#### DESIGN DESIGNATION

A.A.D.T. - 2022 = 1,530A.A.D.T. - 2045 = 2,120D.H.V. = 11% V = 55 M.P.H. D = 50.8%/49.2%

FUNCTIONAL CLASSIFICATION- MINOR ARTERIAL

#### NORMAL RIGHT OF WAY

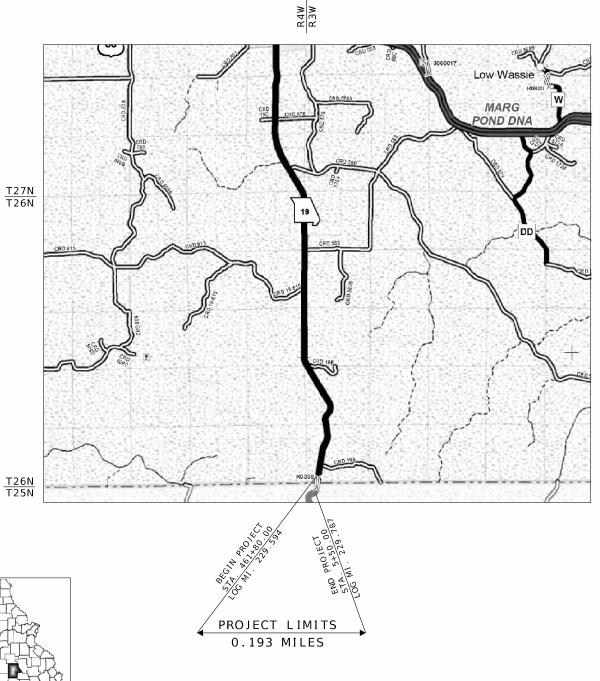
## CONVENTIONAL SYMBOLS

EXISTING NE	•
BUILDINGS AND STRUCTURES  GUARD RAIL  GUARD CABLE  CONCRETE RIGHT-OF-WAY MARKER  STEEL RIGHT-OF-WAY MARKER  LOCATION SURVEY MARKER	
UTILITIES	<del>√</del>
MANHOLE HYD	
FIRE HYDRANT	
WATER VALVE	
WATER METER "™⊕	
DROP INLET	
DITCH BLOCK	
GROUND MOUNTED SIGN	
LIGHT POLE	
H-FRAME POWER POLE	
TELEPHONE PEDESTAL APPED	
CHAIN LINK — V — V WOVEN WIRE — X — GATE POST   □ V — V — V — V — V — V — V — V — V — V	
BENCHMARK &	

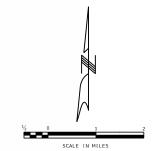
NOTE: DASHED OR OPEN SYMBOLS INDICATE EXISTING FEATURES

## MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

## PLANS FOR PROPOSED STATE HIGHWAY SHANNON COUNTY



THE EXISTENCE AND APPROXIMATE LOCATION OF UTILITY FACILITIES KNOWN TO EXIST, AS SHOWN ON THE PLANS, ARE BASED ON THE BEST INFORMATION AVAILABLE TO THE COMMISSION AT THIS TIME. THIS INFORMATION IS PROVIDED BY THE COMMISSION "AS-IS" AND THE COMMISSION EXPRESSLY DISCLAIMS ANY REPRESENTATION OR WARRANTY AS TO THE COMPLETENESS, ACCURACY, OR SUITABILITY OF THE INFORMATION FOR ANY USE. RELIANCE UPON THIS INFORMATION IS DONE AT THE RISK AND PERIL OF THE USER, AND THE COMMISSION SHALL NOT BE LIABLE FOR ANY DAMAGES THAT MAY ARISE FROM ANY ERROR IN THE INFORMATION. IT IS, THEREFORE, THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY THE EXISTENCE, LOCATION AND STATUS OF ANY FACILITY. SUCH VERIFICATION INCLUDES DIRECT CONTACT WITH THE LISTED UTILITIES.



## INDEX OF SHEETS

DESCRIPTION	SHEET NUMBER
TITLE SHEET	1
TYPICAL SECTIONS (TS) (1 SHEET)	2
QUANTITIES (QU) (3 SHEETS)	3
PLAN-PROFILE (PP)	4 - 7
COORDINATE POINTS (CP)	8
TRAFFIC CONTROL SHEET (TC)	9
EROSION CONTROL SHEETS (EC)	10-12
SIGNING/PAVEMENT MARKING (SN/PM)	13-16
CULVERT SECTIONS (CS)	17
BRIDGE DRAWINGS (B)	
A9309	1 - 35
CROSS SECTIONS (XS)	1-29

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MICHELE R. KEAL NUMBER

#### LENGTH OF PROJECT

BEGINNING OF PROJECT STA. 461 + 80.00 END OF PROJECT STA. 5 + 50.00 -45,630 FEET APPARENT LENGTH

EQUATIONS AND EXCEPTIONS:

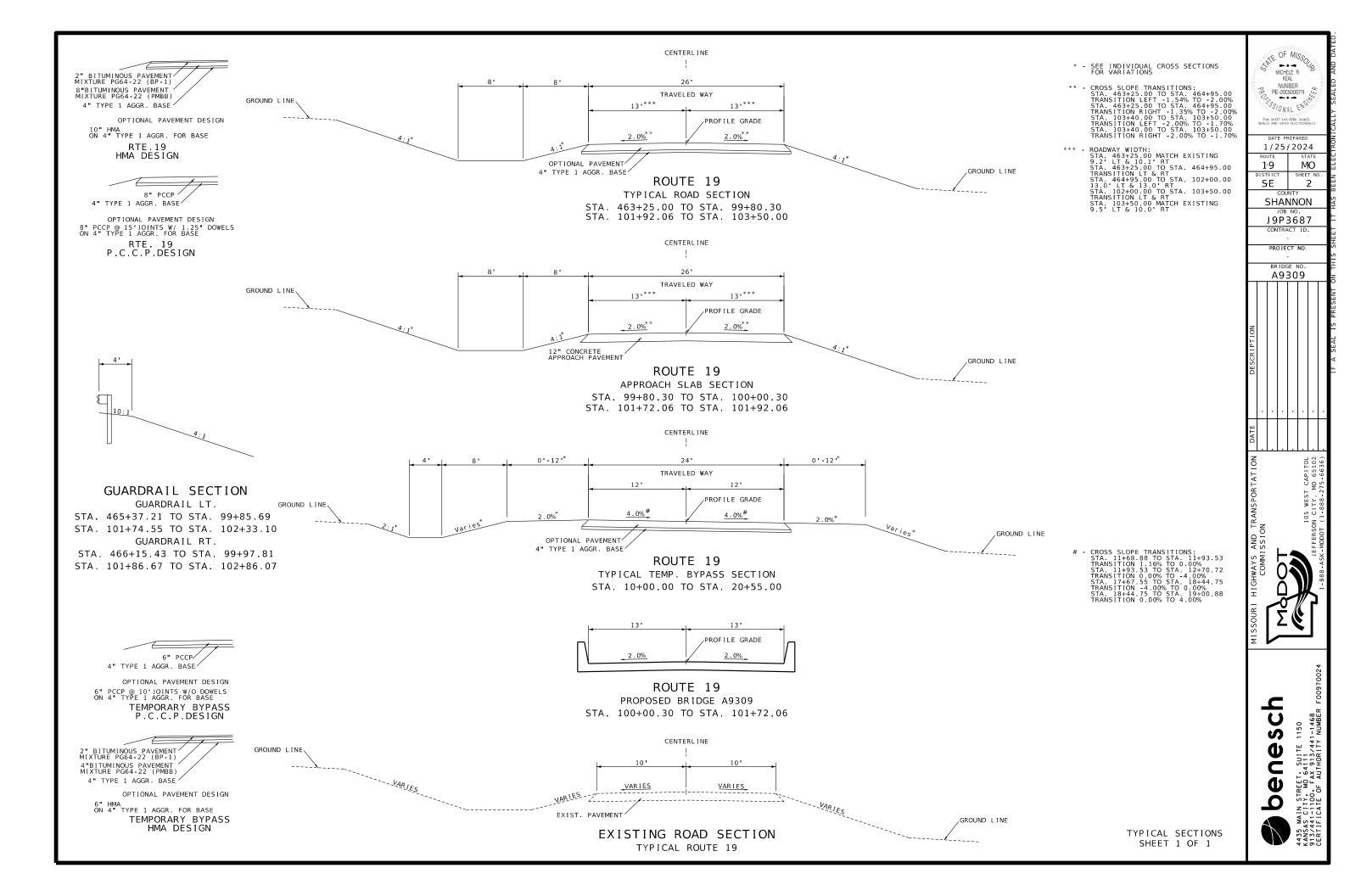
STA. 466+20.90 BK. = STA. 99+70.00 STA. 105+50.00 BK. = STA. 5+50.00



<del>C</del> e

TOTAL CORRECTIONS	0.00	FEET
NET LENGTH OF PROJECT	1020.90	FEET
STATE LENGTH	0.193	MILES
FOR INFORMATION ONLY		

ESTIMATED DISTURBED ACRES 2.60 ACRES



			REMOVAL OF IM	PROVEMEN	ITS		
SHEET	STATION	STATION	LOCATION	SIDE	DESCRIPTION	QUANTITY	UNITS
4	463+25.00	103+50.00	RTE 19	СТ	PAVEMENT	1834.9	SY
4	462+98.38		RTE 19	LT	SIGN	1	EA
4	464+16.94		RTE 19	RT	SIGN	1	EA
4	465+03.84		RTE 19	LT	SIGN	1	EA
4	466+05.57		RTE 19	LT	SIGN	1	EA
4	466+09.29		RTE 19	СТ	CMP	42	LF
4	466+05.00	100+34.11	RTE 19	LT	PAVEMENT	215.5	SY
4	466+26.24		RTE 19	LT	SIGN	1	EA
4	466+29.80		RTE 19	RT	SIGN	1	EA
4	466+46.71		RTE 19	RT	SIGN	1	EA
4	466+48.16		RTE 19	LT	SIGN	1	EA
4	466+62.03		RTE 19	RT	SIGN	1	EA
4	101+60.01		RTE 19	LT	SIGN	1	EA
4	101+71.12		RTE 19	RT	SIGN	1	EA
4	101+81.18		RTE 19	LT	SIGN	1	EA
4	101+91.30		RTE 19	RT	SIGN	1	EA
4	102+00.93		RTE 19	LT	SIGN	1	EA
4	102+12.36		RTE 19	RT	SIGN	1	EA
4	101+59.91	102+49.79	RTE 19	RT	PAVEMENT	147.3	SY
4	103+50.33		RTE 19	LT	SIGN	1	EA
5	10+00.00	20+55.00	TEMPORARY BYPASS	CT	PAVEMENT	2708	SY
			TOTAL			1	LS

	SEEDING AND MULCHING												
	BEGIN	END			COOL SEASON								
SHEET	STATION	STATION	LOCATION	SIDE	MIXTURES	MULCHING	REMARKS						
					(AC)	(AC)							
10	461+38.21	100+03.95	RTE 19	LT.	0.3	0.3	PHASE 1						
10	101+10.32	105+23.73	RTE 19	LT.	0.3	0.3	PHASE 1						
11	463+25.00	102+18.00	RTE 19	LT.	0.2	0.2	PHASE 2						
11	463+25.00	103+50.00	RTE 19	RT.	0.2	0.2	PHASE 2						
12	461+38.21	466+48.65	RTE 19	LT.	0.4	0.4	PHASE 3						
12	101+22.86	105+23.68	RTE 19	LT.	0.1	0.1	PHASE 3						
			TOTAL		1.5	1.5							

			EARTHWOR	RK			
BEGIN	END		CLASS A	COMPACTING	EMBANKMENT	EXCESS	
STATION	STATION	LOCATION	EXCAVATION	EMBANKMENT	IN PLACE	CLASS A EXC.	REMARKS
			(CY)	(CY)	(CY)	(CY)	
10+00.00	20+55.00	TEMPORARY BYPASS	237	189	6,643	0	PHASE 1
461+38.21	105+23.72	RTE 19	561	449	1,514	0	PHASE 2
461+38.21	105+23.72	TEMPORARY BYPASS (REMOVAL)	6,208	224	0	5,928	PHASE 3
		TOTAL	7,005	862	8,157		

	PERMANENT EROSION CONTROL											
SHEET	BEGIN STATION	END STATION	LOCATION	FURNISHING TYPE 2 ROCK BLANKET (CY)	PLACING TYPE 2 ROCK BLANKET (CY)	GEOTEXTILE FABRIC (SY)	PLACING ROCK DITCH LINING TYPE 3 (CY)	FURNISHING ROCK DITCH LINING TYPE 3 (CY)				
4	466+25.06	100+31.33	RTE 19	214.9	214.9	322.0						
4	101+36.11	101+98.21	RTE 19	294.3	294.3	442.0						
11	101+98.4	102+10.21	RTE 19				9.0	9.0				
	·	·	TOTAL	509.2	509.2	764.0	9.0	9.0				
			PAY TOTAL	509	509	764	9	9				

			D	RAINAGE S	TRUCTL	IRES				
				CULVERT	GRO	UP B	GROUP C	24" GROUP B		
				SECTION	PI	PE	PIPE	FLARED	ROCK	CLASS 3
SHEET	STATION	LOCATION	SIDE	SHEET	18"	24"	96"	END SECTIONS	LINING	EXC.
				NO.	(FT)	(FT)	(LF)	(EACH)	(CY)	(CY)
5	14+54.28	TEMPORARY BYPASS	CT	19	60					3.9
5	15+33.83	TEMPORARY BYPASS	CT				68			61.5
5	15+45.54	TEMPORARY BYPASS	CT				68			29.0
5	15+57.25	TEMPORARY BYPASS	CT				68			11.8
5	15+68.95	TEMPORARY BYPASS	CT				68			10.5
5	15+80.66	TEMPORARY BYPASS	CT				68			34.4
5	15+92.37	TEMPORARY BYPASS	CT				68			71.5
5	16+04.07	TEMPORARY BYPASS	СТ				68			84.9
6	100+58.48	CR 19-615	CT	19		50		2	2	6.0
				TOTAL	60	50	476	2	2	313.6
				PAY TOTAL	60	50	476	2	2	314.0

	PERMANENT PAVEMENT MARKING										
	BEGIN	END			WATERBORNE PAVEMENT MARKING PAINT TYPE P BEADS 4" SOLID 4" SOLID						
SHEET	STATION	STATION	LOCATION	SIDE	4" SOLID WHITE (LF)	4" SOLID YELLOW (LF)	REMARKS				
13	456+37.00	463+25.00	RTE 19	LT/RT		1,376.00	DOUBLE CENTER LINE				
13	461+39.00	463+25.00	RTE 19	LT	186.0		EDGE LINE				
13	461+39.00	463+25.00	RTE 19	RT	186.0		EDGE LINE				
13	463+25.00	466+40.00	RTE 19	LT/RT		630.0	DOUBLE CENTER LINE				
13	463+25.00	464+94.96	RTE 19	LT	170.0		EDGE LINE				
13	463+25.00	464+94.96	RTE 19	RT	170.0		EDGE LINE				
13	464+94.96	101+87.98	RTE 19	LT	341.9		EDGE LINE				
13	464+94.96	101+98.18	RTE 19	RT	354.0		EDGE LINE				
13	101+87.98	103+50.00	RTE 19	LT	164.0		EDGE LINE				
13	101+98.18	103+50.00	RTE 19	RT	151.9		EDGE LINE				
13	466+90.00	103+50.00	RTE 19	RT		360.0	SOLID CENTER LINE				
13	466+90.00	103+50.00	RTE 19	LT		90.0	INTERMITTENT CENTER LINE				
13	103+50.00	110+15.00	RTE 19	RT		665.0	SOLID CENTER LINE				
13	103+50.00	110+15.00	RTE 19	LT		665.0	INTERMITTENT CENTER LINE				
13	103+50.00	105+24.00	RTE 19	RT	174.0		EDGE LINE				
13	103+50,00	105+24.00	RTE 19	LT	174,0		EDGE LINE				
			TOTAL		2,072	3,786					

	TEMPORARY PAVEMENT MARKING											
SHEET	LOCATION	SIDE	TEMPORARY PAVEMENT MARKING PAINT 4 IN.		REMARKS							
			WHITE (LF)	YELLOW (LF)								
9	TEMPORARY BYPASS	LT	1,055.40		EDGE LINE							
9	TEMPORARY BYPASS	RT	1,053.10		EDGE LINE							
9	TEMPORARY BYPASS	CT		2,107.40	DOUBLE YELLOW CENTERLINE							
TOTAL			4,2	216								

	POROUS BACKFILL												
SHEET	STATION	STATION	LOCATION	SIDE	POROUS BACKFILL	REMARKS							
					(CY)								
4	95+00.30	100+00.30	RTE. 19	CL	31.4	ASSUMED 5' x 28.7' x 5.9'							
4	101+72.06	101+77.06	RTE. 19	CL	31.4	ASSUMED 5' x 28.7' x 5.9'							
			s	UBTOTAL	62.8								
			63										

MOHELE R
KEAL
NIMBER
PE-2005000711 DATE PREPARED

1/25/2024

ROUTE

1:9

MO

DISTRICT SHEET NO.

S:E

3 STE J S COUNTY SHANNON JOB NO. J 9 P 3 6 8 7 CONTRACT ID. PROJECT NO. BRIDGE NO.



	OPTIONAL PAVEMENT												
				MAIN	ILINE	TEMPORA	RY BYPASS						
				OPTIONAL A	OPTIONAL B	OPTIONAL A	OPTIONAL B						
					8" PCCP								
SHEET	BEGIN	END	LOCATION		15' JOINTS		6" PCCP	GRAVEL (A)	TYPE 1				
	STATION	STATION		10"	1.25" DIA.	6"	10' JOINTS	CRUSHED	AGGREGATE	REMARKS			
				HMA	DOWELS	HMA	W/O DOWELS	STONE (B)*	BASE (4")				
				(SY)	(SY)	(SY)	(SY)	(TON)	(SY)				
4	463+25.00	466+31.20	RTE 19	888.0	888.0				888.0				
4	101+92.06	103+50.00	RTE 19	596.0	596.0				596.0				
5	10+00.00	20+55.00	TEMPORARY BYPASS			2,707.5	2,707.5		2,707.5				
5	16+38.24	16+74.96	TEMPORARY BYPASS					1.4		ENTRANCE			
6	0+13.00	1+40.00	CR 19-615					8.6					
7	102+17.96	103+26.60	RTE 19					5.7		ENTRANCE			
			TOTAL	1484.0	1484.0	2,707.5	2,707.5	15.7	4,191.5				
			PAY TOTAL	148	34.0	27	07.5	16	4,192				

<sup>\*</sup> COMPUTED AT A RATE OF 156 LBS/CF

CONTRACTOR FURN	IISHED SURVEYING & STAKING
	4 LUMD CUM

TEMPORARY SEEDING & MULCHING									
TEMP. SEEDING MULCHING									
	(AC)	(AC)							
TOTALS	0.4	0.4							

<sup>\*</sup> QNTY is 25% of Permanent Seed and Sod quantity

	CLEARING AND GRUBBING										
	BEGIN END										
SHEET	STATION	STATION	(AC)								
4	461+40.00	105+25.00	0.8								
		TOTAL	0.8								
		PAY TOTAL	1								

MOBILIZATION	
1 LUMP SUM	

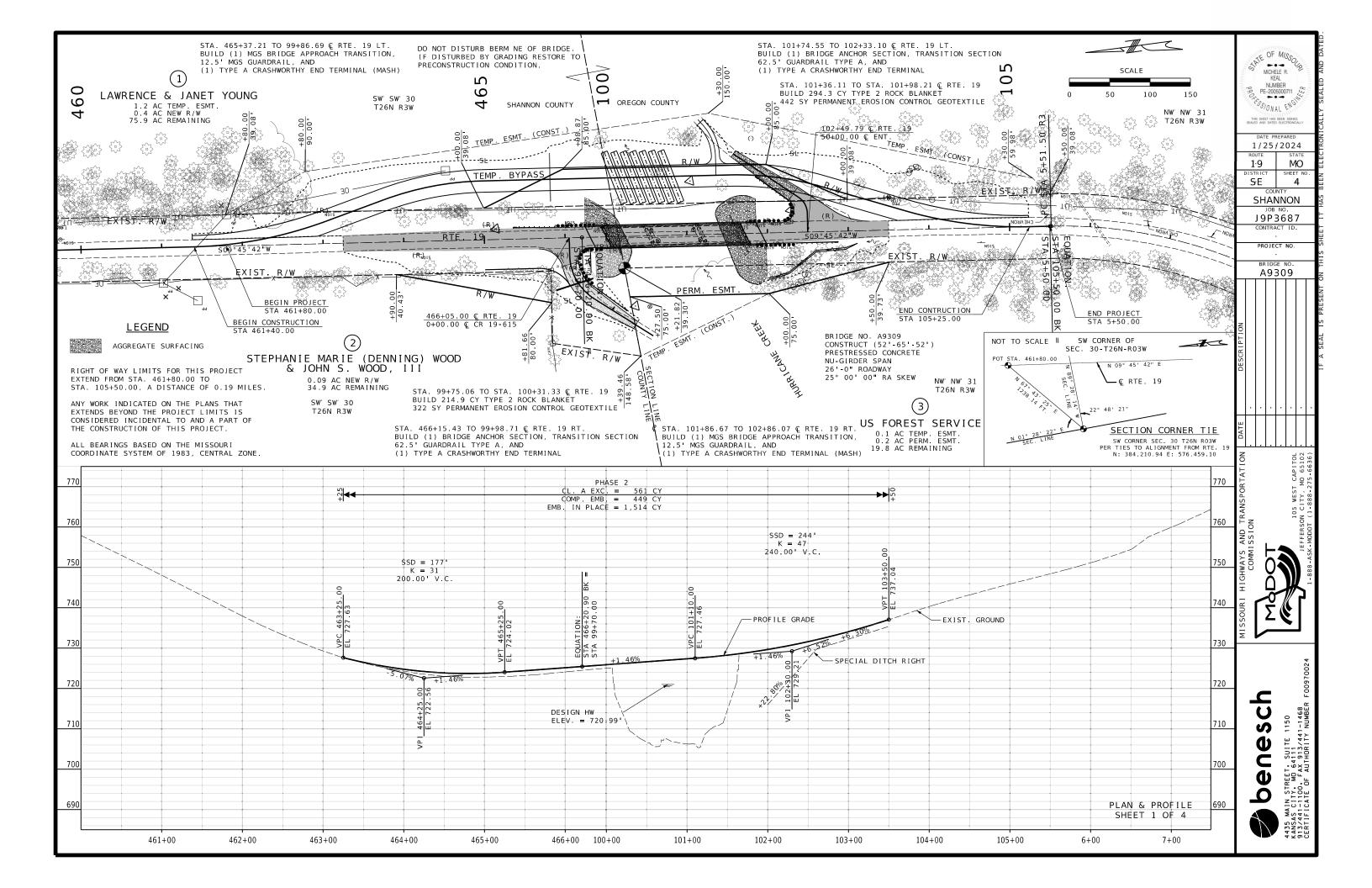
						EROSIO	ON CONTROL				
						TYPE 3B					
	BEGIN	END			SILT	EROSION	ALTERNATE	SEDIMENT	TYPE C		
SHEET	STATION	STATION	LOCATION	SIDE	FENCE	CONTROL	DITCH CHECK	REMOVAL	BERM		REMARKS
						BLANKET					
					(LF)	(SY)	(LF)	(CY)	(LF)		
10	462+18.87	466+36.82	RTE 19	LT	435.9			4		PHASE 1	
10	461+94.00	462+13.00	RTE 19	LT			12	3		PHASE 1	3 DITCH CHECKS AT 7' SPACING
10	463+15.00	464+13.00	RTE 19	LT			64	16		PHASE 1	16 DITCH CHECKS AT 6' SPACING
10	99+78.00		RTE 19	LT			4	1		PHASE 1	
10	465+28.23	99+84.00	RTE 19	LT		167.2				PHASE 1	
10	99+77.00		RTE 19	LT			4	1		PHASE 1	
10	101+24.37	105+38.59	RTE 19	LT	427.1			4		PHASE 1	
10	101+10.32	102+33.62	RTE 19	LT		90.9				PHASE 1	
10	102+08.00	102+16.00	RTE 19	LT			8	2		PHASE 1	2 DITCH CHECKS AT 8' SPACING
10	102+24.00	102+84.00	RTE 19	LT			40	10		PHASE 1	10 DITCH CHECKS AT 7' SPACING
10	104+87.03	105+23.73	RTE 19	LT		110.5				PHASE 1	
11	463+22.23	100+30.39	RTE 19	RT	381.8			4		PHASE 2	
11	463+26.00	464+95.00	RTE 19	RT		142.7				PHASE 2	
11	463+38.00	464+89.00	RTE 19	LT			68	17		PHASE 2	17 DITCH CHECKS AT 9' SPACING
11	99+72		RTE 19	LT			4	1		PHASE 2	
11	100+25.44		RTE 19	СТ					114.0	PHASE 2	
11	101+42.87		RTE 19	СТ					130.0	PHASE 2	
11	101+84.22	103+49.79	RTE 19	RT	171.3			2		PHASE 2	
11	102+64.00	103+06.00	RTE 19	RT			32	4		PHASE 2	4 DITCH CHECKS AT 11' SPACING
			TOTAL		1416.1	511.3	236.0	69.0	244.0		
	·		PAY TOTAL		1416	511	236	69	244		

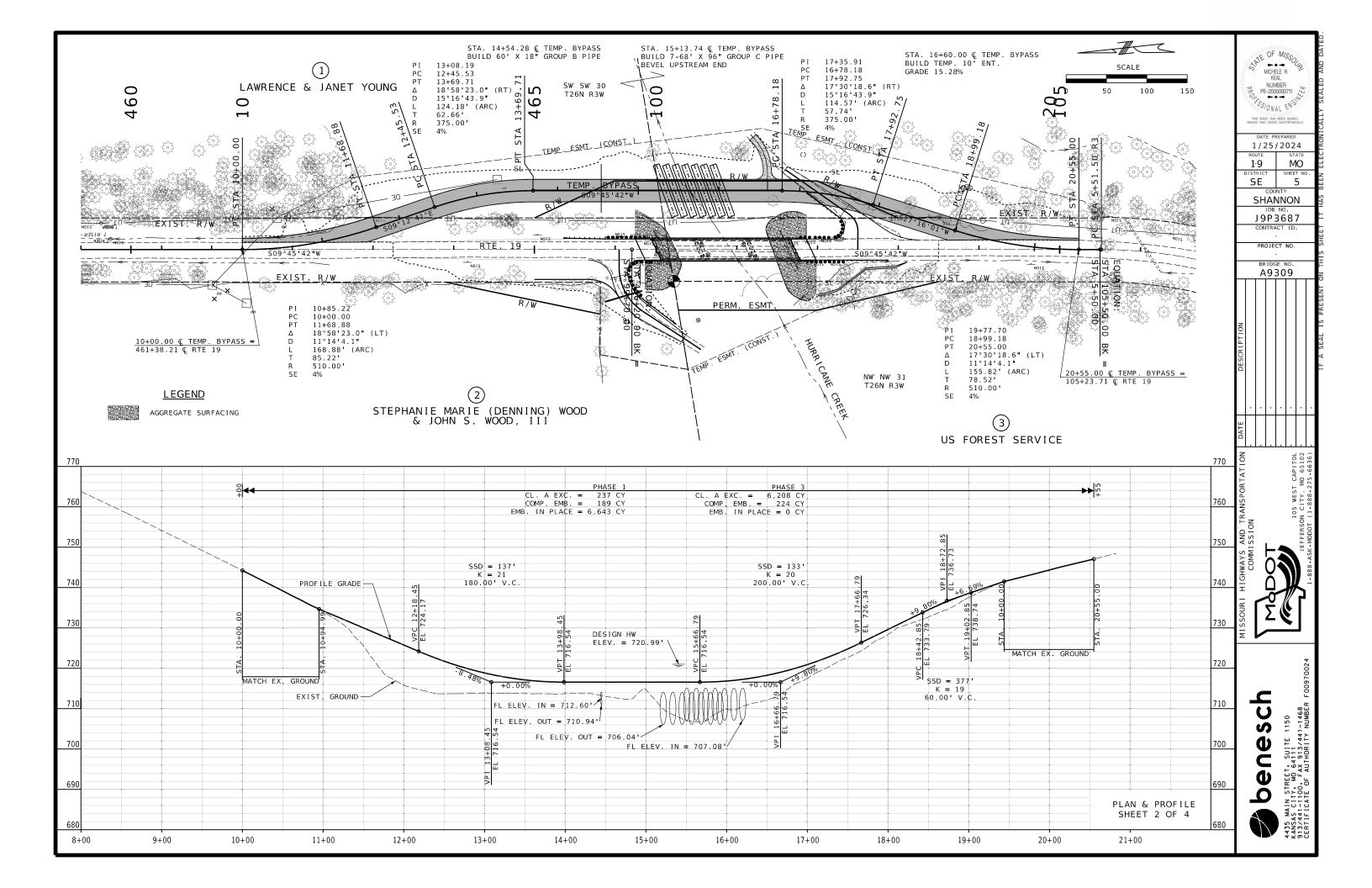
							GUARDRAIL					
SHEET	BEGIN STATION	END STATION	LOCATION	SIDE	MGS GUARDRAIL	GUARDRAIL TYPE A	TYPE A CRASHWORTHY END TERMINAL	TYPE A CRASHWORTHY END TERMINAL (MASH)	BRIDGE ANCHOR SECTION (CURB TYPE)	TRANSITION SECTION 6.5 FT POSTS	MGS BRIDGE APPROACH TRANSITION (REGULAR/NO CURB)	REMARKS
	465+37.21	466+36,59	DTE 10	<del>  ,                                   </del>	(LF)	(LF)	(EA)	(EA)	(EA)	(EA)	(EA)	+
4			RTE 19	LT.	12.5			1			1	
4	466+15.43	466+48.71	RTE 19	RT.		62.5	1		1	1		
4	101+74.55	102+33.10	RTE 19	LT.		62.5	1		1	1		
4	101+86.67	102+86.07	RTE 19	RT.	12.5			1			1	
				TOTAL	25.0	125.0	2	2	2	2	2	
	•		P/	AY TOTAL	25	125	2	2	2	2	2	

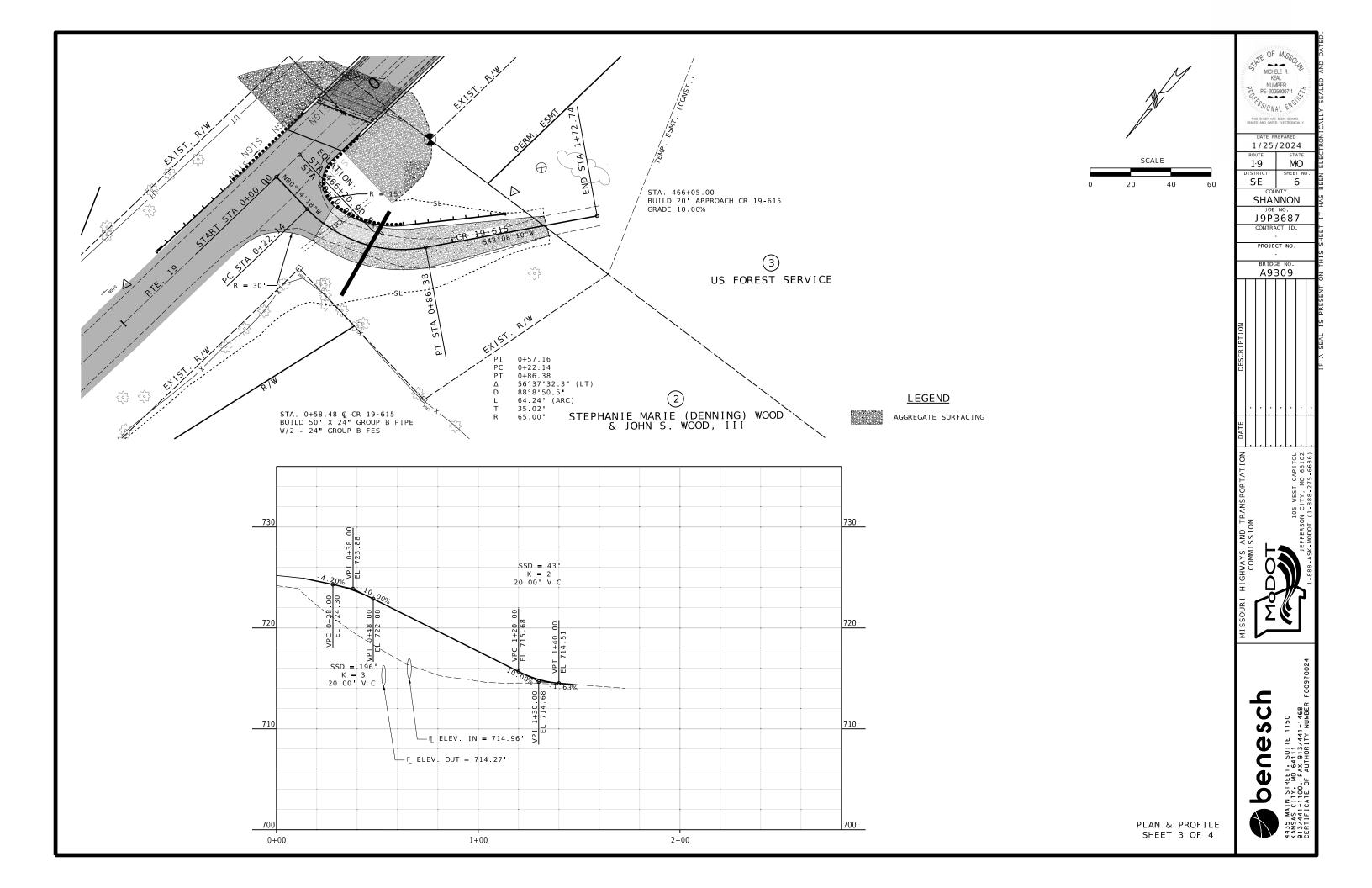
SHE OF MISSOLUTION MICHELE R. KEAL.
NUMBER PE-2005000711 7/10/2024 route state 1.9 MO DISTRICT SE SHEET NO COUNTY SHANNON JOB NO.
J9P3687
CONTRACT ID. PROJECT NO. BRIDGE NO.

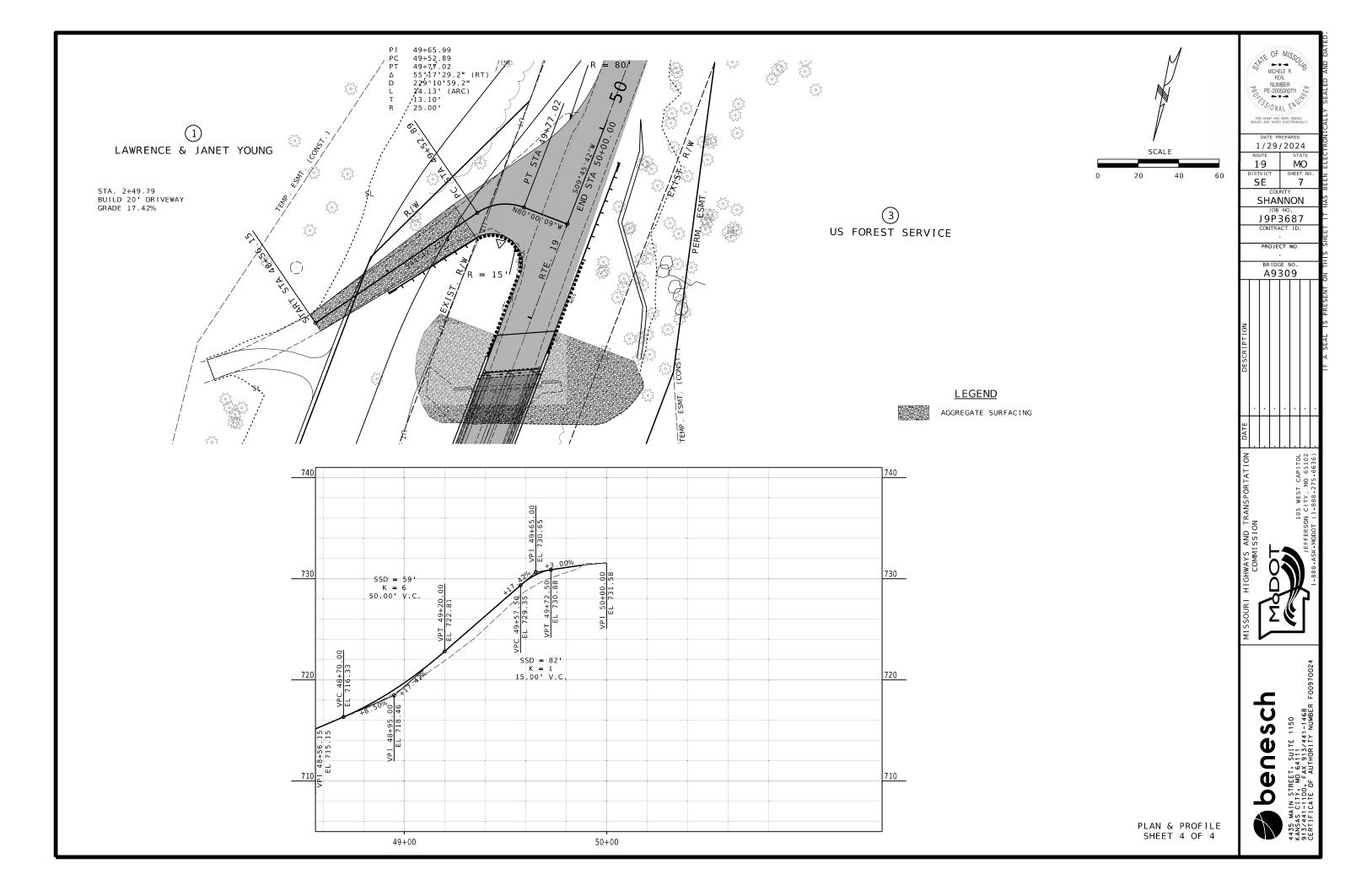


										EFFECTIVE: 04-01-2023	Т,			٦
TOTAL QTY TOT					1 '	TOTALSIG						TE OF A	MISSOL	
SIZE AREA QTY AREA RELOCREL	I I				TOTAL RELOC		1.	II	L		1	Sh, MICHELE	- ~	
SIGN IN SQ.FT EACH SQ.FT EACH SQ.	<u> </u>	SIGN	IN SQ.			SQ.FT.	- DESCRIPTION		TOTAL	DECCRIPTION	1	KEAL NUMBE		
WARNING SIGNS   WO1-1L   48X48   16.00	DESCRIPTION TURN (SYMBOL LEFT ARROW)	E05-1	36X48 12.		DE SIGNS	Г	DESCRIPTION  GORE EXIT	6122008	<del></del>	DESCRIPTION IMPACT ATTENUATOR 40 MPH (SAND BARRELS)		PE-200500	000711	70
WO1-1E 48X48 16.00	TURN (SYMBOL RIGHT ARROW)	E05-2	48X36 12				EXIT OPEN	6122009		IMPACT ATTENUATOR 45 MPH (SAND BARRELS)	11	ESSIONAL	ENG	
WO1-2L 48X48 16.00	CURVE (SYMBOL LEFT ARROW)	E05-2a	48X36 12.				EXIT CLOSED	6122010		IMPACT ATTENUATOR 50 MPH (SAND BARRELS)		THIS SHEET HAS BEE SEALED AND DATED EL		
WO1-2R 48X48 16.00	CURVE (SYMBOL RIGHT ARROW)	GO20-1	60X24 10.				ROAD WORK NEXT XX MILES	6122012		IMPACT ATTENUATOR 55 MPH (SAND BARRELS)	١L			_
WO1-3L 48X48 16.00	REVERSE TURN (SYMBOL LEFT ARROW)	GO20 - 2	48X24 8.				END ROAD WORK	6122014		IMPACT ATTENUATOR 60 MPH (SAND BARRELS)	∤ <b>I</b>	1/25/2		
WO1-3R 48X48 16.00 WO1-4L 48X48 16.00	REVERSE TURN (SYMBOL RIGHT ARROW) REVERSE CURVE (SYMBOL LEFT ARROW)		36X18 4. 42X30 8.				PILOT CAR FOLLOW ME PILOT CAR IN USE WAIT & FOLLOW	6122017		IMPACT ATTENUATOR 65 MPH (SAND BARRELS)  IMPACT ATTENUATOR 70 MPH (SAND BARRELS)	┨┞	ROUTE	STATE	-
WO1-4R 48X48 16.00	REVERSE CURVE (SYMBOL RIGHT ARROW)		18X12 1				PILOT CAR IN USE WAIT & FOLLOW	6122020		REPLACEMENT SAND BARREL	1 ∟	1.9	МО	_
WO1-4bL 48X48 16.00	DOUBLE ARROW REVERSE CURVE (SYMBOL LT ARROWS)	GO20-5aF	36X24 6.	00 4	24.00		WORK ZONE (PLAQUE)	6122030		IMPACT ATTENUATOR (RELOCATION)	1 I "	SE	SHEET NO.	
WO1-4bR 48X48 16.00	DOUBLE ARROW REVERSE CURVE (SYMBOL RT ARROWS)	MO4-8a	24X18 3.				END DETOUR	6123000		TRUCK OR TRAILER MOUNTED ATTENUATOR (TMA)	ŀ⊢	COUNT		-
WO1 -4cL 48X48 16.00	TRIPLE ARROW REVERSE CURVE (SYMBOL LT ARROWS)	MO4 - 9L MO4 - 9R	48X36 12				DETOUR (LEFT ARROW)	6161008		ADVANCED WARNING RAIL SYSTEM	ł∟	SHAN		_
WO1-4CR 48X48 16.00 WO1-6 60X30 12.50 2 50.00	TRIPLE ARROW REVERSE CURVE (SYMBOL RT ARROWS) HORIZONTAL ARROW (SYMBOL)	MO4-9R MO4-9P	48X36 12 48X12 4				DETOUR (RIGHT ARROW) STREET NAME (PLAQUE)	6161012		BUOYS (BOATS KEEP OUT) BUOYS (NO WAKE)	∤ <b>I</b>	лов N <b>J 9 P 3 (</b>		
WO1-6a 72X36 18.00	HORIZ. ARROW (SYMBOL ON PERMANENT BARRICADE)	<b></b>	48X18 6				DETOUR (ARROW LEFT)	6161014		SPECIAL SIGN ASSEMBLY (BOATS KEEP OUT)	t⊢	CONTRACT		-
WO1-7 60X30 12.50	DOUBLE HEAD HORIZONTAL ARROW (SYMBOL)	<b></b>	48X18 6.				DETOUR (ARROW RIGHT)	6161025	15	CHANNELIZER (TRIM LINE)	1 ∟			
WO1-7a 72X36 18.00	DOUBLE HEAD HORIZ. ARROW (SYMBOL ON PERM. BARR.)				ULATORY SIG	VS.		6161030		TYPE III MOVEABLE BARRICADE	11	PROJECT .	NO.	
WO1-8 18X24 3.00	CHEVRON (SYMBOL)	R1-1	48X48 13.				STOP	6161033		DIRECTION INDICATOR BARRICADE	<b>∤</b>	BRIDGE		-
WO1-8a 30X36 7.50 WO3-1 48X48 16.00	CHEVRON (SYMBOL FOR DIVIDED HIGHWAYS) STOP AHEAD (SYMBOL)	R1-2 R1-2a	48TRI 6.		+ + -		TO ONCOMING TRAFFIC (PLAQUE)	6161040		FLASHING ARROW PANEL TYPE III OBJECT MARKER	┧┞	A930	09	╛
WO3-2 48X48 16.00	YIELD AHEAD (SYMBOL)	R1 - 3P	30X30 9.				ALL WAY (PLAQUE)	6161047		SEQUENTIAL FLASHING WARNING LIGHT	<b>1                                    </b>			
WO3-3 48X48 16.00	SIGNAL AHEAD (SYMBOL)	R2 - 1	36X48 12		60.00		SPEED LIMIT XX	6161070		TUBULAR MARKER	1 <b> </b>	,		
WO3-4 48X48 16.00	BE PREPARED TO STOP	R3-1	48X48 16.				NO RIGHT TURN (SYMBOL)	6161095		RADAR SPEED ADVISORY SYSTEM	] [			
WO3-5 48X48 16.00 2 32.00	SPEED LIMIT AHEAD	R3-2	48X48 16.				NO LEFT TURN (SYMBOL)	1		CHANGEABLE MESSAGE SIGN,	<sub>Z</sub>	,		
WO4-1L 48X48 16.00 WO4-1R 48X48 16.00	MERGE (SYMBOL FROM LEFT)  MERGE (SYMBOL FROM RIGHT)	R3-3 R3-4	36X36 9. 48X48 16.		+		NO TURNS NO U-TURN (SYMBOL)	6161096	1	COMMISSION FURNISHED/RETAINED CHANGEABLE MESSAGE SIGN W/O COMM.	┨╣╟	,		
WO4-1R   48X48   16.00	MERGE (SYMBOL FROM RIGHT)  MERGE (ARROW SYMBOL)	R3-4 R3-7L	30X30 6.		+ + -		LEFT LANE MUST TURN LEFT	6161098	2	INTERFACE - CONTRACTOR FURNISHED/RETAINED	MI P			
WO4-1aR 48X48 16.00	MERGE (ARROW SYMBOL)	R3 - 7R	30X30 6.				RIGHT LANE MUST TURN RIGHT	1		CHANGEABLE MESSAGE SIGN WITH COMM.	SCI	.		
WO5-1 48X48 16.00	ROAD/BRIDGE/RAMP NARROWS	R4-1	36X48 12.	00 2	24.00		DO NOT PASS	6161099	1	INTERFACE - CONTRACTOR FURNISHED/RETAINED		.		
WO5-3 48X48 16.00	ONE LANE BRIDGE	R4 - 2	36X48 12.				PASS WITH CARE	6162000		WORK ZONE TRAFFIC SIGNAL SYSTEM	↓ <b>I</b> ∣	.		
WO5-5 48X48 16.00 WO6-1 48X48 16.00	NARROW LANES DIVIDED HIGHWAY (SYMBOL)	R4-8a R4-7a	36X48 12.				KEEP LEFT (HORIZONTAL ARROW) KEEP RIGHT (HORIZONTAL ARROW)	6162002	!	TEMPORARY LONG-TERM RUMBLE STRIPS TEMPORARY TRAFFIC BARRIER	∤ <b>∦</b> ∣	.		
WO6-2 48X48 16.00	DIVIDED HIGHWAY END (SYMBOL)	R5 - 1	30X30 6.				DO NOT ENTER	6173600		CONTRACTOR FURNISHED/RETAINED		$ \cdot \cdot \cdot \cdot$	.   .   .	
WO6-3 48X48 16.00	TWO WAY TRAFFIC (SYMBOL)	R5 - 1a	36X24 6.				WRONG WAY	1	1	TEMPORARY TRAFFIC BARRIER	▎▐▃▍		$\top$	٦
WO7-3a 30X24 5.00	NEXT XX MILES (PLAQUE)	R6-1L	54X18 6.	75			ONE WAY ARROW (LEFT)	6173602	В	CONTRACTOR FURNISHED/COMMISSION RETAINED	JAT	.		
WO8-1 48X48 16.00	BUMP	R6-1R	54X18 6.				ONE WAY ARROW (RIGHT)	6174000		TEMP. TRAFFIC BARRIER HEIGHT TRANSITION	┨Ҵ	<u></u>	<u>.   .   .  </u>	
WO8-2 48X48 16.00 WO8-3 48X48 16.00	DIP PAVEMENT ENDS	R6-2L R6-2R	24X30 5.				ONE WAY (LEFT) ONE WAY (RIGHT)	6175010	4	RELOCATING TEMPORARY TRAFFIC BARRIER TEMPORARY TRAFFIC BARRIER	l s		0.2 0.2	٥,
WO8-4 48X48 16.00	SOFT SHOULDER	R9-9	24X12 2				SIDEWALK CLOSED	6176000	В	COMMISSION FURNISHED/RETAINED	Ιij		APITOL 65102	00.
WO8-5 48X48 16.00	SLIPPERY WHEN WET (SYMBOL)						SIDEWALK CLOSED AHEAD,			TEMP. TRAFFIC BARRIER HEIGHT TRANSITION	1 [ ≿		2 €	2
WO8-6 48X48 16.00	TRUCK CROSSING (WITH FLAGS)	R9-11L	24X18 3.	00			(ARROW LEFT) CROSS HERE	6177000		COMMISSION FURNISHED/RETAINED			EST Y,	ά.
WO8-6c 48X48 16.00	TRUCK ENTRANCE		24710	00			SIDEWALK CLOSED AHEAD,	6208064		TEMPORARY RAISED PAVEMENT MARKER	NS		5 W	3
WO8-7 36X36 9.00 WO8-7a 36X36 9.00	LOOSE GRAVEL FRESH OIL/LOOSE GRAVEL	R9-11R R10-6	24X18 3. 24X36 6.				(ARROW RIGHT) CROSS HERE STOP HERE ON RED (45^ ARROW)	9029400		TEMPORARY TRAFFIC SIGNALS TEMPORARY TRAFFIC SIGNALS AND LIGHTING	<b>┨</b> ⋛	z	105 W ERSON CIT	-
WO8-9 48X48 16.00	LOW SHOULDER		48X30 10		20.00		ROAD CLOSED	1 3023401		TEM ONANT MATTE STOWARD AND ETGITTING	<b>1</b>	ION	ERS	3
WO8-11 48X48 16.00	UNEVEN LANES						ROAD CLOSED XX MILES AHEAD				]   ¥	SS 1 4	EFF.	۲
WO8-12 48X48 16.00	NO CENTER LINE		60X30 12.				LOCAL TRAFFIC ONLY				ζ		= 2	Α'n
WO8-15 48X48 16.00	GROOVED PAVEMENT	<b></b>	60X30 12.				ROAD CLOSED TO THRU TRAFFIC	<b>-</b>			ĕ   ĕ	3 ()∢	<b>3</b>	ů,
WO8-15P 30X24 5.00 WO8-17 48X48 16.00	MOTORCYCLE (PLAQUE) SHOULDER DROP-OFF (SYMBOL)		60X48 20. 56X12 4.				FINE SIGN SPEEDING/PASSING (PLATE)	-			<b>┨</b> ┋	Ă		5
WO8-17 40X40 10.00 WO8-17P 30X24 5.00	SHOULDER DROP-OFF (PLAQUE)	1 00031-37	4 JON 12   4.		CELLANEOUS	SIGNS	STEEDING/TASSING (TEATE)	1			<b>'</b>   ∃	٦	<i>#</i> /\	,
W10-1 42RND. 9.62	RAILROAD CROSSING	CONST-5	48X36 12.				POINT OF PRESENCE	]			٦ I	<u> </u>	<i>[[</i> [	
WO12-1 24X24 4.00	DOUBLE DOWN ARROW (SYMBOL)	<b></b>	96X48 32.				POINT OF PRESENCE	1			00.	<u> </u>   <u> </u>	<b>&amp;</b>	
W012-2 48X48 16.00	LOW CLEARANCE (SYMBOL)		48X24 8.		16.00		RATE OUR WORK ZONE	-			155			
W012-2X 24X18 3.00 W012-2a 84X24 14.00	LOW CLEARANCE (PLAQUE)   OVERHEAD LOW CLEARANCE (FEET AND INCHES)		72X36 18. 48X36 12.		24.00		RATE OUR WORK ZONE  WORK ZONE NO PHONE ZONE	-			Σ			_
WO12-4 120X60 50.00	LOW CLEARANCE XX FT XX IN XX MILES AHEAD	1	10,730 12.				ZONE NO THORE ZONE	1						
WO12-5 120X60 50.00	WIDTH RESTRICTION XX FT XX IN XX MILES AHEAD							]					5.4	
WO13-1 30X30 6.25 2 12.50	ADVISORY SPEED (PLAQUE)							4					700	
W016-2 30X24 5.00	XXX FEET (PLAQUE)	1						4					900	,
WO16-3 30X24 5.00 WO20-1 48X48 16.00 2 32.00	X MILE (PLAQUE)  ROAD/BRIDGE/RAMP WORK AHEAD	+			<del>                                     </del>			-					~ E	
WO20-1 48X48 16.00 2 32.00 WO20-2 48X48 16.00	DETOUR AHEAD	+						1				U	68 BEF	į
WO20-3 48X48 16.00	ROAD CLOSED AHEAD							j				S	TE 1150 1/441-1468 11TY NUMBE	
WO20-4 48X48 16.00	ONE LANE ROAD AHEAD	616-10			TOTAL			-					E 1	
WO20-5 48X48 16.00	RIGHT/CENTER/LEFT LANE CLOSED AHEAD		UCTION S	IGNS		TOTAL						W	11.1 3.5 11.18	
WO20-5a 48X48 16.00 WO20-6a 48X48 16.00	2 RIGHT/CENTER/LEFT LANES CLOSED AHEAD	$\frac{1}{1}$ 616-10		ıc		TOTAL 0							SUI 4111 913,	
WO20-6a 48X48 16.00 WO20-7a 48X48 16.00	RIGHT/CENTER/LEFT LANE CLOSED FLAGGER (SYMBOL, WITH FLAGS)	RELUCA	TED SIGN	13		U						7.	ET. O 64 FAX AUTI	
WO21-2 36X36 9.00	FRESH OIL	1										Ψ	AIN STREET CITY, MO 1-1100, F/	
WO21-5 48X48 16.00	SHOULDER WORK AHEAD	]										9	. S. T. S. T. E. S. T	
WO22-1 48X48 16.00	BLASTING ZONE AHEAD	4											MAIN S CI 41-1 FICA	
WO22-2 42X36 10.50 WO22-3 42X36 10.50	TURN OFF 2-WAY RADIO AND PHONE	-											5 M SAS 744 FIF	
WO24-1 48X48 16.00 2 32.00	END BLASTING ZONE  DOUBLE REVERSE CURVE	1								SUMMARY OF QUANTITIES	, I		4435 R KANSA 913/4 CERTII	
		_								SHEET 3 OF 3			1 2 0,0	









ALL PROJECT COORDINATES HAVE BEEN PROJECTED FROM THE MISSOURI STATE PLANE COORDINATE (SPC) SYSTEM OF 1983 USING AN AVERAGE PROJECT PROJECTION (GRID TO GROUND) FACTOR. TO GET BACK TO STATE PLANE COORDINATES MULTIPY THE PROJECT COORDINATES BY THE AVERAGE GRID FACTOR AS SHOWN IN THE "REFERENCE CONTROL INFORMATION" PORTION OF THIS TABLE.

PROJECT COORDINATE INFORMATION							
COORDINATE SYSTEM	STATE PLAN	E					
HORIZONTAL DATUM	NAD 83 (20	NAD 83 (2011)					
VERTICAL DATUM	NAVD 88						
GEOID MODEL	12B						
ELEVATIONS	XX						
DETERMINED BY	^^						
PROJECT PROJECTION	FACTOR	1.000000					
REFERENCE CONTROL INFORMATION							

DETERMINED BY								
PROJECT PROJECTI	ON	FAG	CTOR	1.000000				
REFERENCE CONTROL INFORMATION								
COORDINATE SYSTE	М	МО	COORDIN	ATE :	SYSTEM	OF	1983	
CONTROL STATION								
DESIGNATION								
CORS_ID								
PID								
LATITUDE	36	53	22.951	90				
LONGITUDE	91	19	43.969	37			·	
NORTHING (M)	11	753	4.473					
EASTING (M)	17	611	7.744					
ZONE	ĒΑ	·SΤ						

PROJECT AVERAGE GRID FACTOR 0.99996591

PROJECT NORTHING X AVERAGE GRID FACTOR
= STATE PLANE NORTHING

EXAMPLE OF PROJECT COORDINATE TO S.P.C.

PROJECT EASTING X AVERAGE GRID FACTOR = STATE PLANE EASTING

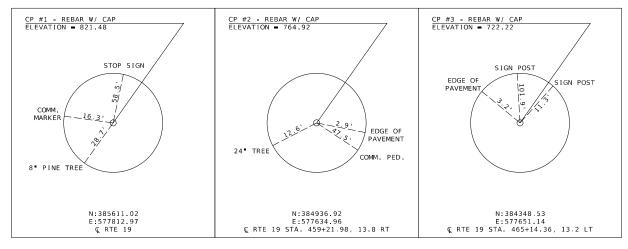
EXAMPLE: CONTROL POINT #2

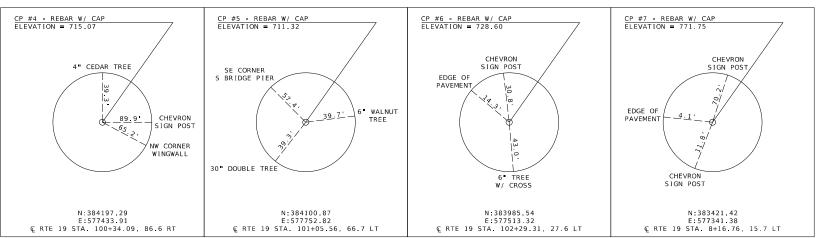
N 384936.92 X 1.00000 = N 384936.92 E 577634.96 X 1.00000 = E 577634.96

#### LINEAR UNIT CONVERSION

1 METER = 3.280833333 US SURVEY FEET (USFT)

	COORDINATE POINT LISTING													
					MOD I F I E	D STATE PLANE (	GROUND)							
				OFFSET	NORTHING	EASTING	ELEVATION		GPK					
SH	HEET NO	STATION	LOCATION	(USFT)	(US SURVEY FT)	(US SURVEY FT)	(US SURVEY FT)	DESCRIPTION	POINT ID					
PRO	OJECT CO	NTROL POINTS	5											
		•	RTE 19		385611.02	577812.97	821.48	REBAR W/ CAP	1					
		459+21.98	RTE 19	13.8	384936.92	577634.96	764.92	REBAR W/ CAP	2					
		465+14.36	RTE 19	- 13 . 2	384348.53	577561.14	722.22	REBAR W/ CAP	3					
1 📖		100+34.09	RTE 19	86.6	384197.29	577433.91	715.07	REBAR W/ CAP	4					
1		101+05.56	RTE 19	-66.7	384100.87	577572.82	711.32	REBAR W/ CAP	5					
ᅥᄔ		102+29.31	RTE 19	-27.6	383985.54	577513.32	728.60	REBAR W/ CAP	6					
┧┕		8+16.76	RTE 19	- 15 . 7	383421.42	577341.38	771.75	REBAR W/ CAP	7					
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AL:	I GNMENTS													
		457+51.16	RTE 19	•	385102.92	577677.54		POT	EX · 19					
		5+51.50	RTE 19		383672.69	577431.48		PC	EX · 19					
		11+11.81	RTE 19		383247.02	577101.99		POT	EX · 19					
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COORDINATE POINT SHEET SHEET 1 OF 1



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MICHELE R.
KEAL
NUMBER
PE-2005000711

1/25/2024

JOB NO. J 9P3687

PROJECT NO.

A9309

MO

SHEET NO

8

1.9

SE

APPROVED BY ENGINEER.

- TRAFFIC CONTROL NOTES:
  1. CONTRACTOR SHALL PLACE TRAFFIC CONTROL DEVICES AS SHOWN ON PLAN. CONTRACTOR SHALL COVER OR REMOVE ANY TRAFFIC CONTROL DEVICES OR PAVEMENT MARKING THAT CONFLICT WITH THE TRAFFIC CONTROL PLAN.

  2. TRAFFIC CONTROL DEVICES SHALL MEET CURRENT MODOT STANDARDS AND
- SPECIFICATIONS.
- SPECIFICATIONS.

  3. COVER EXISTING W1-4L AND W13-1 SIGN AT APPROXIMATE STA 455+28.

  4. CONTRATOR SHALL INSTALL WO1-4R SIGN WITHOUT BLOCKING EXISTING CHEVRON SIGNS (W1-8). CONTRACTOR SHALL COORDINATE WITH ENGINEER FOR APPROVAL OF SIGN LOCATION.

  5. CONTRACTOR SHALL USE FLAGGER OPERATION FOR CONSTRUCTION AND REMOVAL OF TEMPORARY BYPASS. SEE MODOT STANDARD PLAN 616.20.

  6. CONTRACTOR SHALL MAINTAIN ACCESS TO ENTRANCES AT ALL TIMES, AS APPROVED BY ENGINEER

PHASE 1: CONSTRUCT TEMPORARY BYPASS

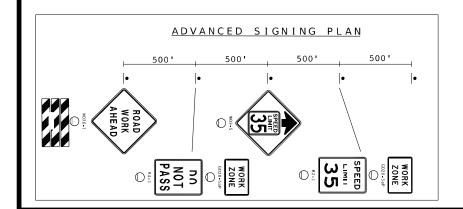
CONSTRUCTION PHASING

PHASE 2: SHIFT TRAFFIC TO BYPASS
BUILD BRIDGE AND APPROACH PAVEMENT
PHASE 3: SHIFT TRAFFIC TO NEW ROADWAY

REMOVE TEMPORARY BYPASS



09 46 2 0 INSTALL CHANNELIZERS— (35' SPACING) 4" TEMPORARY WHITE EDGE LINE 9 Mo Mo 20 4" TEMPORARY DOUBLE YELLOW CENTERLINE-MO.RTE 19 N N -INSTALL CHANNELIZERS (35' SPACING) INSTALL CHANNELIZERS -SEE ADVANCE SIGNING PLAN (35' SPACING) -TYPE III BARRICADE TYPE III BARRICADE-<sub>₹</sub>မ္ဟ



### TRAFFIC CONTROL LEGEND

WORK AREA

- SIGN (SINGLE SIDED)
- CHANNELIZER
- ] TYPE III MOVEABLE BARRICADE



ADVANCED WARNING RAIL SYSTEM

TRAFFIC CONTROL SHEET 1 OF 1 TEMPORARY BYPASS

MICHELE R. KEAL NUMBER PE-2005000711 FSSIONAL ENGIN

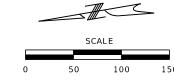
1/25/2024								
ROUTE	STATE							
1.9	MO							
DISTRICT	SHEET NO.							
SE	9							

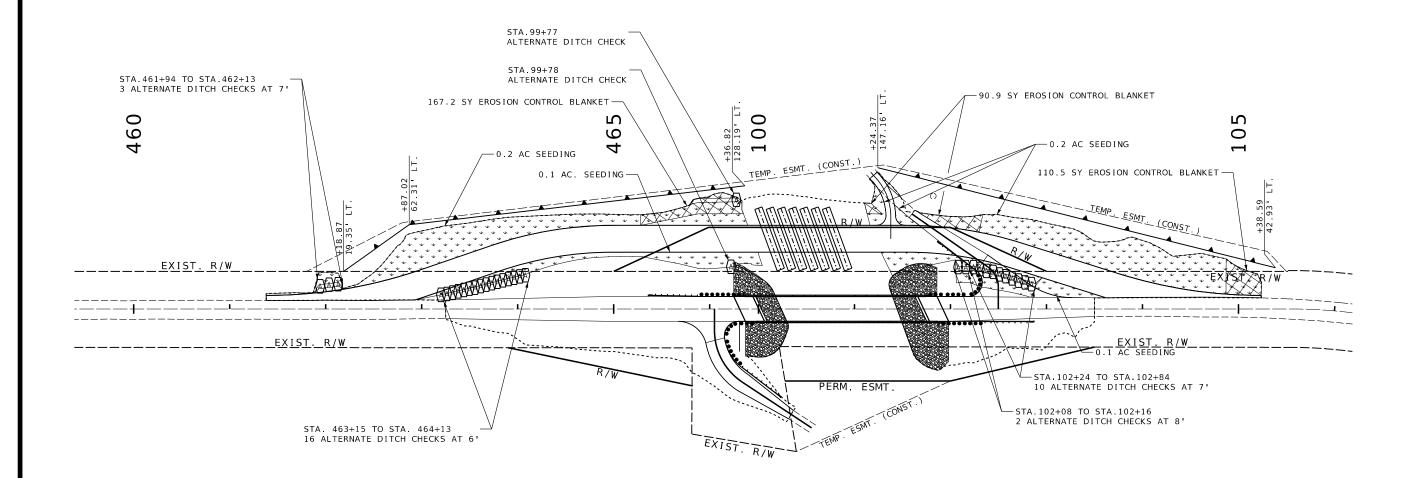
SHANNON J9P3687

PROJECT NO.

A9309

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## TEMPORARY EROSION CONTROL LEGEND

R ROCK DITCH CHECK

ALTERNATE DITCH CHECK

SILT FENCE

EROSION CONTROL BLANKET

TEMPORARY SEEDING AND MULCHING

TYPE C BERM

EROSION CONTROL PHASE 1 SHEET 1 OF 3 MCHELE R.
KEAL
NUMBER
PE-2005000711
THE SPEET FINE BEEDS SOUND.
SEALED AND DATE PREPARED
1/25/2024
ROUTE STATE
1-9 MO
DISTRICT SHEET NO.
SE 1-0
COUNTY
SHANNON
JOB NO.
J 9P 36 87
CONTRACT ID.

PROJECT NO.

BRIDGE NO.
A9309

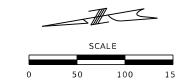
MISSOURI HIGHWAYS AND TRANSPORTATION DATE

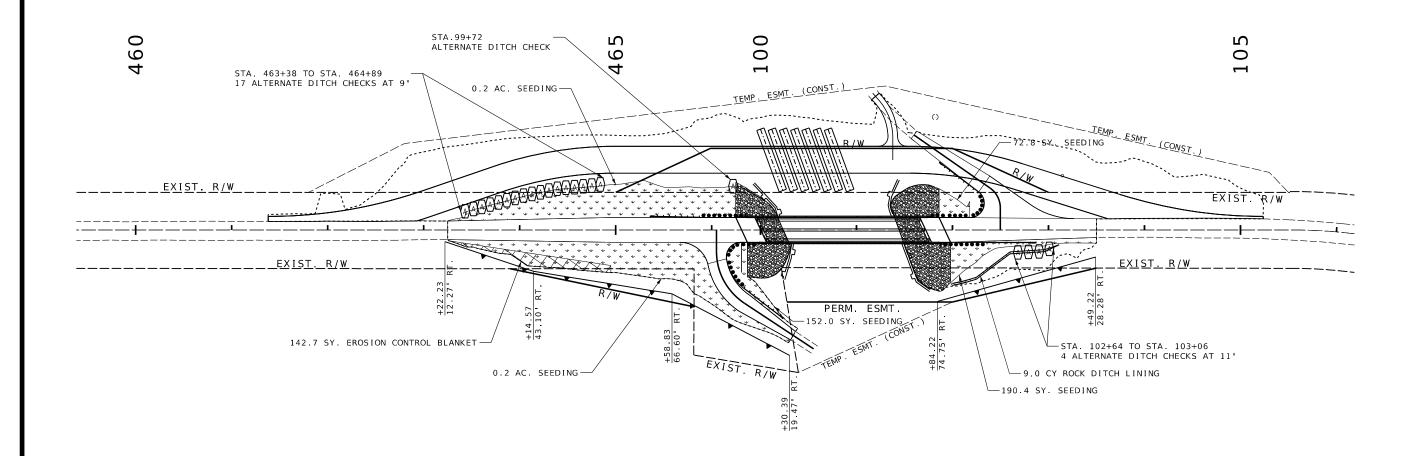
COMMISSION

105 WEST CAPITOL

HEFFERSON CITY, MO 65102

benesch 4435 MAIN STREET. SUITE 1150 KANSAS CITY. MG 64111 913.7441-1100, FAX 913.7441-1468





## TEMPORARY EROSION CONTROL LEGEND

R	ROCK DITCH CHECK
(A)	ALTERNATE DITCH CHECK
	SILT FENCE
	EROSION CONTROL BLANKET
, , , , , , , , , , , , , , , , , , ,	TEMPORARY SEEDING AND MULCHING
	TYPE C BERM

## PERMANENT EROSION CONTROL LEGEND



ROCK DITCH LINING TYPE 3

EROSION CONTROL PHASE 2 SHEET 2 OF 3

MICHELE R. KEAL NUMBER PE-2005000711 CONTE-2005000711

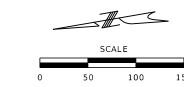
1/25/2024 1.9 МО SE 1.1

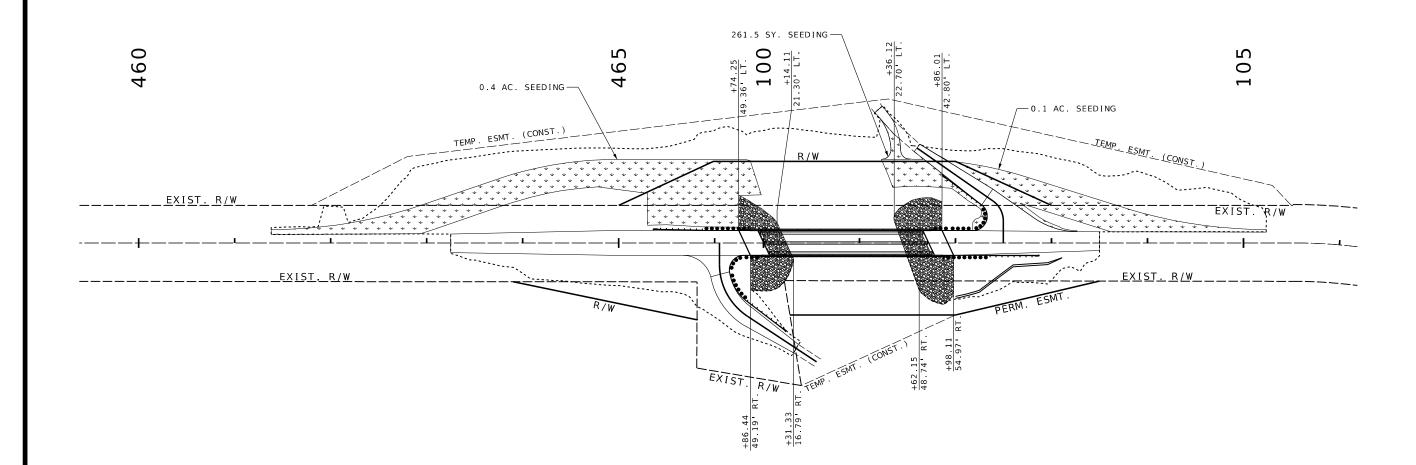
SHANNON J9P3687

PROJECT NO.

A9309

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## TEMPORARY EROSION CONTROL LEGEND

ROCK DITCH CHECK

ALTERNATE DITCH CHECK

SILT FENCE

EROSION CONTROL BLANKET

TEMPORARY SEEDING AND MULCHING

EROSION CONTROL PHASE 3 SHEET 3 OF 3



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DATE PE	REPARED
1/25	2024
ROUTE	STATE
1.9	МО
STRICT	SHEET NO.
SE	1.2
COU	NTY
SHA	NON
JOB	NO.
	3687
CONTRA	CT ID.

PROJECT NO.

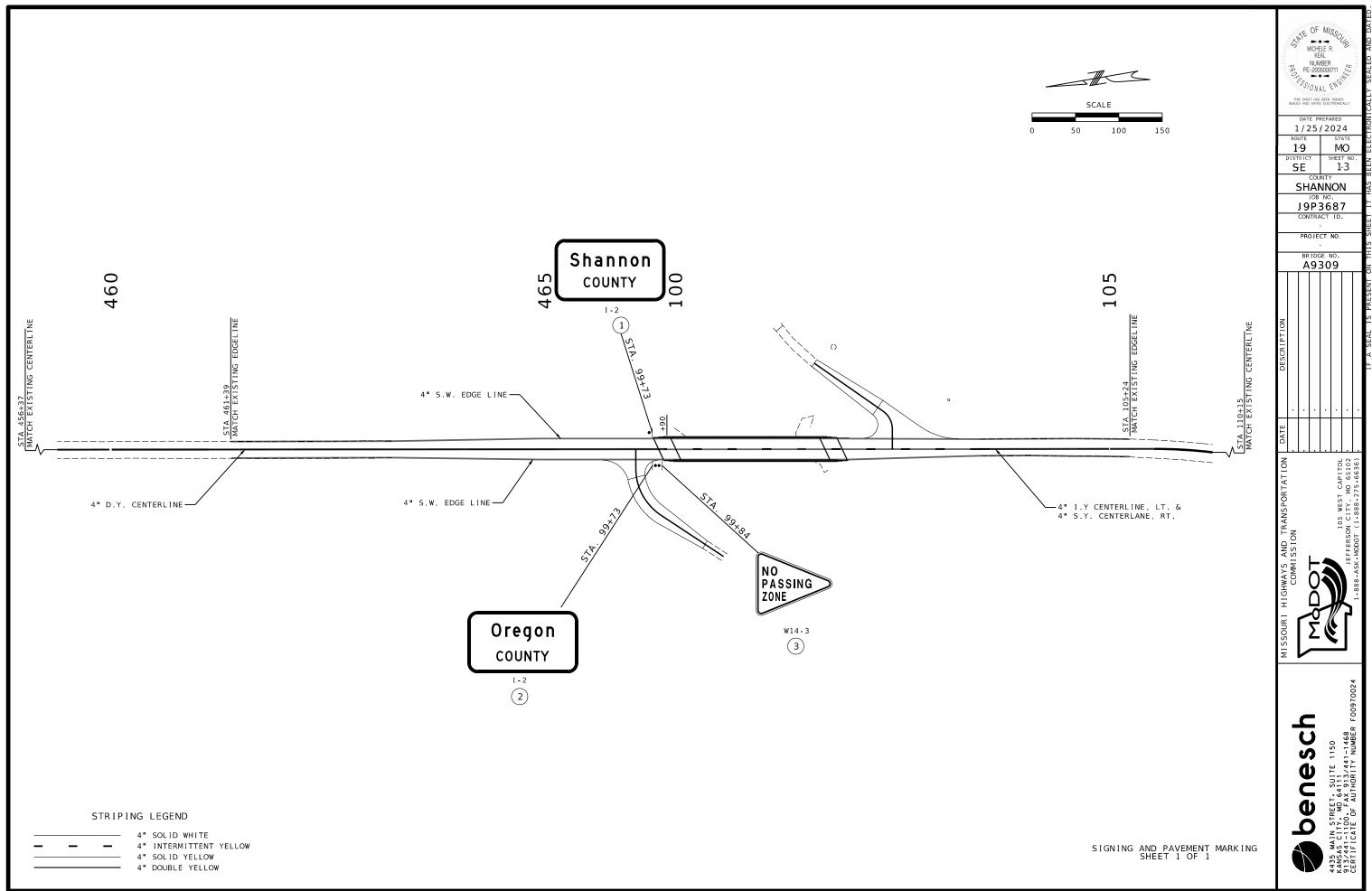
BRIDGE NO.

A9309



**benesch**35 MAIN STRET, SUITE 1150

313/481-1100, FAX 913/41-1468



																																		EFFECTIVE 07-01-2022	MAN THE
		SI	GNS				CONCRETE FOOT INGS	S	TRU	CTUR/ POS	AL S' TS *	TEEL *		Р	IPE P	OSTS	*	BA BA	CK I	NG **	U- CHANNE				2 111		RFOR	ATED	SQUARE						SPIE OF MISS
02 SIG	NAL S	IGNS TAE	BULATED	ON D-	-37A S⊦		EMBEDDED			. 05											POST				2 IN.	ANC	CHORS				2.25"	ANCHORS	BREAK- AWAY	AND	KEAL NUMBER PE-200500071
GN SI	GN ZE	STATION	HORZ CLEAR IF NO STD	LOCA	NOITA	SIGN OTL. SHT. NO.	ITEM NO. 9031010 CY	POST DES NO.	POST NO.1	POST P NO. 2 N	POST L NO.3 P	BS TC PER ITE FT 903	TAL F M NO. 5 31210 .BS	PIPE P SIZE N	POST POS	T LBS 2 PER FT	TOTAL ITEM NO. 9031220 LBS	@ 2.55	X [] 5 LBS TH TO	BARS PER FT DTAL TOTAL LF LBS	- ITEM NO. - 9031250A LF	POS NO.	T POST 1 NO.2	TOTAL ITEM NO. 9031270A	12- ITEM	GA 7 NO ITE 271A 903	GA M NO	CONCRETE 7 - GA ITEM NO 9031274 EA	POST POST NO.1 NO.2	TOTAL ITEM NO. 9031280 LF	INSERT (6 FT) ITEM NO. 9031272A EA	7-GA. 7-GA.	ASSEMBLY ITEM NO. 9031241 EA	OTHER REQUIRED ITEMS	THE SHEET HAS BEEN SE
32" 32"		99+73 99+73	-		∃ 19 ∃ 19		· ·				-											9		9	1			-			-				DATE PREPAR 1/25/20
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					TOTAL		* RREAKAWAV	~~~~	~~~~	S INCH	~~~~			~~~~	STEEL	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\		<u> </u>	<u> </u>	**			<u> </u>	2:7	3	3	•				•		·		
						*	* BREAKAWAY ** BACKING BA	RS ARE	E Tot	ALED W	ITH ST	- POK S FRUCTUI	RAL S	TEEL (	OR PIPE	POSTS	E PUST	J.					STI POST	RUCTU	RAL	STEEL	PO:	ST AN	D FOOTI	NG DA		BLE			
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								WEIG FT	LBS/		STUB LENGTH	+ □	DIA.	OT I NG	TH	ONCRET C.Y.	E				1 W6 2 W6 3 W8	15	. 0 5 . 0 3 . 0	0.75 1.25 1.50	3 0 4 0 4 6	24	4	'-0" 0 '-0" 0	.47 4'-2" .71 4'-8"	0.15 0.50 0.73	4 3 4 9	0.16 3'-6" 0.51 4'-6" 0.74 5'-0"	0.17 0.54 0.78		435 V
						F	3 7.	.79 .58 .79	0.48		$ \begin{array}{cccc} 4 & 3\frac{1}{2} \\ 4 & 3\frac{1}{2} \\ 5 & 3\frac{1}{2} \end{array} $	•	12	4~6 4~6	5	0.13 0.13 0.36					W10 W10 W12		2.0	1.83 2.17 2.92	5 0 5 0	36° 36°	5	0 1	31 5 2	1.36	5 3	1 39 5 6 1 43 5 9 1 1 56 6 3 1	1.45	D-29	1

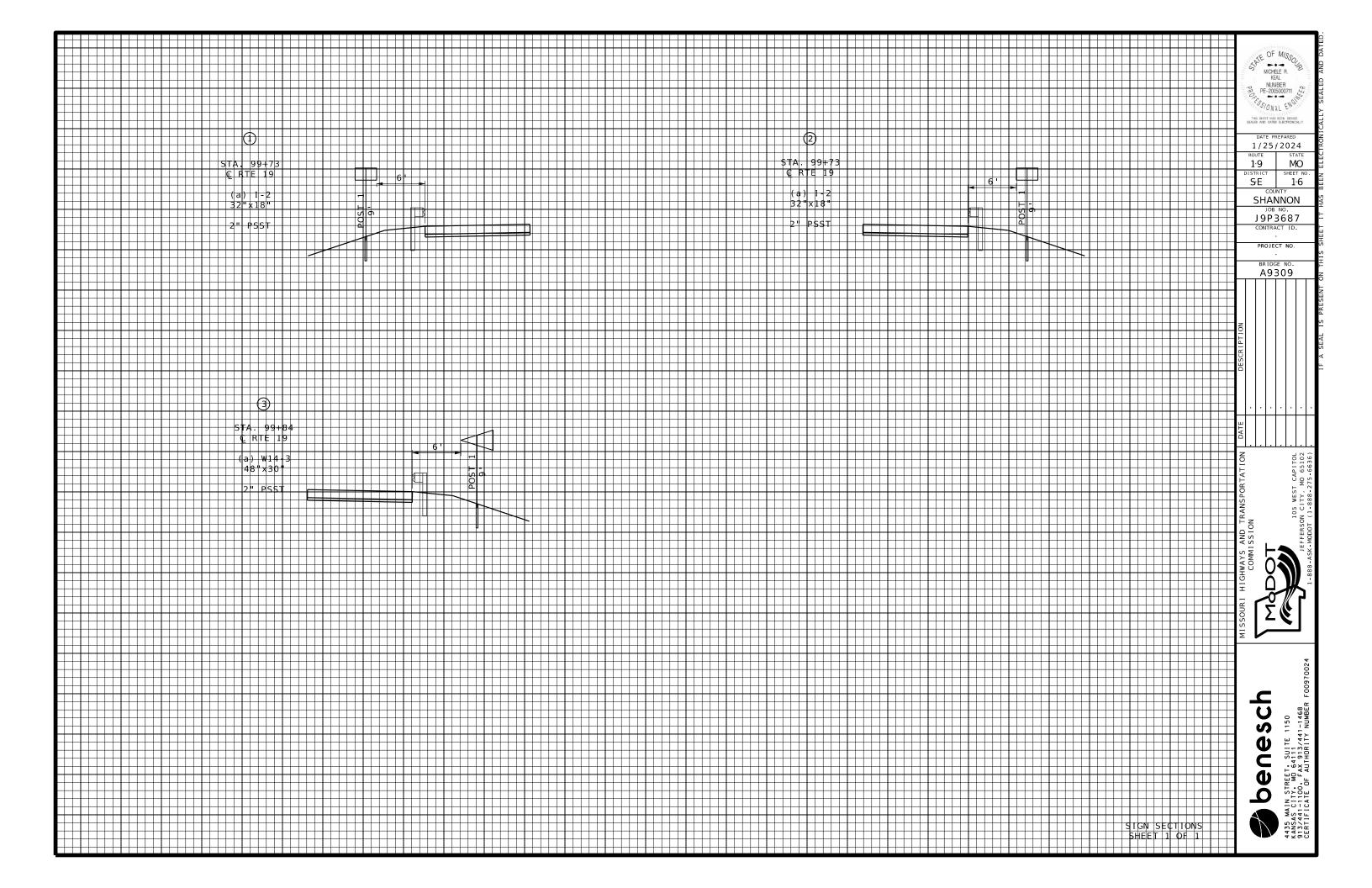
		S	TANDA	RD SI	GN ASSEN	/BLI	ES			SIGN SUMMARY								
						TYPE							SIZE, TYPE & SQUARE FEET					
NUMBER	STATION	LOCATION	SH	SH SIGN D	ESCRIPTION	, SIZE	ES & N	NUMBER	OF EACH	STANDARD SIGN OR SPECIAL SIGN NUMBER	SIGN DETAIL SHEET NO.	NO. EACH	SIZE	FLAT SHEET SH	FLAT SHEET FLUORESCENT SHF *	STRUCTURAL ST	STRUCTURAL FLUORESCENT STF *	
SIGN			I - 2											ITEM NO. 9035004A	ITEM NO. 9035069A	ITEM NO. 9035011A	ITEM NO. 9035071A	
S			32"x18	W14-3 "48"x30"						SHANNON COUNTY I-2 OREGON COUNTY I-2		1	32"x18" 32"x18"	4				
1	99+73	RTE 19	1				-			 NO PASSING ZONE W14-3		1	48"x30"	5				
2	99+73	RTE 19	1	1														
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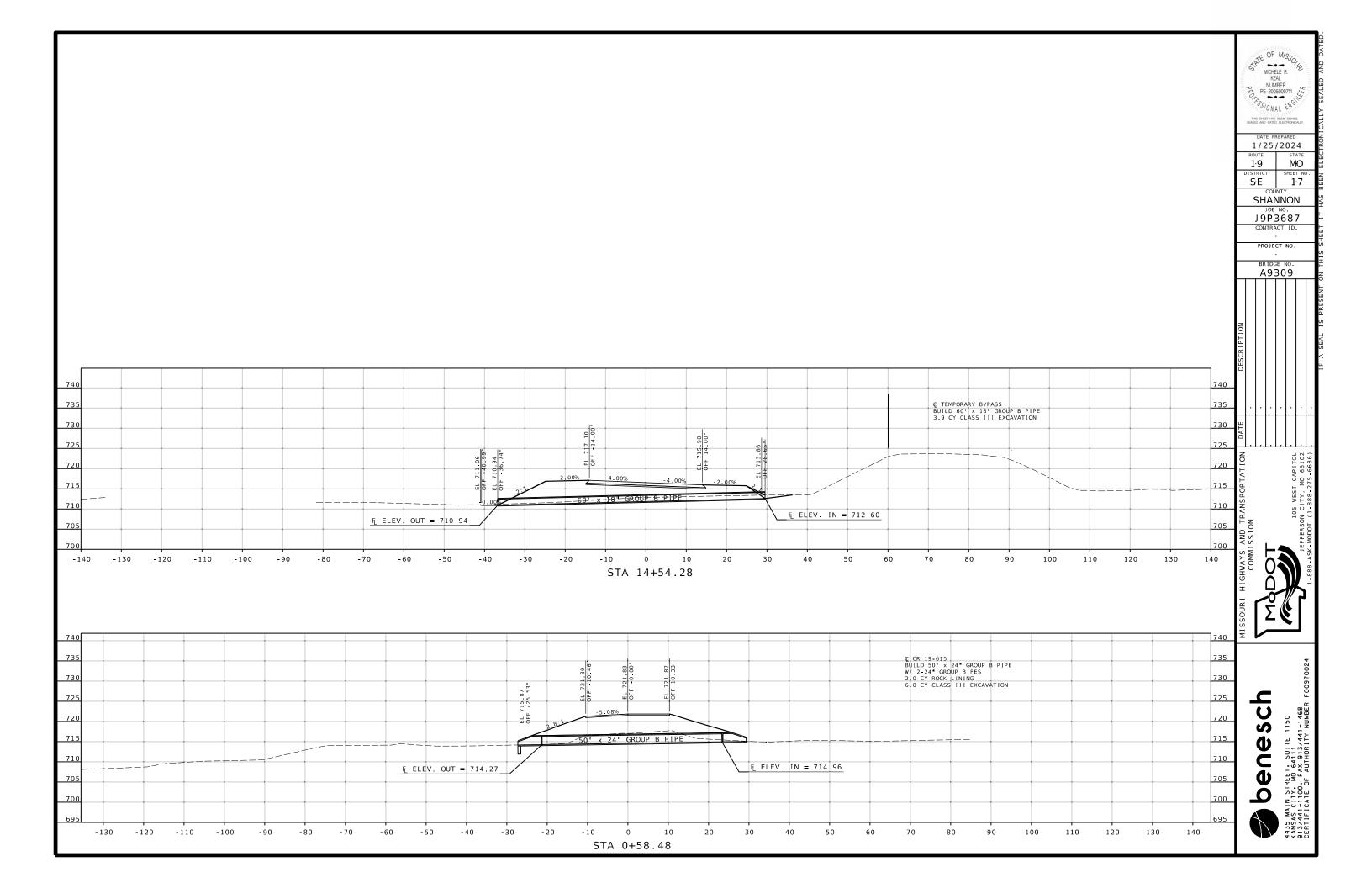
\* ORANGE, YELLOW & YELLOW/GREEN

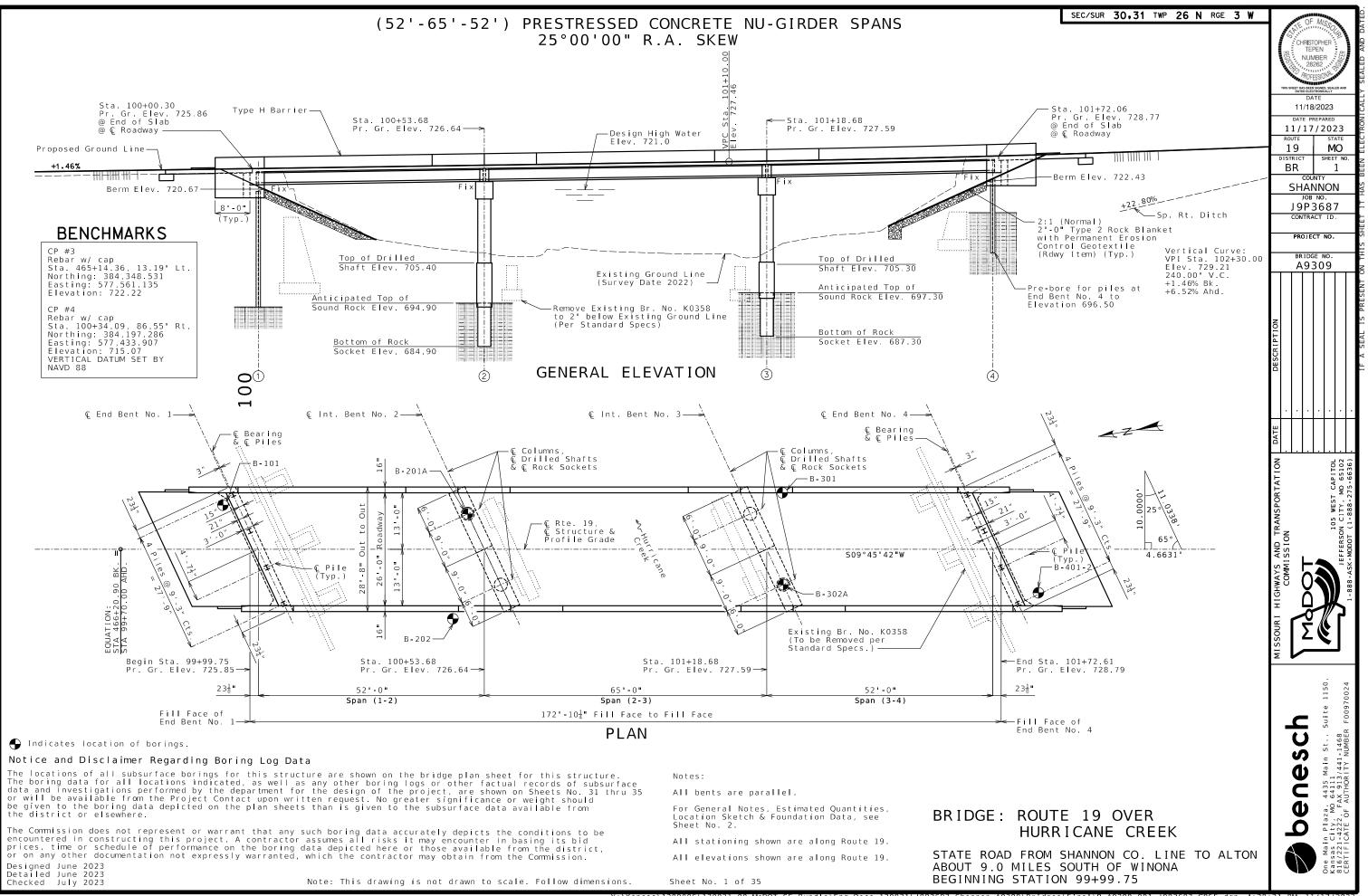












	Estimated	Quantiti	es		
	I t em		Substr.	Superstr	Total
	Class 1 Excavation	cu. yard	80		80
	Bridge Removal (K0358)	lump sum			1
	Bridge Approach Slab (Minor Road)	sq. yard		118	118
	Drilled Shafts (3'-6" O.D.)	linear foot	37.0		37.0
	Rock Sockets (3'-0" Dia.)	linear foot	40.0		40.0
	Video Camera Inspection	each	4		4
	Foundation Inspection Holes	linear foot	80.0		80.0
	Sonic logging Testing	each	4		4
*	Galvanized Structural Steel Piles (12 in)	linear foot	216		216
	Pre-Bore for Piling	linear foot	100		100
	Pile Point Reinforcement	each	8		8
	Class B Concrete (Substructure)	cu. yard	78.2		78.2
	Type H Barrier	linear foot		378	378
**	Slab on Concrete NU-Girder	sq. yard		547	547
	NU 35, Prestressed Concrete NU-Girder	linear foot		506	506
	Reinforcing Steel (Bridges)	pounds	18,070		18,070
	Slab Drain	each		12	12
	Vertical Drain at End Bents	each			2
	Laminated Neoprene Bearing Pad	each		9	9
	Laminated Neoprene Bearing Pad (Tapered)	each		9	9

Hydrologic Data
Drainage Area = 54.1 mi'
Design Flood Frequency = 50 years
Design Flood Discharge = 12,800 cfs
Design Flood (D.F.) Elevation = 721.0
Base Flood (100-year)
Base Flood Elevation = 721.9
Base Flood Discharge = 15,000 cfs
Estimated Backwater = 1.8 ft
Average Velocity thru Opening = 10.0 ft/s
Freeboard (50-year)
Freeboard = 1.4 ft
Roadway Overtopping
Overtopping Flood Discharge = >17,000 cfs
Overtopping Flood Frequency = 200 years
200-Year Flood Elevation = 723.7

- \* Cost of L4x4 ASTM A709 Grade 36 HP pile anchors and 3/4-inch diameter ASTM F3125 Grade A325 Type 1 bolts, complete in place, will be considered completely covered by the contract unit price for Galvanized Structural Steel Piles (12 in.).
- \*\* Type H Barrier shall be cast-in-place or slip-form option.

All concrete above the construction joint in the end bents is included in the Estimated Quantities for Slab on Concrete NU-Girder.

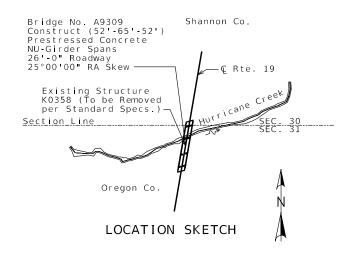
All reinforcement in the end bents is included in the Estimated Quantities for Slab on Concrete NU-Girder.

Note: This drawing is not drawn to scale. Follow dimensions.

All reinforcement in the intermediate bent concrete diaphragms except reinforcement embedded in the beam cap is included in the Estimated Quantities for Slab on Concrete NU-Girder.

All concrete above the intermediate beam cap is included in the Estimated Quantities for Slab on Concrete NU-Girder.

	Foundat	ion Data			
			Bent I	Number	
Туре	Design Data	1	2	3	4
	Pile Type and Size	HP 12×53			HP 12×53
	Number ea	4			4
	Approximate Length Per Each ft	27			27
	Pile Point Reinforcement ea	AII			AII
	Min. Galvanized Penetration (Elev.) ft	Full Length			Full Length
Load	Est. Max. Scour Depth 100 (Elev.) ft	714			711
Bearing Pile	Minimum Tip Penetration (Elev.) ft	694			697
' ' ' '	Criteria for Min. Tip Penetration	Min. Embed.			Min. Embed.
	Pile Driving Verification Method	DF			DF
	Resistance Factor	0.4			0.4
	Minimum Nominal Axial Compressive Resistance kip	345.0			345.0
	Number ea		2	2	
	Foundation Material		Strong Rock	Strong Rock	
Rock	Elevation Range ft		692-668	694-662	
Socket	Minimum Nominal Axial Compressive Resistance (Side Resistance) ksf		26	26	
	Minimum Nominal Axial Compressive Resistance (Tip Resistance) ksf		220	220	



Sheet No. 2 of 35

DF = FHWA-modified Gates Dynamic Pile Formula

Load Bearing Pile

Minimum Nominal Axial Compressive Resistance = Maximum Factored Loads
Resistance Factor

Rock Socket (Drilled Shafts):

Minimum Nominal Axial Compressive Resistance = <u>Maximum Factored Loads</u>
(Side Resistance + Tip Resistance)

Resistance Factor

Prebore for piles at Bent No. 4 to Elevation 696.50.

All piles shall be galvanized to down to the minimum galvanized penetration (elevation).

Designed June 2023 Detailed June 2023 Checked July 2023 Pile Point reinforcement need not be galvanized. Shop drawings will not be required for pile point reinforcement.

The contractor shall make every effort to achieve the minimum galvanized penetration (elevation) shown on the plans for all piles. Deviations in penetration less than 5 feet of the minimum will be considered acceptable provided the contractor makes the necessary corrections to ensure the minimum penetration is achieved on subsequent piles.

Thickness of permanent steel casing shall be as shown on the plans and in accordance with Sec. 701.

Sonic logging testing shall be performed on all drilled shafts and rock sockets.

HP piles are anticipated to be driven to refusal on rock. Review all borings for depth of rock and restrict driving as appropriate to comply with hard rock driving criteria in accordance with Sec. 702.

Estimated Quantities for Slab on Concrete NU-Girder

3145 011 601161616 110 611461	
I t em	Total
Class B-2 Concrete cu. yard	145.0
Reinforcing Steel (Epoxy Coated) pound	42,650

The table of Estimated Quantities for Slab on Concrete NU-Girder represents the quantities used by the State in preparing the cost estimate for concrete slabs. The area of the concrete slab will be measured to the nearest square yard longitudinally from end of slab to end of slab and transversely from out to out of bridge slab (or with the horizontal dimensions as shown on the plan of slab). Payment for prestressed panels, conventional forms, all concrete and epoxy coated reinforcing steel will be considered completely covered by the contract unit price for the slab. Variations may be encountered in the estimated quantities but the variations cannot be used for an adjustment in the contract unit price.

Method of forming the slab shall be as shown on the plans and in accordance with Sec 703. All hardware for forming the slab to be left in place as a permanent part of the structure shall be coated in accordance with ASTM A123 or ASTM B633 with a thickness class SC 4 and a finish type 1, 11 or 111.

The Estimated Quantities for Slab on Concrete NU-Girder are based on skewed precast prestressed end panels.

Class B-2 Concrete quantity is based on minimum top flange thickness and minimum joint material thickness.

The prestressed panel quantities are not included in the table of Estimated Quantities for Slab on Concrete NU-Girder.

### General Notes:

Design Specifications:
2020 AASHTO LRFD Bridge Design Specifications (9th Ed.)
2011 AASHTO Guide Specifications for LRFD Seismic Bridge Design
(2nd Ed.) and 2014 Interim Revisions (Seismic Details)
Seismic Design Category = B
Design earthquake response spectral accleration coefficient at
1.0 second period, SD1 = 0.252g
Acceleration Coefficient (effective peak ground acceleration
coefficient), As = 0.247g

Design Loading:
 Vehicular = HL-93
 Future Wearing Surface = 35 lb/sf
 Earth = 120 lb/cf
 Equivalent Fluid Pressure = 45 lb/cf
 Superstructure: Simply-Supported, Non-Composite for dead load.
 Continuous Composite for live load.

Design Unit Stresses:
Class B Concrete (Substructure, except Drilled f'c = 3,000 psi
Shafts & Rock Sockets)

Class B-2 Concrete (Drilled Shafts & Rock f'c = 4,000 psi Sockets)

Class B-2 Concrete (Superstructure, except f'c = 4,000 psi

Prestressed NU-Girders and Type H Barrier)

Class B-1 Concrete (Type H Barrier) f'c = 4,000 psi

Structural Steel HP Pile (ASTM A709 Grade 50S) fy = 50,000 psi

For precast prestressed panel stresses, see Sheet No. 20.

For prestressed NU-Girder stresses, see Sheets No. 15 thru 18.

Laminated Neoprene Bearing Pads shall be 60 durometer and shall be in accordance with Sec. 716.

Laminated Neoprene Bearing Pads (Tapered) shall be 60 durometer and shall be in accordance with Sec. 716.

oint Filler: All joint filler shall be in accordance with Sec 1057 for preformed sponge rubber expansion and partition joint filler, except as noted.

Reinforcing Steel: Minimum clearance to reinforcing steel shall be  $1\frac{1}{2}$ ",

Traffic Handling: Structure to be closed during construction. Traffic to be maintained on Temporary Bypass during construction. See Roadway Plans for traffic control and for low water bypass

Roadway Plans for traffic control and for low water bypass crossing details.

MoDOT Construction personnel will indicate the type of joint filler option used under the precast panels for this structure:

Constant Joint Filler

Variable Joint Filler

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unless otherwise shown.

Reinforcing Steel (Grade 60)

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BR 2 COUNTY SHANNON

SHEET N

ЈОВ NO. Ј**9**Р3687

PROJECT NO.

BRIDGE NO.
A9309

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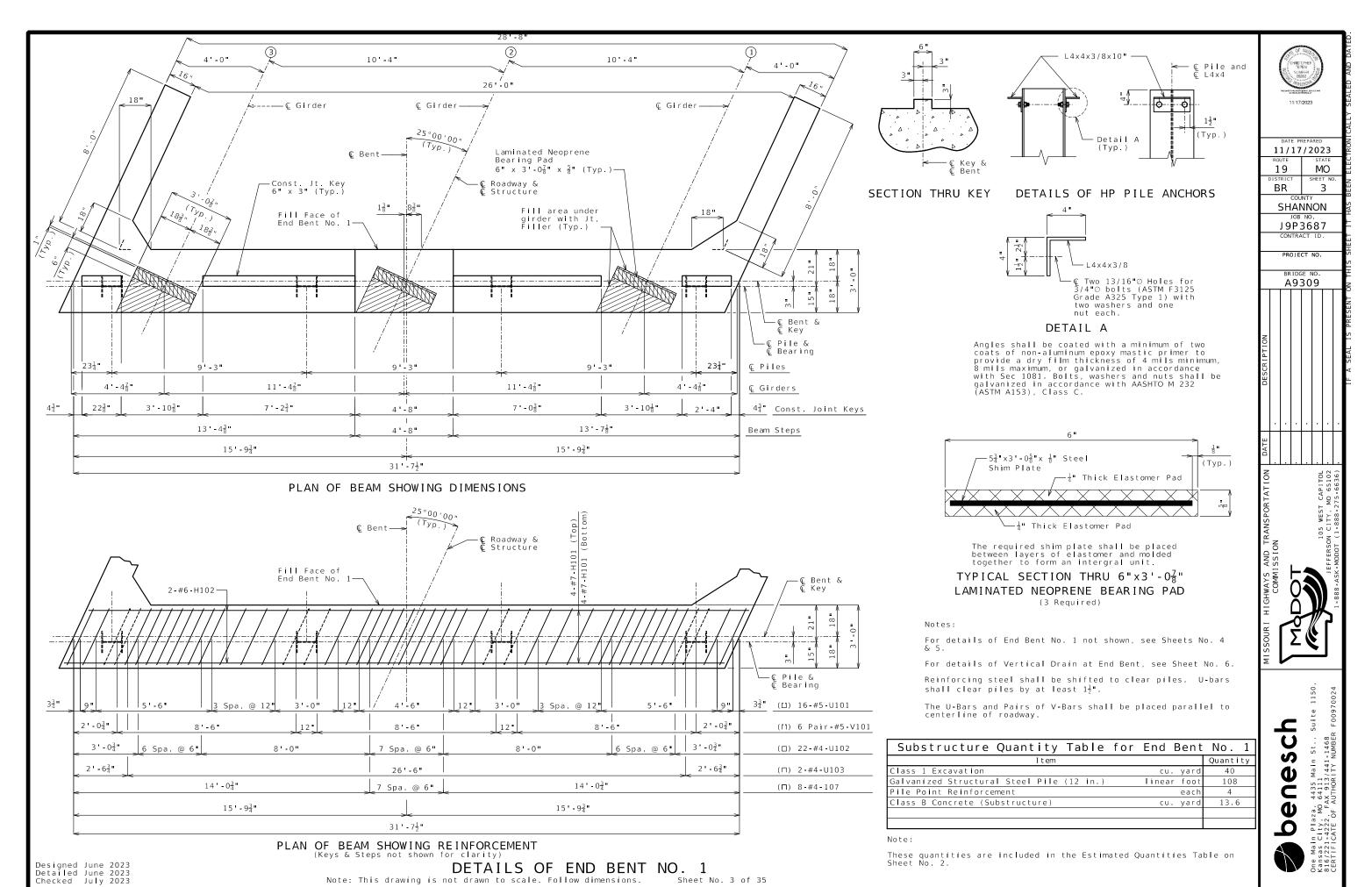
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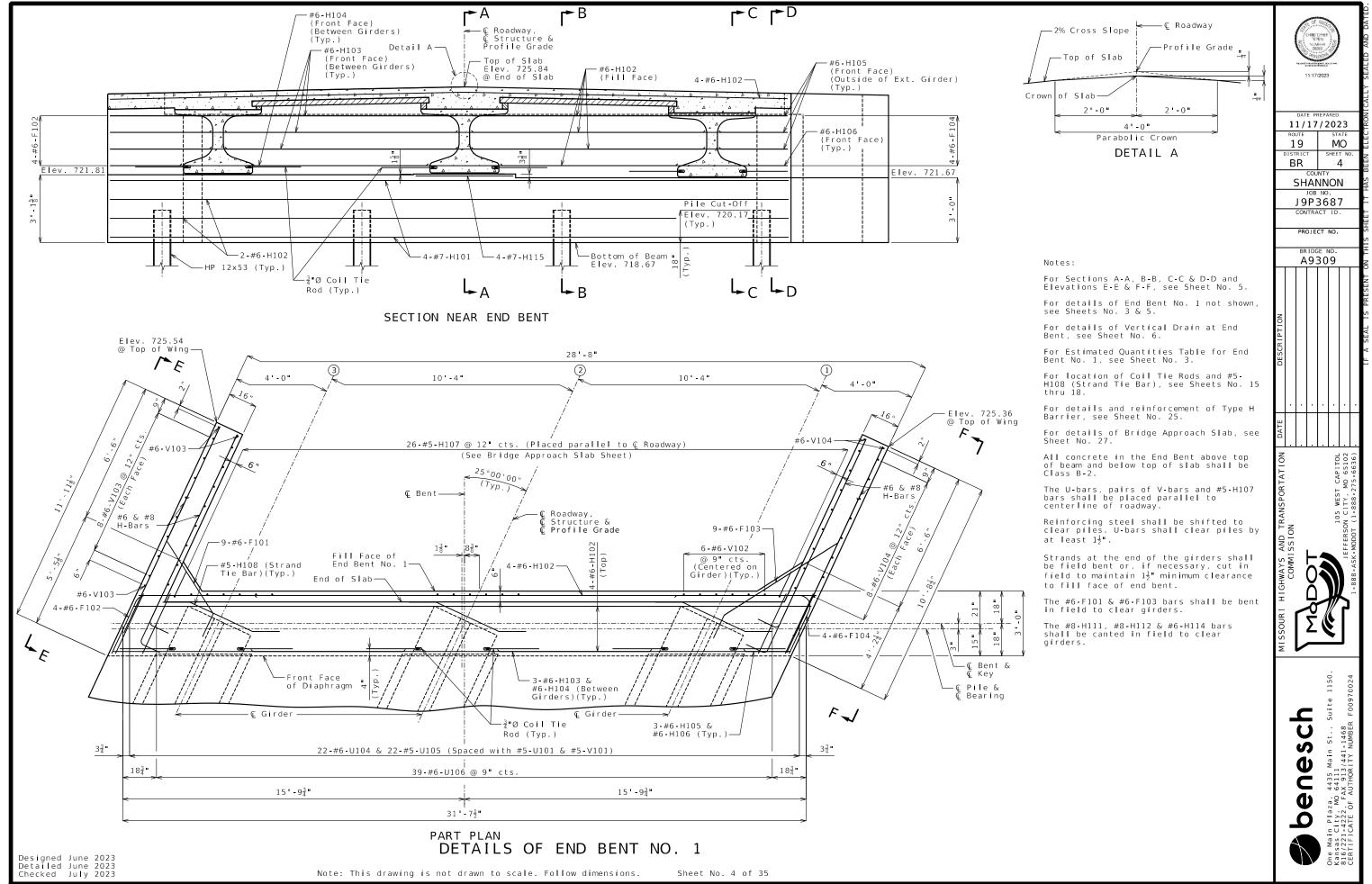
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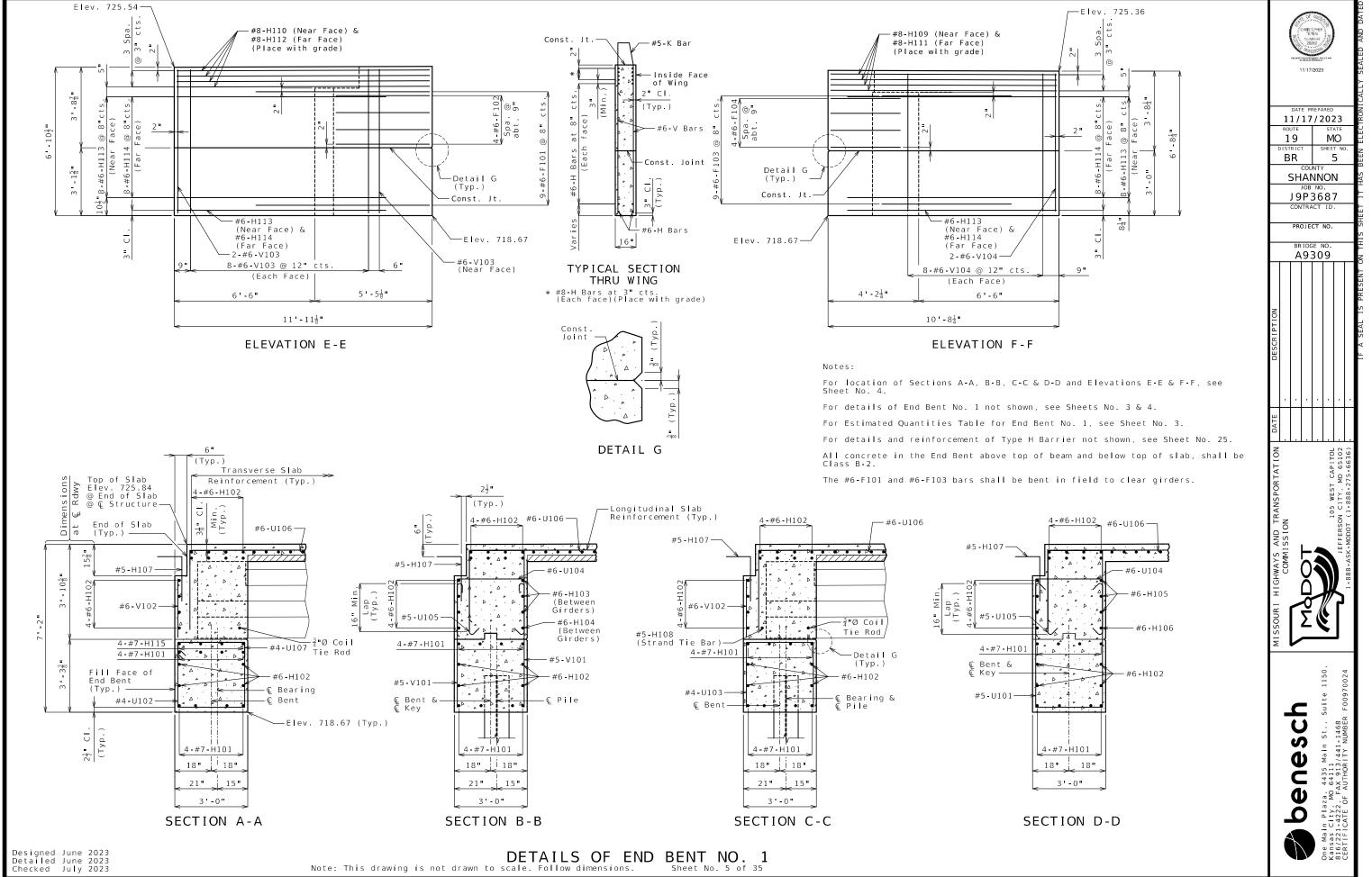
MISSOURI HIGHWAYS AND TRANS
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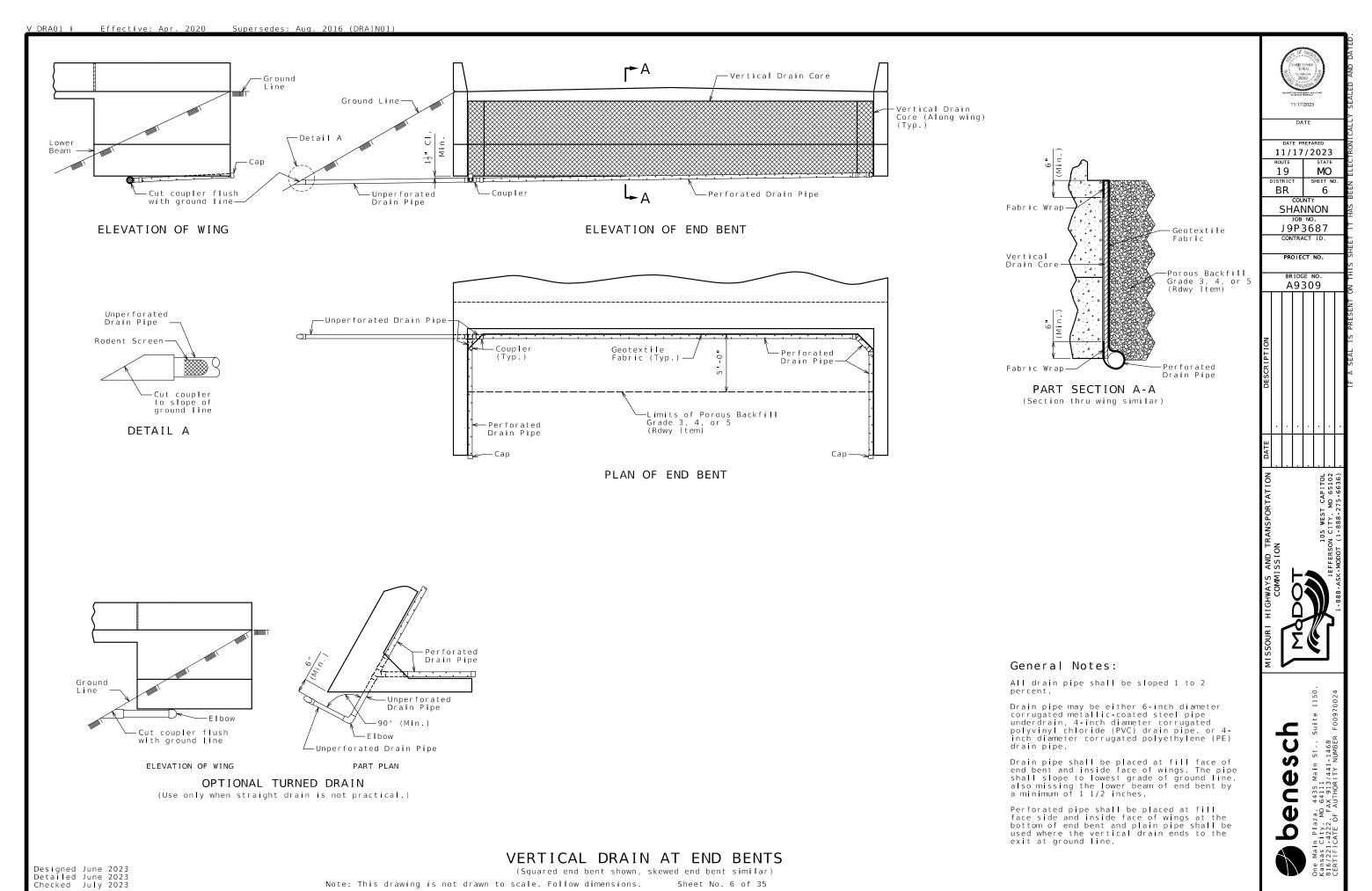
fy = 60,000 psi

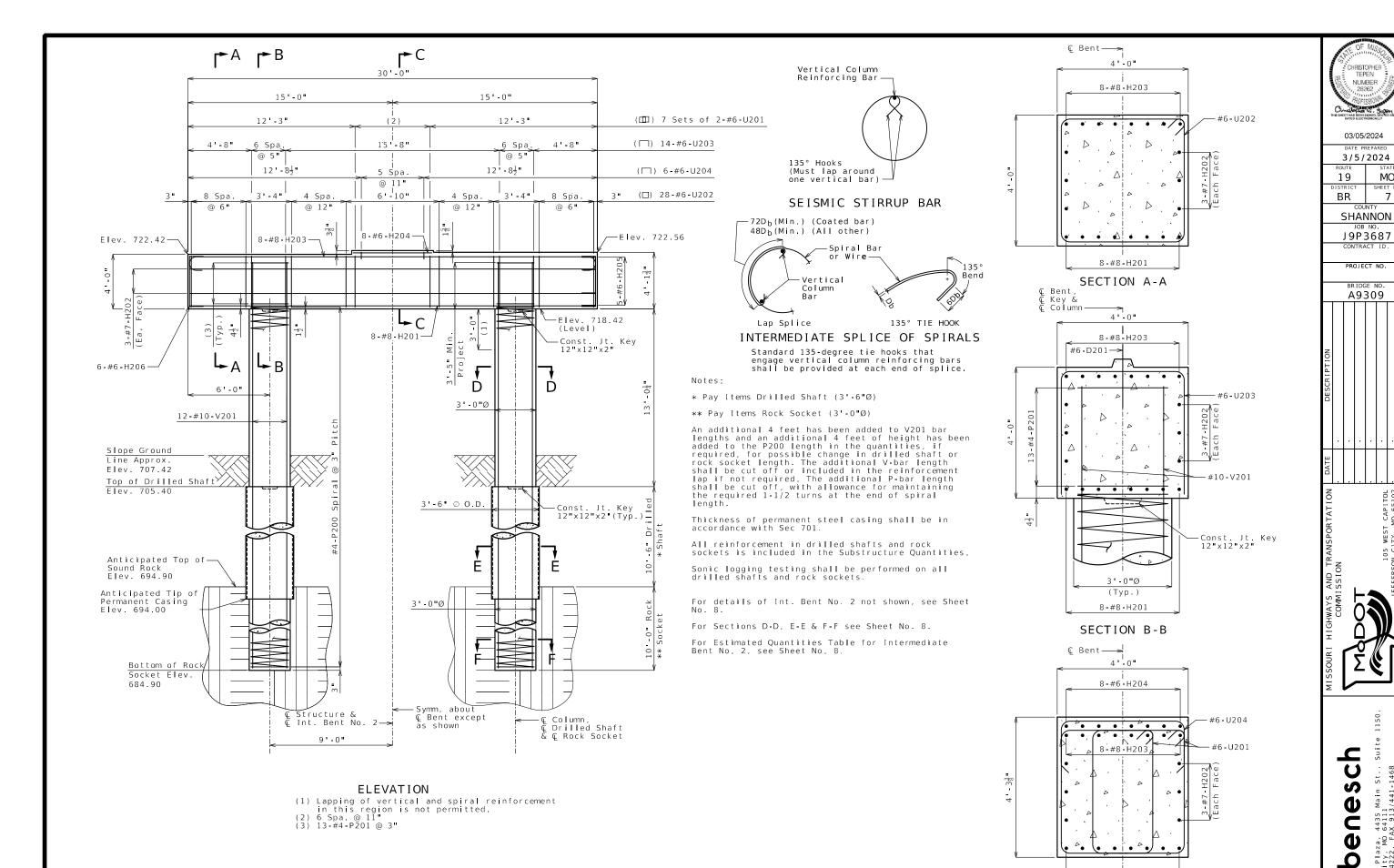
benesch Main Plaza, 4435 Main St., Suite 11











DETAILS OF INTERMEDIATE BENT NO. 2

Designed June 2023 Detailed June 2023 Checked July 2023

8-#8-H201

SECTION C-C

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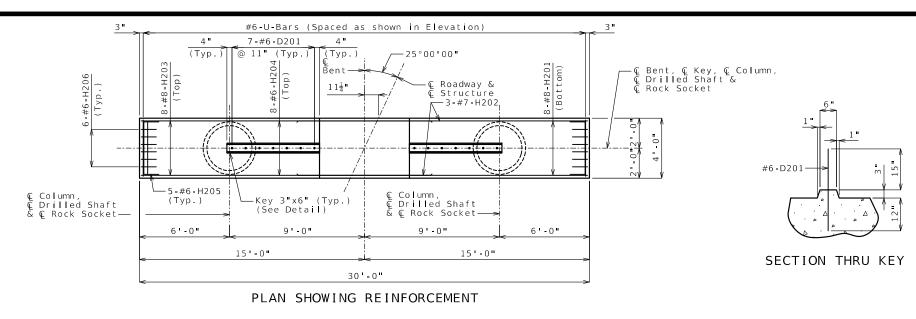
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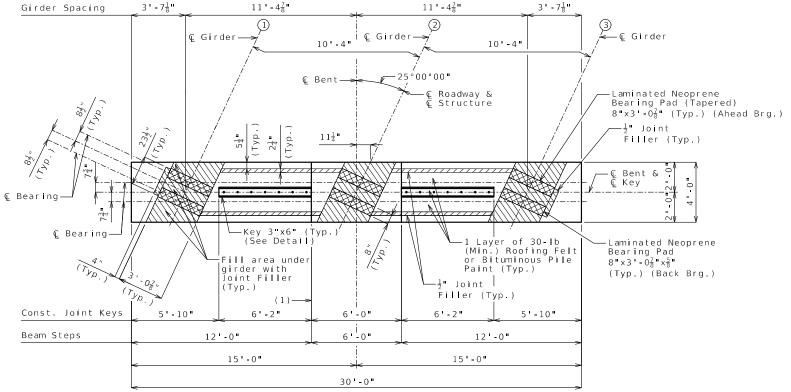
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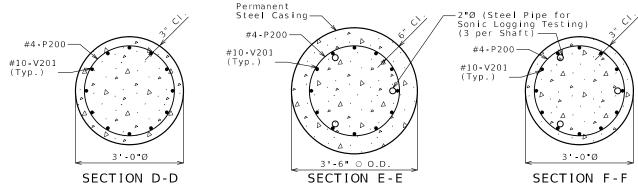
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## PLAN OF BEAM

(1) For steps 2° or more, use  $2\frac{1}{4}$ " $x\frac{1}{2}$ " Joint Filler up Vertical Face (Typ.)



Note: This drawing is not drawn to scale. Follow dimensions.

DETAILS OF INTERMEDIATE BENT NO. 2

Sheet No. 8 of 35

Substructure Quantity Table for Int. Bent No. 2 Quantity I t em linear f Rock Socket (3'-0" Dia.) linear ft 20.0 Video Camera Inspection eacl Foundation Inspection Holes linear ft 40.0 Sonic Logging Testing eac Class B Concrete (Substructure) cu. yar 25.1 Reinforcing Steel (Bridges) 9,240 poun

For details of Int. Bent No. 2 not shown, see Sheet No. 7.

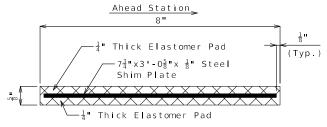
Notes:

These quantities are included in the Estimated Quantities Table on Sheet No. 2.

Ahead Station\_ 8" ¼" Thick Elastomer Pad (Typ.) ·7¾"x3'-0½" Tapered Steel Shim Plate - 1 Thick Elastomer Pad The required shim plate shall be placed between layers of elastomer and molded together to form an intergral unit.

(SPAN 2-3)

TYPICAL SECTION THRU  $8"x3'-0\frac{7}{8}"$  LAMINATED NEOPRENE BEARING PAD (TAPERED) (AHEAD BRG.)



The required shim plate shall be placed between layers of elastomer and molded together to form an intergral unit.

(SPAN 1-2)

TYPICAL SECTION THRU 8"x3'-07" LAMINATED NEOPRENE BEARING PAD (BACK BRG.)



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SHANNON

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PROJECT NO.

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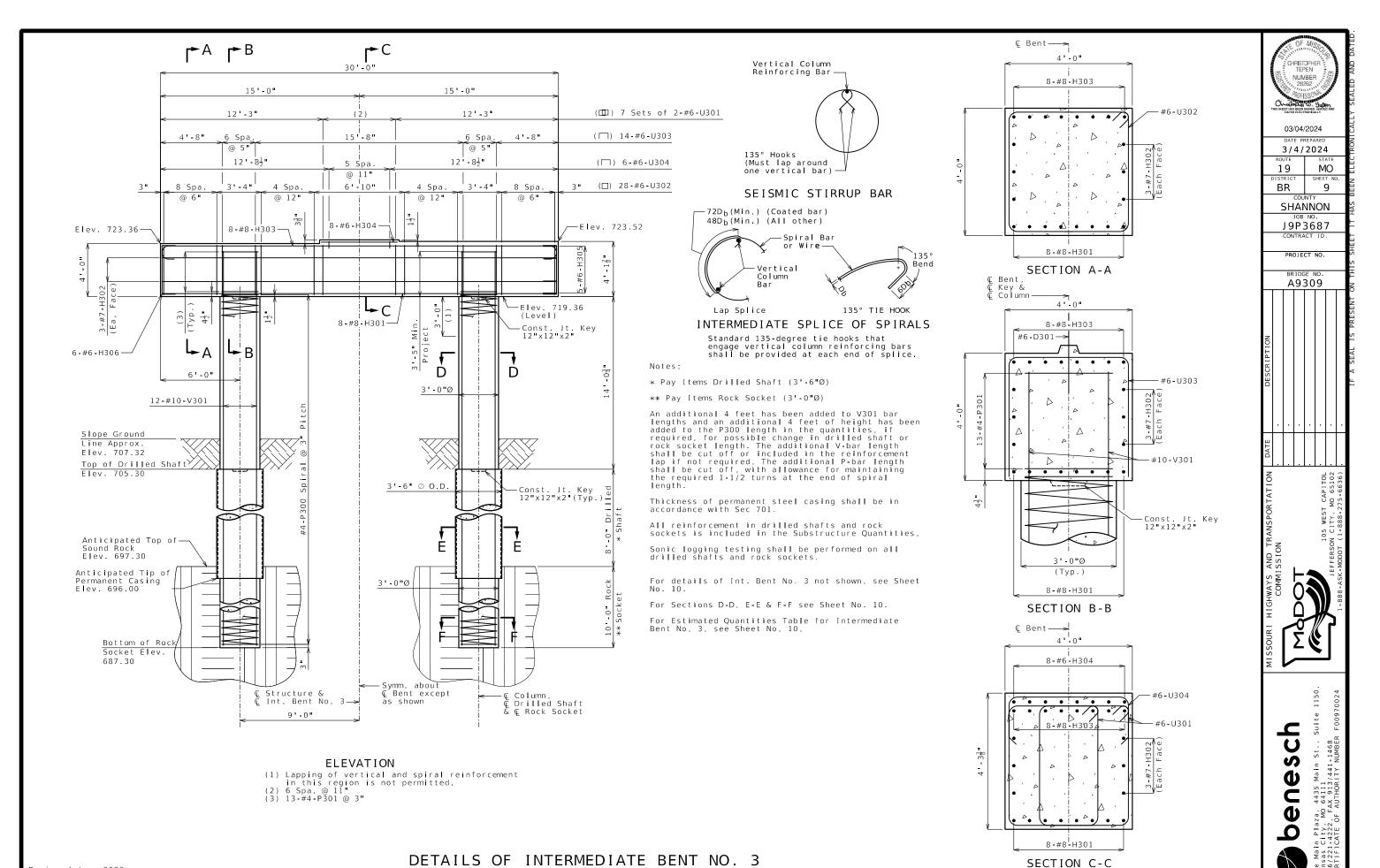
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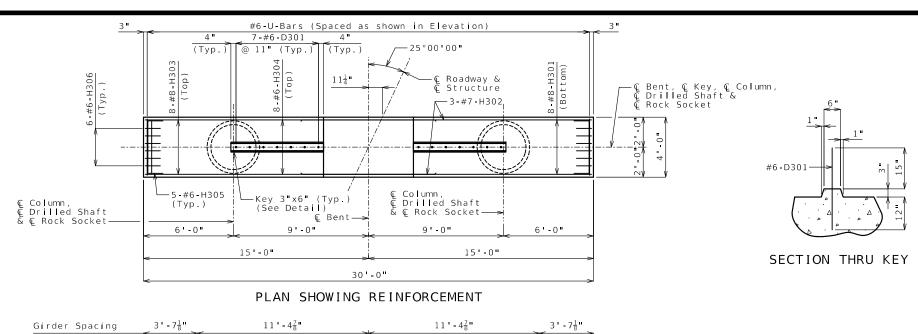
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Designed June 2023 Detailed June 2023 Checked July 2023



Designed June 2023 Detailed June 2023 Checked July 2023



#### 2 −Ç Girder Ç Girder— 10'-4" -25°00'00" € Bent— -Laminated Neoprene Roadway & € Structure Bearing Pad (Tapered) $8"x3'-0\frac{7}{8}"$ (Typ.)(Ahead Brg.) -1 Joint $11\frac{1}{4}$ " Filler (Typ.) Ç̃ Key € Bearing −Key 3"x6" (1 (See Detail) € Bearing-(Typ.) - Laminated Neoprene Bearing Pad 8"x3'-0<sup>7</sup>8"x<sup>5</sup>8" ·1 Laver of 30-lb (Min.) Roofing Felt ill area unde or Bituminous Pile Paint (Typ.) girder with Joint Filler (Typ.)(Back Brg.) —∃" Joint | (Typ.) Filler (Typ.) (1)-Const. Joint Keys 6'-2" 6'-0" 6'-2" 5'-10" 6'-0" Beam Steps 12'-0" 12'-0"

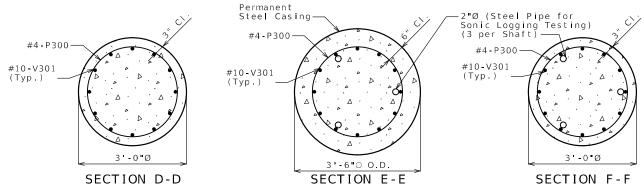
# PLAN OF BEAM

(1) For steps 2° or more, use 2¼"x½" Joint Filler up Vertical Face (Typ.)

15'-0"

Designed June 2023 Detailed June 2023

Checked July 2023

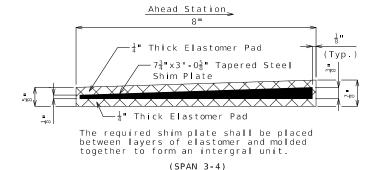


15'-0"

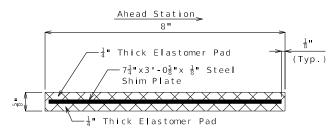
## DETAILS OF INTERMEDIATE BENT NO. 3

Note: This drawing is not drawn to scale. Follow dimensions.

Sheet No. 10 of 35



TYPICAL SECTION THRU  $8"x3'-0\frac{7}{8}"$  LAMINATED NEOPRENE BEARING PAD (TAPERED)(AHEAD BRG.)



The required shim plate shall be placed between layers of elastomer and molded together to form an intergral unit.

(SPAN 2-3)

TYPICAL SECTION THRU 8"x3'-078" LAMINATED NEOPRENE BEARING PAD (BACK BRG.)

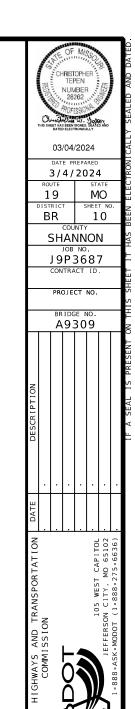
#### Notes:

For details of Int. Bent No. 3 not shown, see Sheet No. 9.

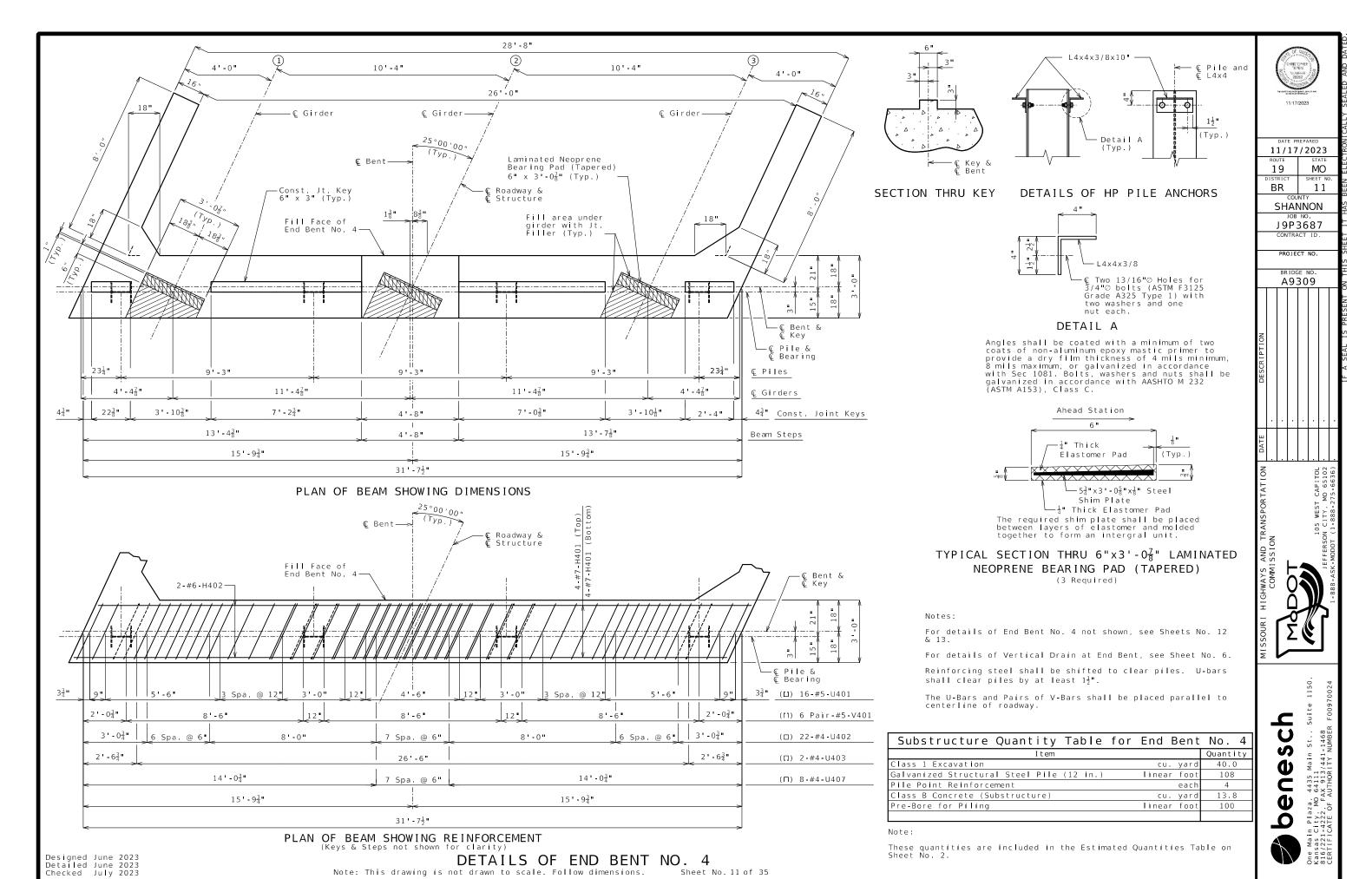
Substructure Quantity Table	for Int. Bent	t No. 3
I t em		Quantity
Drilled Shaft (3'-6" Dia.)	linear ft.	16.0
Rock Socket (3'-0" Dia.)	linear ft.	20.0
Video Camera Inspection	each	2
Foundation Inspection Holes	linear ft.	40.0
Sonic Logging Testing	each	2
Class B Concrete (Substructure)	cu. yard	25.7
Reinforcing Steel (Bridges)	pound	9,030

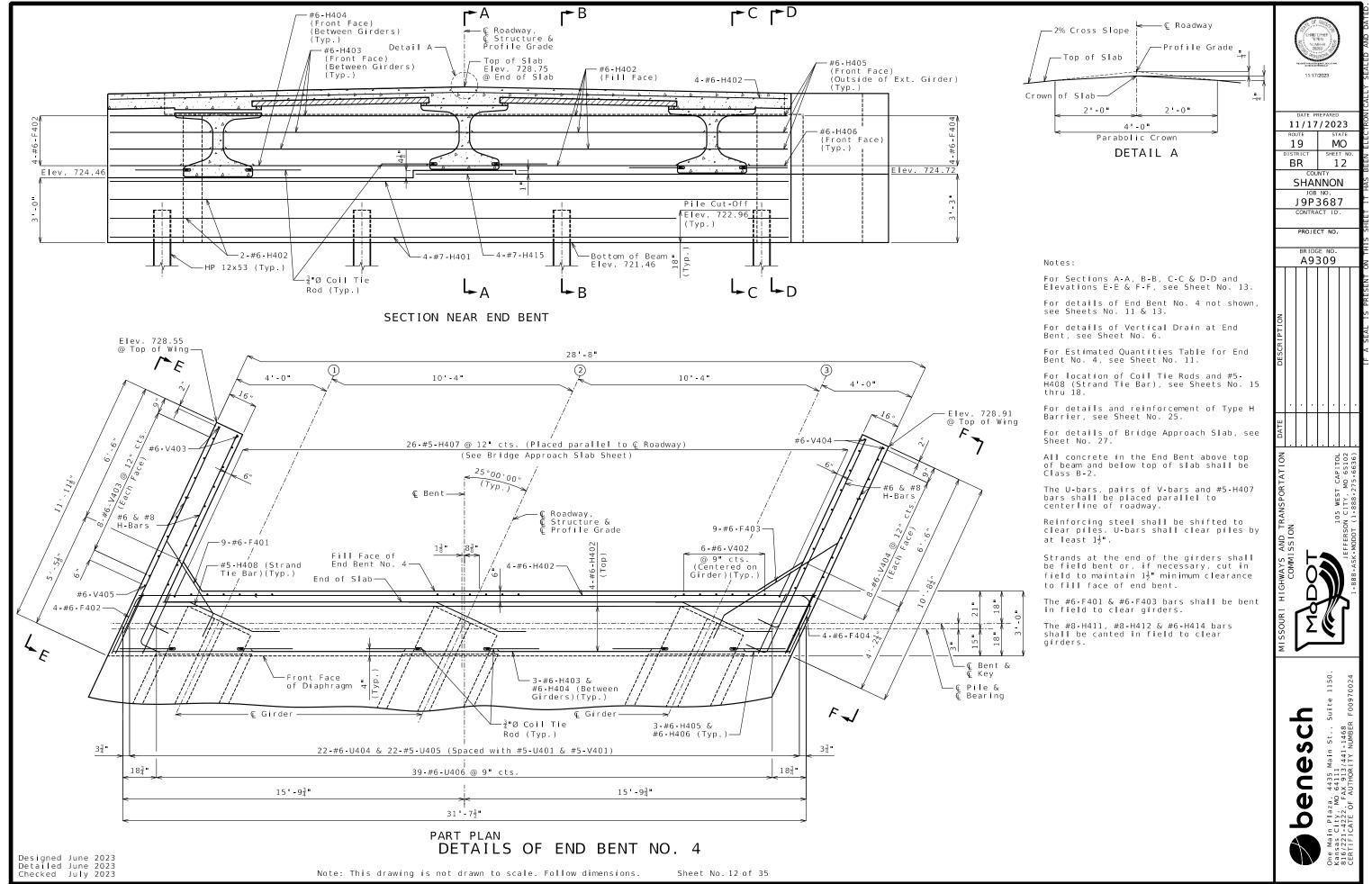
Note:

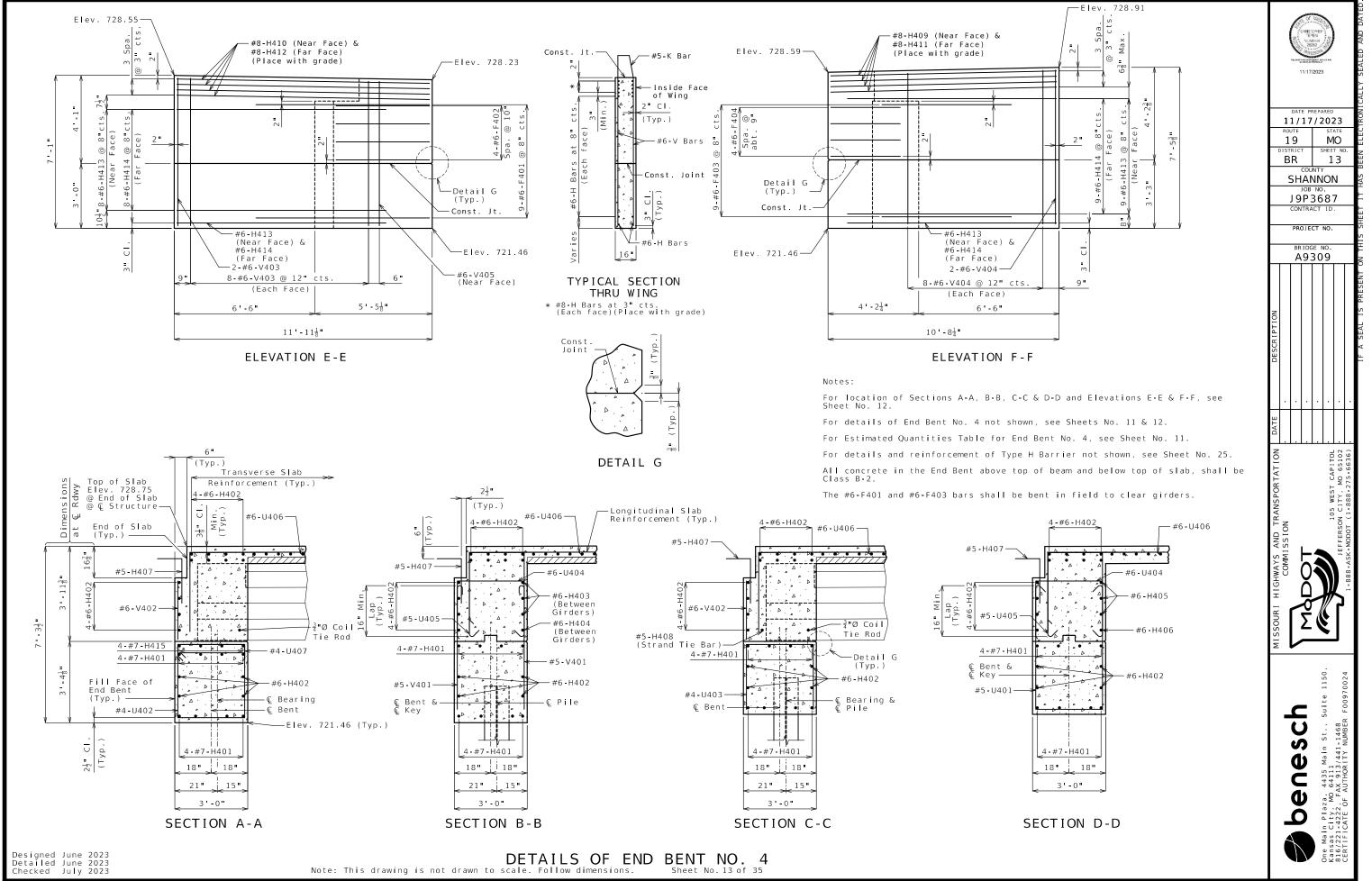
These quantities are included in the Estimated Quantities Table on Sheet No. 2.



one Main Plaza, 4435 Main St., Suite 1150, Kanas City, MO 6411, MO 1141468 E16721, 422, FAS 913, 441-1468 E1671F1CATE OF AUTHORITY NUMBER F00970024









SHANNON

JOB NO.

J9P3687

CONTRACT ID.

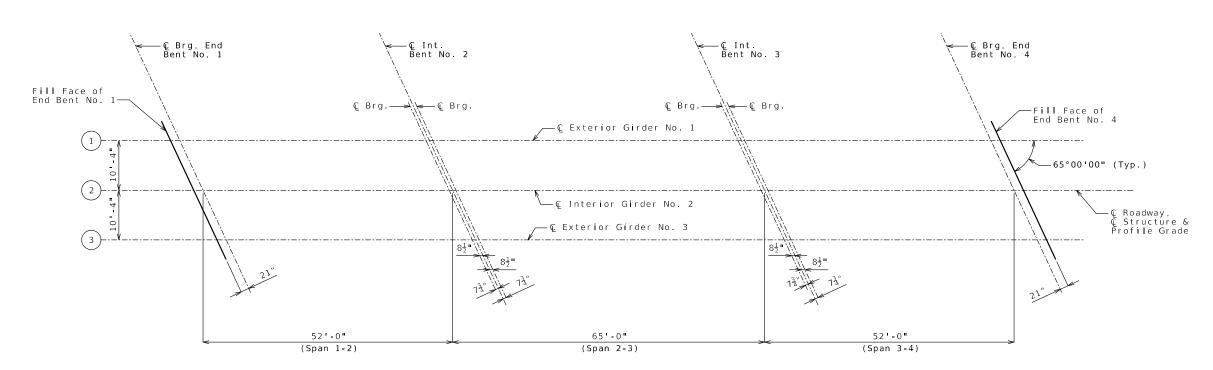
PROJECT NO.

BRIDGE NO.
A9309

HIGHWAYS AND TRANSPORTATION
COMMISSION
105 WEST CAPITOL

benesch Main Plaza, 4435 Main St., Suite 1150,

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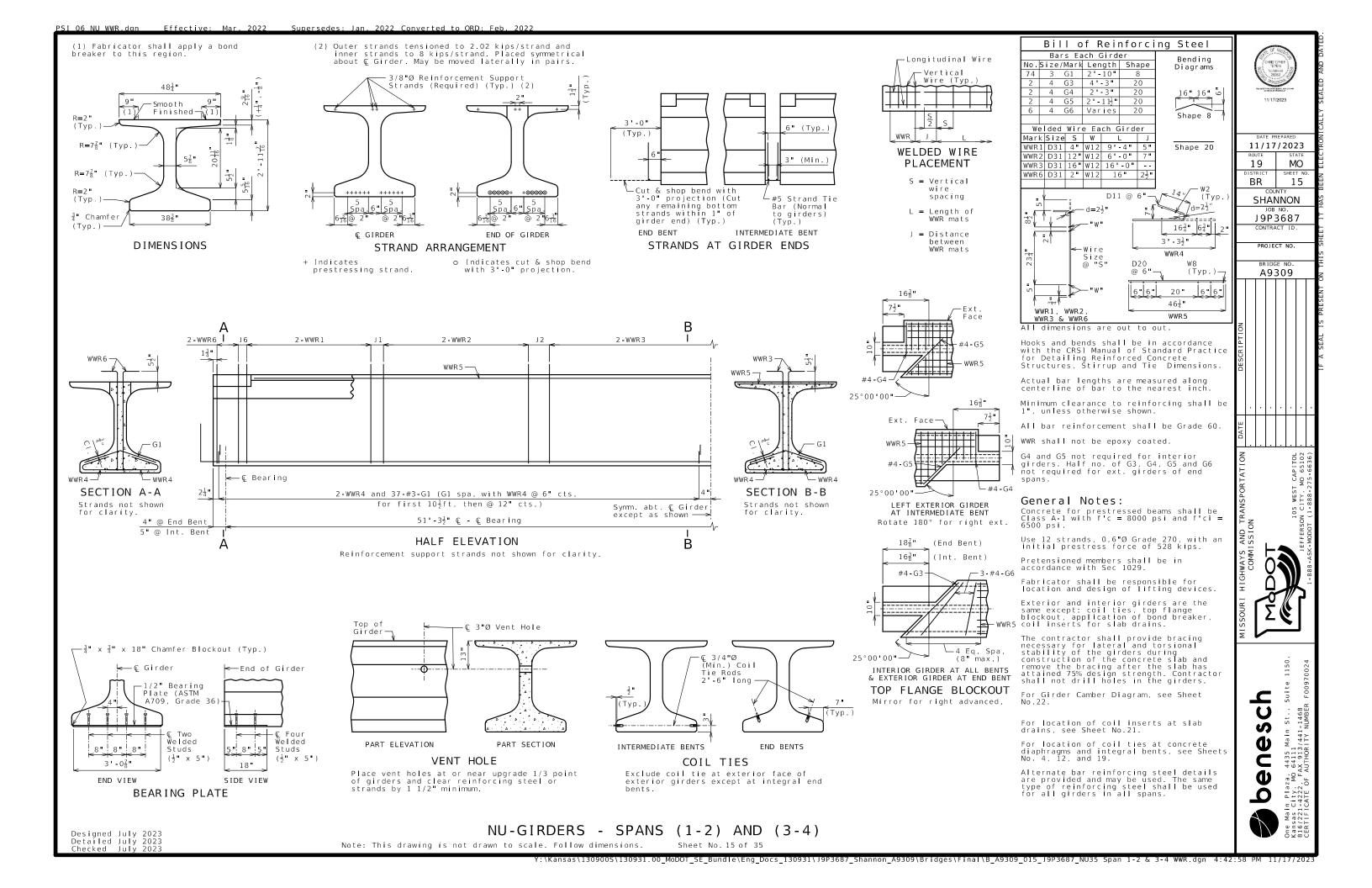
LAYOUT PLAN

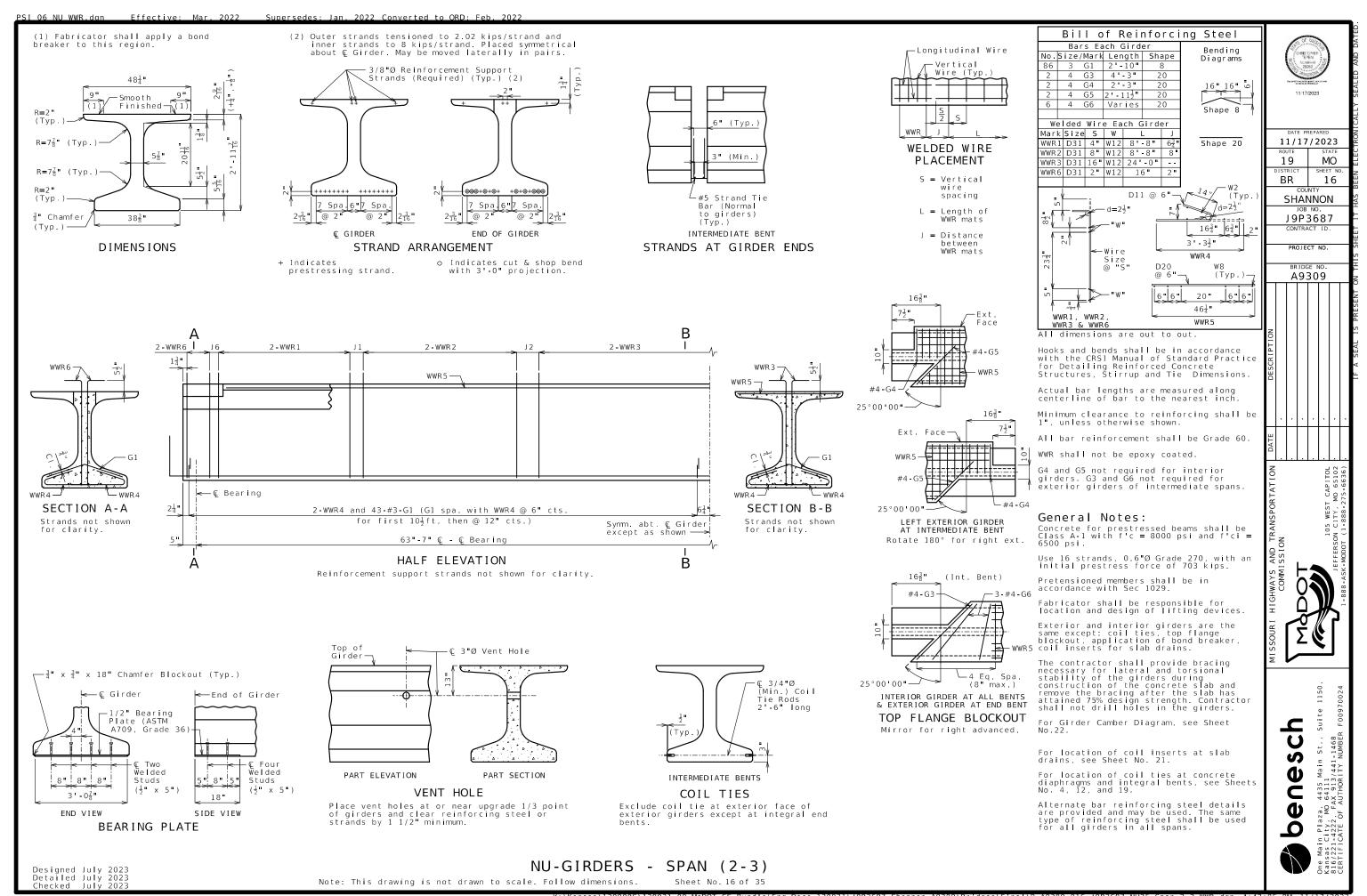
Note

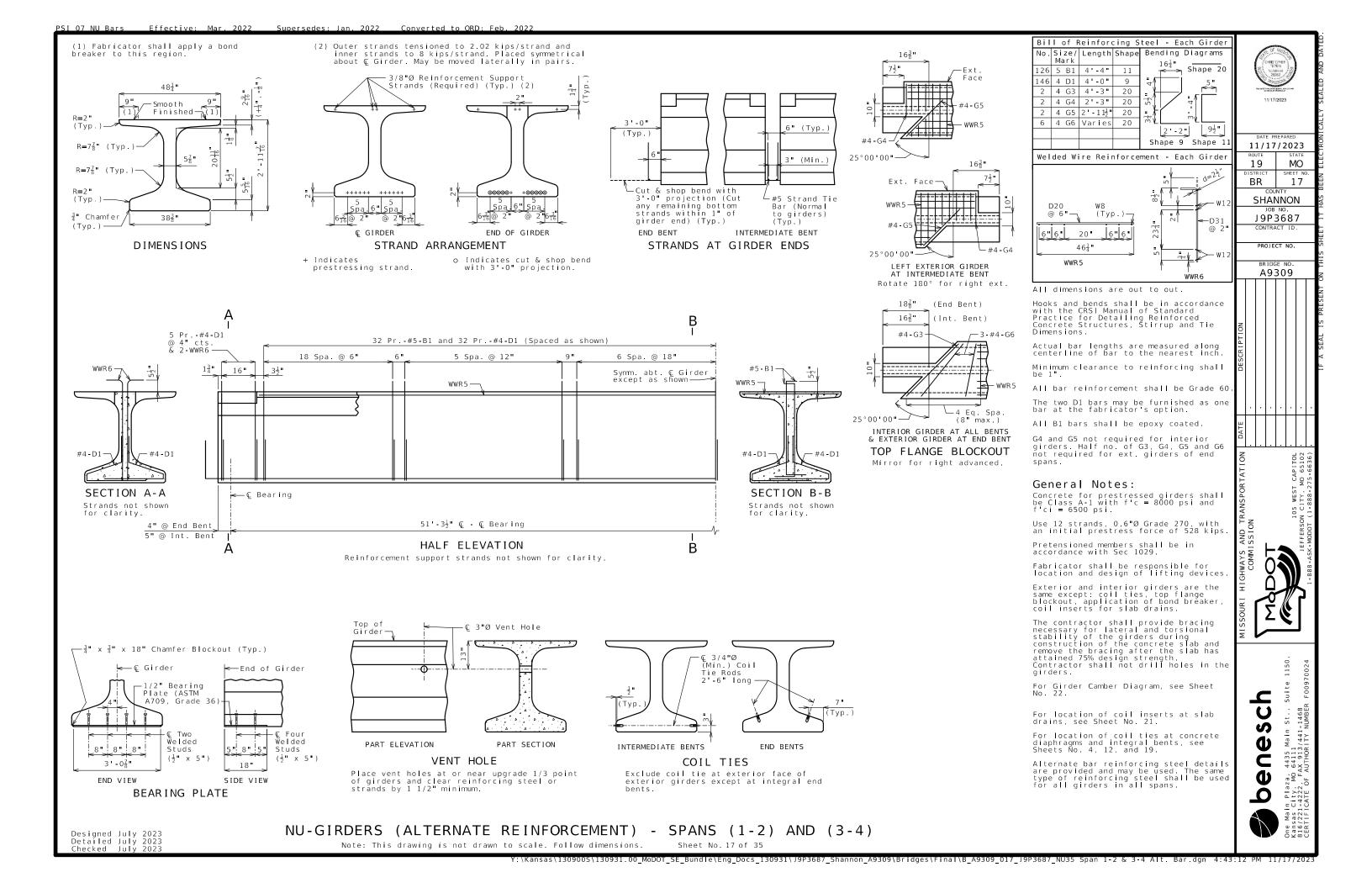
All dimensions are measured horizontal.

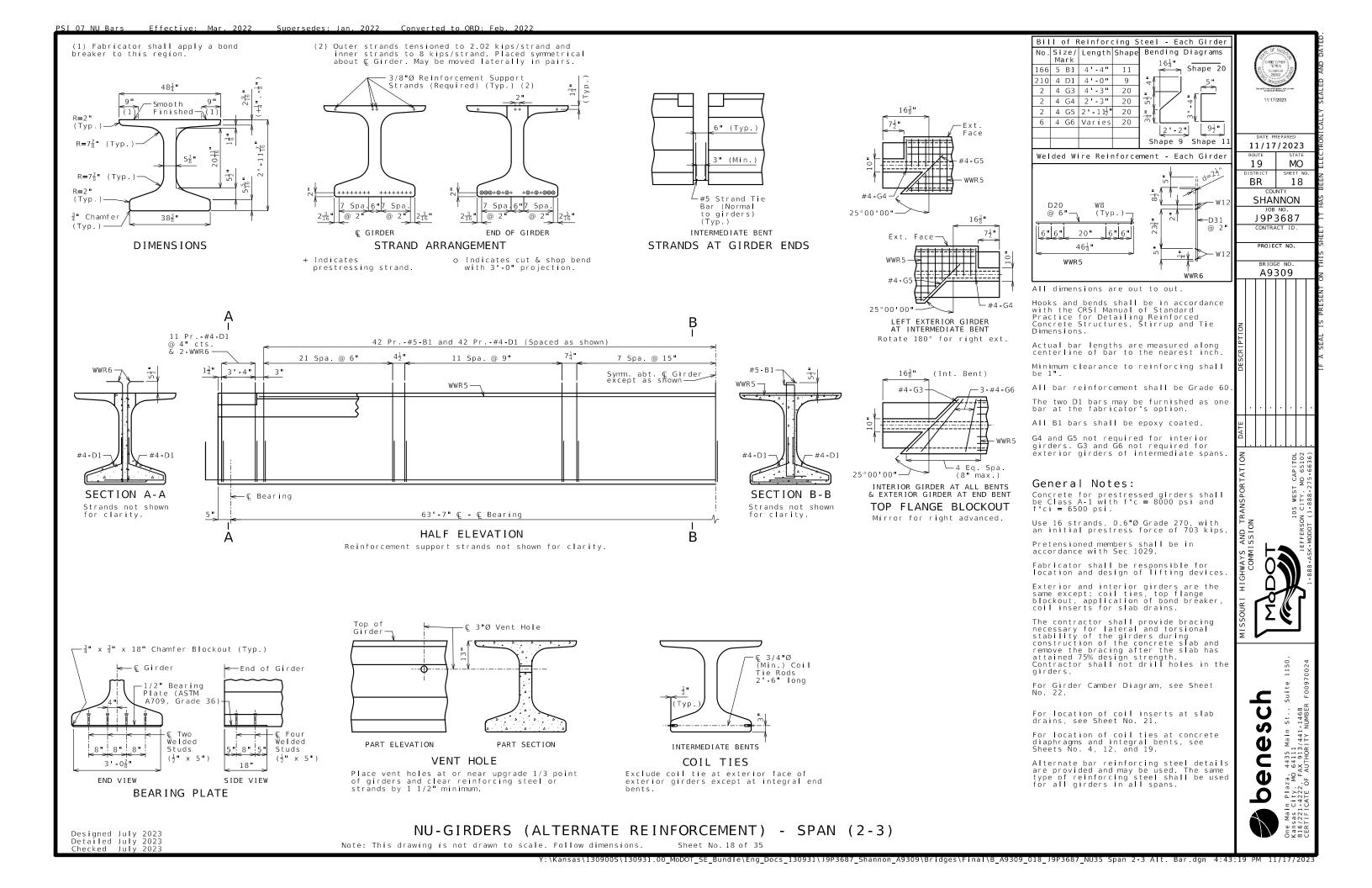
PLAN SHOWING PRESTRESSED CONCRETE NU-35 GIRDER LAYOUT

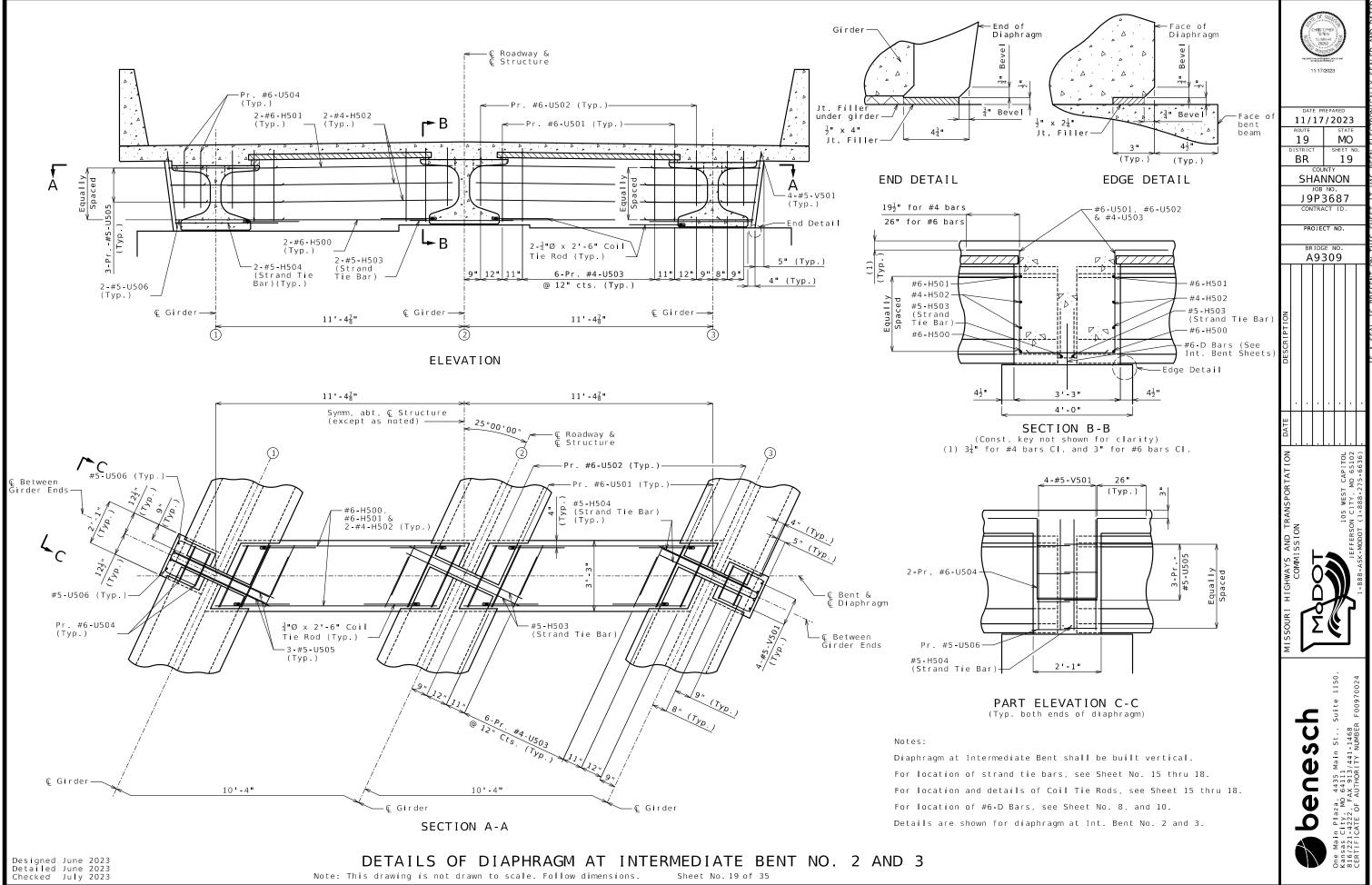
Designed June 2023 Detailed June 2023 Checked July 2023



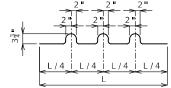






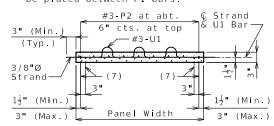


#### SQUARED END PANELS OR TRUNCATED END PANELS

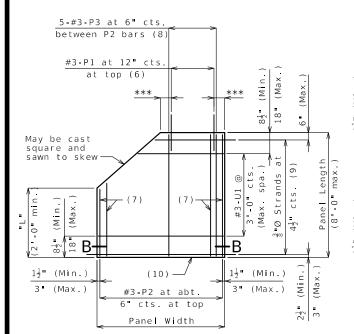


#### BENDING DIAGRAM FOR U1 BAR

U1 Bars may be oriented at right angles to location and spacing shown. U1 Bars shall be placed between P1 bars.



SECTION B-B

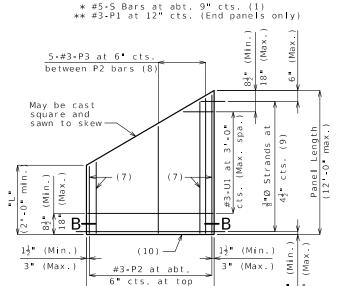


PLAN OF OPTIONAL TRUNCATED END PANEL

\*\*\* 3" (Min.), 6" (Max.)

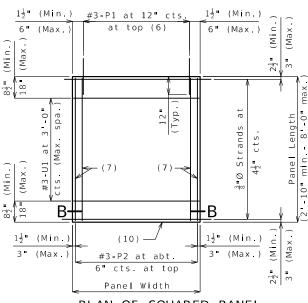
Designed June 2023 Detailed June 2023 Checked July 2023

#### PLAN SHOWING PANEL PLACEMENT



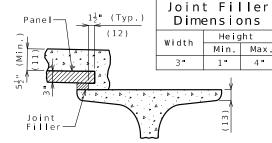
#### PLAN OF OPTIONAL SKEWED END PANEL

Panel Width



#### PLAN OF SQUARED PANEL

### SKEWED END PANELS



#### SECTION A-A

#### Reference Notes:

Plan of Panel Placement: (1) S-bars shown are bottom steel in slab between

panels and used with squared and truncated end (2) Extend S-bars 18 inches beyond the front face

of end bents and int. bents for squared and truncated end panels only.

(3) Extend S-bars 9 inches beyond edge of girder

(4) End panels shall be dimensioned 1/2" min. to 1 1/2" max. from the inside face of diaphragm.

(5) For truncated end panels, use a min. of #5-S bars at 6" crossings in openings, or min. 4x4-

(6) For end panels only, P1 bars shall be 2'-0" in length and embedded 12". P1 bars will not be required for panels at squared integral end bents.

(7) #3-P2 bars near edge of panel at bottom (under strands).

(8) Use #3-P3 bars if panel is skewed  $45^{\circ}$  or greater.

(9) Any strand 2'-0" or shorter shall have a #4 reinforcing bar on each side of it, centered between strands. Strands 2'-0" or shorter may then be debonded at the fabricator's option.

(10) Optional 1/2" x 45° Chamfer one or both sides at bottom.

#### Section A-A:

(11) Slab thickness over prestressed panels varies due to girder camber. In order to maintain minimum slab thickness, it may be necessary to raise the grade uniformly throughout the structure. No payment will be made for additional labor or materials required for necessary grade

(12) Contractor shall ensure proper consolidation under and between panels.

(13) At the contractor's option, the variation in slab thickness over prestressed panels may be eliminated or reduced by increasing and varying the girder top flange thickness. Dimensions shall be shown on the shop drawings.

#### PRESTRESSED PANELS

Note: This drawing is not drawn to scale. Follow dimensions.

Sheet No 20 of 35

#### General Notes:

Prestressed Panels:

Concrete for prestressed panels shall be Class A-1 with f'c = 6,000 psi, f'ci = 4,000 psi.

The top surface of all panels shall receive a scored finish with a depth of scoring of  $1/8\,\text{"}$  perpendicular to the prestressing strands in the panels.

Prestressing tendons shall be high-tensile strength, uncoated, seven-wire, low-relaxation strands for prestressed concrete in accordance with AASHTO M 203 Grade 270, with nominal diameter of strand = 3/8" and nominal area = 0.085 sq.in. and minimum ultimate strength = 22.95 kips (270 ksi). Larger strands may be used with the same spacing and initial tension.

Initial prestressing force = 17.2 kips/strand.

The method and sequence of releasing the strands shall be shown

Suitable anchorage devices for lifting panels may be cast in panels, provided the devices are shown on the shop drawings and approved by the engineer. Panel lengths shall be determined by the contractor and shown on the shop drawings.

When squared end panels are used at skewed bents, the skewed portion shall be cast full depth. No separate payment will be made for additional concrete and reinforcing required.

Support from diaphragm forms is required under the optional skewed end until cast-in-place concrete has reached 3,000 psi compressive strength

Prestressed panels shall be brought to saturated surface-dry (SSD) condition just prior to the deck pour. There shall be no free standing water on the panels or in the area to be cast.

The prestressed panel quantities are not included in the table of estimated quantities for the slab.

#### Reinforcing Steel:

All dimensions are out to out.

Hooks and bends shall be in accordance with the CRSI Manual of Standard Practice for Detailing Reinforced Concrete Structures, Stirrup and Tie Dimensions.

Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise shown.

If U1 bars interfere with placement of slab steel, U1 loops may be bent over, as necessary, to clear slab steel.

Deformed welded wire reinforcement (WWR) providing a minimum area of reinforcing perpendicular to strands of 0.22 sq in./ft, with spacing parallel to strands sufficient to ensure proper handling, may be used in lieu of the #3-P2 bars shown. Wire diameter shall not be larger than 0.375 inch. The above alternative reinforcement criteria may be used in lieu of the #3-P3 bars, when required, and placed over a width not less than 2 feet.

The following reinforcing steel shall be tied securely to the strands with the following maximum spacing in each direction: #3-P2 bars at 16 inches. WWR at 24 inches.

The #3-U1 bars shall be tied securely to #3-P2 bars, to WWR or to strands (when placed between P1 bars) at about 3-foot centers.

Minimum reinforcement steel length shall be 2'-0"

All reinforcement other than prestressing strands shall be epoxy

Precast panels may be in contact with stirrup reinforcing in

S-bars are not listed in the bill of reinforcing.

Cost of S-bars will be considered completely covered by the contract unit price for the slab.

#### Joint Filler:

Joint filler shall be preformed fiber expansion joint material in accordance with Sec 1057 or expanded or extruded polystyrene bedding material in accordance with Sec 1073

Use Slab Haunching Diagram on Sheet No. 22 for determining thickness of joint filler within the limits noted in the table of Joint Filler Dimensions.

Thicker material may be used on one or both sides of the girder to reduce cast-in-place concrete thickness to within tolerances.

The same thickness of preformed fiber expansion joint material shall be used under any one edge of any panel except at locations where top flange thickness may be stepped. The maximum change in thickness between adjacent panels shall be 1/2 inch. polystyrene bedding material may be cut with a transition to match haunch height above top of flange.

Joint filler shall be glued to the girder. When thickness exceeds 1 1/2 inches, the joint filler shall be glued top and bottom. The glue used shall be the type recommended by the joint filler manufacturer.

Edges of panels shall be uniformly seated on the joint filler before slab reinforcement is placed.



11/17/2023

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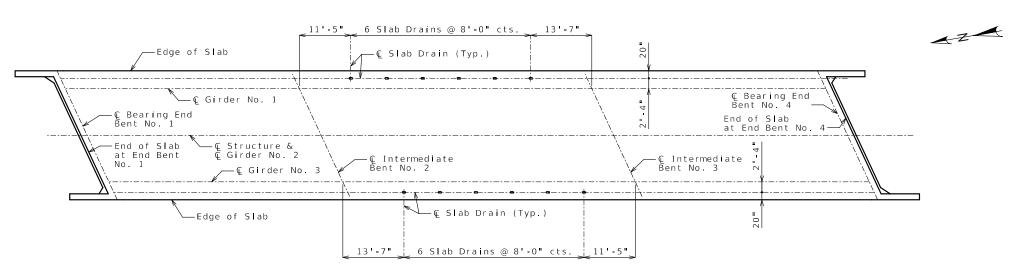
SHANNON

J9P3687

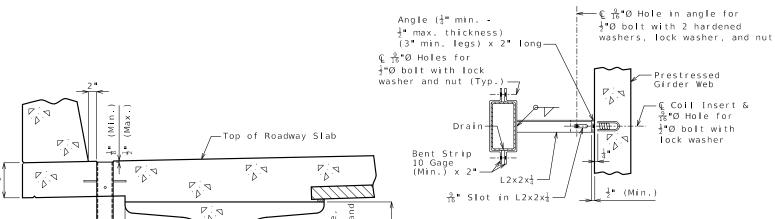
PROJECT NO

A9309

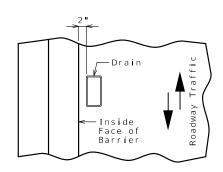
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PLAN OF SLAB SHOWING SLAB DRAIN LOCATIONS



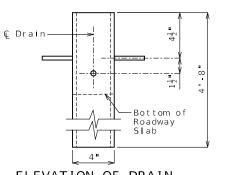
PART SECTION SHOWING BRACKET ASSEMBLY



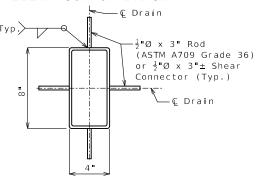
PART PLAN OF SLAB AT DRAIN

PART SECTION NEAR DRAIN

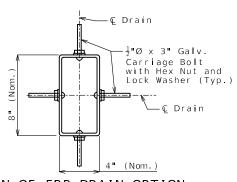
1" (Min.)



ELEVATION OF DRAIN



PLAN OF STEEL DRAIN OPTION



PLAN OF FRP DRAIN OPTION

#### General Notes:

Contractor shall have the option to construct either steel or FRP slab drains. All drains shall be of same type.

Slab drain bracket assembly shall be ASTM A709 Grade 36 steel

Locate drains in slab by dimensions shown in Part Section Near Drain.

Reinforcing steel shall be shifted to

The coil inserts and bracket assembly shall be galvanized in accordance with ASTM A123.

All bolts, hardened washers, lock washers and nuts shall be galvanized in accordance with AASHTO M 232 (ASTM A153),

All 1/2"Ø bolts shall be ASTM A307.

Shop drawings will not be required for the slab drains and the bracket assembly.

The coil insert required for the bracket assembly attachment shall be located on the prestressed girder shop drawings.

Coil inserts shall have a concrete pullout strength (ultimate load) of at least 2,500 pounds in 5,000 psi concrete.

The bolt required to attach the slab drain bracket assembly to the prestressed girder web shall be supplied by the prestressed girder fabricator

#### Notes for Steel Drain:

Slab drains may be fabricated of either 1/4" welded sheets of ASTM A709 Grade 36 steel or from 1/4" structural steel tubing ASTM A500 or A501.

Outside dimensions of drains are 8" x 4".

The drains shall be galvanized in accordance with ASTM A123.

#### Notes for FRP Drain:

Drains shall be machine filament-wound thermosetting resin tubing meeting the requirements of ASTM D2996 with the following exceptions:

Shape of drains shall be rectangular with outside nominal dimensions of  $8\ \times\ 4\ \cdot$ .

Minimum reinforced wall thickness shall be

The resin used shall be ultraviolet (UV) resistant and/or have UV inhibitors mixed throughout. Drains may have an exterior coating for additional UV resistance.

The color of the slab drain shall be gray (Federal Standard 26373). The color shall be uniform throughout the resin and any coating used.

The combination of materials used in the manufacture of the drains shall be tested for UV resistance in accordance with ASTM D4329 Cycle A. The representative material shall withstand at least 500 hours of testing with only minor discoloration and without any physical deterioration. The contractor shall furnish the results of the required ultraviolet testing prior to acceptance of the slab drains.

At the contractor's option, drains may be field cut. The method of cutting FRP slab drain shall be as recommended by the manufacturer to ensure a smooth, chip free



DATE PREPARED								
11/17/2023								
ROUTE	STATE							
19	MO							
STRICT	SHEET NO.							
BR	21							

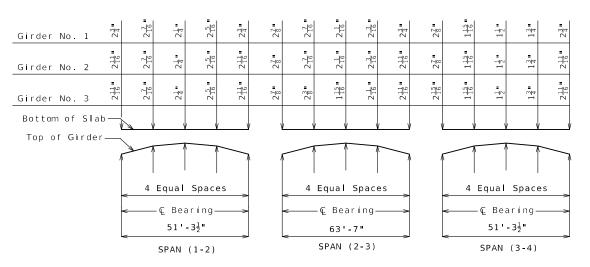
SHANNON

J9P3687

PROJECT NO

A9309

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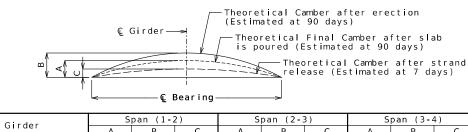
#### THEORETICAL SLAB HAUNCHING DIAGRAM (ESTIMATED AT 90 DAYS)

If girder camber is different from that shown in the camber diagram, in order to maintain minimum slab thickness, an adjustment of the slab haunches, an increase in slab thickness or a raise in grade uniformly throughout the structure shall be necessary. No payment will be made for additional labor or materials required for variation in haunching, slab thickness or grade adjustment.

Concrete in the slab haunches is included in the Estimated Quantities for Slab On Concrete NU-Girder.

	Theoretical Bottom of Slab Elevations at Centerline of Girder (Prior to forming for slab) (Estimated at 90 days)														
Girder Span (1-2) (51'-3½" © Brg © B						Span (2-3) (63'-7" @ Brg @ Brg.)						3-4) (51	'-3 <del>1</del> " Ç B	rg. <b>- (</b> 2	Brg.)
Number	€ Brg	. 25	.50	.75	© Brg.	€ Brg.	. 25	. 50	. 75	€ Brg.	€ Brg.	. 25	.50	. 75	€ Brg.
1	724.90	725.10	725.30	725.48	725.65	725.66	725.93	726.19	726.41	726.59	726.61	726.84	727.10	727.39	727.69
2	725.17	725.38	725.58	725.76	725.93	725.94	726.21	726.47	726.69	726.88	726.89	727.14	727.42	727.71	728.02
3	725.04	725.24	725.44	725.62	725.79	725.80	726.07	726.33	726.55	726.75	726.77	727.03	727.31	727.62	727.95

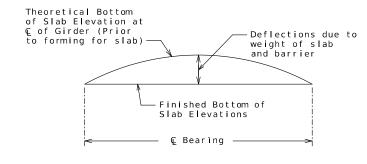
Elevations are based on a constant slab thickness of 8 1/2" and include allowance for theoretical dead load deflections due to weight of slab (including precast panel) and barrier.



011 461	Α	В	С	Α	В	С	Α	В	C	
Exterior Girder 1 & 3	1/2 m	<u>3</u>	<u>.7</u>	<u>3</u> =	1.7."	<u>13</u>	<u>1</u> 11	<u>3</u> •	<u>_7</u> ,,	
Interior Girder 2	<u>7</u> <b>≡</b>	4	16	11 m 16	116	16	7 <sub>16</sub>	4	16	

#### GIRDER CAMBER DIAGRAM

Conversion Factors for Girder Camber (Estimated at 90 days):  $0.25 \text{ pt.} = 0.7125 \times 0.5 \text{ pt.}$ 



TYPICAL SLAB ELEVATIONS DIAGRAM



11/17/2023

SHANNON

J9P3687

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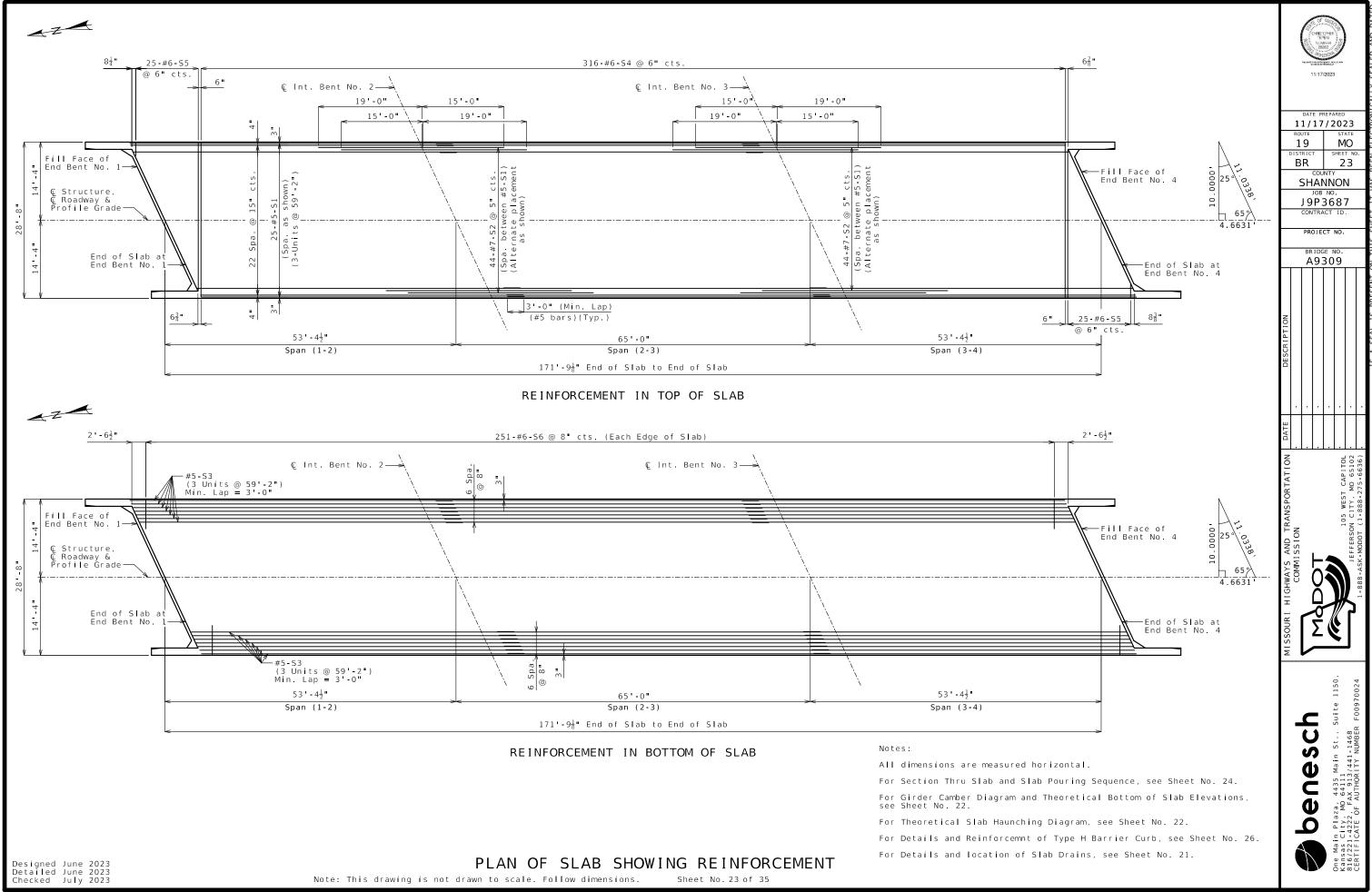
22

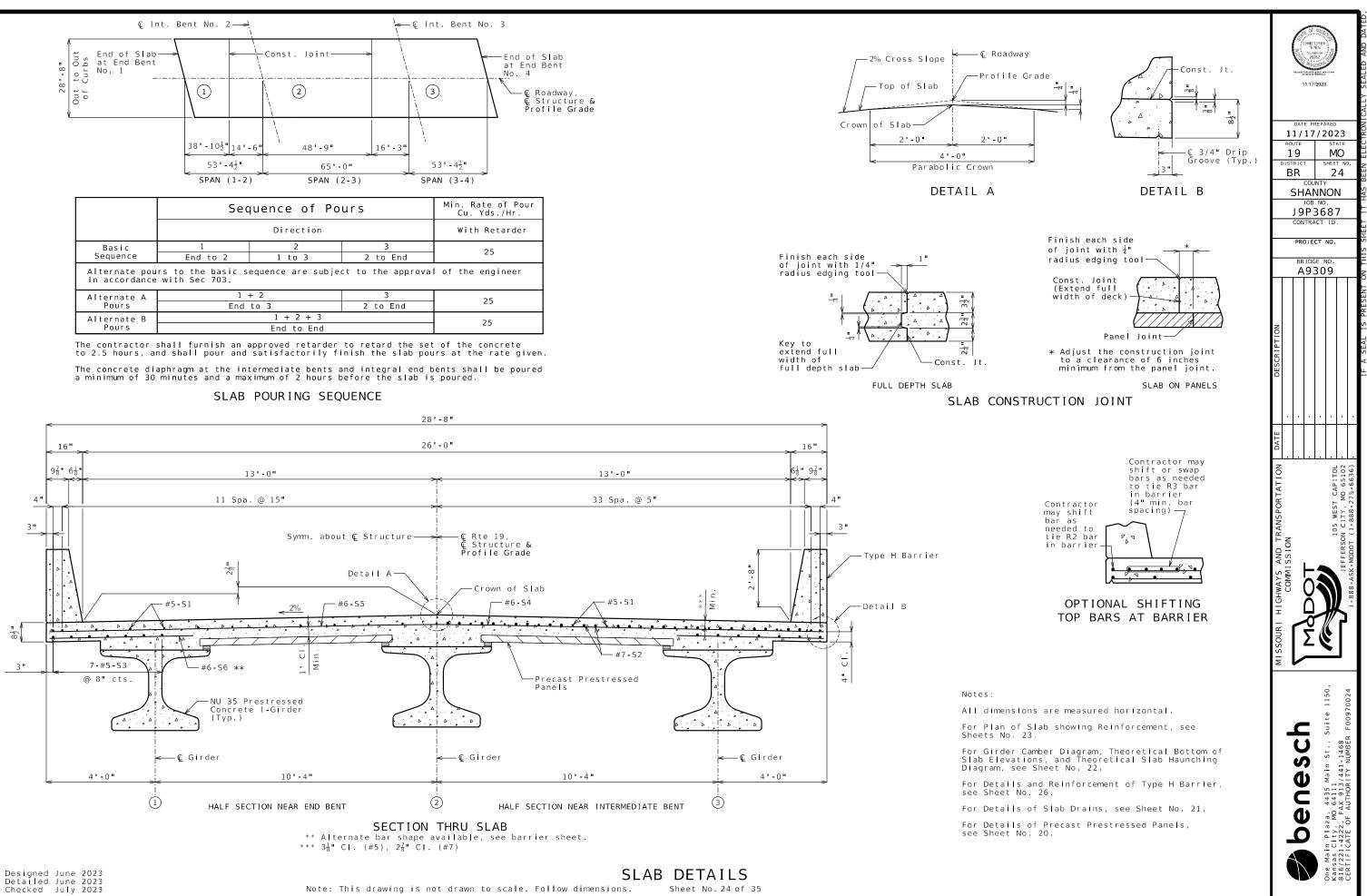
19

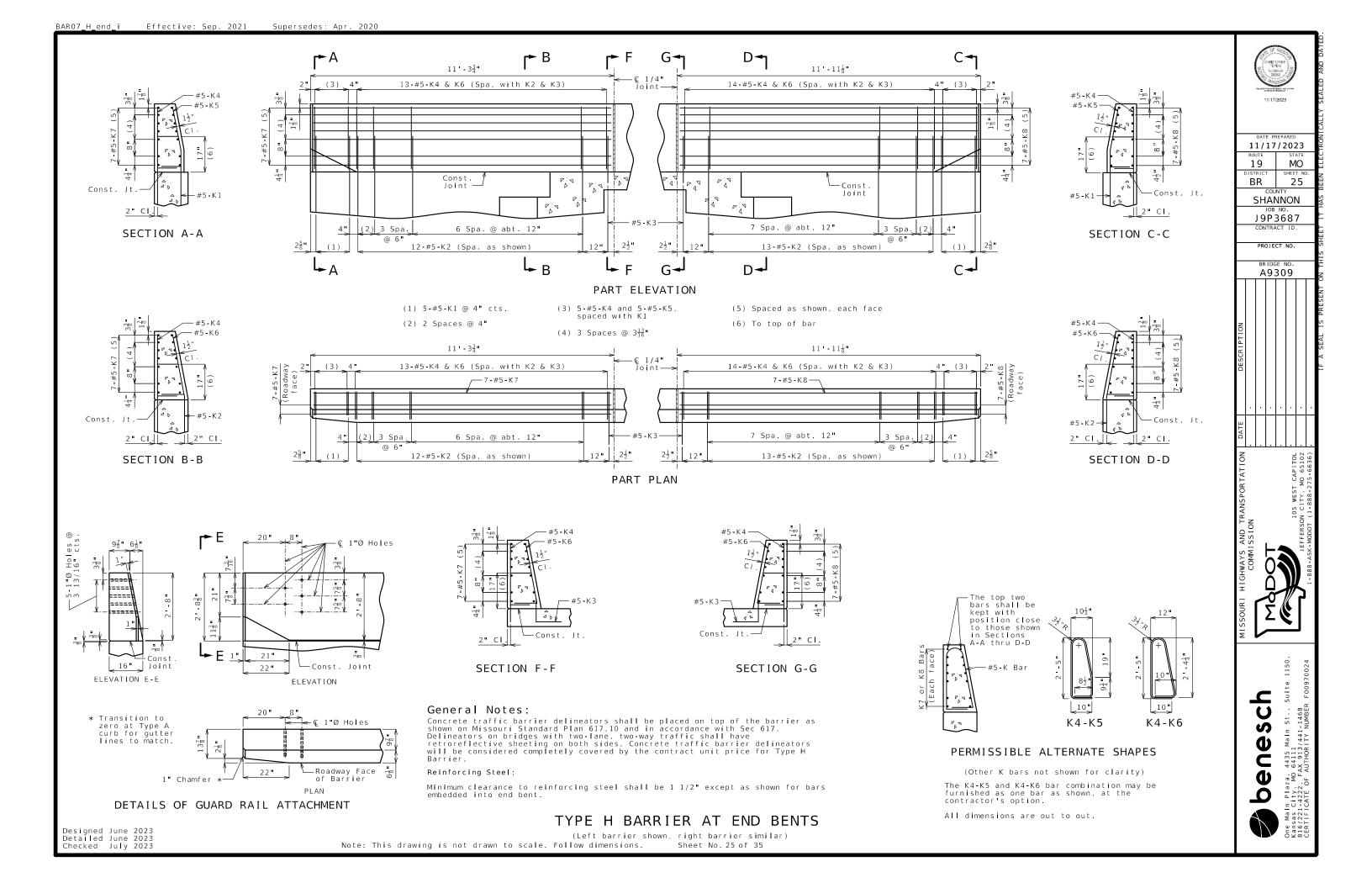
BR

HAUNCHING AND CAMBER DIAGRAMS

Designed June 2023 Detailed June 2023 Checked July 2023







#### PART ELEVATION OF BARRIER (1) Four feet long, centered on joint,

slip-formed option only

Designed June 2023 Detailed June 2023

Checked July 2023

The cross-sectional area above the slab is 2 89 square feet

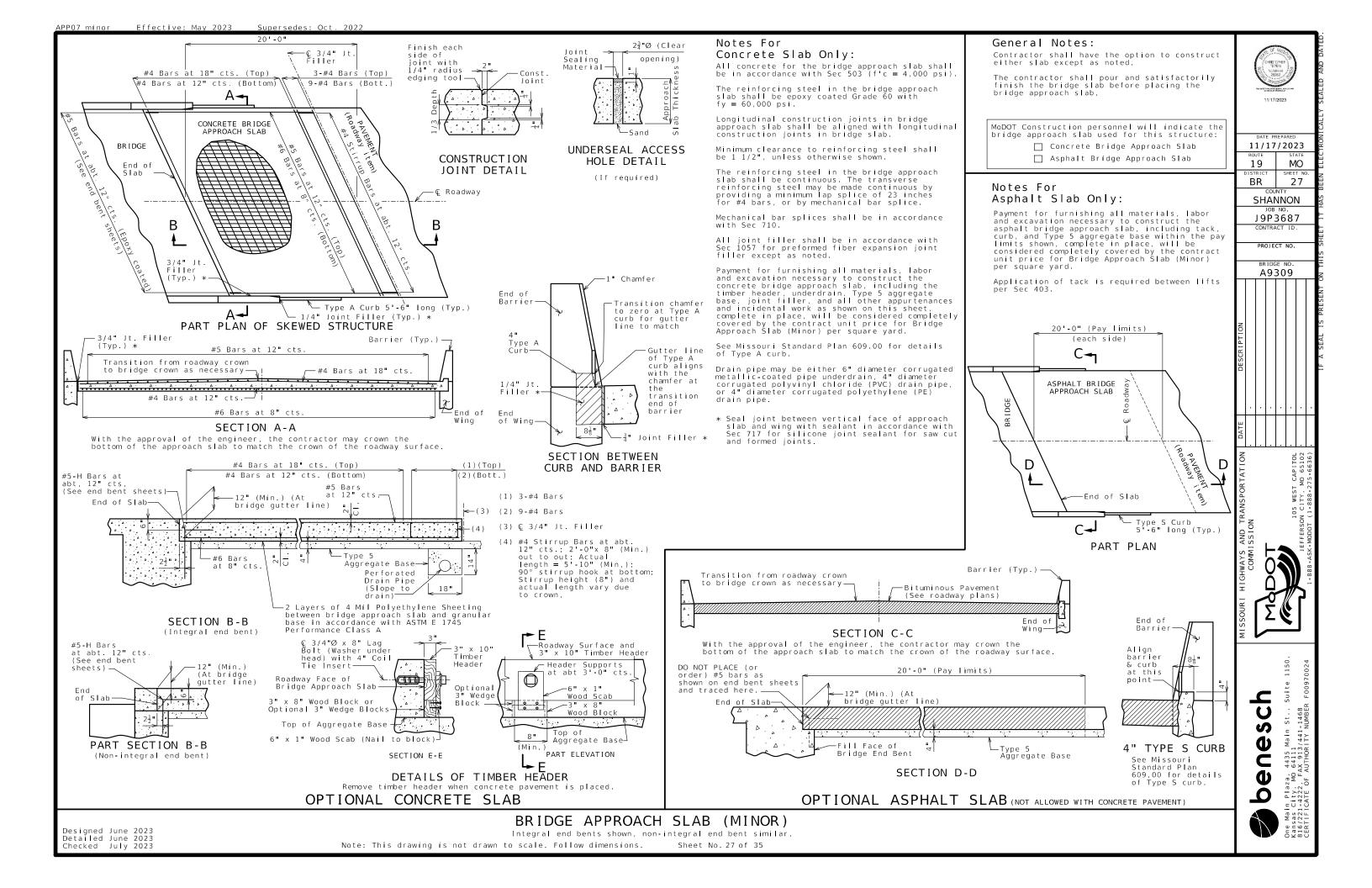
(2) To top of bar

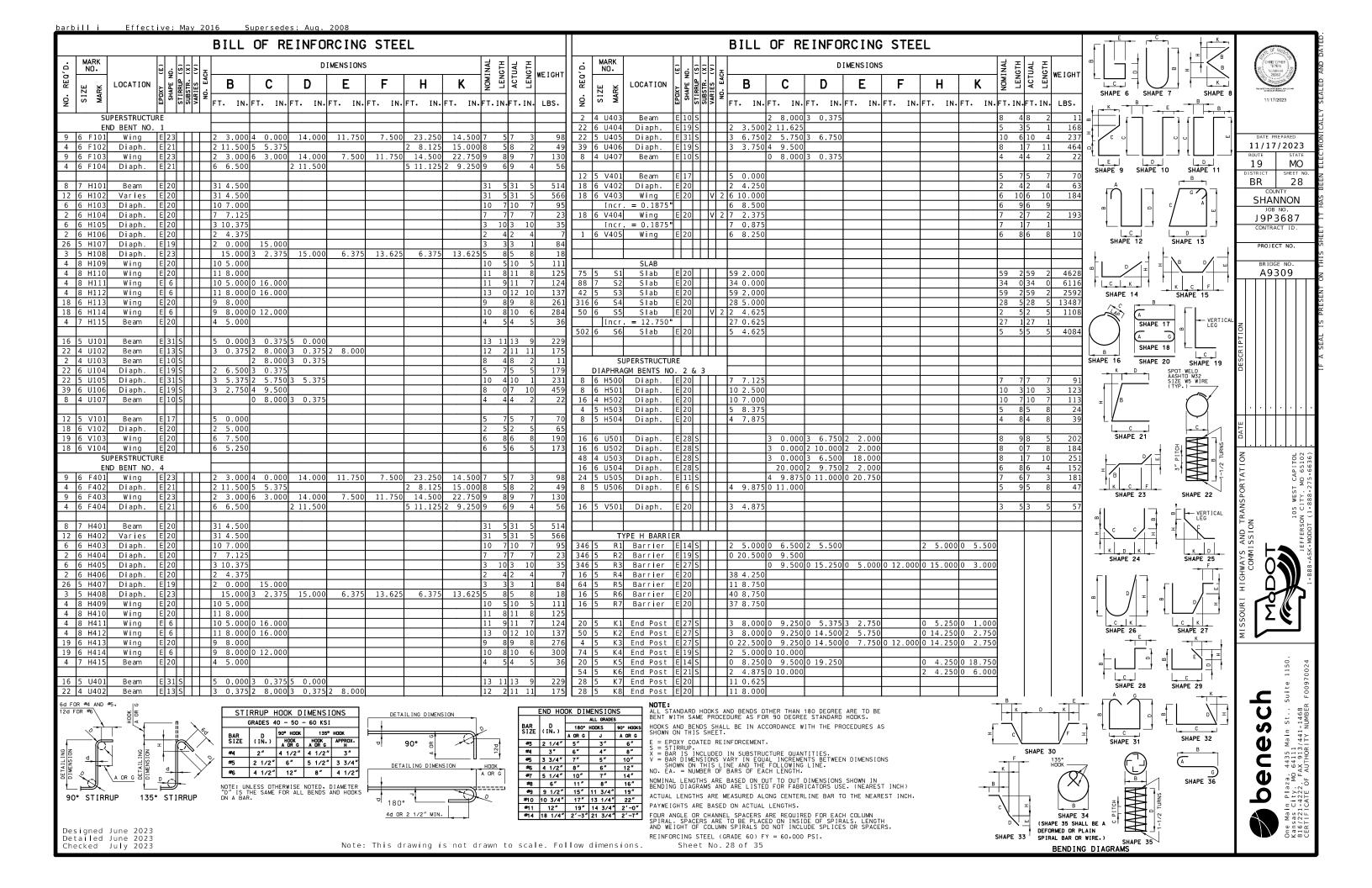
- forming is not used. (All dimensions are out to out.)
- (4) The R2 bar and #5 bottom transverse slab bar in cantilever (prestressed panels only) combination may be furnished as one bar as shown, at the contractor's option.

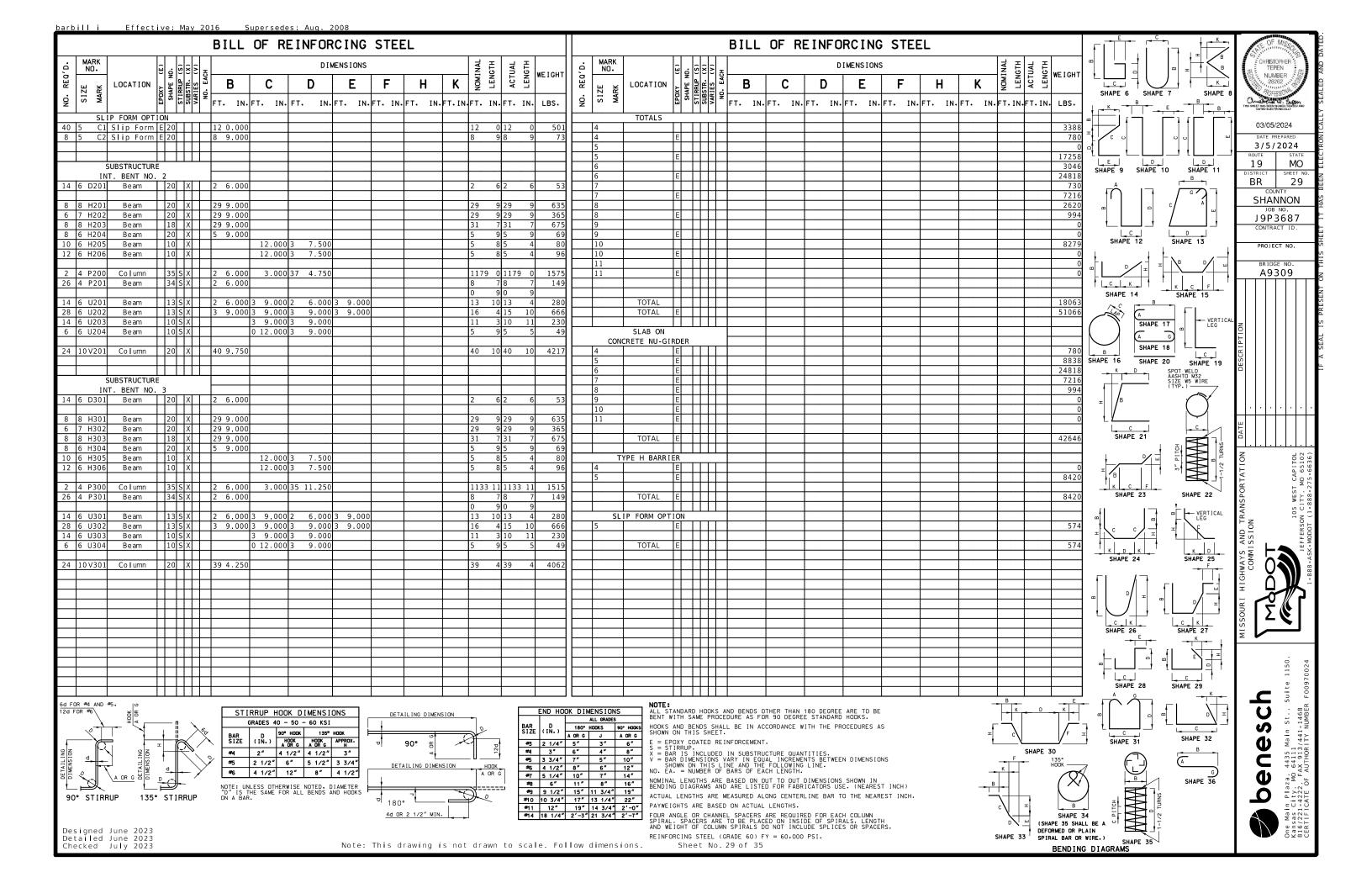
For slip-formed option, both sides of barrier shall have a vertically broomed finish and the top shall have a transversely broomed finish.

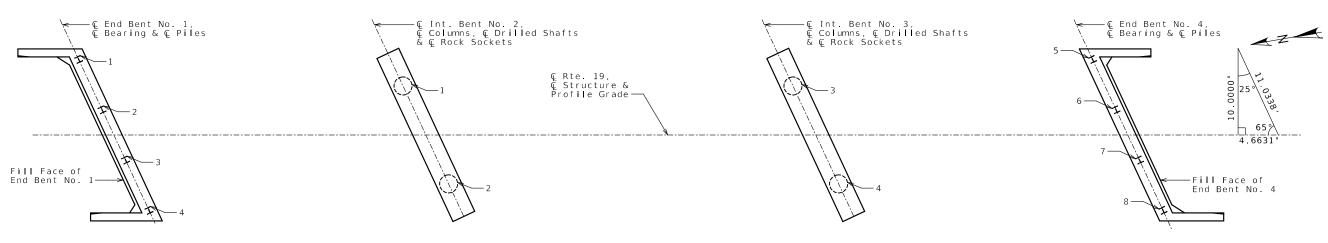
# C Ф ൧

#### TYPE H BARRIER









PART PLAN SHOWING PILE AND DRILLED SHAFT NUMBERING FOR RECORDING AS-BUILT PILE DATA AND DRILLED SHAFT DATA

			As-Built Pile Data				As-Built	Drilled	Shaft Data
Pile No.	Length in Place (ft)	Computed Nominal Axial Compressive Resistance (kips)	Remarks	s	haft No.	Top of Sound Rock (Elev.)	Tip of Casing (Elev.)	Bottom of Rock Socket (Elev.)	R emarks
	( , , ,	(,,	End Bent No. 1	F					Int. Bent No. 2
1				L	1				
2				L	2				
3				┡					
4				┝					
				⊢					
			End Bent No. 4	$\vdash$					Int. Bent No. 3
			Liid Bellt NO. 4	$\vdash$					THE DELLE NO. 3
5				$\vdash$	3				
6				F	4				
7				F					
8									
				L					
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				F					
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I				- 1					

Note: Indicate in remarks column: A. Pile type and grade B. Batter C. Driven to practical refusal

This sheet to be completed by MoDOT construction personnel.

DATE

DATE

DATE

DATE

T1/17/2023

ROUTE

19 MO

DISTRICT SHEET NO.

BR 30

COUNTY

SHANNON

JOB NO.

J 9 P 3 6 8 7

CONTRACT ID.

PROJECT NO.

BRIDGE NO.

A9309

NO.

A9309

Sch MoD n St., Suite 1150,

benescity, Mo 6411

Designed June 2023 Detailed June 2023 Checked July 2023

Mado	McDOT-Geotechnical Section 1617 Missouri Blvd Jefferson City, MO 65109	KEY TO SYMBOLS
CLIENT		PROJECT NAME _Bridge Replacement
PROJECT N	NUMBER J9P3687	PROJECT LOCATION
54-55X N115-644444	OLOGIC SYMBOLS	SAMPLER SYMBOLS
(Unifi	ied Soil Classification System)	Rock Core Barrel
	Asphalt	Nock Gole Ballel
	Boulders and cobbles	Grab Sample
	USCS High Plasticity Clay	Split-Spoon Sampler
	USCS High Plasticity Gravelly Clay	
	USCS Low Plasticity Clay	
	USCS Low Plasticity Gravelly Clay	
	USCS Low Plasticity Silty Clay	WELL CONSTRUCTION SYMBOLS
	USCS Low Plasticity Sandy Clay	WELL CONSTRUCTION STIMBOLS
	Dolomite	
	USCS Clayey Gravel	
	USCS Poorly-graded Gravel	
	USCS Poorly-graded Sandy Gravel	
· (X°	Highly Weathered Dolomite	
	USCS Silty Sand	
	USCS Poorly-graded Sand	
。 。 ()	USCS Poorly-graded Gravelly Sand	
		ABBREVIATIONS
PI W DD NP -200	- LIQUID LIMIT (%) - PLASTIC INDEX (%) - MOISTURE CONTENT (%) - DRY DENSITY (PCF) - NON PLASTIC - PERCENT PASSING NO. 200 SIEVE - POCKET PENETROMETER (TSF)	TV -TORVANE PID -PHOTOIONIZATION DETECTOR UC -UNCONFINED COMPRESSION ppm -PARTS PER MILLION  ☑ Water Level at Time of Drilling
	- UNCONFINED COMPRESSIVE STRENGT	H (PSF)  ▼ Water Level at End of Drilling  ▼ Water Level after Drilling

DATE PREPARED  11/17/2023  ROUTE  19  MO										
		3 OI 10	1 N							
			93	E №	9					
DESCRIPTION				•						
DATE										
MISSOURI HIGHWAYS AND TRANSPORTATION	COMMISSION				105 WEST CAPITOL	JEFFERSON CITY, MO 65102	1-888-ASK-MODOT (1-888-275-6636)			
			こうつうこうつ		In Plaza, 4435 Main St., Suite 1150, Citv, MO 64111	4222, FAX 913/441-1468	AIE OF AUTHORITI NUMBER FUU9/0024			

BORING DATA LEGEND

#### **BORING NO. 101-2 (T-22-64) Missouri Department of Transportation** Page 1 of 2 **Construction and Materials** Job No.: J9P3687 County: Shannon Route: 19 Design: A9309 Skew: 25 RA Location: Over Hurricane Creek Operator: Kenneth Tuttle Bent: 1 Logged By: Matthew Kistler Northing: 384221 Date of Work: 11/17/22-11/21/22 Easting: <u>577534.6</u> Offset: Depth to Water: Elevation: 724.2 Requested Northing: 384218.0 Depth Hole Open: Requested Station: 466+47 Requested Easting: 577537.5 Time Change: Requested Offset: 12.0 L Equipment: CME 45 ,Grab Sample, Split-Spoon Sampler, NX Requested Elevation: 724.5 Location Note: Auxillary Hole Drill No.: G-9577 Hammer Efficiency: 87% Drilling Method: Hollow Stem Auger Depth (ft) Rad Description 0.0-0.4' ASPHALT 0.4-1.8' Black, GRAVEL, dry, fine to medium grained, base rock 1.8-6.8' Reddish brown, GRAVELLY FAT CLAY scattered cobbles, medium stiff, moist 5.0-6.5' gravel in SPT shoe 4-5-2 (10) 6.8-7.5' COBBLES 7.5-15.0' Reddish brown, GRAVELLY FAT 7-3-3 40 CLAY, stiff, moist 10.0-11.5' Gravel in SPT shoe 3-2-4 27 y sat = 128 pcf Sieve Analysis 15.0-18.6' Brown, SILTY SAND, loose, moist, 1-2-1 87 Sieve # % Passing fine grained 639 18.6-29.9' Light brown, CLAYEY GRAVEL 20 with chert gravel, medium dense to dense, 8-10-13 (33) 700 33 (13) 695 29.9-38.9' Cherty Dolomite, light gray, thin Qu Test Results bedded to medium bedded, medium strong UCS = 488 ksf MC = 0% 100 (78) rock to very strong rock, moderately weathered to slightly weathered, very fine **y** <sub>moist</sub> = 162.4 pcf grained 690

N<sub>so</sub> = (Em/60)Nm N<sub>so</sub> - Corrected N value for standard 60% SPT efficiency; Em - Measured hammer efficiency in percent; Nm - Observed N-value (1) = Assumed, (2) = Actual

Coordinate System: U.S. State Plane 1983 Coordinate Zone: Missouri East Coordinate Proj. Factor: \_\_\_

Coordinate Datum: NAD 83 (CONUS) Coordinate Units: U.S. Survey Feet

\* Persons using this information are cautioned that the materials shown are determined by the equipment noted and accuracy of the "log of materials" is limited thereby and y judgement of the operator. THIS INFORMATION IS FOR DESIGN PURPOSES ONLY.

(Continued Next Page)

#### **Missouri Department of Transportation Construction and Materials**

BORING NO.	101-2 (T-22-64)
	Page 2 of 2

Job No.: _J	9P3687	County: Sha	annon				Route: 19				
Design: A	9309	<b>Skew:</b> 25 R/	4				Location: Over Hurricane Creek				
Bent: _1		Logged By: _	Matthe	ew Ki	stler		Operator: Kenneth Tuttle				
Station:	Northing: <u>384221</u>						Date of Work: 11/	17/22-11/21/22			
Offset:		Easting: 577	7534.6				Depth to Water:				
Elevation: _	724.2	Requested No	orthing	j: <u>3</u> 8	34218.0		Depth Hole Open: _				
Requested	Station: _466+47	Requested Ea	asting:	_57	7537.5		Time Change:				
Requested	Offset: 12.0 L	Equipment: _	CME 4	15 ,G	rab Samı	ole, Split-Spo	on Sampler, NX				
Requested	Elevation: 724.5	Location Note	e: <u>Au</u> :	xillary	/ Hole						
Drill No.: _C	G-9577	Hammer Effic	iency:	879	%		Drilling Method: <u>H</u>	ollow Stem Aug	<u>er</u>		
Depth (ft) (ft) (ft) Graphic	Description		Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N <sub>60</sub> )	Shear Data	Field Tests	Index Tests		
	29.9-38.9' Cherty Dolomite, light bedded to medium bedded, med rock to very strong rock, moders weathered to slightly weathered grained (continued)  Bottom of borehole at 38	dium strong ately , very fine			100 (96)		Qu lest Results UCS = 571 ksf MC = 0%				

 $N_{\infty}$  = (Em/60)Nm  $N_{\infty}$  - Corrected N value for standard 60% SPT efficiency; Em - Measured hammer efficiency in percent; Nm - Observed N-value (1) = Assumed, (2) = Actual

Coordinate System: U.S. State Plane 1983	Coordinate Zone:	Missouri East	Coordinate Proj. Factor:
Coordinate Datum: NAD 83 (CONUS)	Coordinate Units:	U.S. Survey Feet	

BORING DATA NEAR END BENT NO. 1

Note: For locations of borings, see Sheet No. 1.

Designed June 2023 Detailed June 2023 Checked July 2023

Note: This drawing is not drawn to scale. Follow dimensions. Sheet No. 32 of 35

11/17/2023 19 MO BR 32

SHANNON

J9P3687

PROJECT NO. A9309

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<sup>\*</sup> Persons using this information are cautioned that the materials shown are determined by the equipment noted and accuracy of the "log of materials" is limited thereby and by judgement of the operator. THIS INFORMATION IS FOR DESIGN PURPOSES ONLY

### **Missouri Department of Transportation**

#### **BORING NO. 202 (T-22-63)** Page 1 of 2

	Construction and Materials	
Job No.: <u>J9P3687</u>	County: Shannon	Route: 19
Design: <u>A9309</u>	Skew: 25 RA	Location: Over Hurricane Creek
Bent: 2	Logged By: Matthew Kistler	Operator: Gary Degraffenreid
Station:	Northing: <u>384171</u>	Date of Work: 11/21/22-11/22/22
Offset:	Easting: _577500	Depth to Water:
Elevation: 708.6	Requested Northing: 384160.9	Depth Hole Open:
Requested Station: 0+57.5	Requested Easting: 577507.2	Time Change:
Requested Offset: 8.2 R	Equipment: Acker Renegade, Grab Sample	, NX

F	Reque	ested I	Elevation: _707.0 Location Not	e: <u>Off</u>	set to	avoid e	existing Structure				
_	Drill N	lo.: _@	9-9667 Hammer Effic	iency:	929	%	Dı	Drilling Method: Hollow Stem Auger			
	O Depth (ft)	Graphic	Description	Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N <sub>60</sub> )	Shear Data	Field Tests	Index Tests	
ANECREEK.GPJ	5 -		0.0-8.0' Brown, SILTY SAND, dense, moist, fine grained  8.0-12.8' Dolomite, very weak rock, highly weathered	705	•					LL = 16 PL = 13	
LES\J9P3687-A9309_19_HURRIC	15 -		12.8-13.7' Dolomite, weak rock, moderately weathered 13.7-20.4' Cherty Dolomite, light gray, thin bedded, strong rock, slightly weathered, very fine grained	695  - 690		98 (72)		Qu Test Results UCS = 543 ksf MC = 0% Y most = 164.2 pcf			
- Z:\SG\GINT\PROJECT FI	- - 25		20.4-22.2' Chert, light gray and white, very thin bedded, strong rock, slightly weathered, very fine grained  22.2-26.7' Cherty Dolomite, light gray, thin bedded, medium strong rock, slightly weathered, very fine grained, Vuggy	685		82 (36) 100 (78)		Qu Test Results UCS = 253 ksf MC = 0%			
OOT 20150728.GDT - 1/12/23 15:20	30 -		26.7-27.8' Chert, blue and grayish white, very thin bedded, very strong rock, slightly weathered, very fine grained 27.8-35.4' Cherty Dolomite, light gray, thin bedded, extremely strong rock, slightly weathered, very fine grained, Pitted	680   675		92 (74)		y moist = 158.3 pcf Qu Test Results UCS = 1550 ksf MC = 0% y moist = 167 pcf Qu Test Results UCS = 2370 ksf MC = 0% y moist = 165.2 pcf			

N<sub>50</sub> = (Em/60)Nm N<sub>50</sub> - Corrected N value for standard 60% SPT efficiency; Em - Measured hammer efficiency in percent; Nm - Observed N-value (1) = Assumed, (2) = Actual

Coordinate System: U.S. State Plane 1983 Coordinate Zone: Missouri East Coordinate Proj. Factor: \_

Coordinate Datum: NAD 83 (CONUS) Coordinate Units: U.S. Survey Feet

\*Persons using this information are cautioned that the materials shown are determined by the equipment noted and accuracy of the "log of materials" is limited thereby and by judgement of the operator. THIS INFORMATION IS FOR DESIGN PURPOSES ONLY.

(Continued Next Page)

#### Missouri Department of Transportation Construction and Materials

BORING	NO.	202	(T-22-63)
			Page 2 of

Job No.:	County: Shannon	Route: 19			
<b>Desig</b> n: _A9309	<b>Skew:</b> 25 RA	Location: Over Hurricane Creek			
Bent: 2	Logged By: Matthew Kistler	Operator: Gary Degraffenreid			
Station:	Northing: <u>384171</u>	Date of Work: 11/21/22-11/22/22			
Offset:	Easting: <u>577500</u>	Depth to Water:			
Elevation: 708.6	Requested Northing: 384160.9	Depth Hole Open:			
Requested Station: 0+57.5	Requested Easting: 577507.2	Time Change:			
Requested Offset: 8.2 R	Equipment: Acker Renegade ,Grab Sample, NX				
Requested Elevation: _707.0	Location Note: Offset to avoid existing Structure				

		tation: _0+57.5	Requested E	asting	<u> 57</u>	7507.2		me Change:		
Requested Offset: 8.2 R Equipment: Acker Renegade, Grab Sample, NX										
	equested Elevation: _707.0 Location Note: Offset to avoid existing Structure									
Drill N	<b>lo.</b> : <u>G</u>	-9667	Hammer Effic	ciency	929	%	Dı	rilling Method: <u></u>	Hollow Stem Auge	<u>r</u>
Depth (ff)	Graphic	Description		Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N <sub>60</sub> )	Shear Data	Field Tests	Index Tests
· -		35.4-40.7' Cherty Dolomite, I blue, thin bedded, extremely slightly weathered, very fine	strong rock, -	  670		94 (14)				
40 _		40.7-41.5' Clay Seam 41.5-41.9' Cherty Dolomite, I		 		75 (19)				
		blue, thin bedded, extremely slightly weathered, very fine 41.5-41.9' Core Barrel went of Seam. Boring Terminated Bottom of borehole a	grained, Vuggy crooked after Clay							

통 N<sub>so</sub> = (Em/60)Nm N<sub>so</sub> - Corrected N value for standard 60% SPT efficiency; Em - Measured hammer efficiency in percent; Nm - Observed N-value 및 (1) = Assumed, (2) = Actual

Coordinate System:	U.S. State Plane 1983	Coordinate Zone:	Missouri East	Coordinate Proj. Factor:	
Coordinate Datum:	NAD 83 (CONUS)	Coordinate Units:	U.S. Survey Feet		

BORING DATA NEAR INT. BENT NO. 2

11/17/2023 19 MO

BR 33 SHANNON

J9P3687 PROJECT NO.

A9309

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<sup>\*</sup> Persons using this information are cautioned that the materials shown are determined by the equipment noted and accuracy of the "log of materials" is limited thereby and by judgement of the operator. THIS INFORMATION IS FOR DESIGN PURPOSES ONLY.

## Missouri Department of Transportation Construction and Materials

BORING NO. 202-2 Page 1 of 1

Job No.: J9P3687 County: Shannon Route: 19 Design: A9309 Skew: 25 RA Location: Over Hurricane Creek Operator: Kenneth Tuttle Bent: 2 Logged By: Matthew Kistler Northing: 384165.4 Date of Work: 11/16/22-11/16/22 Offset: Easting: <u>577499.3</u> Depth to Water: Elevation: 710.4 Requested Northing: 384160.9 Depth Hole Open: Requested Station: 0+57.5 Requested Easting: 577507.2 Time Change: \_\_

Equipment: CME 45 ,Split-Spoon Sampler, NX Requested Offset: 8.2 R Requested Elevation: 707.0 Location Note: Hole Abandoned due to drill failure Drill No.: G-9577 Hammer Efficiency: 87% Drilling Method: Hollow Stem Auger Depth (ft) Rad Description Field 0.0-3.5' Brown, SILTY SAND, dense, moist, fine grained 3.5-5.5' COBBLES and construction debris, M Visible at 202 Stake 2-13-16 (42) 20 5.5-6.6' BOULDERS, Dolomitic 6.6-7.3' Dolomite, highly weathered, Cut with, 7.3-14.3' Cherty Dolomite, light gray, moderately weathered, Pitted 33 (13) 13.4-13.4' Lost Water While Coring 100 (0) 14.3-14.3' Core Barrel Rod Snapped. Barrel Retrieved. Boring Abandoned and Relocated to 202 (T-22-63).

Bottom of borehole at 13.9 feet.

N<sub>oo</sub> = (Em/60)Nm N<sub>oo</sub> - Corrected N value for standard 60% SPT efficiency; Em - Measured hammer efficiency in percent; Nm - Observed N-value (1) = Assumed, (2) = Actual

 Coordinate System:
 U.S. State Plane 1983
 Coordinate Zone:
 Missouri East
 Coordinate Proj. Factor:

 Coordinate Datum:
 NAD 83 (CONUS)
 Coordinate Units:
 U.S. Survey Feet

\* Persons using this information are cautioned that the materials shown are determined by the equipment noted and accuracy of the "log of materials" is limited thereby and

by judgement of the operator. THIS INFORMATION IS FOR DESIGN PURPOSES ONLY.

#### Missouri Department of Transportation Construction and Materials

BORING NO. 301 (V-22-66) Page 1 of 2

ob No.: _J9P3687	County: Shannon	Route: 19			
esign: _A9309	<b>Skew:</b> 25 RA	Location: Over Hurricane Creek			
ent: <u>3</u>	Logged By: Ricardo Todd	Operator: Gary Degraffenreid			
tation:	Northing: <u>384094.2</u>	Date of Work: 11/16/22-11/21/22			
ffset:	Easting: <u>577517.8</u>	Depth to Water:			
levation: 709.2	Requested Northing: 384101.6	Depth Hole Open:			
equested Station: 1+14.9	Requested Easting: 577513.5	Time Change:			
equested Offset: 8.2 L	Equipment: Acker Renegade ,Split-Spoon Sampler, NWD4				
equested Elevation: 706.3	Location Note: Offset to avoid existing Structure				

Requ	ested C	Offset: 8.2 L	Equipment: _	Acker	Rene	gade ,S	plit-Spoon Sampl	er, NWD4		
-		Elevation: 706.3					xisting Structure			
Drill	No.: <u>G</u>	-9667	Hammer Effic	iency:	929	<u>%</u> 	D	rilling Method: <u> </u>	Hollow Stem Auge	er
Depth (ft)	Graphic	Description		Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N <sub>60</sub> )	Shear Data	Field Tests	Index Tests
000072015731570-1/12/231570-2/36/GNINTPR/UECT FILES/999867-49309 19 HIRRICANECREEK GPJ 20 25 25 25 25 25 25 25 25 25 25 25 25 25		9.8-11.6' Brown, GRAVELLY LI trace cobbles, very stiff, moist  11.6-11.9' Dolomite, highly wea 11.9-35.9' Cherty Dolomite, grastrong rock to extremely strong weathered, fine grained, Vuggy	EAN CLAY thered y, thin bedded, rock, slightly	705 - 700 - 695 - 690 - 685 - 680 - 675		73  100 90 (63)  100 (80)  100 (76)  94 (68)	2-3-4 (11) 8-5-8 (20) 33/0.1'	Qu Test Results UCS = 543 ksf MC = 0% Y mois:= 164.8 pcf  Qu Test Results UCS = 1170 ksf MC = 0% Y mois:= 165.8 pcf  Qu Test Results UCS = 1950 ksf MC = 0% Y moist = 160.1 pcf  Qu Test Results UCS = 721 ksf MC = 0% Y moist = 162.9 pcf  Qu Test Results UCS = 7500 ksf MC = 0% Y moist = 162.9 pcf	PP = 1.00 tsf	MC = 15.4% γ set = 136 pcf <sup>(1)</sup> LL = 20 PL = 13 MC = 13.8% γ set = 139 pcf <sup>(1)</sup>

 $N_{60}$  = (Em/60)Nm  $N_{60}$  - Corrected N value for standard 60% SPT efficiency; Em - Measured hammer efficiency in percent; Nm - Observed N-value (1) = Assumed, (2) = Actual

Coordinate System: U.S. State Plane 1983	Coordinate Zone: Missouri East	Coordinate Proj. Factor:	
Coordinate Datum: NAD 83 (CONUS)	Coordinate Units: U.S. Survey Feet		

(Continued Next Page)

### BORING DATA NEAR INT. BENT NO. 2 AND INT. BENT NO. 3

CHRS CORE ON NUMBER OF THE PROPERTY OF THE PRO

DATE PREPARED

11/17/2023

ROUTE STATE

19 MO

DISTRICT SHEET NO.

BR 34

BR 34

COUNTY

SHANNON

JOB NO.

J9P3687 CONTRACT ID

PROJECT NO.

BRIDGE NO.

A9309

COMMISSION

COMMISSION

105 WEST CAPITOL

benesch in Plaza, 4435 Main St., Suite 1150, City, Mo 6411 14222, FAX, 913/441-1468

<sup>\*</sup> Persons using this information are cautioned that the materials shown are determined by the equipment noted and accuracy of the "log of materials" is limited thereby and by judgement of the operator. THIS INFORMATION IS FOR DESIGN PURPOSES ONLY.

Bottom of borehole at 47.4 feet.

### Missouri Department of Transportation

### **BORING NO. 301 (V-22-66)**

	Construction and Materials					
ob No.:	County: Shannon	Route: 19				
Design:	<b>Skew</b> : 25 RA	Location: Over Hurricane Creek				
Sent: 3	Logged By: Ricardo Todd	Operator: Gary Degraffenreid				
Station:	Northing: <u>384094.2</u>	Date of Work: 11/16/22-11/21/22				
Offset:	Easting: _577517.8	Depth to Water:				
Elevation: 709.2	Requested Northing: 384101.6	Depth Hole Open:				
Requested Station: 1+14.9	Requested Easting: 577513.5	Time Change:				
Requested Offset: 8.2 L	Equipment: _Acker Renegade ,Split-Spoon Sampler, NWD4					

Location Note: Offset to avoid existing Structure Requested Elevation: 706.3 Drill No.: G-9667 Hammer Efficiency: 92% Drilling Method: Hollow Stem Auger Depth (ft) RAD 3 Description Qu Test Results UCS = 2040 ksf MC = 0% 35.9-47.4' Cherty Dolomite, gray, thin bedded, (23)medium strong rock to very strong rock, **y** <sub>moist</sub> = 168.8 pcf moderately weathered, fine grained, Vuggy 670 Qu Test Results UCS = 313 ksf MC = 0% **y** moist = 163.7 pcf 72 (8) 665 Qu Test Results UCS = 1700 ksf MC = 0% \$\fomale\$\_{\text{moist}} = 172.2 \text{pcf} 98 (38)

N<sub>80</sub> = (Em/60)Nm N<sub>80</sub> - Corrected N value for standard 60% SPT efficiency; Em - Measured hammer efficiency in percent; Nm - Observed N-value (1) = Assumed, (2) = Actual

Coordinate System: U.S. State Plane 1983 Coordinate Zone: Missouri East Coordinate Datum: NAD 83 (CONUS) Coordinate Units: U.S. Survey Feet

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## Missouri Department of Transportation

BORING	NO.	401-2	(T-22-62)
			Page 1 of 1

	Constituction and Materials	
ob No.:	County: Shannon	Route: 19
esi <b>g</b> n: _A9309	<b>Skew:</b> 25 RA	Location: Over Hurricane Creek
ent: <u>4</u>	Logged By: Matthew Kistler	Operator: Kenneth Tuttle
ation:	Northing: <u>384038.8</u>	Date of Work: 11/16/22-11/16/22
ffset:	Easting: <u>577486.2</u>	Depth to Water:
evation: <u>728.3</u>	Requested Northing: 384044.5	Depth Hole Open:
equested Station: 1+76.3	Requested Easting: 577483.3	Time Change:
equested Offset: 12.0 R	Equipment: CMF 45 Split-Spoon Sampler N	IX

Requ	ested	Offset: 12.0 R Equipment:	CME	45 ,S	plit-Spoc	n Sampler, NX			
Requ	ested I	Elevation: 728.0 Location No	Location Note: Auxillary Hole						
Drill N	No.: _@	G-9577 Hammer Effi	ciency:	: <u>87</u>	%	Dı	rilling Method: <u></u>	Hollow Stem Auge	er
O Depth	Graphic	Description	Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N <sub>60</sub> )	Shear Data	Field Tests	Index Tests
   5		O.0-0.5' ASPHALT     O.5-1.8' Black, GRAVEL, dry, fine to medium grained, base rock     1.8-12.6' Reddish brown, FAT CLAY scattered fine gravel, scattered fine sand, stiff, moist	725		40	5-3-3		PP = 0.75 tsf	MC = 40.9%
   5 10			720			(9) 1-2-5			γ sat = 112 pcf <sup>(1)</sup>
		\ 12.6-12.7' Dolomite, moderately weathered \int 12.7-16.2' Dolomite, light gray, thin bedded,	715		40 (100)	(10) (10)		PP = 0.75 tsf	γ sat = 119 pcf <sup>(1)</sup>
15	// // //	medium strong rock, slightly weathered, very fine grained  16.2-23.7' Cherty Dolomite, light gray, thin bedded, medium strong rock, slightly weathered, very fine grained, Pitted	710		98 (12)				
20			- - - -		50 (0)				
25		23.7-27.7' Cherty Dolomite, light gray, thin bedded, strong rock, slightly weathered, very fine grained	705		84 (20)				
20 20 20 20 20 20 20 20 20 20 20 20 20 2		Bottom of borehole at 27.7 feet.							

 $N_{e0}$  = (Em/60)Nm  $N_{e0}$  - Corrected N value for standard 60% SPT efficiency; Em - Measured hammer efficiency in percent; Nm - Observed N-value (1) = Assumed, (2) = Actual

Coordinate System:	U.S. State Plane 1983	Coordinate Zone:	Missouri East	Coordinate Proj. Factor:
Coordinate Datum:	NAD 83 (CONUS)	Coordinate Units	U.S. Survey Feet	

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BORING DATA NEAR INT. BENT NO. 3 AND END BENT NO. 4

Note: For locations of borings, see Sheet No. 1.

Designed June 2023 Detailed June 2023 Checked July 2023

Note: This drawing is not drawn to scale. Follow dimensions. Sheet No. 35 of 35

Coordinate Proj. Factor: \_

J9P3687

PROJECT NO. A9309

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