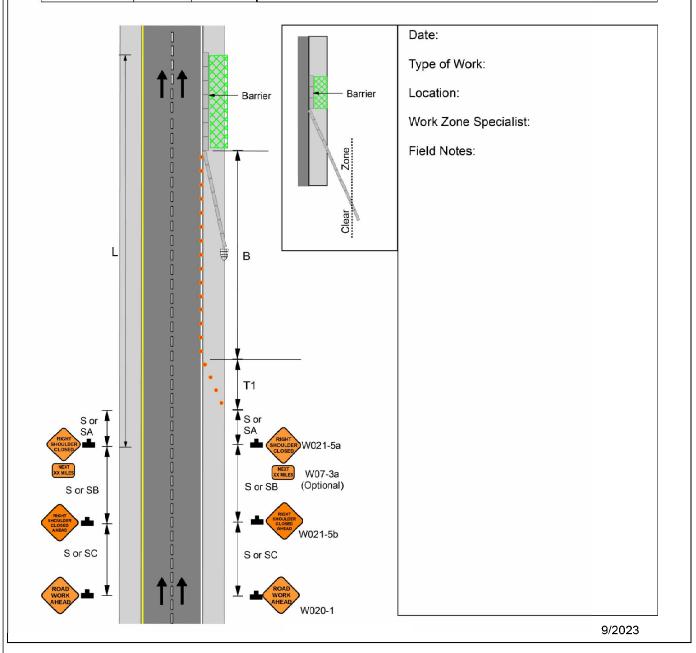
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616.8.5B (TA-5B) Long-Term Shoulder Closure with Temporary Traffic Barrier

| SPEED | SIGN SPACING (ft.) | TAPER LEN | GTH (ft.) | RECOMMENDED | CHANNELIZER | SPACING (ft.) |
|------------------------------|-------------------------------------|----------------------|------------------|-------------------------------|-------------|-----------------------|
| Permanent Posted (mph) | Divided (S) | Shoulder (1) (T1) | Lane (2) (T2) | Buffer Length (ft.) (B) | Tapers | Buffer/ Work Areas |
| 0-35 | 200 | 70 | - | 250 | 35 | 40 |
| 40-45 | 500 | 150 | - | 360 | 40 | 80 |
| 50-55 | 1000 | 185 | ī | 495 | 50 | 80 |
| 60-70 | SA - 1000 SB - 1500 SC - 2640 | 235 | , | 730 | 60 | 120 |

1. Shoulder taper length based on 10 ft. (standard shoulder width) offset. 2. Lane taper length based on 12 ft. (standard lane width) offset.

| | | | 1 1000000000000000000000000000000000000 | | to to outself the least to the | | | Mary desirant |
|-----------------|----------------|------------------------------------|---|-------------|--------------------------------|------|------------------|---------------|
| TYPE OF ROADWAY | SIGN HEIGHT | MAXIMUM WORK ZONE LENGTH (L) | • | Channelizer | | | ∰ Cra | ash Cushion |
| URBAN | 7' Post | 1 Mi. | | | | Sign | | |
| RURAL DIVIDED | 7' Post | 2 Mi. | | Barrier | | | ********* | Work Space |



616.8.5B (TA-5B) Long-Term Shoulder Closure with Temporary Traffic Barrier

Always use advance warning signs:

- 1. Set additional warning signs at each intersection with another state highway within the work zone.
- 2. ROAD NARROWS sign should be used when the lane widths are reduced.
- 3. ROAD WORK AHEAD sign is not needed if shoulder work is located within the limits of an activity area where another ROAD WORK AHEAD sign is already used.
- 4. NEXT XX MILES is required for narrow lane sections more than one mile.
- Always use channelizers to create a buffer area when using temporary traffic barrier.
- **Temporary Traffic Barrier:** Use engineering judgement when determining the use of temporary traffic barriers.
 - 1. A flare rate of 8:1 for the flared concrete barrier is used from the edge of roadway to the limits of the clear zone where the side slope is 6:1 or flatter.
 - 2. Concrete barrier within the clear zone will use an approved crash cushion when the normal posted speed limit is greater than 35 mph.
 - 3. Concrete barrier within the clear zone can use either a barrier height transition or an approved crash cushion when the normal posted speed limit is less than or equal to 35 mph.

For other operations, refer to:

- Stationary:
 - 1. EPG 616.8.3S (TA-3S) Stationary Shoulder Work.
- Long-Term Stationary Operations:
 - 1. EPG 616.6.2.2 Flags and Advance Warning Rail System.
 - 2. EPG 616.6.78 Temporary Markings.
- Speed Limit Guidelines:
 - 1. EPG 616.12 Work Zone Speed Limits.



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4/2/2024

ROUTE STATE
6 MO
DISTRICT SHEET NO.
NE 17

MAR I ON

JNE0183

PROJECT NO

BRIDGE NO.

DATE DESCRIPTION

COMMISSION

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105 WEST CAPITOL

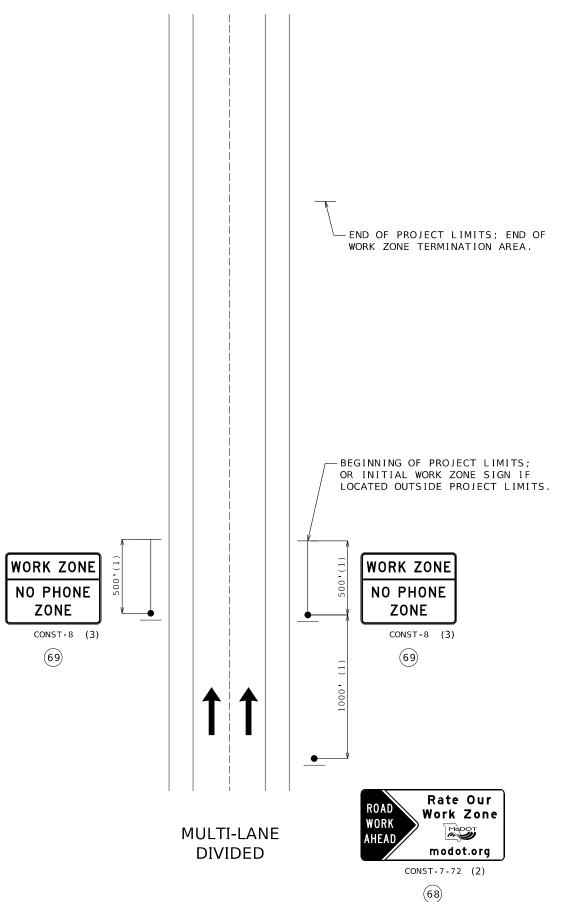
JEFFERSON CITY, MO 65102

MISSOURI HIGHWAYS AND TISCOMMISSION

HDR Engineering, Inc.

TRAFFIC CONTROL PLAN
TYPICAL APPLICATIONS
SHEET 2 OF 4

BEGIN/END OF PROJECT SIGNING



NOTES:

SIGN GO20-1 IS REQUIRED PER EPG 616.6.56.

SIGN GO20-2 IS USED ON ALL PROJECTS WHERE SIGN GO20-1 IS USED.

OTHER SIGNS SUCH AS DETOUR OR ALTERNATE ROUTE SIGNING MAY BE USED OUTSIDE THE PROJECT LIMITS.

ANY EXISTING SIGNING THAT CONFLICTS WITH THE TRAFFIC CONTROL SINGING SHALL BE COMPLETELY COVERED OR REMOVED.

BEGIN/END OF PROJECT SIGNING TO BE USED IN PLACE THROUGHOUT THE DURATION OF THE PROJECT.

- (1) DISTANCE MAY BE ADJUSTED ACCORDING TO FIELD CONDITIONS. WHERE TRAFFIC BACKUPS ARE EXPECTED BEYOND THE ADVANCE WARNING AREA, ADDITIONAL SIGNING MAY BE NEEDED.
- (2) SIGN CONST-7-72 OR CONST-7-48 IS PLACED 500 FEET BEFORE THE BEGINNING OF PROJECT LIMITS OR THE ROAD WORK AHEAD SIGN OR ROAD WORK NEXT XX MILES SIGN. IF USED, WHEN THESE SIGNS ARE LOCATED OUTSIDE OF THE PROJECT LIMITS.
- (3) THE "WORK ZONE NO PHONE ZONE" SIGN IS PLACED A MINIMUM OF 500 FEET BEFORE THE ROAD WORK AHEAD SIGN.

SIGNED, SEALED, AND 4/2/2024 6 MO NΕ 18 MAR I ON JNE0183 PROJECT NO BRIDGE NO.

TRAFFIC CONTROL PLAN
TYPICAL APPLICATIONS
SHEET 3 OF 4



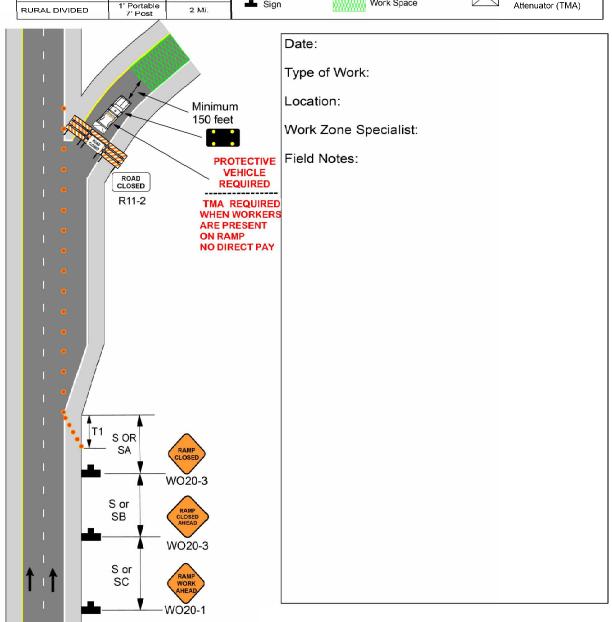
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9/2023

616.8.43S (TA-43S) Stationary Work on a Full Closed Ramp

| | SPEED | SIGN SP. | ACING (ft.) | TAPER LEN | GTH (ft.) | RECOMMENDED | CHANNELIZER | SPACING (ft.) |
|----|----------|-----------|-------------------------------------|--------------|-----------|--------------|-------------|---------------|
| Pe | ermanent | Undivided | Divided | Shoulder (1) | Lane (2) | BUFFER | Tapers | Buffer/ |
| F | Posted | (S) | (S) | (T1) | (T2) | LENGTH (ft.) | | Work Areas |
| | (mph) | | | | | (B) | | |
| | 0-35 | 200 | 200 | 70 | 245 | 250 | 35 | 40 |
| | 40-45 | 350 | 500 | 150 | 540 | 360 | 40 | 80 |
| | 50-55 | 500 | 1000 | 185 | 660 | 495 | 50 | 80 |
| | 60-70 | 1000 | SA - 1000 SB - 1500 SC - 2640 | 235 | 840 | 730 | 60 | 120 |

| | SIGN HEIGHT | MAXIMUM WORK ZONE LENGTH (L) | Channelizer | Flashing Arrow Panel | Protective Vehicle |
|---------------|------------------------|------------------------------------|---------------|--|-----------------------|
| URBAN | 1' Portable 7' Post | 1 Mi. | _ | ************************************** | Truck/Trailer Mounted |
| RURAL DIVIDED | 1' Portable | 2 Mi. | - Sign | Work Space | Attenuator (TMA) |



616.8.43S (TA-43S) Stationary Work on Ramps

• Always use advance warning signs, flashing arrow panels, and channelizers.

- 1. Use EPG 616.33SD (TA-33SD) Short Duration Lane Closure on a Multi-Lane Highway for setting advance warning signs.
- 2. Reduce channelizer spacing to no more than thirty feet at the beginning of the deceleration lane to past the gore point.
- Always use a Protective Vehicle, and a Truck/Trailer Mounted Attenuator (TMA):
 - 1. Activate high intensity rotating, flashing, oscillating, or strobe lights.
 - 2. Position the protective vehicle/TMA a minimum of 150 feet in advance of the work space, if possible.
 - 3. Operate the flashing arrow panel in arrow mode.
- · Always maintain a minimum of ten feet of driving surface. This may include a part of the shoulder if the shoulder is of adequate strength to handle the traffic.

For other operations, refer to:

- Mobile:
 - 1. EPG 616.35M (TA-35M) Mobile Operation on Multi-Lane Highway.
- Short Duration (60 minutes or less):
 - 1. EPG 616.8.33SD (TA-33SD) Short Duration Lane Closure on a Multi-Lane Highway.
- Long-term Stationary Operations:
 - 1. EPG 616.6.2.2 Flags and Advance Warning Rail System.
 - 2. EPG 616.6.78 Temporary Markings.
- Nighttime Operations:
 - 1. EPG 616.6.83 Warning Lights for use of sequential lights.
- Speed Limit Guidance:
 - 1. EPG 616.12 Work Zone Speed Limits for speed limit guidelines.
- Work in the Vicinity of an Exit Ramp:
 - 1. EPG 616.42S (TA-42S) Work in the Vicinity of an Exit Ramp.

SIGNED, SEALED, AND DATED ELECTRONICALLY

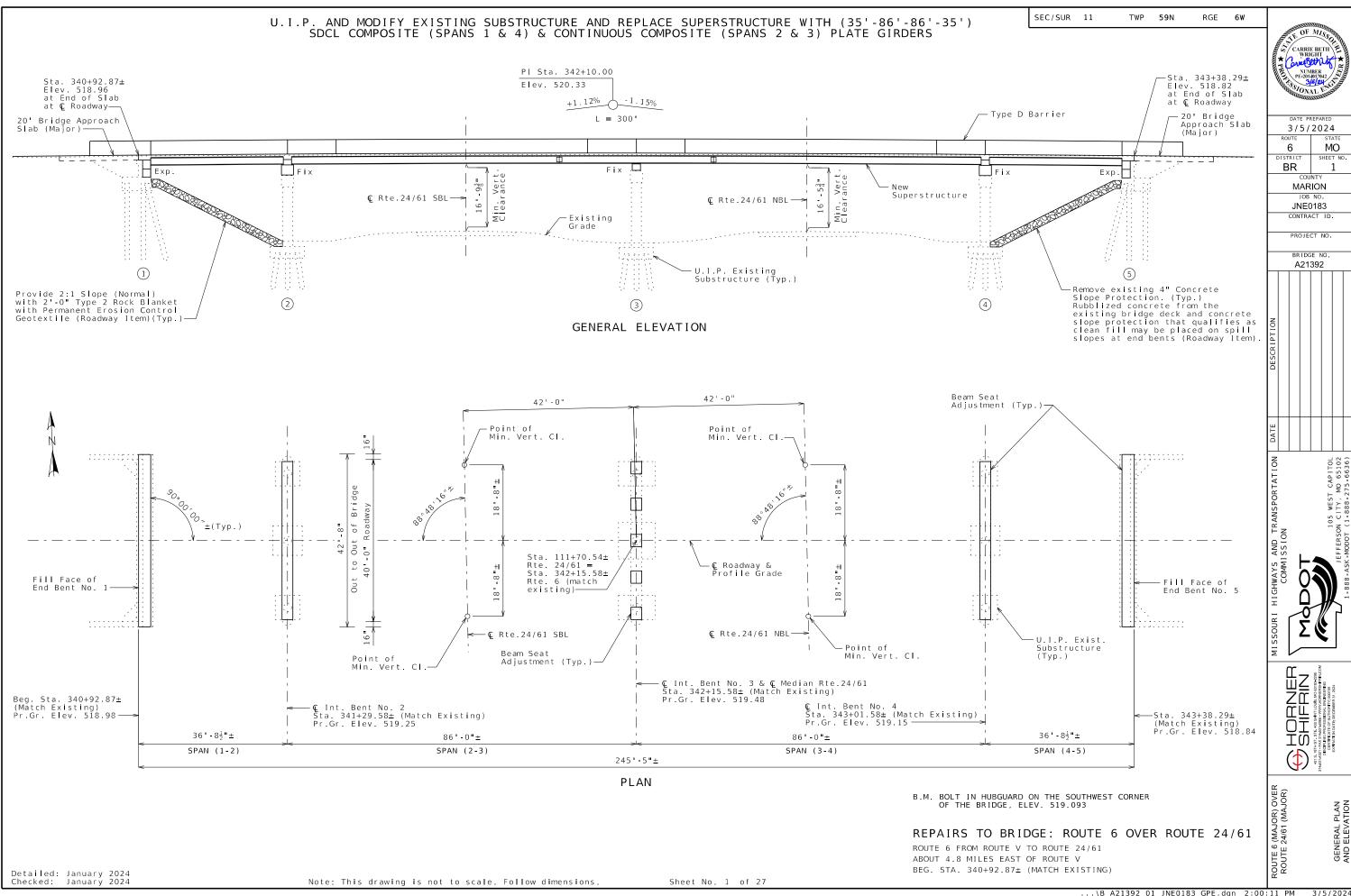
| DATE PREPARED | | | | | |
|---------------|-----------|--|--|--|--|
| 4/2/2024 | | | | | |
| ROUTE | STATE | | | | |
| 6 | MO | | | | |
| DISTRICT | SHEET NO. | | | | |
| NE | 19 | | | | |

MAR I ON JNE0183

PROJECT NO

BRIDGE NO

TRAFFIC CONTROL PLAN TYPICAL APPLICATIONS SHEET 4 OF 4



| Estimated | Quantities | | | |
|--|-------------|---------|-----------|---------|
| I t em | | Substr. | Superstr. | Total |
| Removal of Miscellaneous ACM (Non-Friable) | sq. foot | | 24 | 24 |
| Removal of Existing Superstructure | lump sum | | | 1 |
| Bridge Approach Slab (Major) | sq. yard | | 182 | 182 |
| Class B Concrete (Substructure) | cu. yard | 27.9 | | 27.9 |
| Slab on Steel | sq. yard | | 1,164 | 1,164 |
| Type D Barrier | linear foot | | 539 | 539 |
| Substructure Repair (Formed) | sq. foot | 25 | | 25 |
| Substructure Repair (Unformed) | sq. foot | 19 | | 19 |
| Reinforcing Steel (Epoxy Coated) | pound | 5,680 | | 5,680 |
| Protective Coating-Concrete Bents & Piers (Epoxy) | lump sum | | | 1 |
| Fabricated Structural Low Alloy Steel (Plate Girder) A709, Grade 50 | pound | | 254,380 | 254,380 |
| Intermediate Field Coat (System G) | sq. foot | | 10,200 | 10,200 |
| Finish Field Coat (System G) | sq. foot | | 2,400 | 2,400 |
| Laminated Neoprene Bearing Pad | each | | 30 | 30 |
| Laminated Neoprene Bearing Pad Assembly | each | | 5 | 5 |
| | | | | |
| | | | | |
| | | | | |
| | | | | |
| | | | | |
| | | | | |

All concrete and reinforcement above the Beam Seat Adjustment in the end bents is included in the Estimated Quantities for Slab on Steel

All reinforcement in the intermediate bent concrete diaphragms except reinforcement embedded in the Beam Seat Adjustment is included in the Estimated Quantities for Slab on Steel.

All concrete above the Beam Seat Adjustment in Intermediate Bent concrete diaphragms is included in the Estimated Quantities for Slab on Steel

All concrete and reinforcement in the Beam Seat Adjustment will be paid for as Class B Concrete (Substructure) and Reinforcing Steel (Epoxy Coated).

Cost of any required excavation for bridge will be considered completely covered by the contract unit price for other items

| Estimated Quantities for Slab on Steel | |
|---|--------|
| I t em | Total |
| Class B-2 Concrete cu. yard | 290.0 |
| Reinforcing Steel (Epoxy Coated) pound | 86,270 |
| | |

The table of Estimated Quantities for Slab on Steel represents the quantities used by the State in preparing the cost estimate for concrete slabs. The area of the concrete slab will be measured to the nearest square yard longitudinally from end of slab to end of slab and transversely from out to out of bridge slab (or with the horizontal dimensions as shown on the plan of slab). Payment for stay-in-place corrugated steel forms, conventional forms, all concrete and epoxy coated reinforcing steel will be considered completely covered by the contract unit price for the slab. Variations may be encountered in the estimated quantities but the variations cannot be used for an adjustment in the contract unit

Method of forming the slab shall be as shown on the plans and in accordance with Sec 703. All hardware for forming the slab to be left in place as a permanent part of the structure shall be coated in accordance with ASTM A123 or ASTM B633 with a thickness class SC 4 and a finish type I, II or III.

Slab shall be cast-in-place with conventional forms or stay-inplace corrugated steel forms. Precast prestressed panels will not be permitted

GENERAL NOTES:

Design Specifications: 2002 AASHTO LFD (17th Ed.) Standard Specifications Seismic Performance Category A

Design Loading:

H20-44 (1965), HS20-44 (New Construction) 35 lb/sf Future Wearing Surface Earth = 120 lb/cf. Equivalent Fluid Pressure = 45 lb/cf.(min.) Fatique Stress - Case II structure: Simple for dead load in Spans (1-2) and (4-5). Continuous for dead load in Spans (2-3) and (3-4). Non-composite for dead load in all spans. Continuous composite for live load in all spans.

Design Unit Stresses:

Class B Concrete (Substructure) f'c = 3,000 psiClass B-2 Concrete (Superstructure except Barrier) f'c = 4,000 psif'c = 4,000 psiClass B-1 Concrete (Barrier) Reinforcing Steel (Grade 60) fy = 60,000 psify = 50,000 psiStructural Steel (ASTM A709 Grade 50)

Neoprene Pads:

Neoprene bearing pads shall be 60 durometer and shall be in accordance with Sec 716.

Fabricated Steel Connections:

Field connections shall be made with 3/4" diameter ASTM F3125 Grade A325 Type 1 bolts and 13/16" diameter holes, except as

All joint filler shall be in accordance with Sec 1057 for preformed sponge rubber expansion and partition joint filler,

Reinforcing Steel:
Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise shown.

Structural Steel Protective Coatings:

Protective Coating: System G in accordance with Sec 1081.

Prime Coat: The cost of the inorganic zinc prime coat will be considered completely covered by the contract unit price for the fabricated structural steel.

Field Coat: The color of the field coat shall be Gray (Federal Standard #26373). The cost of the intermediate field coat will be considered completely covered by the contract unit price per sq. foot for Intermediate Field Coat (System G). The cost of the finish field coat will be considered completely covered by the contract unit price per sq. foot for Finish Field Coat (System G).

At the option of the contractor, the intermediate field coat and finish field coat may be applied in the shop. The contractor shall exercise extreme care during all phases of loading, hauling, handling, erection and pouring of the slab to minimize damage and shall be fully responsible for all repairs and cleaning of the coating systems as required by the engineer.

Concrete Protective Coatings:

Protective coating for concrete bents and piers (Epoxy) shall be applied as shown on the bridge plans and in accordance with Sec 711.

Existing Structure:

All concrete repairs shall be in accordance with Sec 704,

Outline of existing work is indicated by light dashed lines. Heavy lines indicate new work.

Contractor shall verify all dimensions in field before ordering new material.

The area exposed by the removal of concrete and not covered with new concrete shall be coated with an approved bituminous paint qualified special mortar in accordance

Traffic Handling:
Structure to be closed during construction. Traffic to be maintained on other routes during construction. See Roadway Plans for traffic control.

Vertical clearance for Route 24/61 traffic during construction shall be 14'-6" minimum over a 24'-0" wide horizontal opening of the roadway in each direction.

Miscellaneous

High strength bolts, nuts and washers will be sampled for quality assurance as specified in Sec 106.

Longitudinal dimensions are based on the original design

Roadway surfacing adjacent to bridge ends shall match new bridge slab surface (roadway item)

Stav-in-Place Forms:

tay-in-Place Forms: Corrugated steel forms, supports, closure elements and accessories shall be in accordance with grade requirement and coating designation G165 of ASTM A653. Complete shop drawings of the permanent steel deck forms shall be required in accordance with Sec 1080.

Corrugations of stay-in-place forms shall be filled with an expanded polystyrene material. The polystyrene material shall be placed in the forms with an adhesive in accordance with the manufacturer's recommendations.

Form sheets shall not rest directly on the top of girder flanges. Sheets shall be securely fastened to form supports with a minimum bearing length of one inch on each end. Form supports shall be placed in direct contact with the flange. Welding on or drilling holes in the girder flanges will not be permitted. All steel fabrication and construction shall be in accordance with Sec 1080 and 712. Certified field welders will not be required for welding of the form supports.

The design of stay-in-place corrugated steel forms is per manufacturer which shall be in accordance with Sec 703 for false work and forms. Maximum actual weight of corrugated steel forms allowed shall be 4 psf assumed for girder loading.



3/5/2024 6 MO SHEET NO BR 2

MARION JNE0183 CONTRACT ID

PROJECT NO

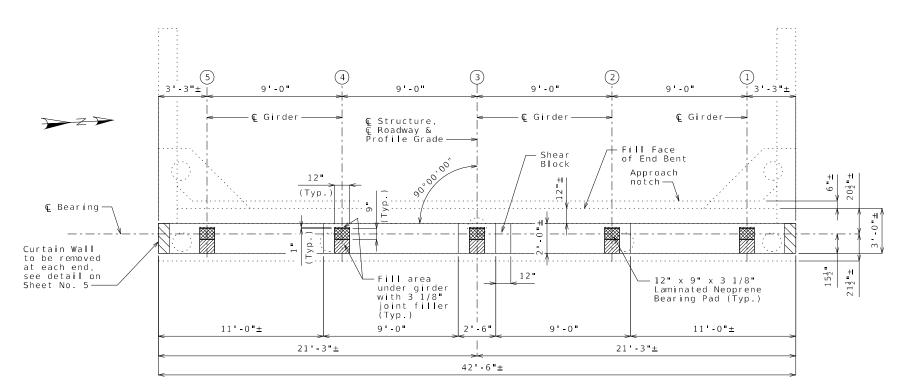
BRIDGE NO A21392

HORNER SHIFFIN (1)

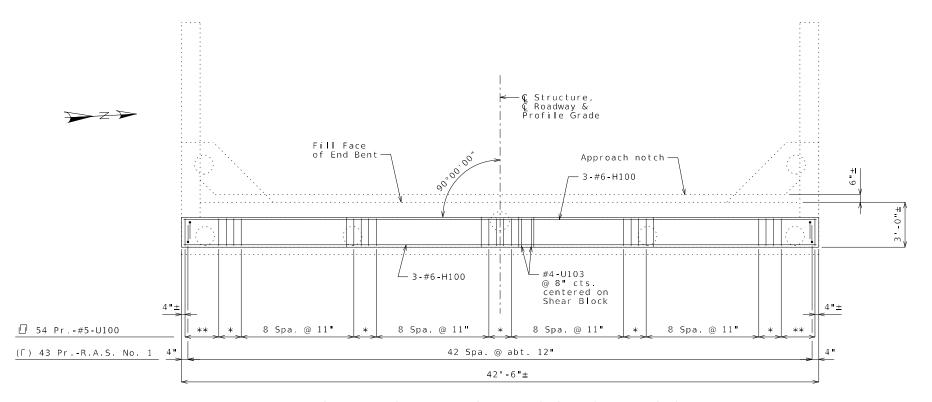
Steel corrugated A→ bridge forms - Form support Fill corrugations with foam (Typ.) VЛ VÀ SECTION A-A

> OPTIONAL STAY-IN-PLACE FORM DETAILS

Detailed: January 2024



PLAN OF BEAM SEAT ADJUSTMENT SHOWING DIMENSIONS

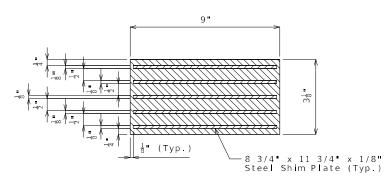


PLAN OF BEAM SEAT ADJUSTMENT SHOWING REINFORCEMENT
* 3 Spa. @ 6"

DETAILS OF END BENT NO. 1

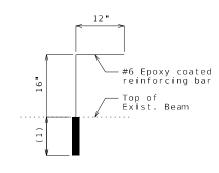
Detailed: January 2024 Checked: January 2024 Note: This drawing is not to scale. Follow dimensions.

Sheet No. 3 of 27



TYPICAL SECTION THRU 9"x12"x3 1/8" LAMINATED NEOPRENE BEARING PAD

(5 required)



R.A.S. NO. 1

(86 required)

(1) Manufacturer's recommended embedment length (5" min.)

Note:

Work this sheet with Sheets No. 4 & 5.

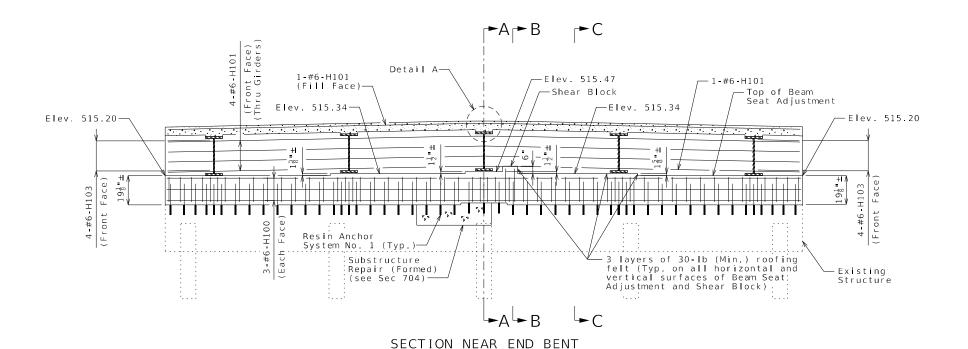


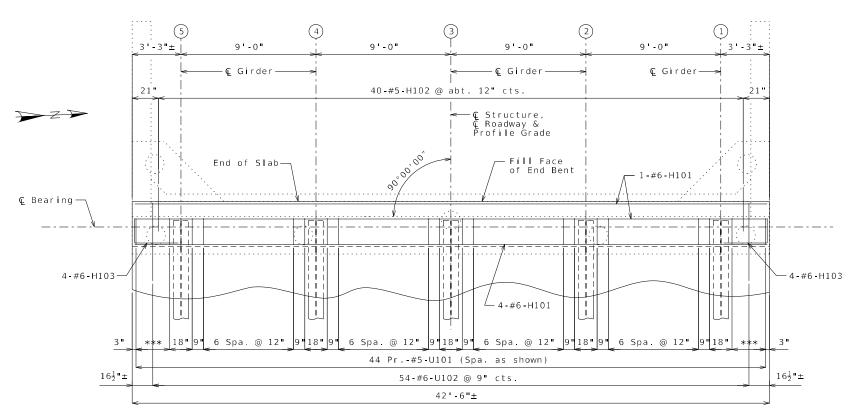


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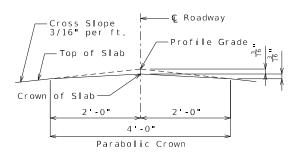


PART PLAN OF END BENT DIAPHRAGM *** 3 Spa. @ abt. 9"

DETAILS OF END BENT NO. 1

Detailed: January 2024 Checked: January 2024 Note: This drawing is not to scale. Follow dimensions.

Sheet No. 4 of 27



DETAIL A

Notes:

Work this sheet with Sheets No. 3 & 5.

All concrete in the End Bent above top of proposed Beam Seat Adjustment and below top of slab shall be Class B-2.

The contractor shall shift reinforcement as necessary to clear shear block by 1 $1/2^{\circ}$.

Beam Seat Adjustment shall be Class B Concrete.

Concrete diaphragms at the end bents shall be poured a minimum of 12 hours before the slab is poured.

Elevations shown are based on existing bridge plan elevations, and a vertical datum shift of -.61'. The contractor shall be solely responsible for verification and any necessary elevation and dimension modifications prior to ordering materials and fabricating girders and bearings. See Special Provisions.

The contractor shall use one of the qualified resin anchor systems in accordance with Sec 1039.

Cost of furnishing and installing the resin anchor systems, complete in place, will be considered completely covered by the contract unit price for Class B Concrete (Substructure).

The minimum embedment depth in concrete with f'c = 4,000 psi for the resin anchor system shall be that required to meet the minimum ultimate pullout strength in accordance with Sec 1039 but shall not be less than 5".

All resin anchor systems shall be spaced to clear existing reinforcing in cap to be used-in-place by a minimum of $1/2^{\circ}$.

An epoxy coated #6 Grade 60 reinforcing bar shall be substituted for the 3/4" threaded rod.

Top of diaphragm for End Bent No. 1 shall be formed to the crown and grade of the roadway.

Area of existing concrete to be covered with new concrete shall be prepared in accordance with Sec 704 and roughened to amplitude of 1/4" before pouring new concrete. Surface preparation of concrete will be considered completely covered by the contract unit price for Class B Concrete (Substructure).

The bars shown in the Beam Seat Adjustment, except bars used with resin anchor system, shall be considered completely covered by the contract unit price for Reinforcing Steel (Epoxy Coated).

All concrete and reinforcement above the Beam Seat Adjustment shall be considered completely covered by the contract unit price for Slab on Steel.

The U-bars shall be placed parallel to ${f C}$ Girders.

For reinforcement of the barrier, see Sheets No. 23 & 24.

For details of Bridge Approach Slab, see Sheet No. 25.



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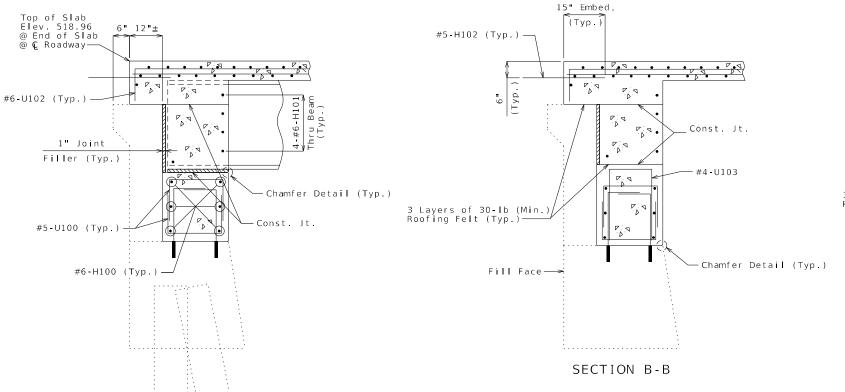
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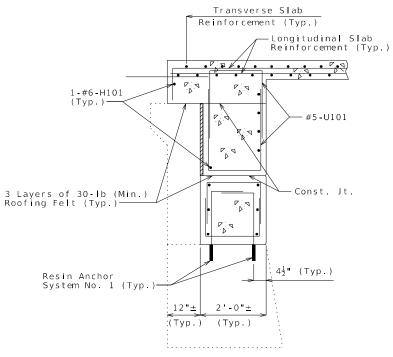
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Work this sheet with Sheets No. 3 & 4.

SECTION C-C

The cost to remove the curtain walls at each end bent will be considered completely covered by the contract unit price for Removal of Existing Superstructure.

| Area to be removed, typ. each end | Area to be removed, typ. each end | CHAMFER DETAIL |
|-----------------------------------|-----------------------------------|----------------|
| ELEVATION | PLAN | |

SECTION A-A

−¢ Exterior Girder

CURTAIN WALL REMOVAL DETAIL

| Substructure | Quantity | Table | for | Bent I | No. 1 |
|------------------------|-----------|-------|-----|----------|----------|
| | I t em | | | | Quantity |
| Class B Concrete (Subs | tructure) | | | cu. yard | 5.3 |
| Substructure Repair (F | ormed) | | | sq. foot | 5 |
| | | | | pound | 840 |
| | | | | | |
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These quantities are included in the Estimated Quantities table on Sheet No. 2.

DETAILS OF END BENT NO. 1

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Detailed: January 2024 Checked: January 2024



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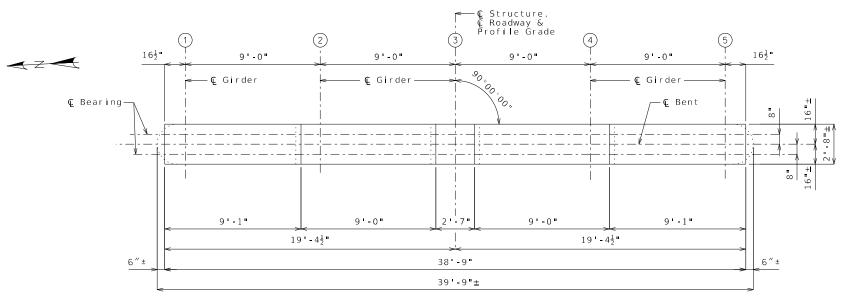
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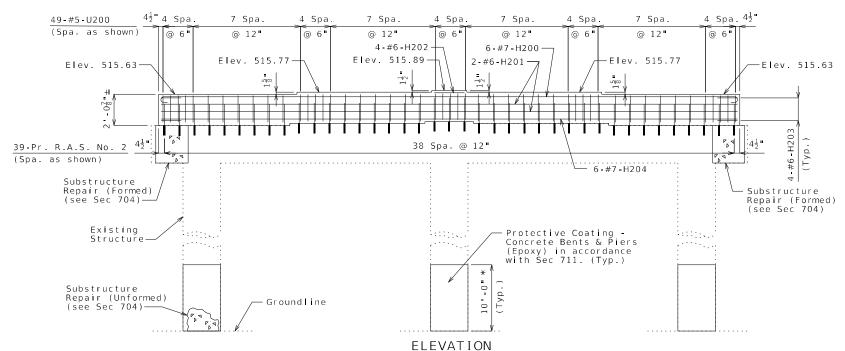
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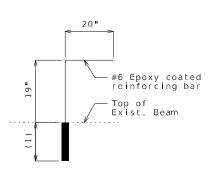
ROUTE 24/61 (MAJOR)



PLAN OF BEAM SEAT ADJUSTMENT SHOWING DIMENSIONS

Note: Reinforcement not shown for clarity.





R . A . S . NO . 2

(1) Manufacturer's recommended embedment length (5" min.)

Detailed: January 2024 Checked: January 2024 Note: Dowel bars and key not shown for clarity.

* Pay limits of Protective Coating-Concrete Bents and Piers (Epoxy) (Typ. around each column)

Resin Anchor Notes:

RAS = Resin Anchor System

The contractor shall use one of the qualified resin anchor systems in accordance with Sec 1039.

Cost of furnishing and installing the resin anchor systems, complete in place, will be considered completely covered by the contract unit price for Class B Concrete (Substructure).

The minimum embedment depth in concrete with f'c = 4,000 psi for the resin anchor systems shall be that required to meet the minimum ultimate pullout strength in accordance with Sec 1039 but shall not be less than 5".

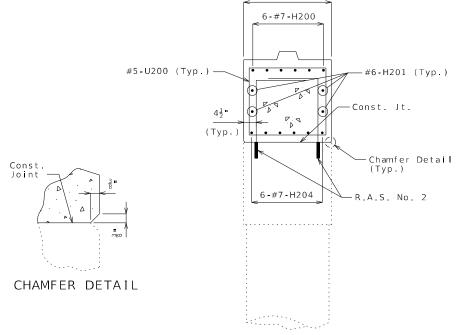
An epoxy coated #6 Grade 60 reinforcing bar shall be substituted for the 3/4" diameter threaded rod.

All resin anchor systems shall be spaced to clear existing reinforcing in cap to be used-in-place by a minimum of 1/2".

DETAILS OF INTERMEDIATE BENT NO. 2

Sheet No. 6 of 27

Note: This drawing is not to scale. Follow dimensions.



2 -8"±

Notes:

Work this sheet with Sheet No. 7.

The bars shown in the Beam Seat Adjustment, except bars used with resin anchor system, shall be included in the contract unit price for Reinforcing Steel (Epoxy Coated).

TYPICAL SECTION

All concrete in Beam Seat Adjustment shall be Class B Concrete.

Existing anchor bolts shall be removed or cut flush with existing concrete. The contractor shall repair any concrete damaged in accordance with Sec 704 and fill holes with an approved grout in accordance with Sec 1066. Removal of existing anchor bolts will be considered completely covered by the contract unit price for Removal of Existing Superstructure.

Beam seat adjustment heights are based on existing bridge plans and a vertical datum adjustment of -0.61'. Contractor shall adjust as necessary.

| Substructure Quantity | Table | for | Bent | No. 2 |
|----------------------------------|-------|-----|----------|----------|
| I t em | | | | Quantity |
| Class B Concrete (Substructure) | | | cu. yard | d 8.1 |
| Substructure Repair (Formed) | | | sq. foo | t 20 |
| Substructure Repair (Unformed) | | | sq. foo | t 4 |
| Reinforcing Steel (Epoxy Coated) | | | pound | d 1,800 |
| | | | | |
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These quantities are included in the Estimated Quantities table on Sheet No. 2.



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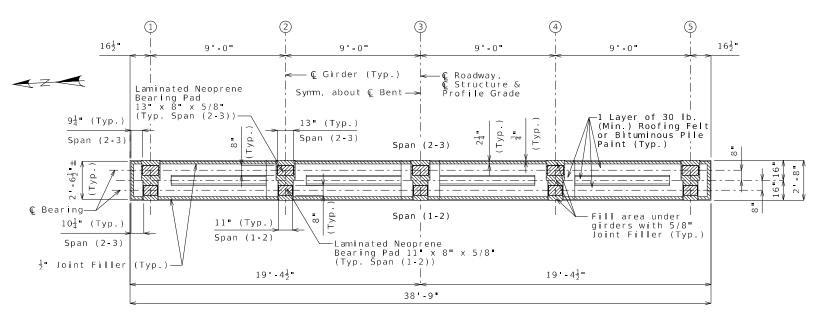


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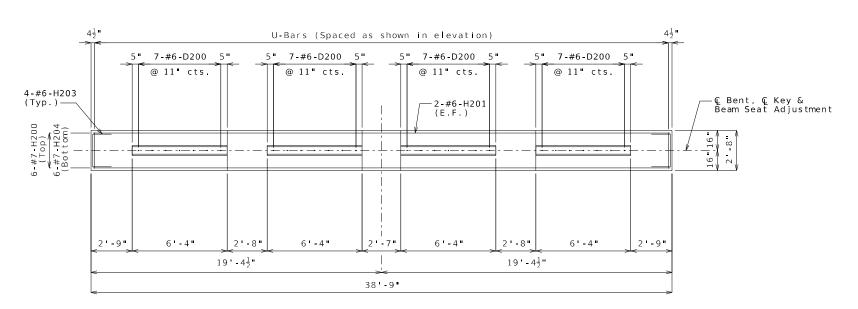


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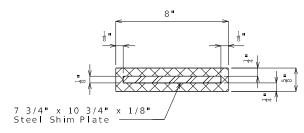
COUTE 6 (MAJOR) OVER ROUTE 24/61 (MAJOR)



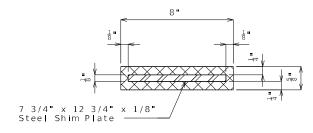
PLAN OF BEAM SEAT ADJUSTMENT



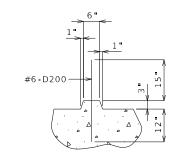
PLAN OF BEAM SEAT ADJUSTMENT SHOWING REINFORCEMENT



TYPICAL SECTION THRU 8"x11"x5/8" LAMINATED NEOPRENE BEARING PAD SPAN (1-2)



TYPICAL SECTION THRU 8"x13"x5/8" LAMINATED NEOPRENE BEARING PAD SPAN (2-3)



SECTION THRU KEY



Notes:

Work this sheet with Sheet No. 6.

For details of diaphragm at intermediate bent, see Sheet No. 18.

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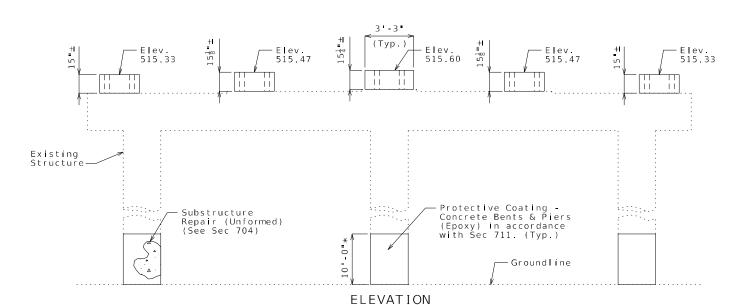
OUTE 6 (MAJOR) OVER ROUTE 24/61 (MAJOR)

DETAILS OF INTERMEDIATE BENT NO. 2

Detailed: January 2024 Checked: January 2024

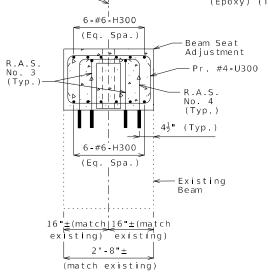
PLAN OF BEAM SEAT ADJUSTMENT SHOWING DIMENSIONS

Note: Reinforcement not shown for clarity.

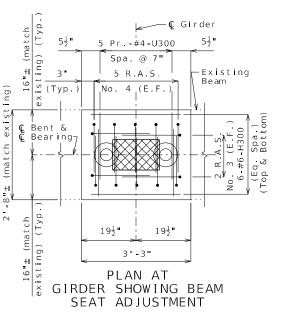


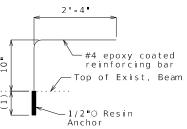
Note: Reinforcement not shown for clarity.

*Pay limits of Protective Coating-Concrete Bents and Piers © Bent− (Epoxy) (Typ. around each column)



SECTION THRU BEAM SEAT ADJUSTMENT (Interior Girders shown Exterior Girders similar)





R.A.S. NO. 3 (20 required)

-Spot weld

AASHTO M32

size W5 wire

-1 1/2 turns

20" -#4 epoxy coated reinforcing bar Top of Exist. Beam 1/2"⊘ Resin Anchor

R.A.S. NO. 4 (50 required)

(1) Manufacturer's recommended embedment length (5" min.)

Resin Anchor Notes:

R.A.S = Resin Anchor System

The contractor shall use one of the qualified resin anchor systems in accordance with Sec 1039.

Cost of furnishing and installing the resin anchor systems, complete in place, will be considered completely covered by the contract unit price for Class B Concrete (Substructure).

The minimum embedment depth in concrete with f'c =4,000 psi for the resin anchor systems shall be that required to meet the minimum ultimate pullout strength in accordance with Sec 1039 but shall not be less than 5".

An epoxy coated #4 Grade 60 reinforcing bar shall be substituted for the 1/2" diameter threaded rod.

All resin anchor systems shall be spaced to clear existing reinforcing in cap to be used-in-place by a minimum of 1/2".

For details of Laminated Neoprene Bearing Pad Assembly, see Sheet No. 14.

Beam seat adjustment heights are based on existing bridge plans and a vertical datum adjustment of -0.61'. Contractor shall adjust as

The bars shown in the Beam Seat Adjustment, except bars used with resin anchor systems, will be included in the contract unit price for Reinforcing Steel (Epoxy Coated).

All concrete in Beam Seat Adjustment shall be Class B Concrete.

Existing anchor bolts shall be removed or cut flush with existing concrete if they do not interfere with new anchor bolts. The contractor shall repair any concrete damaged in accordance with Sec 704 and fill holes with an approved grout in accordance with Sec 1066. Removal of existing anchor bolts will be considered completely covered by the contract unit price for Removal of Existing Superstructure.

All reinforcing bars in the tops of beam seat adjustments shall be spaced to clear anchor bolt wells for bearings by at least 1/2".

Holes for anchor bolts may be drilled into the substructure.

| Substructure Quantity | Table | for | Bent | No. 3 |
|----------------------------------|-------|-----|----------|----------|
| I t em | | | | Quantity |
| Class B Concrete (Substructure) | | | cu. yard | 2.0 |
| Substructure Repair (Unformed) | | | sq. foot | 5 |
| Reinforcing Steel (Epoxy Coated) | | | pound | 430 |
| | | | | |
| | | | | |
| | | | | |

These quantities are included in the Estimated Quantities table on Sheet No. 2.

DETAILS OF INTERMEDIATE BENT NO. 3

Detailed: January 2024

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 8 of 27

Pitch

DETAIL OF SPIRAL REINFORCEMENT

Clear top reinforcement (Tie top of spiral to

longitudinal reinforcement) -

DETAIL OF

ANCHOR BOLT WELLS

- AASHTO M32 Size W5 Wire (Bar Mark W301)



(D)

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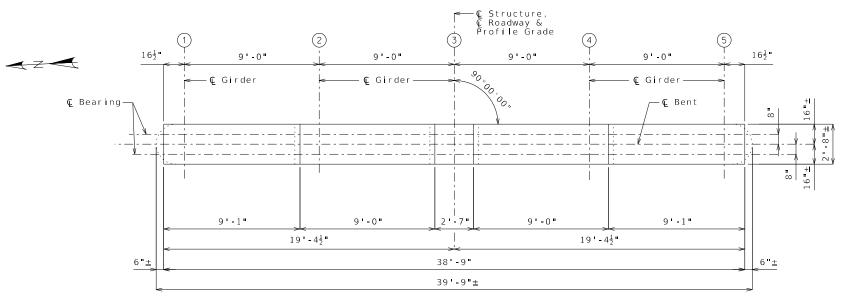
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BRIDGE NO A21392

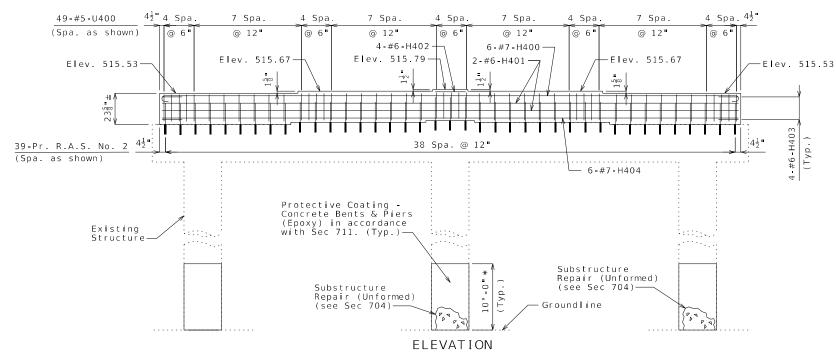
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OUTE 6 (MAJOR) OVER ROUTE 24/61 (MAJOR)



PLAN OF BEAM SEAT ADJUSTMENT SHOWING DIMENSIONS

Note: Reinforcement not shown for clarity.



Note: Dowel bars and key not shown for clarity. * Pay limits of Protective Coating-Concrete Bents

and Piers (Epoxy) (Typ. around each column)

Resin Anchor Notes:

R.A.S. = Resin Anchor System

The contractor shall use one of the qualified resin anchor systems in accordance with Sec 1039.

Cost of furnishing and installing the resin anchor systems, complete in place, will be considered completely covered by the contract unit price for Class B Concrete (Substructure).

The minimum embedment depth in concrete with f'c = 4,000 psi for the resin anchor systems shall be that required to meet the minimum ultimate pullout strength in accordance with Sec 1039 but shall not be less than 5".

An epoxy coated #6 Grade 60 reinforcing bar shall be substituted for the 3/4" diameter threaded rod.

All resin anchor systems shall be spaced to clear existing reinforcing in cap to be used-in-place by a minimum of 1/2".

DETAILS OF INTERMEDIATE BENT NO. 4

Sheet No. 9 of 27

Detailed: January 2024 Checked: January 2024 Note: This drawing is not to scale. Follow dimensions.

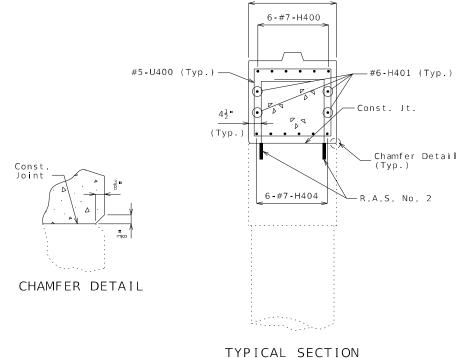
#6 Epoxy coated reinforcing bar

-Topof Exist.Beam

20"

R.A.S. NO. 2

(78 required) (1) Manufacturer's recommended embedment length (5" min.)



2 -8"±

Notes:

Work this sheet with Sheet No. 10.

The bars shown in the Beam Seat Adjustment, except bars used with resin anchor system, shall be included in the contract unit price for Reinforcing Steel (Epoxy Coated).

All concrete in Beam Seat Adjustment shall be Class B

Existing anchor bolts shall be removed or cut flush with existing concrete. The contractor shall repair any concrete damaged in accordance with Sec 704 and fill holes with an approved grout in accordance with Sec 1066. Removal of existing anchor bolts shall be considered completely covered by the contract unit price for Removal of Existing

Beam seat adjustment heights are based on existing bridge plans and a vertical datum adjustment of -0.61'. Contractor shall adjust as necessary.

| Substructure Quantity | Table | for | Bent 1 | No. 4 |
|----------------------------------|-------|-----|----------|----------|
| I t em | | | | Quantity |
| Class B Concrete (Substructure) | | | cu. yard | 7.7 |
| Substructure Repair (Unformed) | | | sq. foot | 10 |
| Reinforcing Steel (Epoxy Coated) | | | pound | 1,790 |
| | | | | |
| | | | | |
| | | | | |

These quantities are included in the Estimated Quantities table on



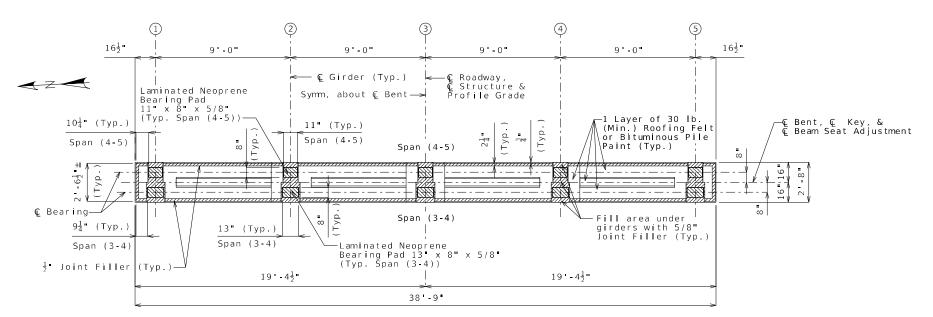
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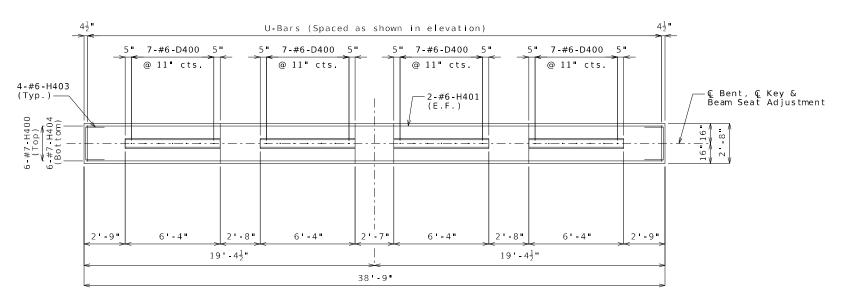
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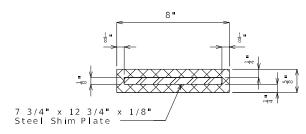
OUTE 6 (MAJOR) OVEF ROUTE 24/61 (MAJOR)



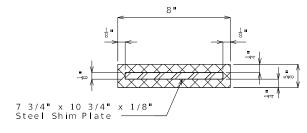
PLAN OF BEAM SEAT ADJUSTMENT



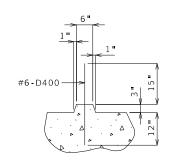
PLAN OF BEAM SEAT ADJUSTMENT SHOWING REINFORCEMENT



TYPICAL SECTION THRU 8"x13"x5/8" LAMINATED NEOPRENE BEARING PAD SPAN (3-4)



TYPICAL SECTION THRU 8"x11"x5/8" LAMINATED NEOPRENE BEARING PAD SPAN (4-5)



SECTION THRU KEY

Notes:

Work this sheet with Sheet No. 9.

For details of diaphragm at intermediate bent, see Sheet No. 18.

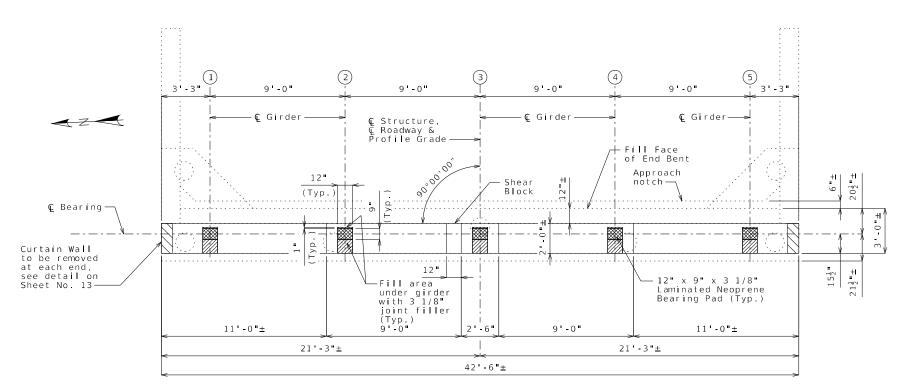
OUTE 6 (MAJOR) OVER ROUTE 24/61 (MAJOR)

DETAILS OF INTERMEDIATE BENT NO. 4

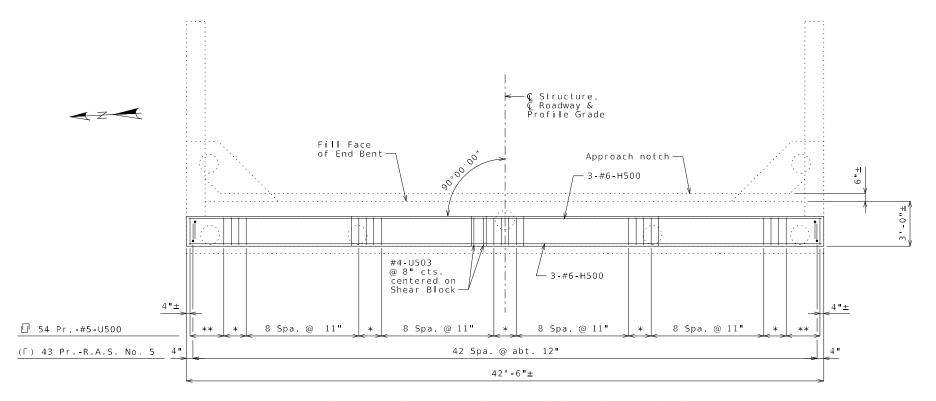
Detailed: January 2024 Checked: January 2024

2/29/2024 6 MO SHEET NO BR 10 MARION JNE0183 CONTRACT ID. PROJECT NO. BRIDGE NO A21392

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PLAN OF BEAM SEAT ADJUSTMENT SHOWING DIMENSIONS



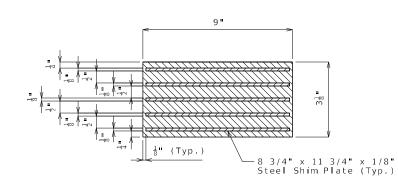
PLAN OF BEAM SEAT ADJUSTMENT SHOWING REINFORCEMENT

* 3 Spa. @ 6" ** 3 Spa. @ 10"

DETAILS OF END BENT NO. 5

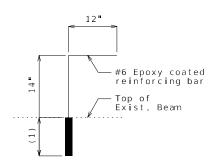
Detailed: January 2024 Checked: January 2024 Note: This drawing is not to scale. Follow dimensions.

Sheet No. 11 of 27



TYPICAL SECTION THRU 9"x12"x3 1/8" LAMINATED NEOPRENE BEARING PAD

(5 required)



R.A.S. NO. 5

(86 required)

(1) Manufacturer's recommended embedment length (5" min.)

Note:



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JNE0183

CONTRACT ID.

PROJECT NO.



SSOURI HIGHWAYS AND TRANSPORTATION

COMMISSION

105 WEST CAPITOL

105 WEST CAPITOL

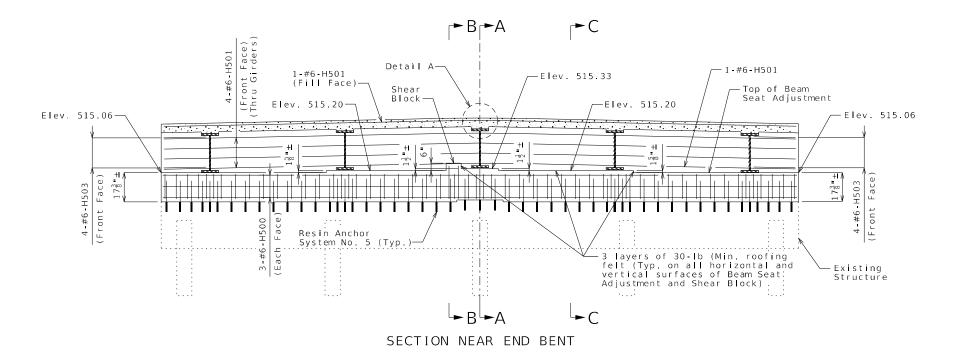
105 WEST CAPITOL

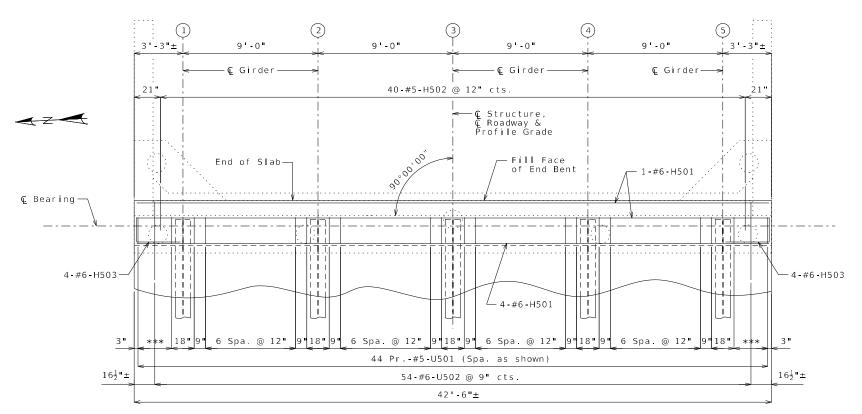
HORNOR SERVICE SERVICE

COUTE 24/61 (MÁJOR)

...\B_A21392_11_JNE0183_End Bent 5.dgn 3:14:53 PM 2/29/2024

Work this sheet with Sheets No. 12 & 13.





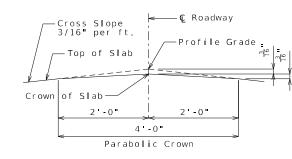
PART PLAN OF END BENT DIAPHRAGM *** 3 Spa. @ 9"

DETAILS OF END BENT NO. 5

Detailed: January 2024

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 12 of 27



DETAIL A

Notes:

Work this sheet with Sheets No. 11 & 13.

All concrete in the End Bent above top of proposed Beam Seat Adjustment and below top of slab shall be Class B-2.

The contractor shall shift reinforcement as necessary to clear shear block by 1 $1/2^{\circ}$.

Beam Seat Adjustment shall be Class B concrete.

Concrete diaphragms at the end bents shall be poured a minimum of 12 hours before the slab is poured.

Elevations shown are based on existing bridge plan elevations, and a vertical datum shift of - .61 contractor shall be solely responsible for verification and any necessary elevation and dimension modifications prior to ordering materials and fabricating girders and bearings. See Special Provisions.

The contractor shall use one of the qualified resin anchor systems in accordance with Sec 1039.

Cost of furnishing and installing the resin anchor systems, complete in place, will be considered completely covered by the contract unit price for Class B Concrete (Substructure).

The minimum embedment depth in concrete with f'c = 4,000 psi for the resin anchor system shall be that required to meet the minimum ultimate pullout strength in accordance with Sec 1039 but shall not be less than 5".

All resin anchor systems shall be spaced to clear existing reinforcing in cap to be used-in-place by a

An epoxy coated #6 Grade 60 reinforcing bar shall be substituted for the 3/4" threaded rod.

Top of diaphragm for End Bent No. 5 shall be formed to the crown and grade of the roadway.

Area of existing concrete to be covered with new concrete shall be prepared in accordance with Sec 704 and roughened to amplitude of 1/4" before pouring new concrete. Surface preparation of concrete will be considered completely covered by the contract unit price for Class B Concrete (Substructure).

The bars shown in the Beam Seat Adjustment, except bars used with resin anchor system, shall be considered completely covered by the contract unit price for Reinforcing Steel (Epoxy Coated).

All concrete and reinforcement above the Beam Seat Adjustment shall be considered completely covered by the contract unit price for Slab on Steel.

The U-bars shall be placed parallel to € Girders.

For reinforcement of the barrier, see Sheets No. 23 & 24.

For details of Bridge Approach Slab, see Sheet No. 25.



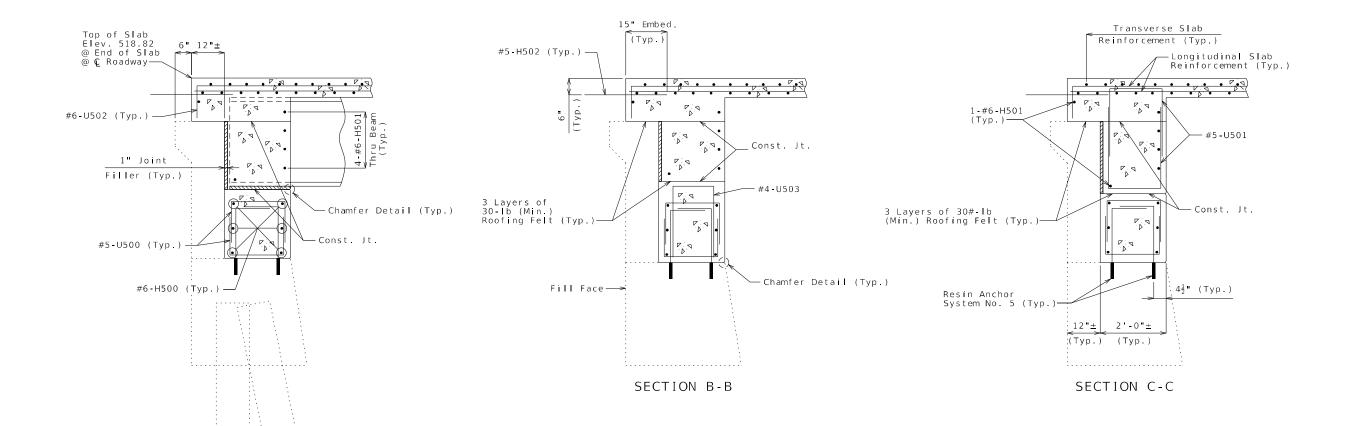
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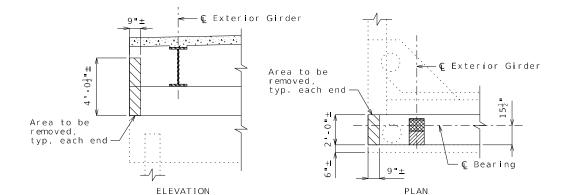
JNE0183 CONTRACT ID

PROJECT NO



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SECTION A-A

Const. CHAMFER DETAIL

CURTAIN WALL REMOVAL DETAIL

Notes:

Work this sheet with Sheets No. 11 & 12.

The cost to remove the curtain walls at each end bent will be considered completely covered by the contract unit price for Removal of Existing Superstructure.

| Substructure | Quantity | Table | for | Bent N | No. 5 |
|------------------------|------------|-------|-----|----------|----------|
| | I t em | | | | Quantity |
| Class B Concrete (Subs | tructure) | | | cu. yard | 4.8 |
| Reinforcing Steel (Epo | xy Coated) | | | pound | 820 |
| | | | | | |
| | | | | | |
| | | | | | |
| | | | | | |

These quantities are included in the Estimated Quantities table on Sheet No. 2.

DETAILS OF END BENT NO. 5

Detailed: January 2024 Checked: January 2024

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 13 of 27



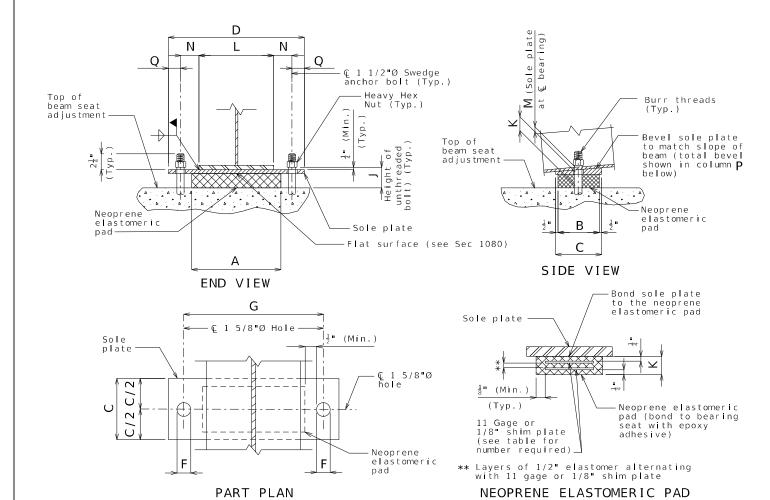
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CONTRACT ID.

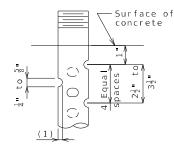


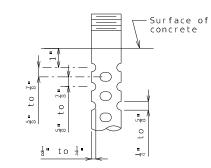
HORNER SHIFFIN

OUTE 6 (MAJOR) OVER ROUTE 24/61 (MAJOR)



| | | | | | | | F | ΙΧΕ | D B | EAF | RIN | GS | | | |
|--------------|---------------|--------------|--------------|-------|------------------|--------------|------------------|------|--------------|--------------|------------------|--------|---------------------------------|----------------------------|--------------------|
| BENT NO. | Α | В | С | D | F | G | J | K | L | М | N | Р | Q | NUMBER OF SHIM PLATES * | NUMBER REQUIRED |
| 3 | 21" | 12" | 13" | 28½" | 1 5 " | 24" | 6 ³ " | 5 " | 19" | 1½" | 2 1 " | 0 " | 2 ¹ / ₄ " | 8 | 5 |
| | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | |
| * The | requ | uire of e | d sh last | im p | lat | e sh d mo | all Ided | be p | lace ethe | d be r to | twee | n n | | TOTAL BEAR I NGS | 5 |
| l a y a n | ers (inte | ote gral | uni | ome r | an | d mo | Ided | tog | ethe | r to | tori | m | l | DEARTINGS | |





DETAIL OF 3/4"Ø THRU 2 1/2"Ø ANCHOR BOLTS

OPTIONAL DETAIL OF 1 3/8"Ø THRU 2 1/2"Ø ANCHOR BOLTS

SWEDGE ANCHOR BOLT DETAILS

(1) $\frac{1}{8}$ " for $\frac{3}{4}$ "Ø thru $1\frac{1}{4}$ "Ø anchor bolts

 $\frac{1}{8}$ " to $\frac{1}{4}$ " for $1\frac{3}{8}$ "Ø thru $2\frac{1}{2}$ "Ø anchor bolts

GENERAL NOTES:

Anchor bolts shall be 1 1/2"Ø ASTM F1554 Grade 55 swedged bolts and shall extend 18" into the concrete with ASTM A563 Grade A Heavy Hex nuts. Actual manufacturer's certified mill test reports(chemical and mechanical) shall be provided. Swedging shall be 1" less than extension into the concrete.

Anchor bolts and heavy hex nuts shall be coated with a minimum of two coats of inorganic zinc primer to provide a total dry film thickness of 4 mils minimum, 6 mils maximum, or galvanized in accordance with Sec 1081.

Neoprene Elastomeric Pads shall be 60 Durometer.

Structural steel for sole plate shall be ASTM A709 Grade 50 and shall be coated with a minimum of two coats of inorganic zinc primer to provide a total dry film thickness of 4 mils minimum, 6 mils maximum.

Laminated Neoprene Bearing Pad Assembly shall be in accordance with Sec $716. \,$



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PROJECT NO.

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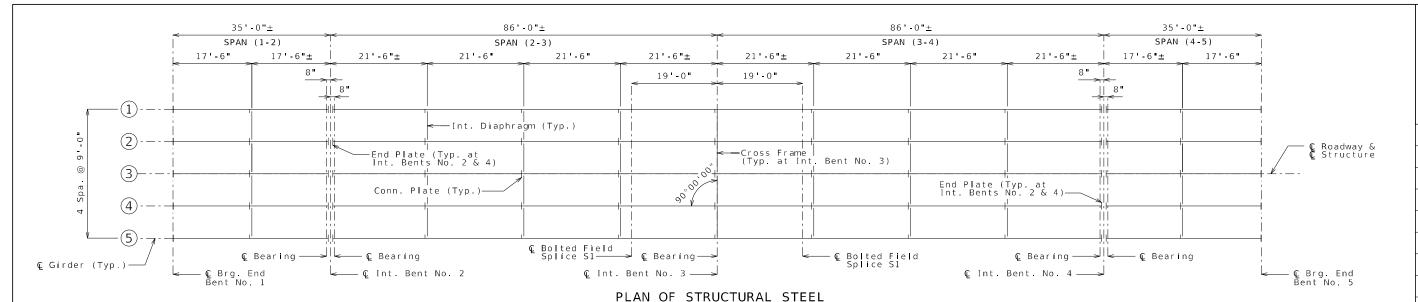
HIGHWAYS AND TRANSPORTATION DATE
COMMISSION

105 WEST CAPITOL
JEFFERSON CITY, MO 65102

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ROUTE 24/61 (MAJOR)

LAMINATED NEOPRENE BEARING PAD ASSEMBLY



Intermediate diaphragm connection plate and diaphragm spacing may vary from plan dimensions by a maximum of 3" for diaphragm to connect to the intermediate

Field connections shall be made with 3/4 diameter ASTM F3125 Grade A325 Type 1 bolts and 13/16 diameter holes, except as noted.

At the contractor's option, holes in the diaphragm plate of non slab bearing diaphragms may be made 3/16" larger than the nominal diameter of the bolt. A hardened washer shall be used under the bolt head and nut when this option is used. Holes in the girder diaphragm connection plate or transverse web stiffener shall be standard size.

Longitudinal dimensions are horizontal dimensions and shall be field verified.

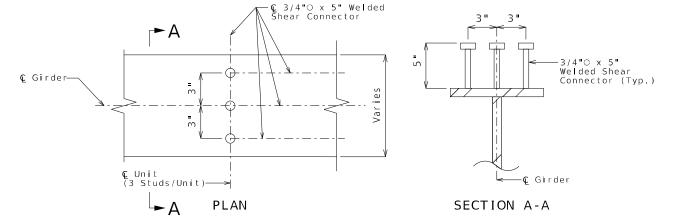
For additional steel details, see Sheets No. 16 & 17.

Fabricated structural steel shall be ASTM A709 Grade 50. Weight of all structural steel is included in the weight of Fabricated Structural Low Alloy Steel (Plate Girder) A709, Grade 50.

Plate girders shall be fabricated to be in accordance with the camber diagram shown on Sheet No. 19.

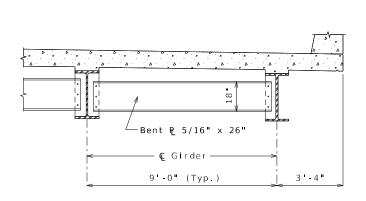
All web plates shall be subject to notch toughness requirements.

The flange and web splice plates shall be subject to notch toughness requirements when notch toughness is required for flanges on both sides

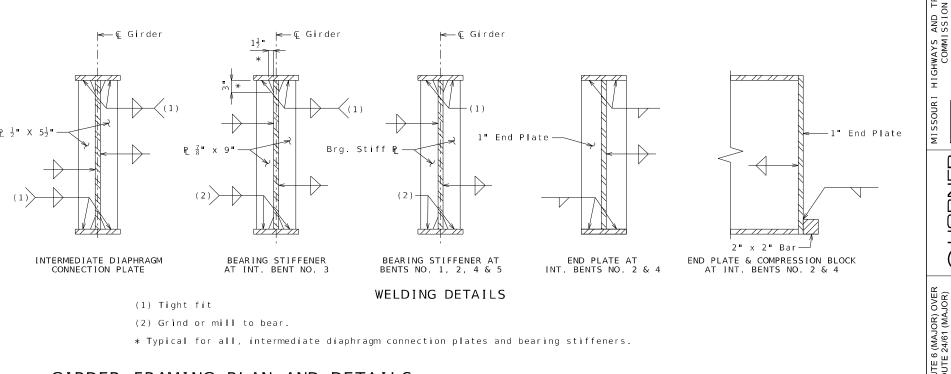


DETAILS OF SHEAR CONNECTORS

Weight of 2,860 pounds of shear connectors is included in the weight of Fabricated Structural Low Alloy Steel (Plate Girder) A709, Grade 50. Shear connectors shall be in accordance with Sec 712, 1037, and 1080.



TYPICAL PART SECTION SHOWING INTERMEDIATE DIAPHRAGMS AND CROSS FRAMES



(1) Tight fit

(2) Grind or mill to bear.

st Typical for all, intermediate diaphragm connection plates and bearing stiffeners.

GIRDER FRAMING PLAN AND DETAILS

Detailed: January 2024 Checked: January 2024 3/1/2024

MARION JNE0183 CONTRACT ID

PROJECT NO.

BRIDGE NO A21392

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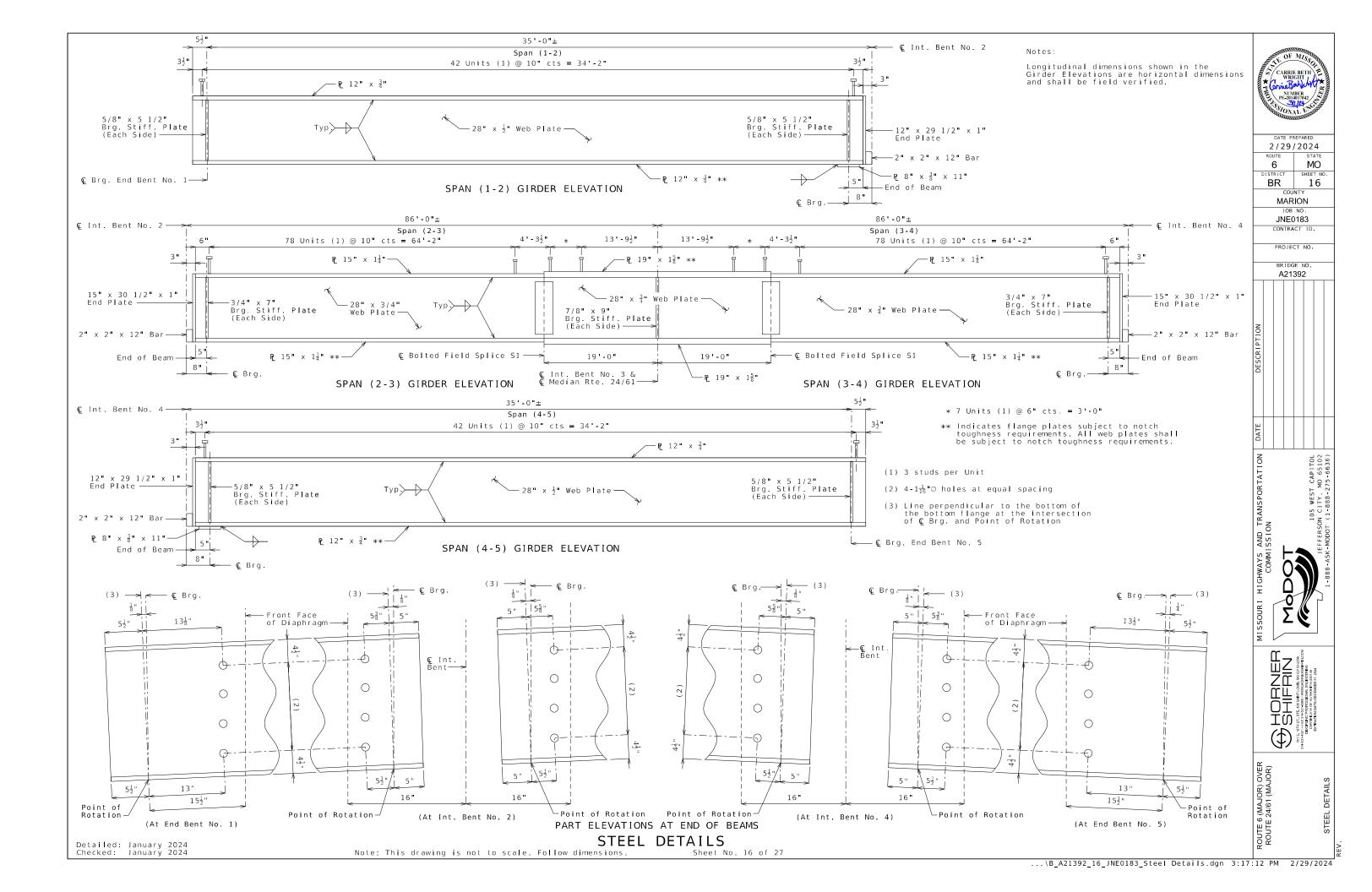
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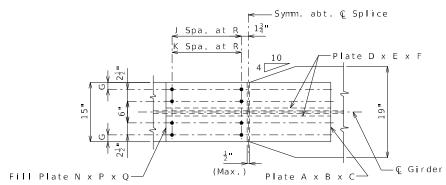
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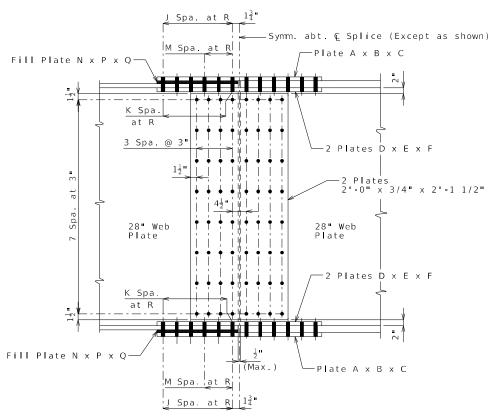
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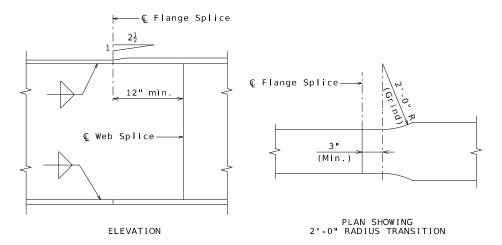
PLAN OF FLANGE SPLICE S1 SPAN (2-3) SPAN (4-3)



DETAIL OF BOLTED FIELD SPLICE

Bolts shall be 7/8-inch diameter ASTM F3125 Grade A325 Type 1 in 15/16-inch diameter holes.

Contact surfaces shall be in accordance with Sec 1081 for surface preparation.



WELDED SHOP WEB AND FLANGE SPLICE

Welded shop web and flange splices may be permitted when detailed on the shop drawings and approved by the engineer. No additional payment will be made for optional welded shop web and flange splices.

| | T. | ABL | E OF | DIM | IENS | SIONS | - F | ΙEΙ | D: | SPL | I CE | : | | |
|----------|-----|-------------|-----------|-----|-------------|--------|-----|-----|----|-----|------|-------|-----|----|
| LOCATION | А | В | С | D | Е | F | G | J | K | М | N | Р | Q | R |
| S1 | 15" | <u>3</u> II | 3 ' - 6½" | 6½" | <u>7</u> II | 3'-6½" | 2 • | 6 | 6 | - | 15" | o)(o) | 21" | 3" |
| | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | |

Note: Work this sheet with Sheet No. 15 & 16.

PLATE GIRDER SPLICE DETAILS Detailed: January 2024 Checked: January 2024

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 17 of 27

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PLATE GIRDEF SPLICE DETAIL

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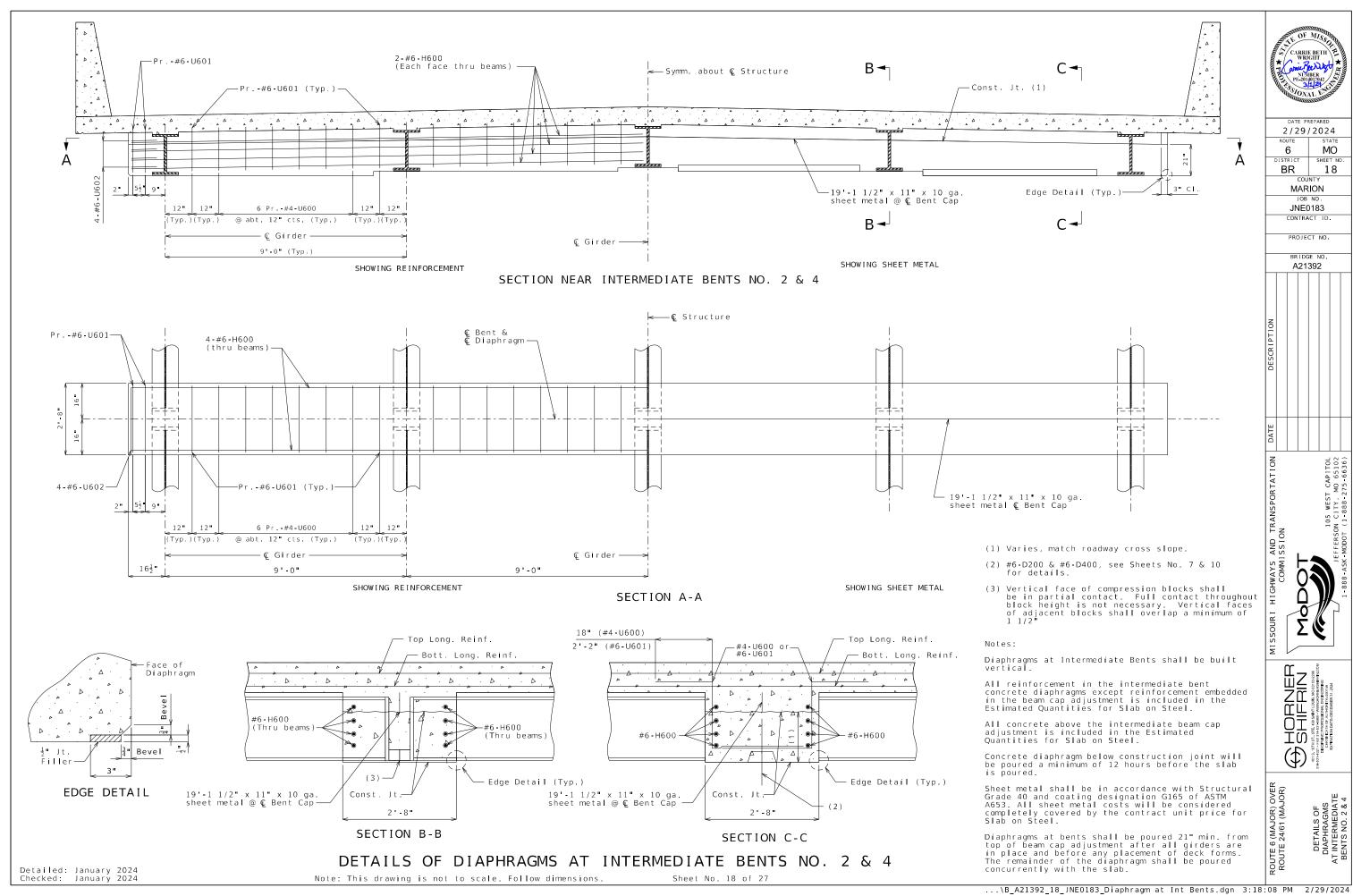
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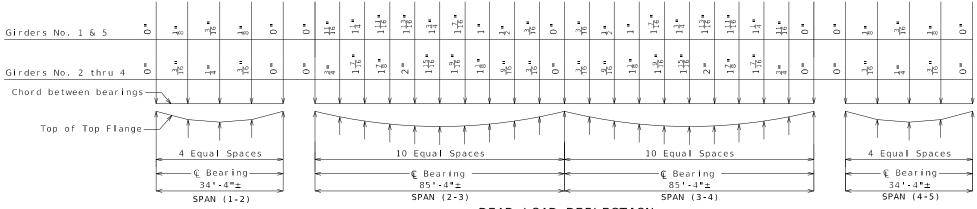
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DEAD LOAD DEFLECTION

Dead load deflection includes weight of structural steel, concrete slab (including stay-in-place forms) and barrier. 13% of dead load deflection is due to weight of structural steel in Spans (1-2) & (4-5). 20% of dead load deflection is due to weight of structural steel in Spans (2-3) & (3-4).

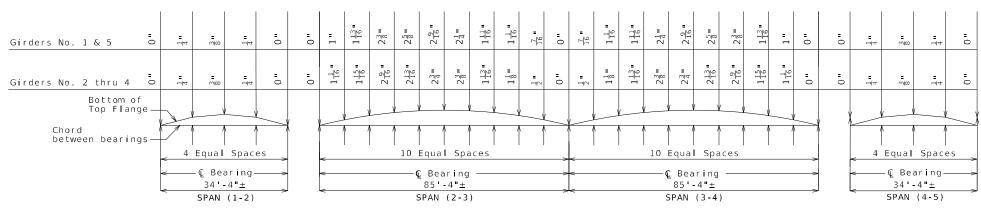
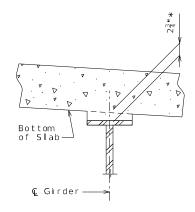


PLATE GIRDER CAMBER DIAGRAM

Camber includes allowance for vertical curve and dead load deflection due to concrete slab, stay-in-place forms, barrier and structural steel.

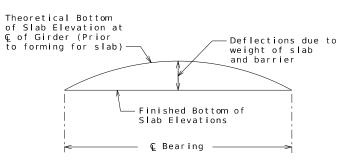


THEORETICAL SLAB HAUNCH

* Dimension (bottom of slab to top of web) may vary if girder camber after erection differs from plan camber by more than the % of Dead Load Deflection due to weight of structural steel. No payment will be made for any adjustment in forming or additional concrete required for variation in haunching.

| | Cei | | | | | | b Elev o form | | | ab) | |
|--------|----------|-----------|----------|-----------|-----------|----------|------------------|----------|----------|---------|--------|
| Girder | Span | (1-2) (34 | 1'-4"± € | Brg Q | Brg.) | | | | | | |
| Number | © Brg. | . 25 | .50 | . 75 | € Brg. | | | | | | |
| 1 & 5 | 518.09 | 518.17 | 518.24 | 518.29 | 518.34 | | | | | | |
| 2 & 4 | 518.23 | 518.31 | 518.38 | 518.43 | 518.48 | | | | | | |
| 3 | 518.35 | 518.43 | 518.51 | 518.56 | 518.60 | | | | | | |
| Girder | | | | Span (2- | 3) (85 | 4"± € Br | g - Q B | rg.) | | | |
| Number | © Brg. | .10 | .20 | .30 | .40 | .50 | .60 | .70 | .80 | .90 | € Brg. |
| 1 & 5 | 518.34 | 518.44 | 518.52 | 518.58 | 518.62 | 518.64 | 518.64 | 518.63 | 518.60 | 518.58 | 518.57 |
| 2 & 4 | 518.48 | 518.58 | 518.67 | 518.73 | 518.78 | 518.80 | 518.79 | 518.78 | 518.75 | 518.73 | 518.71 |
| 3 | 518.61 | 518.71 | 518.79 | 518.86 | 518.90 | 518.92 | 518.92 | 518.90 | 518.87 | 518.85 | 518.84 |
| Girder | | • | | Span (3- | 4) (85 | 4"± € Br | g Q. B | rg.) | • | | |
| Number | € Brg. | .10 | .20 | .30 | .40 | .50 | .60 | .70 | .80 | .90 | € Brg. |
| 1 & 5 | 518.57 | 518.57 | 518.58 | 518.60 | 518.60 | 518.59 | 518.56 | 518.51 | 518.44 | 518.35 | 518.25 |
| 2 & 4 | 518.71 | 518.72 | 518.73 | 518.75 | 518.75 | 518.75 | 518.72 | 518.67 | 518.59 | 518.50 | 518.39 |
| 3 | 518.84 | 518.84 | 518.86 | 518.87 | 518.88 | 518.87 | 518.84 | 518.79 | 518.72 | 518.62 | 518.51 |
| Girder | Span | (4-5) (34 | 1'-4"± © | Brg Q | Brg.) | | | | • | | |
| Number | € Brg. | .25 | .50 | . 75 | € Brg. | | | | | | |
| 1 & 5 | 518.24 | 518.18 | 518.12 | 518.04 | 517.95 | | | | | | |
| 2 & 4 | 518.38 | 518.33 | 518.26 | 518.18 | 518.09 | | | | | | |
| 3 | 518.50 | 518.45 | 518.39 | 518.31 | 518.21 | | | | | | |
| Elova | tions or | 0 62504 | on 2 con | stant sla | ah thickr | oss of T | 7 1/2° an | d includ | o allowa | nco for | |

Elevations are based on a constant slab thickness of 7 1/2 and include allowance for theoretical dead load deflections due to weight of slab (including stay-in-place forms) and barrier.



TYPICAL SLAB ELEVATIONS DIAGRAM

Note:

Longitudinal dimensions are horizontal from centerline bearing to centerline

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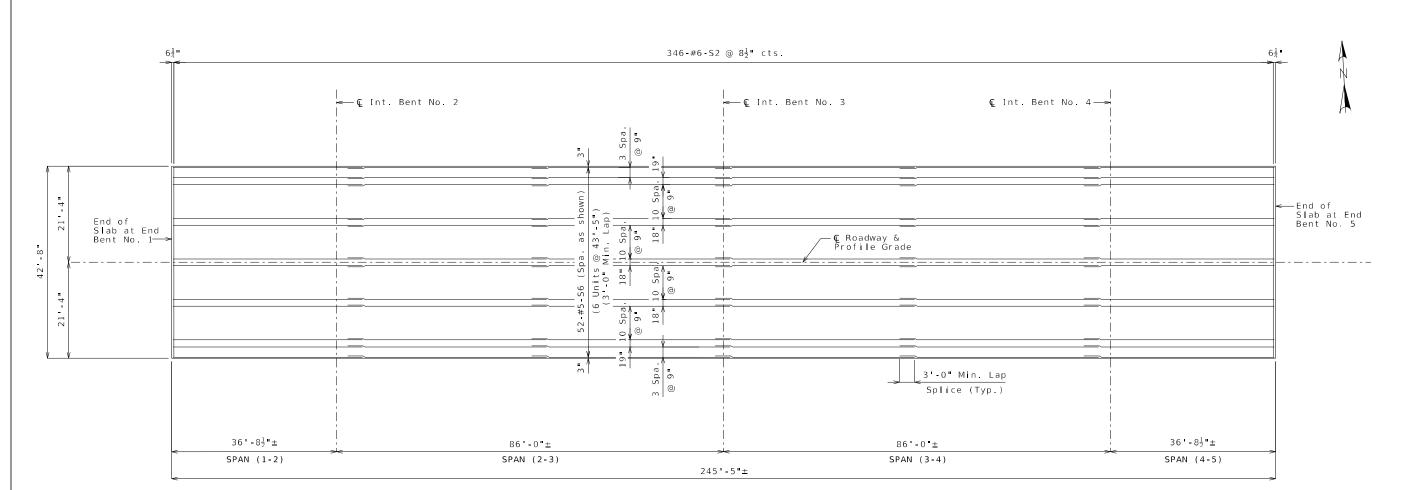
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HORNER SHIFFIN

(OUTE 6 (MAJOR) OVEF ROUTE 24/61 (MAJOR)

STEEL DETAILS



PLAN

Notes:

Work this sheet with Sheets No. 21 & 22.

Longitudinal slab dimensions shown are measured horizontally.

For Section Thru Slab and Slab Pouring Sequence, see Sheet No. 22.

For Details and Reinforcement of Type D Barrier, see Sheets No. 23 thru 24.

For Theoretical Bottom of Slab Elevations and Slab Haunching Diagram, see Sheet No. 19.

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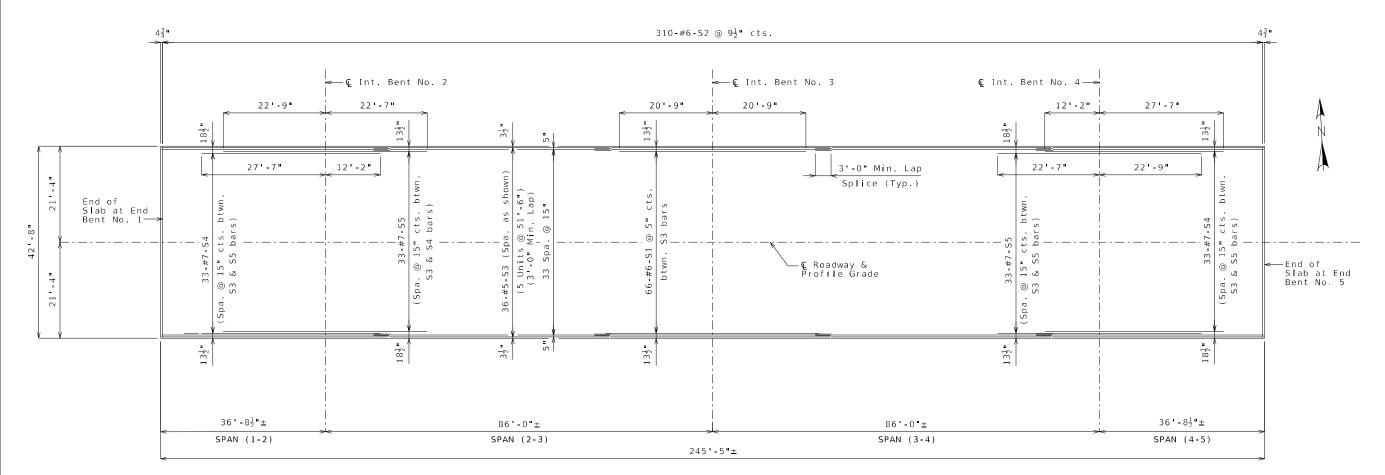
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ROUTE 6 (MAJOR) OVER ROUTE 24/61 (MAJOR)

SLAB PLAN SHOWING BOTTOM REINFORCEMENT



PLAN

Notes:

Work this sheet with Sheets No. 20 & 22.

Longitudinal slab dimensions shown are measured horizontally.

For Section Thru Slab and Slab Pouring Sequence, see Sheet No. 22.

For Details and Reinforcement of Type D Barrier, see Sheets No. 23 thru 24.

For Theoretical Bottom of Slab Elevations and Slab Haunching Diagram, see Sheet No. 19.

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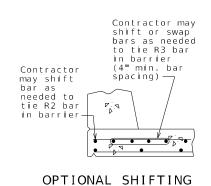


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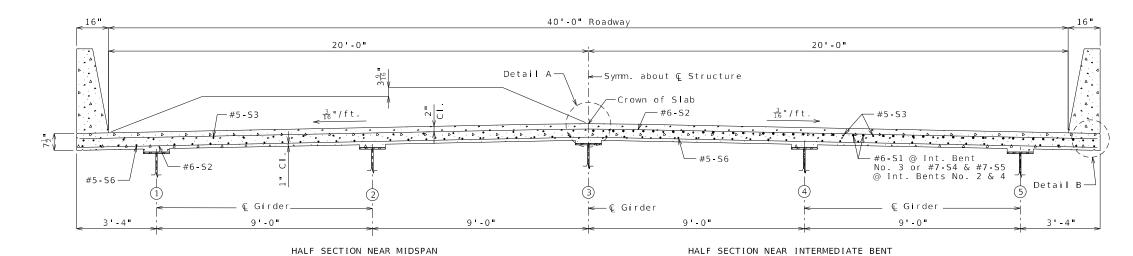
ROUTE 6 (MAJOR) OVER ROUTE 24/61 (MAJOR)

SLAB PLAN SHOWING TOP REINFORCEMENT

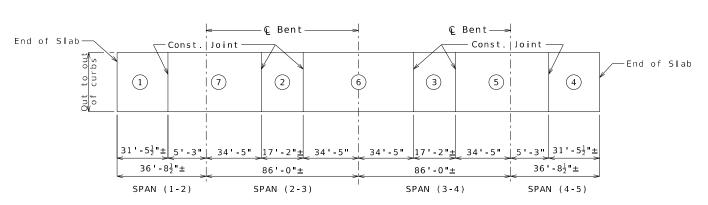
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TOP BARS AT BARRIER



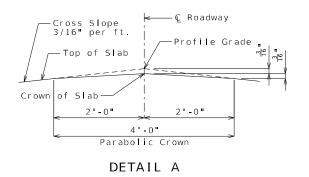
SECTION THRU SLAB

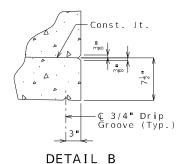


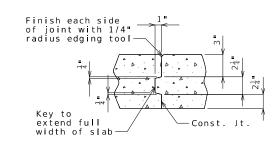
| | | 9 | Sequen | ice d | of Pou | ırs | | | | e of Pour Is./Hr. |
|----------------------|------------|--------------------------------------|--------|-------------|------------------|-------------|-------------------------------|-------------------|------------------|----------------------|
| | | | ı | Direct | ion | | | | With Retarder | No Retarder |
| Basic | 1 | 2 | 3 | 4 | 5 | | 6 | 7 | 2.5 | 2.5 |
| Sequence | | | Eit | her Di | irection | | | | 25 | 25 |
| Alternate A | 1 | 1 7 + 2 | | | | | | | | |
| | | | | | | | | + 4 | 26 | 43 |
| Pours | End t | : 0 7 | 1 to (| | 6 + 3 2 to 5 | | | + 4 o End | - 26 | 43 |
| Pours Alternate B | | t o 7 7 7 + 2 | | | 2 to 5 | | | o End | | |
| | 1 + | | | 6 | 2 to 5 | 5 | 3 t | o End 4 | 26 | 43 |
| Alternate B | 1 + End | 7 + 2 | 1 to (| 6 + | 2 to 5 3 5 | 5 | 3 t | o End 4 End | 26 | 43 |
| Alternate B Pours | 1 + End | 7 + 2 d to 6 | 1 to (| 6 + | 2 to 5 3 | 3 + | 3 t 5 + 3 3 to E | o End 4 End | | |
| Alternate B Pours | 1 + End | 7 + 2 d to 6 1 + 7 + End to | 1 to 6 | 6 + 2 to | 2 to 5 3 | 3 + 2 to | 3 t 5 + 3 to E 5 + 4 | o End 4 End | 26 | 43 |

The contractor shall pour and satisfactorily finish the slab pours at the rate given. Retarder, if used, shall be an approved type and retard the set of concrete to 2.5 hours.

SLAB POURING SEQUENCE







SLAB CONSTRUCTION JOINT

Notes:

For reinforcement of barrier not shown, see Sheet No. 23.

For Theoretical Bottom of Slab Elevations, Girder Camber Diagram and Theoretical Slab Haunching Diagram, see Sheet No. 19.

For Plan of Slab Showing Reinforcement, see Sheets No. 20 & 21.

| DATE P | REPARED | | | | |
|-----------|-----------|--|--|--|--|
| 2/29/2024 | | | | | |
| ROUTE | STATE | | | | |
| 6 | MO | | | | |
| DISTRICT | SHEET NO. | | | | |
| BR | 22 | | | | |
| COUNTY | | | | | |

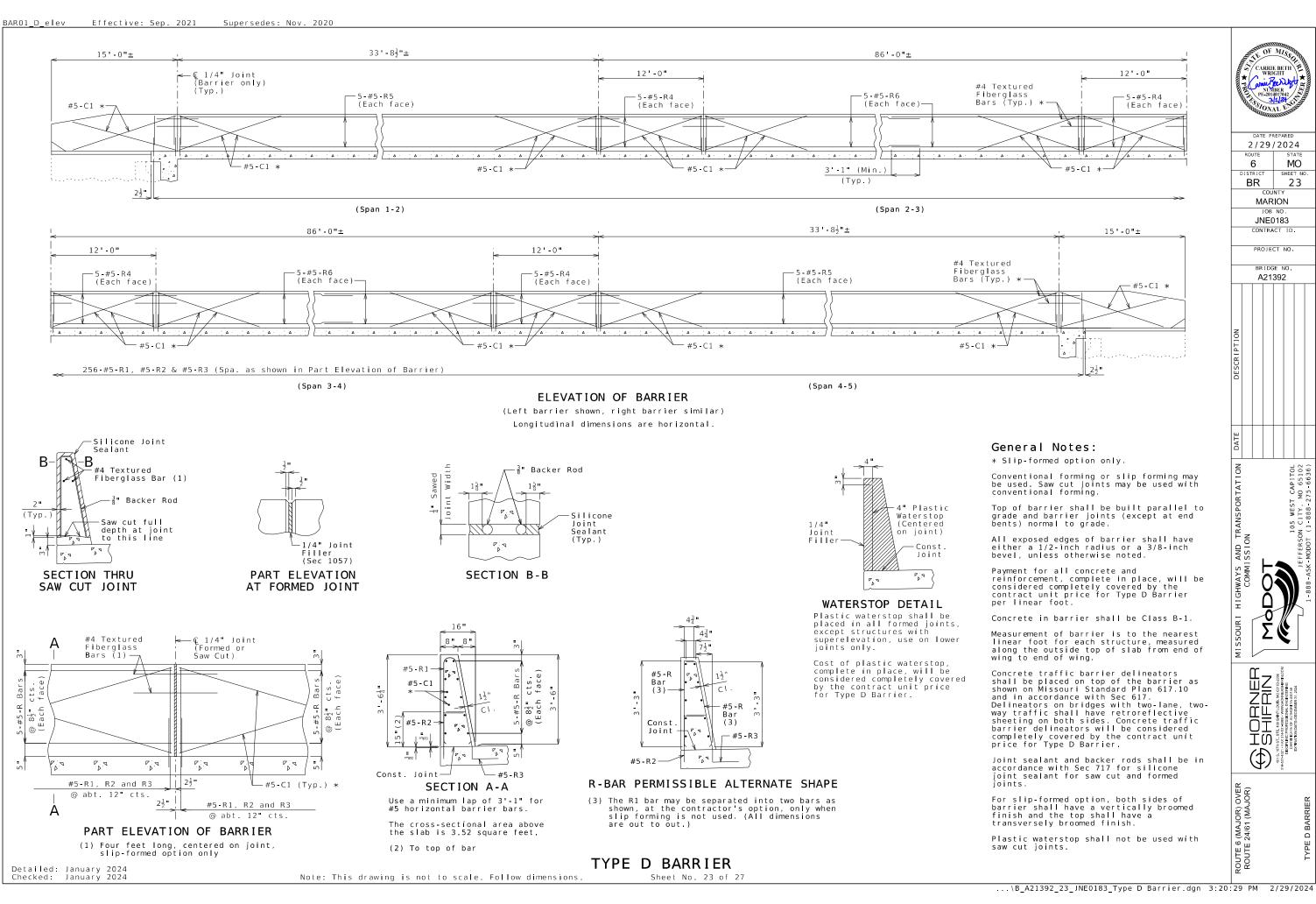
MARION JNE0183 CONTRACT ID.

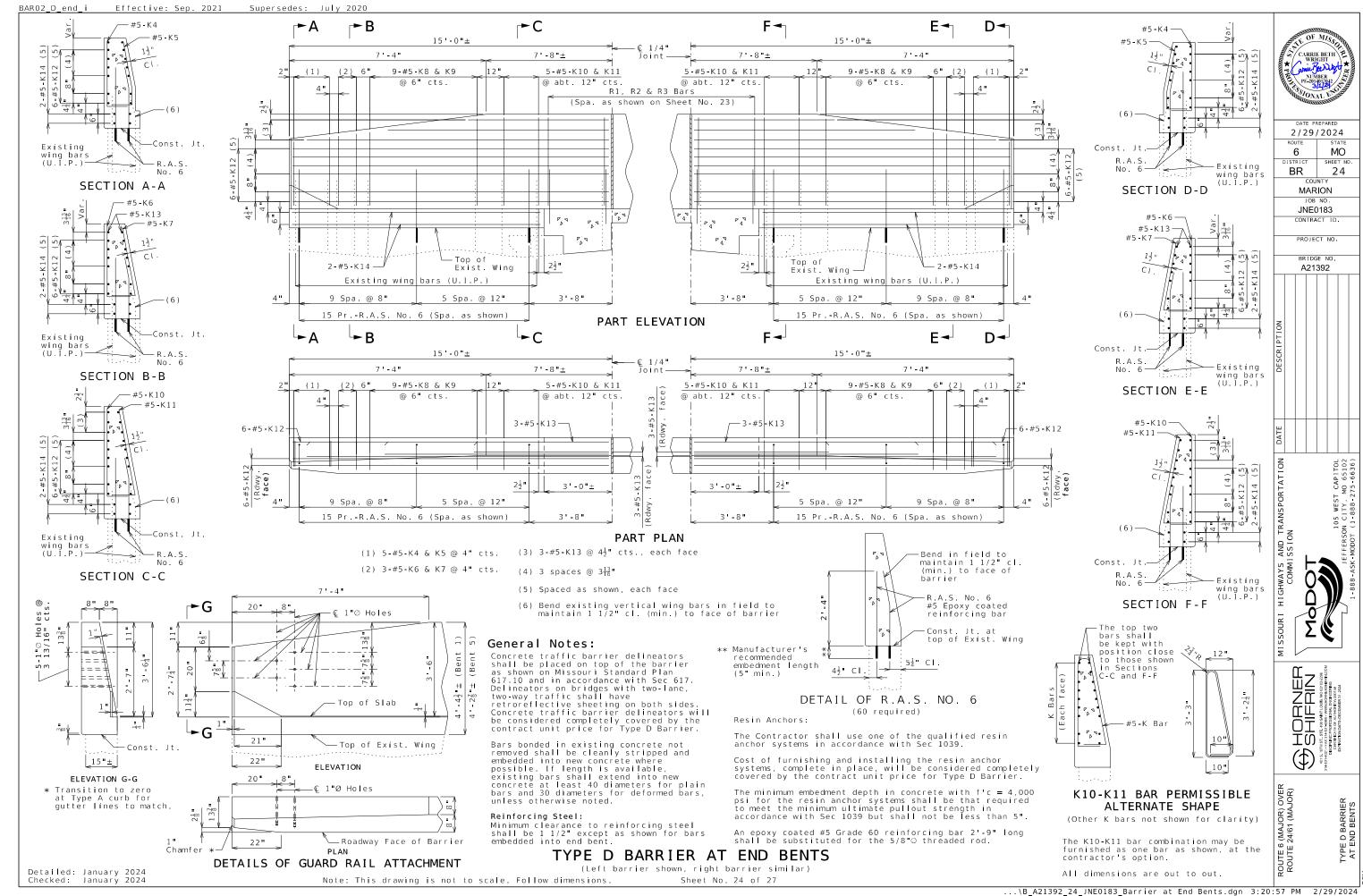
PROJECT NO.

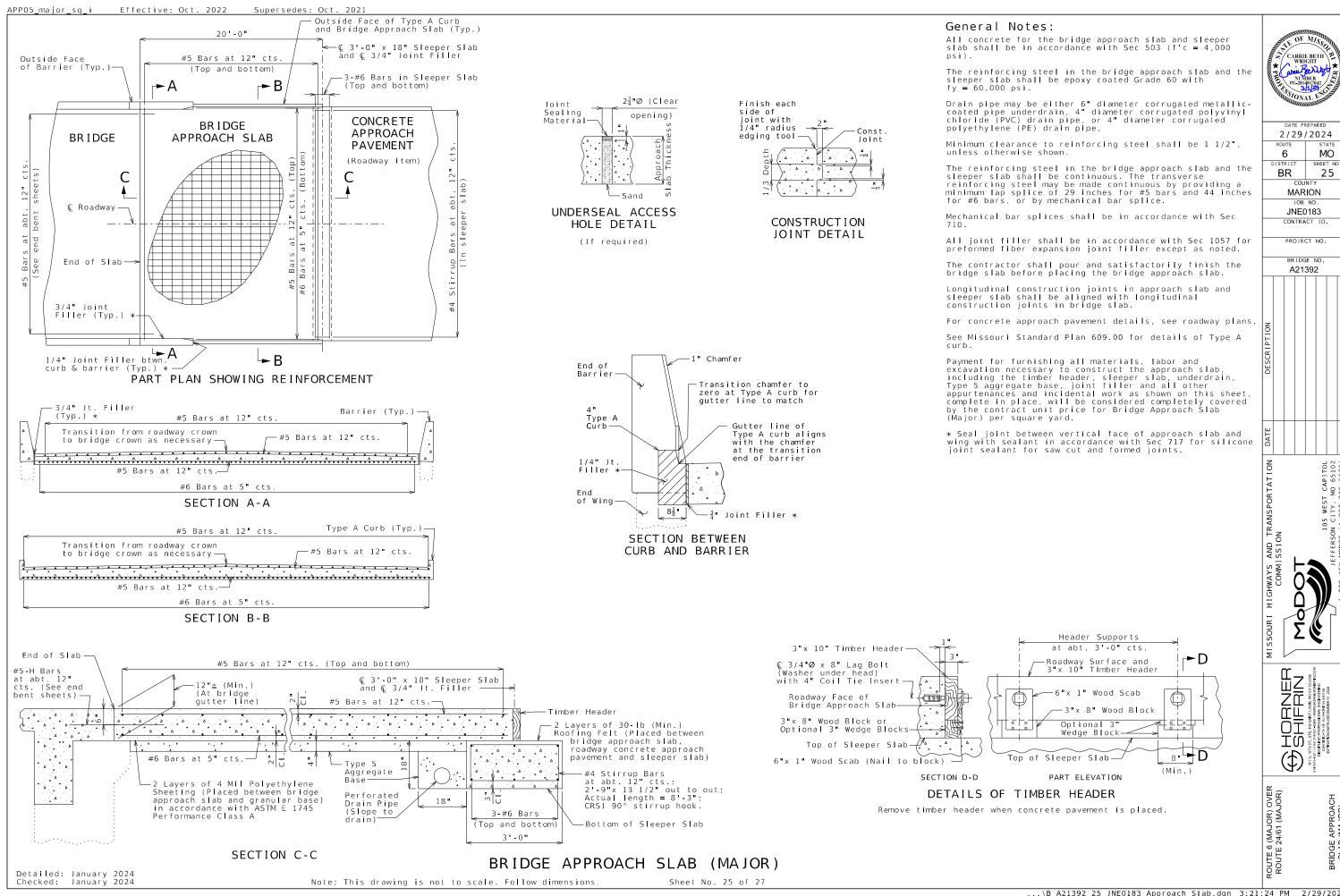
BRIDGE NO A21392

HORNER SHIFBIN

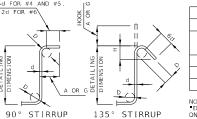
SLAB DETAILS Sheet No. 22 of 27





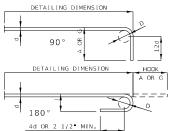


| Convert to ORD: Jan 20 |)22 ´ | | | | · | | | | | | |
|--|--------------|----------------------|-------------|--------------|--|---|--|--------------------------------------|------------|--------------------|--|
| BILL OF REINFORCING STEEL | | | | | BILL OF REINFORCING STEEL | | | | | | |
| · MARK | | | | D | DIMENSIONS Y E E | · MARK | MARK THE | | | | DIMENSIONS & E & E & E & B & TOP MISSON |
| O NO | N S X | _ AC | | | | ON ON ON | 0 | 114 | - | | = □ □ □ □ □ □ □ □ □ □ □ □ □ □ □ □ □ □ □ |
| SIZE MARK TOCATION | HAPE | B B | С | D | E F H K QV | FOCATION | HAPE IRRU BSTR | B B | С | D | E F H K 2 4 5 4 5 SHAPE 6 SHAPE 7 SHAPE 8 SHAPE 8 |
| NO. SI. | STII STII | FT. IN. | FT. IN | FT. IN. | N. FT. IN. FT. IN. FT. IN. FT. IN FT. IN FT. IN LBS. | NO. | ST | FT. IN. | FT. IN. | FT. IN. | N.FT. IN.FT. IN.FT. IN.FT. IN.FT.IN.FT.IN. LBS. |
| SUBSTRUCTURE | | | | | | END BENT NO 5 | | | | | |
| END BENT NO 1 | | | | | | 6 6 H501 DIAPHRAGM E | 20 | 42 2.000 | | | 42 2 42 2 380 T C U U U DATE PREPARED |
| | | | | | | 40 5 H502 DIAPHRAGM E | 20 | 2 6.000 | | | 2 6 2 6 104 0 |
| 6 6 H100 BM SEAT ADJ E | 20 X | 42 2.000 |) | | 42 2 42 2 380 | 8 6 H503 DIAPHRAGM E | 19 5 | 2 10.000 | 1 8.000 | | 4 6 4 4 52 F F F F F F F F F F F F F F F F F F |
| 108 5 U100 BM SEAT ADJ E | | | | 1 9.000 | | | | | | 1 8.000 | |
| 2 4 U103 SHEAR KEY E | 10 S X | | 1 9.000 | 1 6.000 | 000 5 0 4 10 6 | 54 6 U502 DIAPHRAGM E | 19 S | 0 10.000 | 5 2.000 | | 0 0 3 10 473 (COUNTY |
| INT BENT NO 2 | | | | | | | | | | | marion of the second of the se |
| 28 6 D200 SHEAR KEY E | 20 X | 2 3.000 | | | 2 3 2 3 95 | 16 6 H600 DIAPHRAGM E | 20 | 38 5.000 | | | 38 5 38 5 923 JNE0183 |
| | | | | | | 96 4 U600 DIAPHRAGM E | | | | | 00 1 6.000 6 3 6 1 390 CONTRACT ID. |
| 6 7 H200 BM SEAT ADJ E 4 6 H201 BM SEAT ADJ E | | 38 5.000 38 5.000 | | | | 48 6 U601 DIAPHRAGM E 16 6 U602 DIAPHRAGM E | | | | 2 7.000 2 5.000 | 00 Z 2.000 PROJECT NO. |
| 4 6 H202 BM SEAT ADJ E | 20 X | | | | 2 3 2 3 14 | | | | | | D T BRIDGE NO. |
| 8 6 H203 BM SEAT ADJ E 6 7 H204 BM SEAT ADJ E | - | 38 5.000 | | 2 3.000 | 000 4 7 4 3 51 38 5 38 5 471 | SLAB | | | | | A21392 |
| | | | | | | 66 6 S1 SLAB E | 20 | 41 6.000 | | | 41 6 41 6 4114 SHAPE 14 SHAPE 15 |
| 49 5 U200 BM SEAT ADJ E | 13 S X | 2 5.000 | 1 8.000 | 2 5.000 | 000 1 8.000 9 1 8 9 447 | | 20 | 42 5.000 51 6.000 | | | 42 5 42 5 41794 51 6 51 6 9669 |
| NT BENT NO 3 | | | | | | 66 7 S4 SLAB E | 20 | 39 9.000 | | | 39 9 39 9 5362 SHAPE 17 M VERTICAL Z |
| 60 6 H300 BM SEAT ADJ E | 20 X | 2 11.000 | | | 2 11 2 11 263 | | 20 | 45 4.000 43 5.000 | | | 45 4 45 4 6116 43 5 43 5 14128 |
| | | | | 2 5 000 | | | | | | | SHAPE 18 C C S |
| 50 4 U300 BM SEAT ADJ E | 10 S X | | 0 11.000 | 2 5.000 | 000 4 3 4 1 136 | BARRIER | | | | | SHAPE 16 SHAPE 20 SHAPE 19 W C SPOT WELD |
| 10W5 W301 BM SEAT ADJ E | 35 X | 0 9.000 | 0 3.000 | 0.000 | 000 16 7 16 1 27 | K1 NOT USED | | | | | AASHTO M32 SIZE W5 WIRE (TYP.) |
| NT BENT NO 4 | | | | | | K2 NOT USED K3 NOT USED | | | | | |
| 28 6 D400 SHEAR KEY E | 20 V | 2 2000 | | | 2 3 2 3 95 | 20 5 K4 BARRIER E INCREMENT = | 19 S | V 4 2 4.250 2 6.250 | | | 3 2 3 1 4 3 3 66 |
| | | | | | 2 3 2 3 95 | 0.500 INCH | | 2 0.230 | 0 10.000 | | |
| 6 7 H400 BM SEAT ADJ E 4 6 H401 BM SEAT ADJ E | - | 38 5.000 38 5.000 | | | 40 1 40 1 492 38 5 38 5 231 | 20 5 K5 BARRIER E INCREMENT = | 14 S | V 4 0 8.250 0 8.250 | | | |
| 4 6 H402 BM SEAT ADJ E | | 2 3.000 | | | 2 3 2 3 14 | | | 0 8.230 | 0 9.300 | 1 8.300 | 0 4.300 1 0.000 3 2 3 1 03 |
| 8 6 H403 BM SEAT ADJ E 6 7 H404 BM SEAT ADJ E | 10 X | 38 5.000 | | 2 3.000 | 000 4 7 4 3 51 38 5 38 5 471 | 12 5 K6 BARRIER E | 19 S | V 4 2 6.750 2 7.750 | | | 3 5 3 4 TON 3 4 TO THE TOTAL OF |
| 0 7 H404 BN SEAT ABS E | 20 1 | 30 3.000 | | | 36 3 36 3 471 | 0.500 INCH | | | | | ORT 125 |
| 49 5 U400 BM SEAT ADJ E | 13 S X | 2 5.000 | 7.000 | 2 5.000 | 000 1 7.000 8 11 8 7 439 | 12 5 K7 BARRIER E INCREMENT = | 21 5 | V 4 2 6.625 2 7.625 | | | 2 6.000 0 6.250 3 5 3 3 2 7.000 0 6.500 3 6 3 4 41 |
| END BENT NO 5 | | | | | | 0.500 INCH | | | | | Z 7.000 0 0.500 3 0 3 7 71 ml VERTICAL REG 1 80 1 80 1 80 1 80 1 80 1 80 1 80 1 8 |
| 6 6 H500 BM SEAT ADJ E | 20 X | 42 2.000 | | | 42 2 42 2 380 | 36 5 K8 BARRIER E | 19 S | V 4 2 8.500 3 2.500 | | | 3 7 3 5 4 1 3 11 139 x c x x 1 C QNV Y 1000W |
| | | | | | | 0.750 INCH | | | | | |
| 108 5 U500 BM SEAT ADJ E | 10 S X | | | 9.000 | | 36 5 K9 BARRIER E | 21 S | V 4 2 8.500 | 0 10.000 | | 2 7.750 0 6.750 3 6 3 5 3 1.750 0 7.750 4 1 3 11 136 |
| | | | | | | 0.750 INCH | | | | | |
| SUPERSTRUCTUR | | | | | + | 20 5 K10 BARRIER E 20 5 K11 BARRIER E | 19 S | 3 3.000 3 3.000 | | | 4 1 4 0 83 |
| END BENT NO 1 | | | | | | 48 5 K12 BARRIER E | 20 | 14 9.000 | | | 14 9 14 9 738 |
| 6 6 H101 DIAPHRAGM E | 20 | 42 2.000 | | | 42 2 42 2 380 | 24 5 K13 BARRIER E INCREMENT = | 20 | V 8 8 0.000 14 0.000 | | | 14 9 14 9 738 8 0 8 0 14 0 14 0 275 SHAPE 26 SHAPE 27 |
| 40 5 H102 DIAPHRAGM E | 20 | 2 6.000 | | | 2 6 2 6 104 | 3.0 FT 0.00 INCH | 20 | 11 0000 | | | SHAPE 26 SHAPE 27 E |
| 8 6 H103 DIAPHRAGM E | 19 2 | 2 10.000 | 1 8.000 | | 4 6 4 4 52 | 16 5 K14 BARRIER E | 20 | 11 9.000 | | | 11 9 11 9 196 |
| 88 5 U101 DIAPHRAGM E 54 6 U102 DIAPHRAGM E | 10 S | 0 10.000 | | 1 8.000 | | 512 5 R1 BARRIER E 512 5 R2 BARRIER E | 26 | 3 3.000 1 7.500 | | | 3 3.000 0 6.750 6 10 6 9 3605 2 5 2 4 1246 50 0 4.000 1 0.000 1 3.000 0 3.000 3 5 3 3 1736 |
| 54 6 0102 DIAFHRAGM E | 19 3 | 0 10.000 | 3 2.000 | 1 | 6 0 5 10 473 | 512 5 R3 BARRIER E | 27 S | 1 7.300 | | | 2 5 2 4 1246 50 0 4,000 1 0,000 1 3,000 0 3,000 3 5 3 3 1736 SHAPE 28 SHAPE 29 |
| | | | | | | 80 5 R4 BARRIER E | 20 | 11 9.000 | | | SHAPE 28 SHAPE 29 SHA |
| 6d FOR #4 AND #5, | | ST | IRRUP HO | OK DIMENS | NSIONS DETAILING DIMENSION END HOO | NOTE: OK DIMENSIONS ALL STAND | DARD HO | OOKS AND BENDS O | THER THAN | 180_DEGREE | EE ARE TO BE |
| | — n 🔻 | | | - 50 - 60 KS | KSI D BAR D | 180° HOOKS 90° HOOKS HOOKS AND | D BENDS | PROCEDURE AS FO S SHALL BE IN AC | | | |
| | | BAR SIZE | 1 D - | HOOK HOOK | | A OR G J A OR G SHOWN ON E = EPOX | Y COATE | SHEET. ED REINFORCEMENT | | | PROCEDURES AS F T SHAPE 31 SHAPE 32 |
| STON B STON | + | \$ #4 | 2 4 | 1/2 4 1/2 | 1/2 3 #4 3 #4 3 #4 3 #5 2 2/4# | 6 4 8 X = BAR | RUP. IS INCL | LUDED IN SUBSTRU | CTURE OUAN | ITITIES. | SHAPE 30 |
| #4 2' 4 1/2' 4 1/2' 3 3/4 7' 5' 10' #5 3 3/4' 7' 5' 10' #6 4 1/2' 8' 6' 12' 8' 6' 12' 8' 6' 12' 8' 6' 12' 8' 6' 12' 8' 6' 12' 8' 6' 11' 8' 11 | | | | | | | | | | | |
| A OR G O D NOME: UNLESS OTHERWISE NOTED, DIAMETER NOTE: UNLESS OTHERWISE NOTED, DIAMETER B NOMINAL LENGTHS ARE BASED ON OUT TO OUT DIMENSIONS SHOWN IN B NOMINAL LENGTHS ARE BASED ON OU | | | | | | | | | | | |
| 90° STIRRUP 135° | STIRRU | *D** 15 | THE SAME FO | OR ALL BENDS | MDS AND HOOKS #9 9 1/2" #10 10 3/4" | 15 11 3/4 19 ACTUAL LI | ENGTHS | ARE MEASURED AL | ONG CENTER | | TO THE NEAREST INCH. |
| | | | | | #11 12 - | 2 3 21 3/4 2 7 FOUR ANG | LE OR C | BASED ON ACTUAL CHANNEL SPACERS | ARE REQUIR | ED FOR EAC | ACH COLUMN SHAPE 34 SHAPE 34 SHAPE 34 |
| | | | | | | AND WEIGH | HT OF C | S ARE TO BE PLAC COLUMN SPIRALS D | O NOT INCL | .UDE SPLICE: | CES OR SPACERS. |
| Detailed: January 20 Checked: January 20 | | | | | Note: This drawing is not to scale. Follow dir | | | EEL (GRADE 60) F No. 26 of 27 | | PSI. | SHAPE 33 SPIRAL BAR OR WIRE.) SHAPE 35 BENDING DIAGRAMS |
| enceked. January 20 | | | | | | | cet | 20 01 27 | | | \B A21392 26 JNE0183 Barbill.dgn 3:21:53 PM 2/29/2024 |



| STI | RRUP I | HOOK D | IMENSI | ONS | | | |
|-------------------------|--------|----------------|----------------|---------|--|--|--|
| GRADES 40 - 50 - 60 KSI | | | | | | | |
| BAR | D | 90° HOOK | 135° | HOOK | | | |
| SIZE | (IN.) | HOOK A OR G | HOOK A OR G | APPROX. | | | |
| #4 | 2* | 4 1/2 | 4 1/2 | 3 • | | | |
| #5 | 2 1/2 | 6 - | 5 1/2 | 3 3/4 | | | |
| #6 | 4 1/2 | 12 - | 8 - | 4 1/2 | | | |

NOTE: UNLESS OTHERWISE NOTED, DIAMETER "D" IS THE SAME FOR ALL BENDS AND HOOKS ON A BAR.

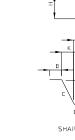


| | END HOOK DIFFERDIGHT | | | | | | | | | | |
|----|----------------------|---------|------------|-----------|--------|--|--|--|--|--|--|
| | | | ALL GRADES | | | | | | | | |
| | BAR SIZE | D (IN.) | 180° | 90° HOOKS | | | | | | | |
| _ | | (114.) | A OR G | J | A OR G | | | | | | |
| ł | #3 | 2 1/4 | 5 | 3* | 6 • | | | | | | |
| Ţ | #4 | 3* | 6 • | 4 • | 8" | | | | | | |
| J | #5 | 3 3/4 | 7 - | 5 • | 10 | | | | | | |
| - | #6 | 4 1/2 | 8 - | 6 - | 12 - | | | | | | |
| G | #7 | 5 1/4 | 10 | 7 • | 14 | | | | | | |
| _! | #8 | 6 • | 11 | 8" | 16 | | | | | | |
| =1 | #9 | 9 1/2 | 15 | 11 3/4 | 19" | | | | | | |
| | #10 | 10 3/4 | 17 | 13 1/4" | 22" | | | | | | |
| | #11 | 12 | 19" | 14 3/4 | 2 0 | | | | | | |
| | #14 | 18 1/4 | 2 3 | 21 3/4 | 2 - 7" | | | | | | |

NOMINAL LENGTHS ARE BASED ON OUT TO OUT DIMENSIONS SHOWN IN BENDING DIAGRAMS AND ARE LISTED FOR FABRICATORS USE. (NEAREST INCH) ACTUAL LENGTHS ARE MEASURED ALONG CENTERLINE BAR TO THE NEAREST INCH. PAYWEIGHTS ARE BASED ON ACTUAL LENGTHS.

FOUR ANGLE OR CHANNEL SPACERS ARE REQUIRED FOR EACH COLUMN SPIRAL. SPACERS ARE TO BE PLACED ON INSIDE OF SPIRALS. LENGTH AND WEIGHT OF COLUMN SPIRALS DO NOT INCLUDE SPLICES OR SPACERS. REINFORCING STEEL (GRADE 60) FY = 60,000 PSI.

Sheet No. 27 of 27



SHAPE 34 DEFORMED OR PLAIN SHAPE 33 SPIRAL BAR OR WIRE.) SHAPE 35 BENDING DIAGRAMS

Detailed: January 2024

Note: This drawing is not to scale. Follow dimensions.