#### DESIGN DESIGNATION ROUTE F

A.A.D.T. 2021 = 82 A.A.D.T. - 2041 = 107 D.H.V. = 10%

T = 13%

V = 55 M.P.H. D = 50% / 50%

FUNCTIONAL CLASSIFICATION- MINOR COLLECTOR

#### US - 59

A.A.D.T. - 2021 = 918A.A.D.T. - 2041 = 1,239

V = 60 M.P.H. D = 50% / 50%

FUNCTIONAL CLASSIFICATION- MINOR ARTERIAL

#### ROUTE T

A.A.D.T. - 2021 = 102 A.A.D.T. - 2041 = 133

V = 55 M.P.H

D = 50% / 50%

US - 59

FUNCTIONAL CLASSIFICATION- MAJOR COLLECTOR

#### NO RIGHT OF WAY ACQUISITION

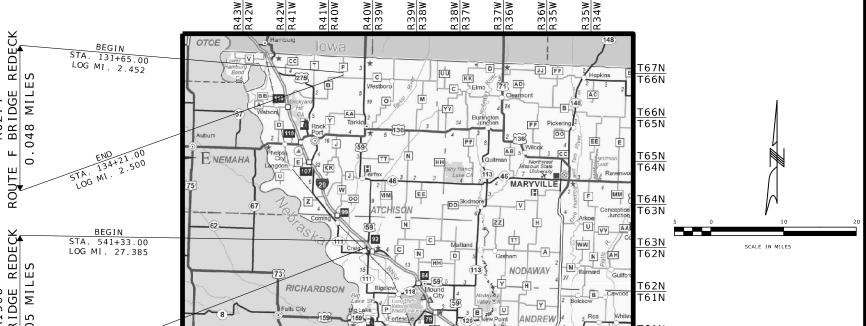
#### CONVENTIONAL SYMBOLS

,	
EXISTING	NEW
0000	••••• ••••
- FOOTVUTV OT UT OE UE S SS G W -	-OTV- -UTV- -OT- -UT- -OE-
Ð	)
	3
	€
₽ <sup>™</sup>	€
٦	
=	⊨
SIGN	-
PED	·—
<sup>BM</sup> ⊗	)
	- FO OTV UTV UT OE UE SS G W - W W W W W W W W W W W - W W W W W W W W W W W - W -

NOTE: DASHED OR OPEN SYMBOLS INDICATE EXISTING FEATURES

## MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

## PLANS FOR PROPOSED STATE HIGHWAY ATCHISON & HOLT COUNTY



JNTIES

es) - Square Miles 69.704

# N0811 ROUTE T BRIDGE REDECK 0.073 MILES

# THE EXISTENCE AND APPROXIMATE LOCATION OF UTILITY FACILITIES KNOWN TO EXIST, AS SHOWN ON THE PLANS, ARE BASED ON THE BEST INFORMATION AVAILABLE TO THE COMMISSION AT THIS TIME. THIS INFORMATION IS PROVIDED BY THE COMMISSION "AS-IS" AND THE COMMISSION EXPRESSLY DISCLAIMS ANY REPRESENTATION OR WARRANTY AS TO THE COMPLETENESS, ACCURACY, OR SUITABILITY OF THE INFORMATION FOR ANY USE. RELIANCE UPON THIS INFORMATION IS DONE AT THE RISK AND PERIL OF THE USER, AND THE COMMISSION SHALL NOT BE LIABLE FOR ANY DAMAGES THAT MAY ARISE FROM ANY FROR IN THE INFORMATION. IT IS, THEREFORE, THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY THE EXISTENCE, LOCATION AND STATUS OF ANY FACILITY. SUCH VERIFICATION INCLUDES DIRECT CONTACT WITH THE LISTED UTILITIES.

#### INDEX OF SHEETS

DESCRIPTION	SHEET NUMBER
TITLE SHEET	1
TYPICAL SECTIONS (TS) (1 SHEET)	2
QUANTITIES (QU) (3 SHEETS)	3
RTE F	
PLAN-PROFILE (PP)	4
TRAFFIC CONTROL SHEETS (TC)	5
EROSION CONTROL SHEETS (EC)	6
SIGNING (SN)	7 - 8
PAVEMENT MARKING (PM)	9
US-59	
PLAN-PROFILE (PP)	10
TRAFFIC CONTROL SHEETS (TC)	11-24
SPECIAL SHEET (I-29)	25
EROSION CONTROL SHEETS (EC)	26
PAVEMENT MARKING (PM)	27
RTE T	
PLAN-PROFILE (PP)	28
TRAFFIC CONTROL SHEETS (TC)	29
EROSION CONTROL SHEETS (EC)	30
SIGNING (SN)	31-32
PAVEMENT MARKING (PM)	33
BRIDGE DRAWINGS (B)	
R0274	1 - 9
A1906	1 - 14
N0811	1 - 10

## LENGTH OF PROJECT

#### ROUTE F

BEGINNING	STA.	131+65.00	
END	STA.	134+21.00	
APPARENT LENGTH		256	FEET
EQUATIONS AND EXCEPTIONS:			
NONE			
TOTAL CORRECTIONS		0.00	FEET

#### 115 - 59

05-3	) )		
BEGINNING	STA.	541+33.00	
END	STA.	546+90.00	
APPARENT LENGTH		557	FEET
EQUATIONS AND EXCEPTIONS:			
NONE			
TOTAL CORRECTIONS		0.00	FEET

#### ROUTE T

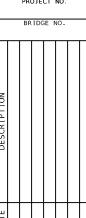
BEGINNING	STA.	124+15.00	
END	STA.	128+00.00	
APPARENT LENGTH		385	FEET
EQUATIONS AND EXCEPTIONS:			
NONE			
TOTAL CORRECTIONS		0.00	FEET

NET LENGTH OF PROJECT	1,198.00	FEET
STATE LENGTH	0.227	MILES
FOR INFORMATION ONLY ESTIMATED DISTURBED ACRES	0.58	ACRES



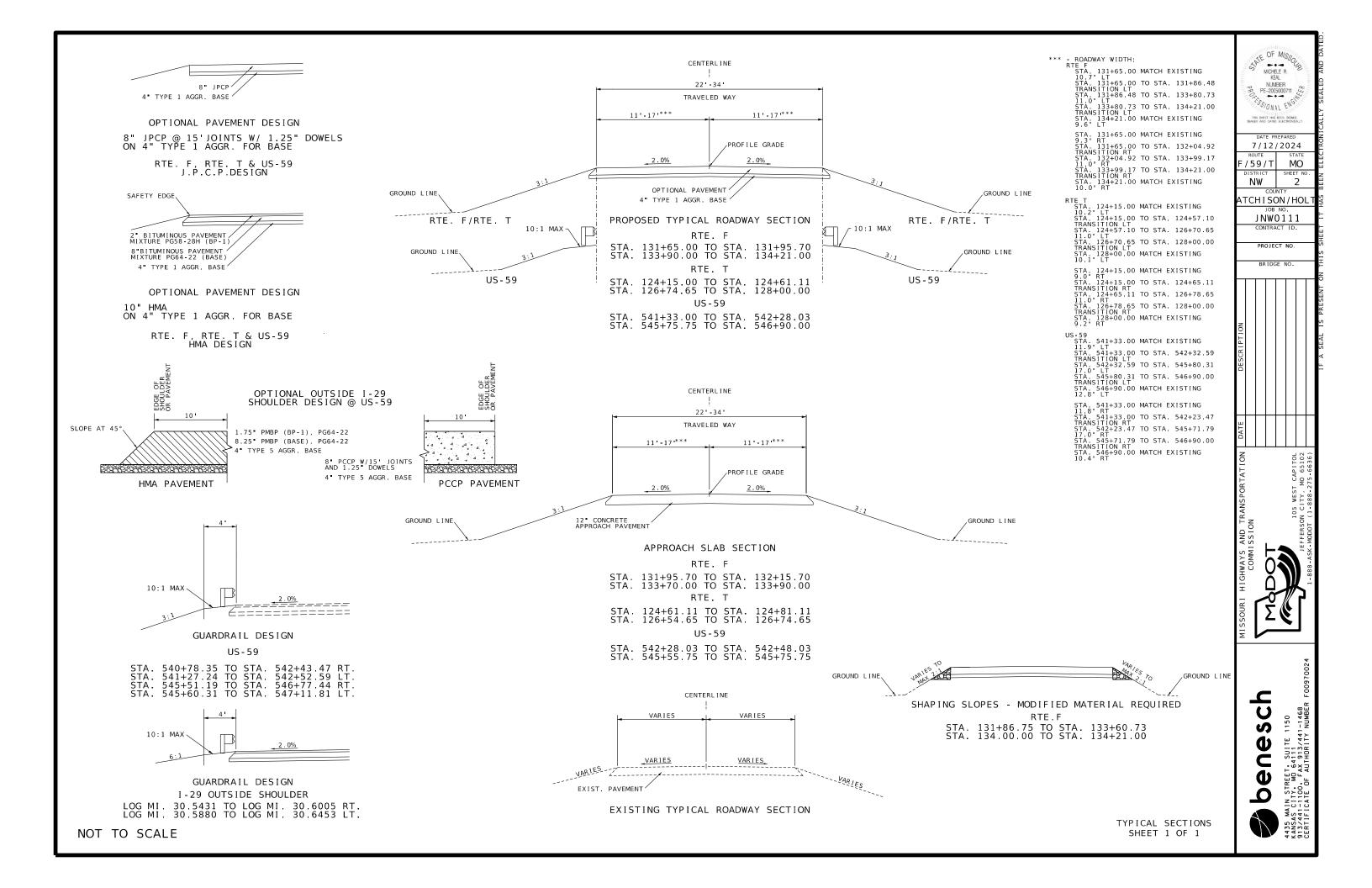
DATE PREPARED				
7/12/2024				
ROUTE	STATE			
/59/T	MO			
ISTRICT	SHEET NO.			
NW 1				

ROUTE	STATE
59/T	MO
ISTRICT	SHEET NO.
NW	1
COU	NTY
CHIS	ON/HOLT





<del>C</del> S 0 0 2



			PROVEMENTS		
STATION	STATION	SIDE	DESCRIPTION	QUANTITY	UNITS
RTE F	_				
131+65.00	132+15.70	CL	PAVEMENT	109.9	SY
131+65.71		LT	SIGN	1	EA
131+85.30		LT	SIGN	1	EA
131+85.42		RT	SIGN	1	EA
132+04.40		RT	SIGN	1	EA
132+04.55		LT	SIGN	1	EA
132+22.85		RT	SIGN	1	EA
133+69.95	134+21.00	CL	PAVEMENT	108.5	SY
133+82.74		RT	SIGN	1	EA
133+99.21		LT	SIGN	1	EA
134+01.10		RT	SIGN	1	EA
134+19.75		LT	SIGN	1	EA
134+21.46		RT	SIGN	1	EA
			SUBTOTAL	1	LS
US-59	<b>.</b>				
540+78.35	542+43.47	RT	GUARDRAIL	54078.4	LF
541+27.01	542+52.59	LT	GUARDRAIL	54127.0	LF
541+33.00	542+28.03	CL	PAVEMENT	350.8	SY
542+28.03	542+48.03	CL	APPROACH SLAB	75.5	SY
545+51.19	546+77.49	RT	GUARDRAIL	54551.2	LF
545+60.31	547+11.81	LT	GUARDRAIL	54560.3	LF
545+55.75	545+75.75	CL	APPROACH SLAB	75.5	SY
545+75.75	546+90.00	CL	PAVEMENT	418.4	SY
		I-29 SB	OUTSIDE SHOULDER	1400.4	SY
		I-29 NB	OUTSIDE SHOULDER	1399.2	SY
			SUBTOTAL	1	LS
RTE T					
124+15.00	124+81.11	CL	PAVEMENT	138.4	SY
124+40.68		LT	SIGN	1	EA
124+44.12		RT	SIGN	1	EA
124+55.77		LT	SIGN	1	EA
124+63.94		RT	SIGN	1	EA
124+74.72		LT	SIGN	1	EA
124+83,78		RT	SIGN	1	EA
126+51.90		LT	SIGN	1	EA
126+54.65	128+00.00	CL	PAVEMENT	308.5	SY
126+61.06	120 - 00.00	RT	SIGN	1	EA
126+80.59		RT	SIGN	1	EA
126+80.00		LT	SIGN	1	EA
126+80.00		LT	SIGN	1	EA
126+96.26		RT	SIGN	1	EA
120797.01	1	J KI	SUBTOTAL	1	LS

OPTIONAL OUTSIDE I-29 SHOULDER					
	OPTIONAL TYPE 5				
BEGIN	END		I-29 OUTSIDE	AGGREGATE	
LOG MILE	LOG MILE	LOCATION	SHOULDER	BASE (4")	
			(SY)	(SY)	
30.4758	30.7145	I-29 SB	1400.4	1400.4	
30.4782	30.7167	I-29 NB	1399.2	1399.2	
TOTAL 2799.6 2799.6					
		PAY TOTALS	2800	2800	

OPTIONAL RUMBLE STRIPS				
BEGIN	END			
LOG MILE	LOG MILE	LOCATION	QUANTITY	
			(100 FT.)	
30.4758	30.7145	I-29 SB	12.6	
30.4782	30.7167	I-29 NB	12.6	
	25.2			

CLEARING AND GRUBBING							
BEGIN STATION	END STATION	(AC)					
RTE F							
131+65.00	134+21.00	0.1					
	SUBTOTAL	0.1					
US-59							
540+45.75	540+45.75 547+39.94						
	SUBTOTAL						
RTE T							
124+15.00	128+00.00	0.1					
	SUBTOTAL	0.1					
	PAY TOTAL	1.0					

MOBILIZATION	
1 LUMP SUM	

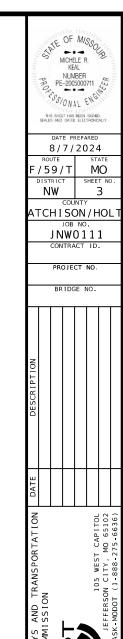
## CONTRACTOR FURNISHED SURVEYING & STAKING 1 LUMP SUM

		EARTHWORK	, L	
BEGIN STATION	END STATION	SIDE	SHAPING SLOPES CLASS III (100 FT)	SHAPING SLOPES MODIFIED MATERIAL REQUIREMENT (100 FT)
RTE F	•			
131+65.00	132+06.48	LT	0.1	0.3
131+65.00	132+26.88	RT	0.0	0.6
133+60.73	134+21.00	LT	0.5	0.1
133+79.17	134+21.00	RT	0.0	0.4
		SUBTOTAL	0.6	1.4
US-59				
540+45.75	542+43.47	RT	2.0	0.0
541+00.00	542+52.59	LT	1.5	0.0
545+51.19	547+22.32	RT	1.7	0.0
545+60.31	547+39.68	LT	1.8	0.0
I-29 SB			3.2	
I-29 NB			3.2	
		SUBTOTAL	13.4	0.0
RTE T				
124+15.00	124+77.10	LT	0.6	0.0
124+15.00	124+85.11	RT	0.7	0.0
126+50.65	128+00.00	LT	1.5	0.0
126+58.65	128+00.00	RT	1.4	0.0
		SUBTOTAL	4.2	0.0
		PAY TOTAL	18	2

OP <sup>-</sup>	OPTIONAL PAVEMENT (F, 59, & T)							
			TYPE 1					
BEGIN	END	OPTIONAL	AGGREGATE					
STATION	STATION	PAVEMENT	BASE (4")					
		(SY)	(SY)					
RTE F								
131+65.00	131+95.70	71.5	71.5					
133+90.00	134+21.00	71.9	71.9					
	SUBTOTALS	143.4	143.4					
US-59								
541+33.00	542+28.03	304.7	304.7					
545+75.75	546+90.00	363.1	363.1					
	SUBTOTALS	667.8	667.8					
RTE T								
124+15.00	124+61.11	103.9	103.9					
126+74.65	128+00.00	272.8	272.8					
	SUBTOTALS	376.7	376.7					
	PAY TOTALS	1188	1188					

_										
	GUARDRAIL									
					TYPE A	MGS				
					CRASHWORTHY	BRIDGE	MGS			
				MGS	END TERMINAL	APPROACH	END			
	LOCA	TION	SIDE	GUARDRAIL	(MASH)	TRANSITION	ANCHOR			
				(LF)	(EA)	(EA)	(EA)			
	US-59									
	BEGIN STATION	END STATION								
	540+78.35	542+43.47	RT.	75.0	1	1				
	541+27.24	542+52.59	LT.	37.5	1	1				
	545+51.19	546+77.44	RT.	37.5	1	1				
	545+60.31	547+11.81	LT.	62.5	1	1				
	I-29									
	BEGIN LOG MILE	END LOG MILE								
	30.5431	30.6005	RT.	237.5	1		1			
1	30.5880	30.6453	LT.	237.5	1		1			
1		SUB	<b>FOTALS</b>	687.5	6	4	2			
-		PAY	TOTALS	688	6	4	2			

SUMMARY OF QUANTITIES SHEET 1 OF 3





	T	EMPORAR	Y EROSION	CONTROL		
				TYPE 3B		
BEGIN	END		SILT	EROSION	TYPE C	SEDIMENT
STATION	STATION	SIDE	FENCE	CONTROL	BERM	REMOVAL
				BLANKET		
			(LF)	(SY)	(LF)	(CY)
RTE F		'				
131+65.00	132+09.90	RT	69.2			1
131+65.00	132+10.00	RT		39.2		
131+65.00	132+26.88	LT		14.4		
131+65.02	132+31.13	LT	45.2			1
132+09.90	132+46.50	CL			148.1	2
132+96.99	133+78.27	CL			143.3	2
133+60,81	134+20.96	LT	60.5			1
133+78.27	134+21.83	RT	43.6			1
133+79.17	134+21.00	RT		10.8		
	SI	JBTOTALS	218.5	64.4	291.4	8
US-59						
540+28.96	542+30.56	RT	207.0			2
540+86.82	542+52.24	LT	179.0			2
545+60.82	547+65.00	LT	213.0			2
545+65.24	547+51.52	RT	202.0			2
I-29		RT	434.2			4
I-29		LT	418.9			4
	SI	JBTOTALS	1654.1			16
RTE T						
124+02.00	124+81.00	LT	83.4			1
124+02.00	124+95.00	RT	97.5			1
124+80.64	125+60.72	CL			194.4	2
126+03.50	126+61.06	CL			148.9	2
126+49.00	128+14.00	LT	170.8			2
126+60.00	128+03.00	RT	148.7			2
	SI	JBTOTALS	500.4		343.3	10
	P/	AY TOTALS	2373	64	635	34

PAVEMENT MARKING							
				STANDARD	WATERBORNE PA	VEMENT	
					MARKING PAINT		
					TYPE P BEADS		
BEGIN	END			4" SOLID	4" INT.	4" SOLID	
						DOUBLE	
STATION	STATION	ROUTE	LENGTH	WHITE	YELLOW	YELLOW	REMARKS
			(FT)	(FT)	(FT)	(FT)	
131+65.00	134+21.00	F	256.00	512.0	64.0	0.0	
541+33.00	546+90.00	US-59	557.00	1114.0	0.0	1114.0	
124+15.00	128+00.00	T	385.00	0.0	0.0	770.0	
			TOTALS	1626.0	64.0	1884.0	
			PAY				
			TOTALS	1,626	1,9	48	
NO	OTE: TEMPORAF	RY AND PERMAI	NENT PAVE	MENT MARKING SHA	ALL BE IN ACCORD	ANCE WITH 620.1	0.

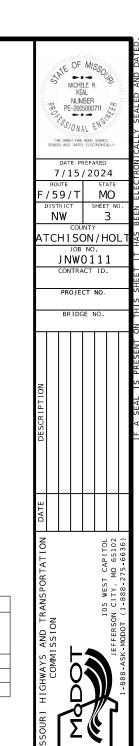
				=					
	PAVEMENT MARKING								
				HIGH	BUILD WATERBOR	NE			
				PAVEN	IENT MARKING PA	INT,			
					TYPE L BEADS				
BEGIN	END			6" INTERMITTENT	6" SOLID	6" SOLID			
LOG MILE	LOG MILE	LOCATION	SIDE	WHITE	WHITE	YELLOW			
				(LF)	(LF)	(LF)			
30.1509	30.7148	I-29 SB	RT	2978.6	2978.6	2978.6			
30.4709	31.0421	I-29 NB	LT	3016.1	3016.1	3016.1			
			TOTAL	1498.7	5994.7	5994.7			
		PA	Y TOTALS	7,49	93	5,995			

	PAVEMENT MARKING REMOVAL								
BEGIN	END			PAVEMENT	EXISTING	TEMP. TRAFFIC			
LOG MILE	LOG MILE	LOCATION	PHASE	MARKING	MARKINGS	CONTROL MARKINGS	REMARKS		
				TYPE	(LF)	(LF)			
30.2566	30.7148	I-29 SB	1	6" INTERMITTENT WHITE	2552.0				
30.4780	30.9263	I-29 NB	1	6" INTERMITTENT WHITE	2574.0				
30.2566	30.7148	I-29 SB	3	4" SOLID WHITE		2978.0			
30.4780	30.9263	I-29 NB	3	4" SOLID WHITE		2999.0			
30.1181	30.6876	I-29 SB	3	4" SOLID WHITE	1179.0				
30.5091	30.9618	I-29 NB	3	4" SOLID WHITE	1197.0				
30.1589	30.7153	I-29 SB	4	4" SOLID WHITE		1177.6			
30.4781	31.0344	I-29 NB	4	4" SOLID WHITE		1197.0			
30.1589	30.7153	I-29 SB	4	4" SOLID YELOW		2940.0			
30.4781	31.0344	I-29 NB	4	4" SOLID YELLOW		2939.0			
30.4776	30.9271	I-29 NB	5	4" SOLID WHITE		2371.0			
30.2566	30.7141	I-29 SB	5	4" SOLID WHITE		2939.0	FINAL REMOVALS		
·		·	TOTAL		7502.0	19540.6			
		PAY	/ TOTALS		7,502	19,541			

TRAFFIC CONTROL PAVEMENT MARKING								
				STANDARI				
BEGIN LOG MILE	END LOG MILE	LOCATION	SIDE	4" SOLID WHITE (LF)	4" INTERMITTENT WHITE (LF)	4" SOLID YELLOW (LF)	REMARKS	
30.2566	30.7148	I-29 SB	RT	2978.0	(Li )	(Li )	PHASE 1	
30.4780	30.9263	I-29 NB	LT	2999.0			PHASE 1	
30.1589	30.7153	I-29 SB	RT	1177.6		2940.0	PHASE 3	
30.4781	31.0344	I-29 NB	LT	1197.0		2939.0	PHASE 3	
30.2566	30.7141	I-29 SB	RT	2413.5			PHASE 4	
30.4776	30.9271	I-29 NB	LT	2371.0			PHASE 4	
			TOTAL	13136.1	0.0	5879.0		
		PAY	TOTALS	13,136	0	5,879		

	SEEDING AND MULCHING						
		COOL SEASON					
BEGIN STATION	END STATION	MIXTURES	MULCHING				
		(AC)	(AC)				
RTE F							
131+65.00	134+21.00	0.1	0.1				
	SUBTOTALS	0.1	0.1				
US-59							
540+44.79	547+40.03	0.2	0.2				
I-29		0.4	0.4				
	SUBTOTALS	0.6	0.6				
RTE T							
124+15.00	128+00.00	0.1	0.1				
	SUBTOTALS	0.1	0.1				
	PAY TOTALS	0.8	0.8				

	LINEAR GRADING (CLASS I)							
BEGIN LOG MILE	END LOG MILE	LOCATION						
30.4758	30.7145	I-29 SB						
30.4782	30.7167	I-29 NB						
		TOTAL						





LINEAR GRADING STA. 12.6 12.6 25.2

																EFFECTIVE: 04-01-2023
			TOTAL	. QTY TO	TAL SIGN						QTY TOTAL	SIGN				
	SIZE	AREA QTY	AREA	RELOCRE	LOC NUM.			SIZE	AREA	QTY TOTAL F	RELOC RELOC	NUM.				
SIGN	IN.	SQ FT EACH	SQ FT	EACH SQ	.FT.		SIGN	IN.	SQ. FT I	EACH SQ FT	EACH SQ.FT.			ITEM	TOTA	L <b> </b>
		WAR	NING S	SIGNS	•	DESCRIPTION			<del> </del>	GUIDE SIGN	IS		DESCRIPTION	NUMBER	QTY	DESCRIPTION
WO1-1L	48X48					TURN (SYMBOL LEFT ARROW)	E05-1	36X48	12.00				GORE EXIT	6122008	<del></del>	IMPACT ATTENUATOR 40 MPH (SAND BARRELS)
WO1-1R	48X48	16.00				TURN (SYMBOL RIGHT ARROW)	E05-2	48X36	12.00				EXIT OPEN	6122009		IMPACT ATTENUATOR 45 MPH (SAND BARRELS)
WO1-2L	48X48	16.00				CURVE (SYMBOL LEFT ARROW)	E05-2a	48X36	12.00				EXIT CLOSED	6122010		IMPACT ATTENUATOR 50 MPH (SAND BARRELS)
WO1-2R						CURVE (SYMBOL RIGHT ARROW)	GO20-1		10.00			1	ROAD WORK NEXT XX MILES	6122012		IMPACT ATTENUATOR 55 MPH (SAND BARRELS)
WO1-3L	48X48					REVERSE TURN (SYMBOL LEFT ARROW)	GO20-2			4 32.00		7	END ROAD WORK	6122014		IMPACT ATTENUATOR 60 MPH (SAND BARRELS)
WO1 - 3R			22.00			REVERSE TURN (SYMBOL RIGHT ARROW)	GO20-4		4.50				PILOT CAR FOLLOW ME	6122017		IMPACT ATTENUATOR 65 MPH (SAND BARRELS)
WO1-4L WO1-4R	48X48		32.00			REVERSE CURVE (SYMBOL BECHT ARROW)	GO20-4a GO20-4a		8 75 1 50				PILOT CAR IN USE WALT & FOLLOW	6122019		IMPACT ATTENUATOR 70 MPH (SAND BARRELS) REPLACEMENT SAND BARREL
WO1-4K WO1-4bL			32.00		12	REVERSE CURVE (SYMBOL RIGHT ARROW) DOUBLE ARROW REVERSE CURVE (SYMBOL LT ARROWS)	GO20-4a			8 48.00		2	PILOT CAR IN USE WAIT & FOLLOW WORK ZONE (PLAQUE)	6122030		IMPACT ATTENUATOR (RELOCATION)
WO1 -4bR						DOUBLE ARROW REVERSE CURVE (SYMBOL RT ARROWS)	MO4 - 8a		3.00				END DETOUR	6123000A		TRUCK OR TRAILER MOUNTED ATTENUATOR (TMA)
WO1-4cL						TRIPLE ARROW REVERSE CURVE (SYMBOL LT ARROWS)	MO4 - 9L		12.00	2 0.00			DETOUR (LEFT ARROW)	6161008		ADVANCED WARNING RAIL SYSTEM
WO1-4cR						TRIPLE ARROW REVERSE CURVE (SYMBOL RT ARROWS)	MO4 - 9R		12.00				DETOUR (RIGHT ARROW)	6161012		BUOYS (BOATS KEEP OUT)
WO1-6	60X30	12.50				HORIZONTAL ARROW (SYMBOL)	MO4-9P	48X12	4.00				STREET NAME (PLAQUE)	6161013		BUOYS (NO WAKE)
WO1-6a	72X36	18.00				HORIZ. ARROW (SYMBOL ON PERMANENT BARRICADE)	MO4 - 10L		6.00				DETOUR (ARROW LEFT)	6161014		SPECIAL SIGN ASSEMBLY (BOATS KEEP OUT)
WO1-7	60X30					DOUBLE HEAD HORIZONTAL ARROW (SYMBOL)	MO4 - 10R	48X18					DETOUR (ARROW RIGHT)	6161025		CHANNELIZER (TRIM LINE)
WO1 - 7a						DOUBLE HEAD HORIZ. ARROW (SYMBOL ON PERM. BARR.)		1407440		REGULATORY	' SIGNS			6161030		TYPE III MOVEABLE BARRICADE
WO1 - 8	18X24					CHEVRON (SYMBOL)	R1-1		13.25				STOP	6161033		DIRECTION INDICATOR BARRICADE
WO1-8a WO3-1	30X36 48X48		1			CHEVRON (SYMBOL FOR DIVIDED HIGHWAYS) STOP AHEAD (SYMBOL)	R1-2 R1-2a		9 00	+			YIELD TO ONCOMING TRAFFIC (PLAQUE)	6161040 6161047		FLASHING ARROW PANEL TYPE III OBJECT MARKER
WO3 - 1	48X48 48X48		1			YIELD AHEAD (SYMBOL)	R1-2a		2.50	+			ALL WAY (PLAQUE)	6161047		SEQUENTIAL FLASHING WARNING LIGHT
WO3 - 2	48X48		1			SIGNAL AHEAD (SYMBOL)	R2-1		12.00	8 96.00		4.8	SPEED LIMIT XX	6161070		TUBULAR MARKER
WO3 - 4	48X48					BE PREPARED TO STOP	R3-1		16.00	1 20.00		., •	NO RIGHT TURN (SYMBOL)	6161095		RADAR SPEED ADVISORY SYSTEM
WO3 - 5	48X48					SPEED LIMIT AHEAD	R3-2		16.00				NO LEFT TURN (SYMBOL)			CHANGEABLE MESSAGE SIGN,
WO4-1L	48X48	16.00				MERGE (SYMBOL FROM LEFT)	R3-3	36X36	9.00				NO TURNS	6161096		COMMISSION FURNISHED/RETAINED
WO4-1R	48X48	16.00				MERGE (SYMBOL FROM RIGHT)	R3-4	48X48	16.00				NO U-TURN (SYMBOL)			CHANGEABLE MESSAGE SIGN W/O COMM.
WO4-1aL						MERGE (ARROW SYMBOL)	R3-7L		6.25				LEFT LANE MUST TURN LEFT	6161098	2**	INTERFACE - CONTRACTOR FURNISHED/RETAINED
WO4-1aR			32.00		13	MERGE (ARROW SYMBOL)	R3 - 7R		6.25				RIGHT LANE MUST TURN RIGHT			CHANGEABLE MESSAGE SIGN WITH COMM.
WO5 - 1	48X48					ROAD/BRIDGE/RAMP NARROWS	R4-1		12.00				DO NOT PASS	6161099		INTERFACE - CONTRACTOR FURNISHED/RETAINED
WO5 - 3 WO5 - 5	48X48 48X48					ONE LANE BRIDGE NARROW LANES	R4-2 R4-8a		12.00	4 48.00		46A	PASS WITH CARE KEEP LEFT (HORIZONTAL ARROW)	6162000 <i>A</i>		WORK ZONE TRAFFIC SIGNAL SYSTEM TEMPORARY LONG-TERM RUMBLE STRIPS
WO6 - 1	48X48					DIVIDED HIGHWAY (SYMBOL)	R4-8a		12.00				KEEP RIGHT (HORIZONTAL ARROW)	6162002	8 * *	TEMPORARY TRAFFIC BARRIER
WO6 - 2	48X48					DIVIDED HIGHWAY END (SYMBOL)	R5 - 1		6.25				DO NOT ENTER	61736000	1360*	* CONTRACTOR FURNISHED/RETAINED
WO6 - 3	48X48					TWO WAY TRAFFIC (SYMBOL)	R5-1a		6.00				WRONG WAY	01730002	1300	TEMPORARY TRAFFIC BARRIER
WO7 - 3a	30X24					NEXT XX MILES (PLAQUE)	R6-1L		6.75				ONE WAY ARROW (LEFT)	6173602E	3	CONTRACTOR FURNISHED/COMMISSION RETAINED
WO8 - 1	48X48	16.00				BUMP	R6-1R	54X18	6.75				ONE WAY ARROW (RIGHT)	6174000A	1	TEMP. TRAFFIC BARRIER HEIGHT TRANSITION
WO8 - 2	48X48					DIP	R6-2L		5.00				ONE WAY (LEFT)	6175010A	1030*	* RELOCATING TEMPORARY TRAFFIC BARRIER
WO8 - 3	48X48					PAVEMENT ENDS	R6-2R		5.00				ONE WAY (RIGHT)			TEMPORARY TRAFFIC BARRIER
WO8 - 4	48X48					SOFT SHOULDER	R9-9	24X12	2.00				SIDEWALK CLOSED	6176000E	3	COMMISSION FURNISHED/RETAINED
WO8 - 5	48X48 48X48					SLIPPERY WHEN WET (SYMBOL) TRUCK CROSSING (WITH FLAGS)	 	24710	3 00				SIDEWALK CLOSED AHEAD,	6177000		TEMP. TRAFFIC BARRIER HEIGHT TRANSITION
WO8-6 WO8-6c	48X48 48X48					TRUCK ENTRANCE	R9-11L	24X18	3.00				(ARROW LEFT) CROSS HERE SIDEWALK CLOSED AHEAD,	6177000E		COMMISSION FURNISHED/RETAINED TEMPORARY RAISED PAVEMENT MARKER
WO8 - 7	36X36					LOOSE GRAVEL	R9-11R	24X18	3.00				(ARROW RIGHT) CROSS HERE	9029400		TEMPORARY TRAFFIC SIGNALS
WO8 - 7a	36X36					FRESH OIL/LOOSE GRAVEL	R10-6		6.00				STOP HERE ON RED (45^ ARROW)	9029401		TEMPORARY TRAFFIC SIGNALS AND LIGHTING
WO8 - 9	48X48	16.00				LOW SHOULDER	R11-2			6 60.00		63	ROAD CLOSED			
WO8 - 11	48X48	16.00				UNEVEN LANES							ROAD CLOSED XX MILES AHEAD			
WO8-12						NO CENTER LINE	-			3 37.50			LOCAL TRAFFIC ONLY			
WO8 - 15			1			GROOVED PAVEMENT	1			2 25.00			ROAD CLOSED TO THRU TRAFFIC			
WO8 - 15P			1			MOTORCYCLE (PLAQUE)	4 <b></b>			4 80.00			FINE SIGN	<u> </u>	-	
WO8-17 WO8-17P			1			SHOULDER DROP-OFF (SYMBOL) SHOULDER DROP-OFF (PLAOUE)	CONST - 3	η 56X12		4   18.68   MISCELLANE	OUS STONE	53	SPEEDING/PASSING (PLATE)			
W10-1			1			RAILROAD CROSSING	CONST - 5	48836		MISCELLANE	OUS SIGNS		POINT OF PRESENCE			
W10-1 W012-1			1			DOUBLE DOWN ARROW (SYMBOL)	CONST-5			+			POINT OF PRESENCE			
WO12-2			1			LOW CLEARANCE (SYMBOL)	CONST - 7						RATE OUR WORK ZONE			
W012-2X			1			LOW CLEARANCE (PLAQUE)	CONST - 7						RATE OUR WORK ZONE		-	TEMPORARY TRAFFIC CONTROL
WO12-2a						OVERHEAD LOW CLEARANCE (FEET AND INCHES)	4 <b></b>			4 48.00		56	WORK ZONE NO PHONE ZONE			CT PAY TOTAL 1 LUMP SUM
WO12-4						LOW CLEARANCE XX FT XX IN XX MILES AHEAD	SP-1			2 130.50			MO. RTE F CLOSED AHEAD		****	INCLUDED IN TEMPORARY TRAFFIC
WO12-5						WIDTH RESTRICTION XX FT XX IN XX MILES AHEAD	1			1 42.75			NB US-59 CLOSED AT I-29			INCLUDED IN TEMPORARY TRAFFIC L-1 LUMP SUM QUANTITY
WO13-1			-			ADVISORY SPEED (PLAQUE)	SP - 3			3 182.25			SB US-59 CLOSED AT I-29			
WO16-2			1			XXX FEET (PLAQUE)	4 <b></b>			3 195.75		E 0 -	MO. RTE T CLOSED AHEAD			ALL TEMPORARY TRAFFIC CONTROL ITEMS
WO16-3 WO20-1			1		6	X MILE (PLAQUE) ROAD/BRIDGE/RAMP WORK AHEAD	-			1 19.50 4 78.00			DETOUR ASSEMBLY NORTH/LEFT DETOUR ASSEMBLY SOUTH/LEFT			CLUDED IN PAY ITEM 616-99.01 LUMP MPORARY TRAFFIC CONTROL. UNLESS
		16.00 3	48 00			DETOUR AHEAD	4 <b></b>			3 58.50			DETOUR ASSEMBLY NORTH/RIGHT		OTHERW	ISE NOTED, QUANTITIES SHOWN ARE
		16.00 5				ROAD CLOSED AHEAD	4 <b></b>			3 58.50			DETOUR ASSEMBLY SOUTH/LEFT			TE ONLY AND SUBJECT TO CHANGE BASED
WO20-4			1.00			ONE LANE ROAD AHEAD	-			7 136.50			DETOUR ASSEMBLY NORTH/STRAIGHT		ON FIE	LD CONDITIONS.
		16.00 4	64.00		9	RIGHT/CENTER/LEFT LANE CLOSED AHEAD	SPECIAL					50 f	DETOUR ASSEMBLY SOUTH/STRAIGHT			
WO20-5a						2 RIGHT/CENTER/LEFT LANES CLOSED AHEAD	GO22-1					59				
		16.00 2	32.00		10	RIGHT/CENTER/LEFT LANE CLOSED										
WO20-7a						FLAGGER (SYMBOL, WITH FLAGS)	616-10			TOTAL						
WO21-2						FRESH OIL			ON SIGN	NS 2006.19	1=5=::					
WO21-5						SHOULDER WORK AHEAD	616-10		LCNC		TOTAL					
WO22-1 WO22-2			1			BLASTING ZONE AHEAD TURN OFF 2-WAY RADIO AND PHONE	RELOCA	VIED S	כאטוס							
WO22-2 WO22-3			1			END BLASTING ZONE										SUMMARY OF QUANTITIES
WO24 1			+			DOUBLE DEVENCE CURVE	ł									SHEET 3 OF 3

DOUBLE REVERSE CURVE

WO24-1 48X48 16.00

TRUCK OR TRAILER MOUNTED ATTENUATOR (TMA) SPECIAL SIGN ASSEMBLY (BOATS KEEP OUT) 28 SEQUENTIAL FLASHING WARNING LIGHT CHANGEABLE MESSAGE SIGN W/O COMM. 2\*\* INTERFACE - CONTRACTOR FURNISHED/RETAINED CHANGEABLE MESSAGE SIGN WITH COMM. INTERFACE - CONTRACTOR FURNISHED/RETAINED WORK ZONE TRAFFIC SIGNAL SYSTEM 2 8\*\* TEMPORARY LONG-TERM RUMBLE STRIPS CONTRACTOR FURNISHED/COMMISSION RETAINED TEMP. TRAFFIC BARRIER HEIGHT TRANSITION DA 1030\*\* RELOCATING TEMPORARY TRAFFIC BARRIER TEMP. TRAFFIC BARRIER HEIGHT TRANSITION TEMPORARY RAISED PAVEMENT MARKER TEMPORARY TRAFFIC SIGNALS AND LIGHTING NOTE: ALL TEMPORARY TRAFFIC CONTROL ITEMS ARE INCLUDED IN PAY ITEM 616-99.01 LUMP SUM TEMPORARY TRAFFIC CONTROL. UNLESS ESTIMATE ONLY AND SUBJECT TO CHANGE BASED

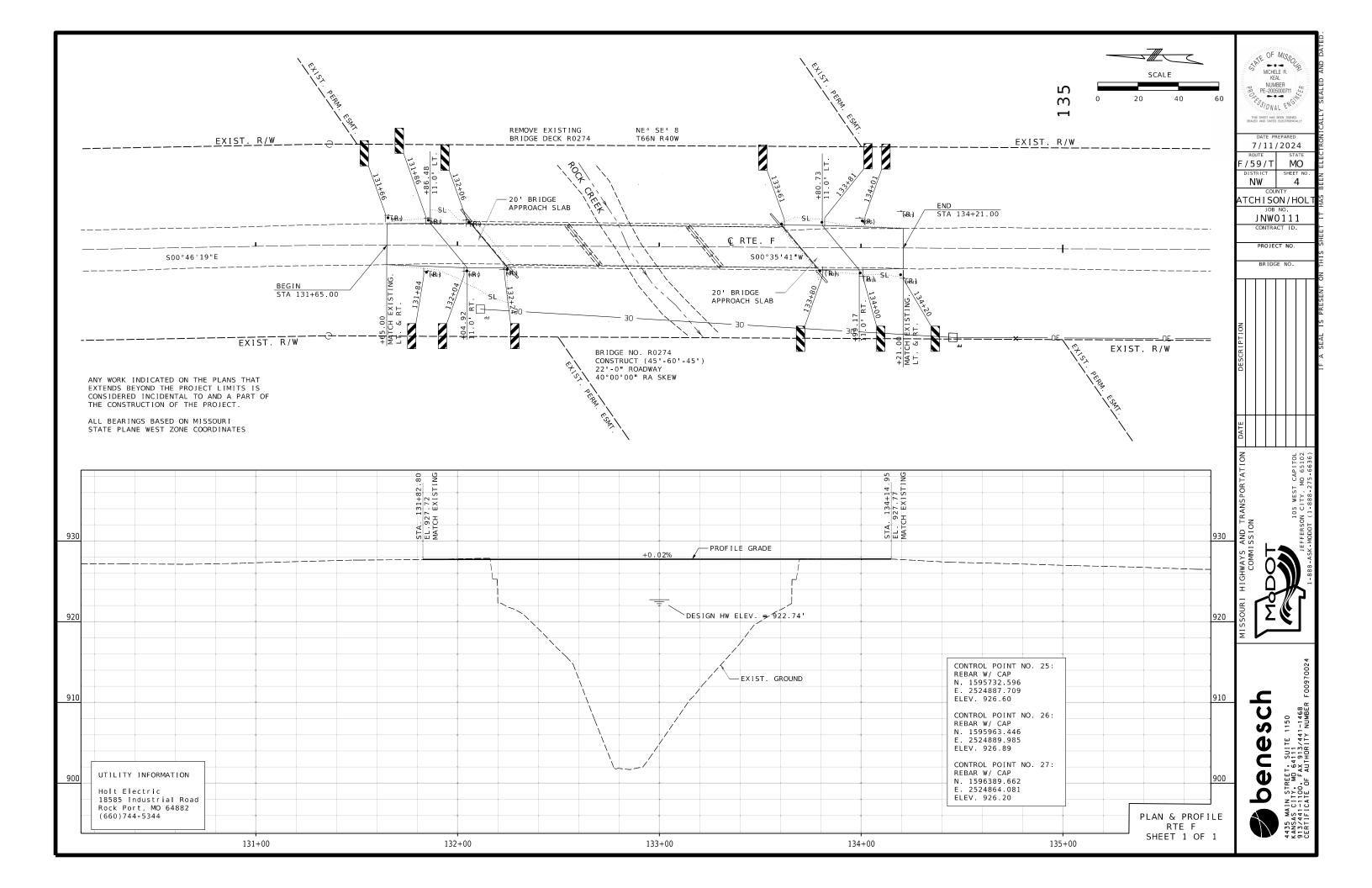
SATE OF MISSOLY MICHELE R. KEAL NUMBER PE-2005000711 FE-2005000711 E THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY.

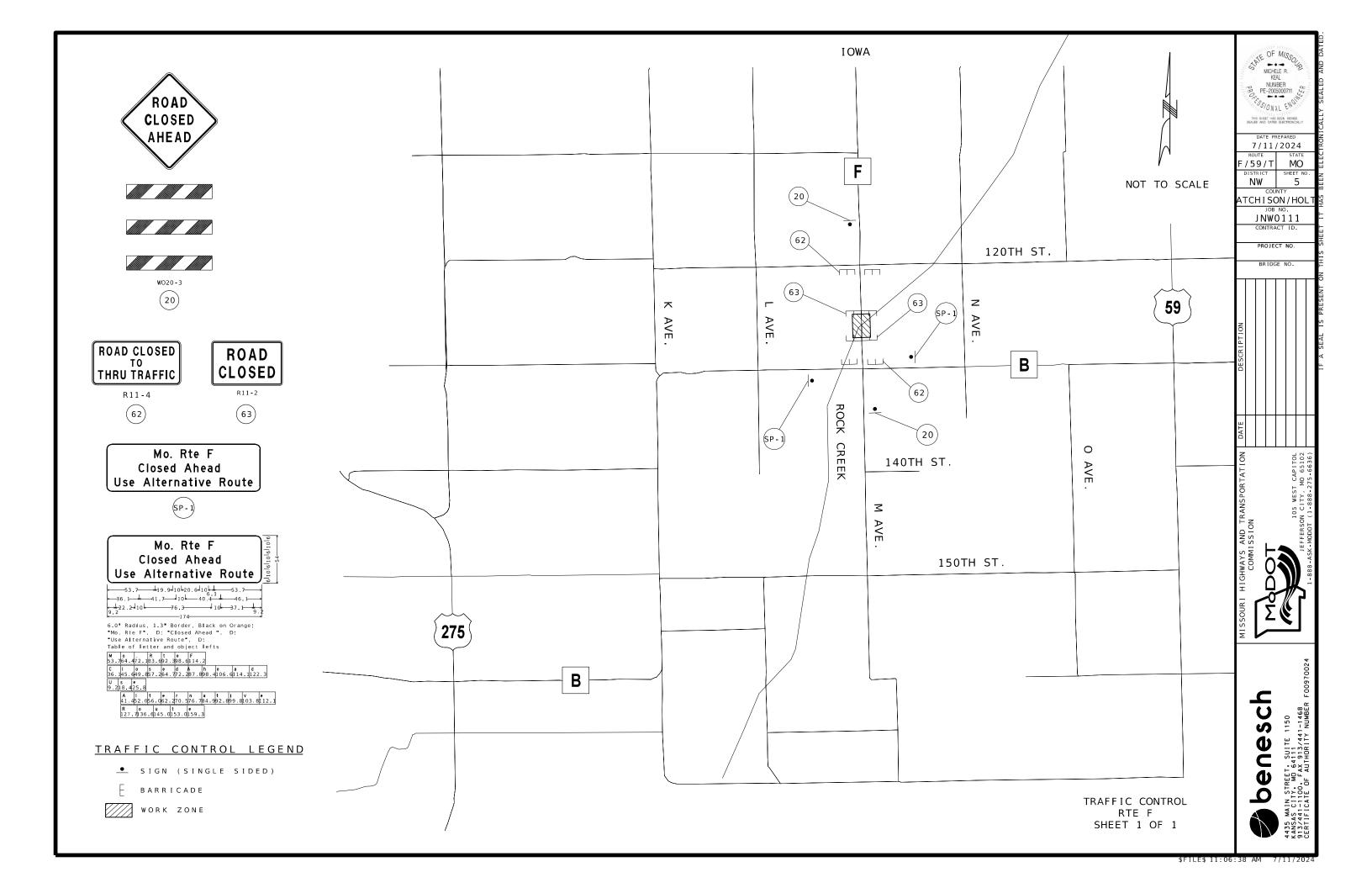
9/19/2024 F/59/T MO SHEET NO N⋅W 3

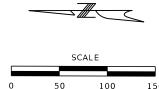
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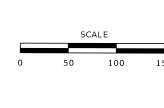
> PROJECT NO. BRIDGE NO.

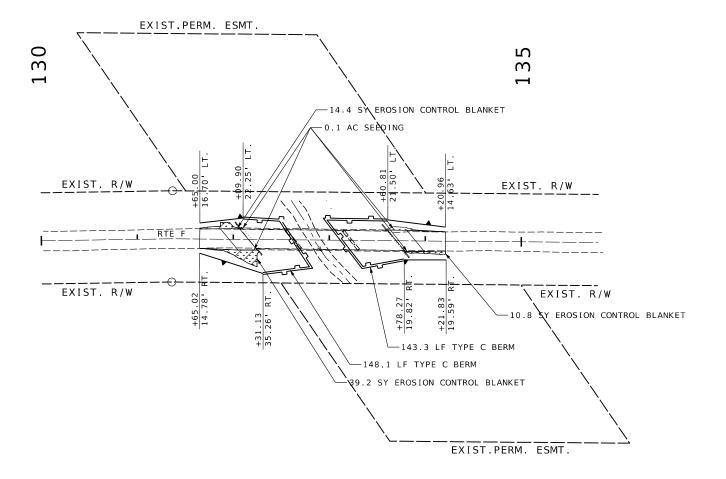
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TEMPORARY EROSION CONTROL LEGEND

EROSION CONTROL BLANKET SILT FENCE

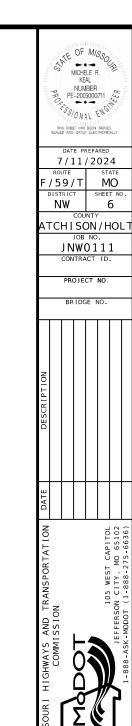
TEMPORARY TYPE C BERM

PERMANENT EROSION CONTROL LEGEND



PERMANENT SEEDING AND MULCHING

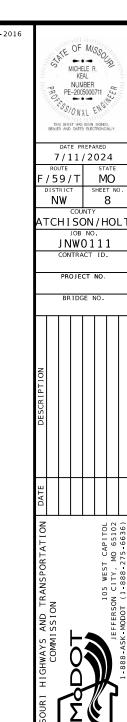
EROSION CONTROL RTE F SHEET 1 OF 1

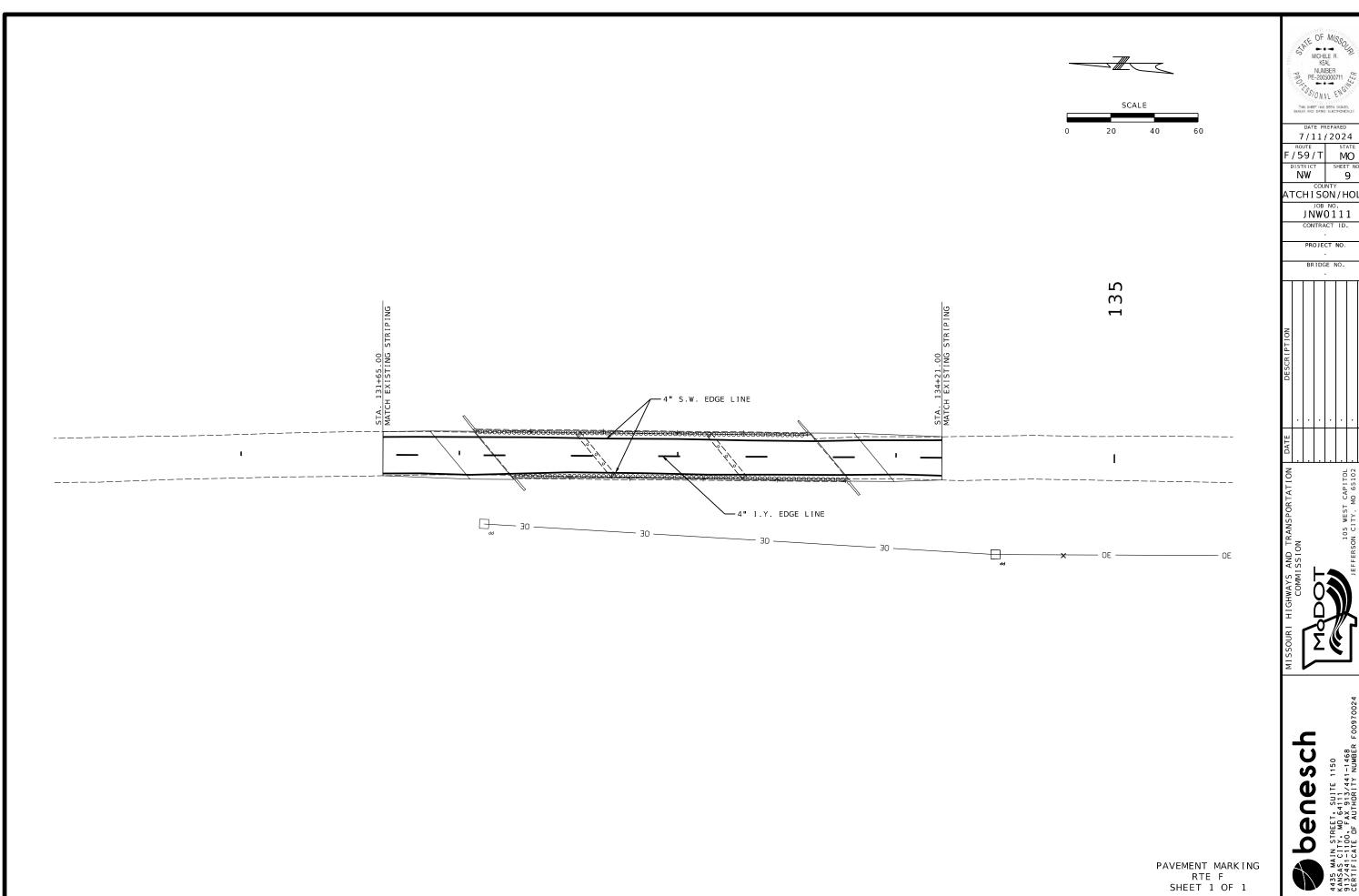


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902 SIGNAL	SIGNS T	ΓABUL <i>A</i>	ATED ON D-374	A SHEET	EMBEDDED														POST				2 IN. PO	ANCHORS		1		5 IN PC	ANCI	HORS	BREAK- AWAY	REMARKS AND	KEAL NUMBER PF-2005000
IGN SIGN SIZE	STATIO	ON	LOCATION	SIGN DTL. SHT. NO.	I TEM NO. 9031010 CY	POST DES NO.	POST NO.1	POST I NO. 2	POST I NO.3 F	LBS T PER 11 FT 9	TOTAL TEM NO. 031210 LBS	PIPE SIZE IN.	POST POS NO.1 NO	ST LBS	S TOTAL ITEM NO 9031220 LBS	0 2 5 NO LO EACH 1	X [] I 55 LBS GTH TC	PER FT	ITEM NO AL 9031250, LF	NO.	ST POS 0.1 NO.	T TOTAL 2 ITEM NO 9031270A	12-GA	N DRIVEN 7-GA 0. ITEM NO. 1A 9031273A EA	7 - GA I TEM NO	NO 1 NO	2 ITEM NO. 9031280	(6 FT) ITEM NO.	ITEM NO.	CONCRETE 7-GA ITEM NO. 9031285 EA	ASSEMBLY ITEM NO. 9031241 EA	OTHER REQUIRED ITEMS	NUMBER PE-2005000
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					ROUND	PID	F PC	ST ^	MD	FOOT	LING	דאם	Δ ΤΛΓ	RIF				POST	T DES NOM	1. L	WE I	GHT LBS/IN	STUB LENGTH		LEVEL GR	OUND 6		4:1	GRADE I C.Y.	3:1 OR 2	:1 GRADE C.Y.		وم
					NOM.SIZE	WE I	GHT		STUB		FC	OTING		CONCRE					1 W6 2 W6		9.0 15.0	0.75 1.25	3 - 0 4 - 0	15	3 0"		-2 0.15	3'-3"	0.16 0.51	3'-6" 4'-6"	0.17		
					2½ 5	.79	0.48	3	4 3½	<u>;•</u>	DIA.	4~	-6"	0.13	3				3 W8 4 W10	) 1	18.0 22.0	1.50	4 - 6 5 - 0	36"	5 0	1.31 5	-2"   1.36	5 3	0.74	5'-6"	0.78		
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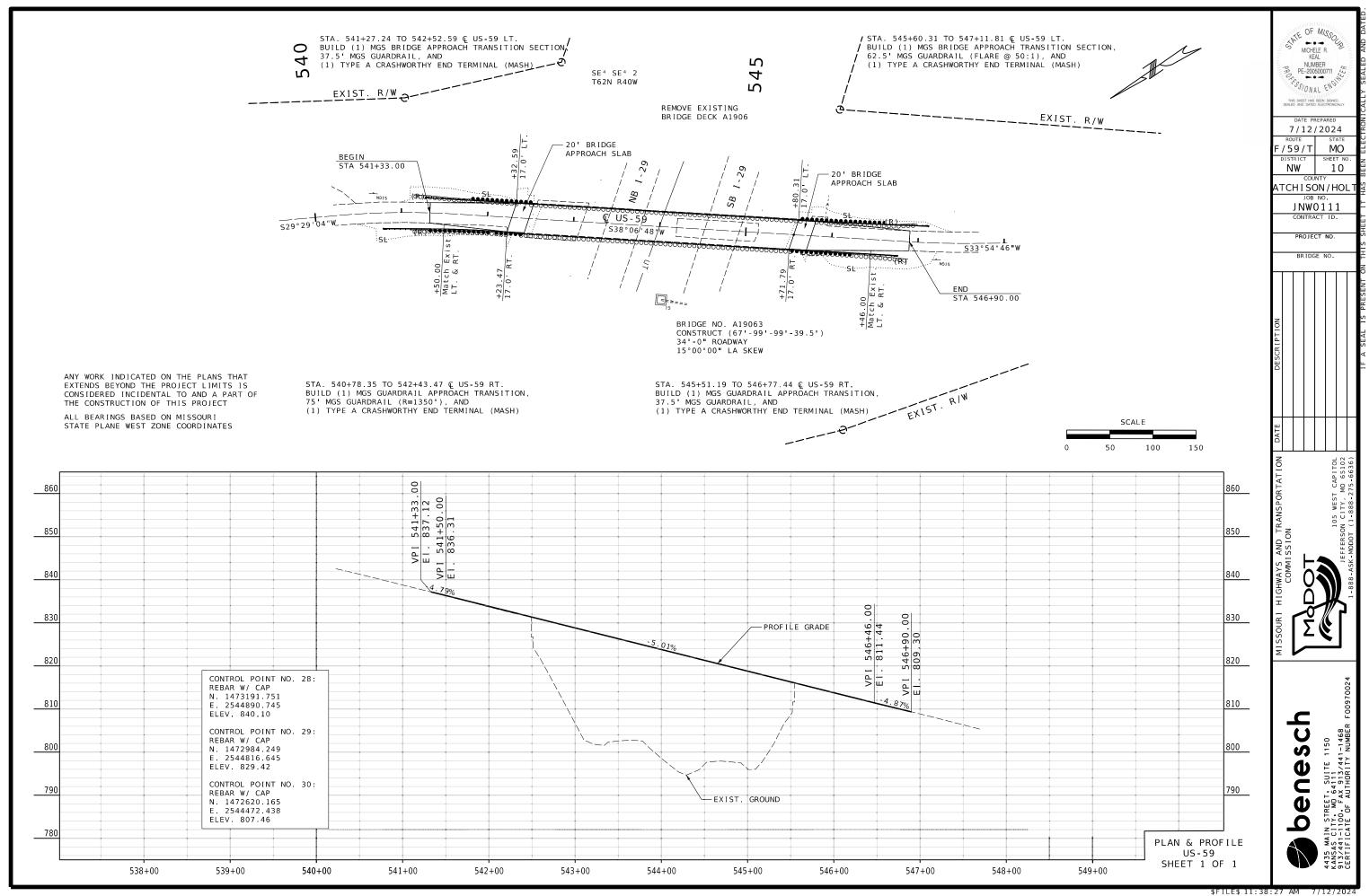
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STATION				SIGN	N DE	SCRI	PTIC	N G	SIZE	ى، ؟	NI IMB	RER C	DE E	ACH		_	STANDARD OR			SIGN DETAIL SHEET	NO. EACH	6175		FLAT SHEET SH	FL/ FLU	AT SHEET JORESCENT	STRU	CTURAL ST	STRUCTURAL FLUORESCENT
STATION	LOCATION	OM3 - L 12 × X36		310		Jerr		1	1		TOPIE			\(\text{CII}\)		_ SPE	CIAL SIG	IN NUMB	ER	NO.		SIZE		ITEM NO.	1	SHF *	ITE	EM NO.	STF * ITEM NO. 9035071A
																OM3 - L - T)	YPE 3 OBIE	CT MARKE	R LEFT		6	12"X36"	+	9035004A		18.00	903	5011A	9035071A
		OM3 L 12 X36	OM3 R 12 X36													OM3-R - TY	PE 3 OBJE	CT MARKE	R, RIGHT		6	12"X36"				18.00			
131+66	RTE F, 13.64' L	T 1																											
131+84	RTE F, 13.30 R	T 1	1																										
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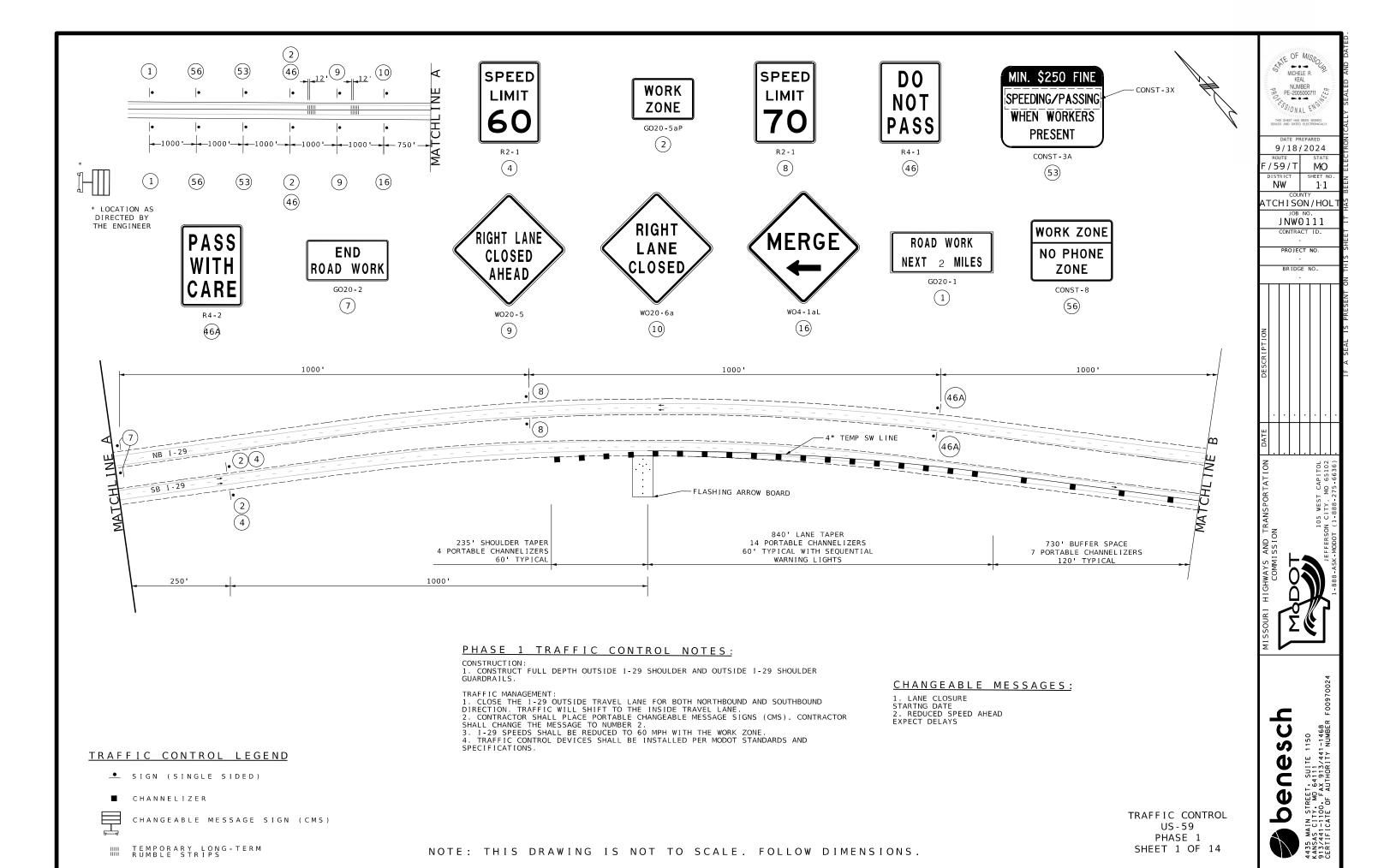
\* ORANGE, YELLOW & YELLOW/GREEN

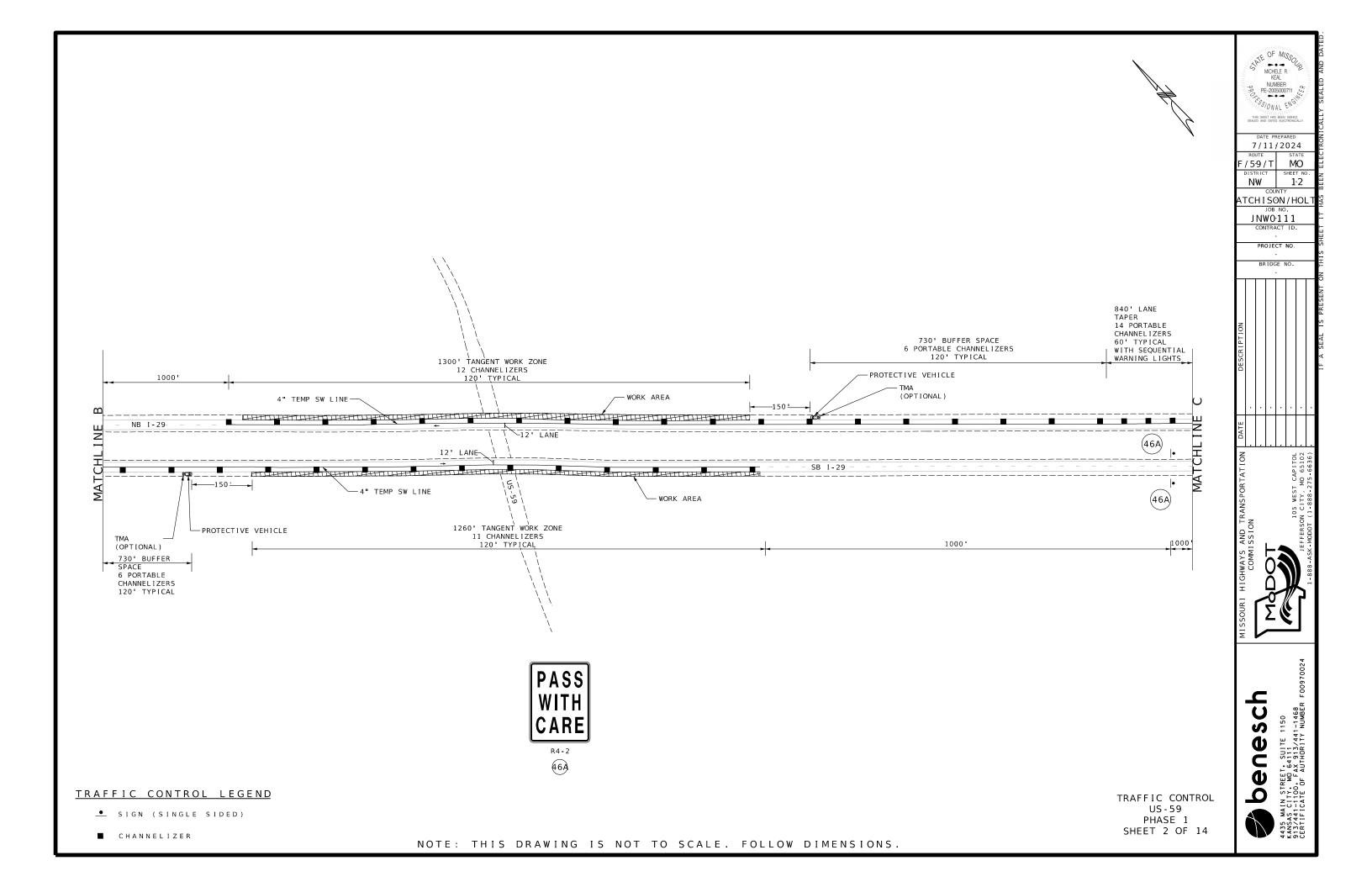


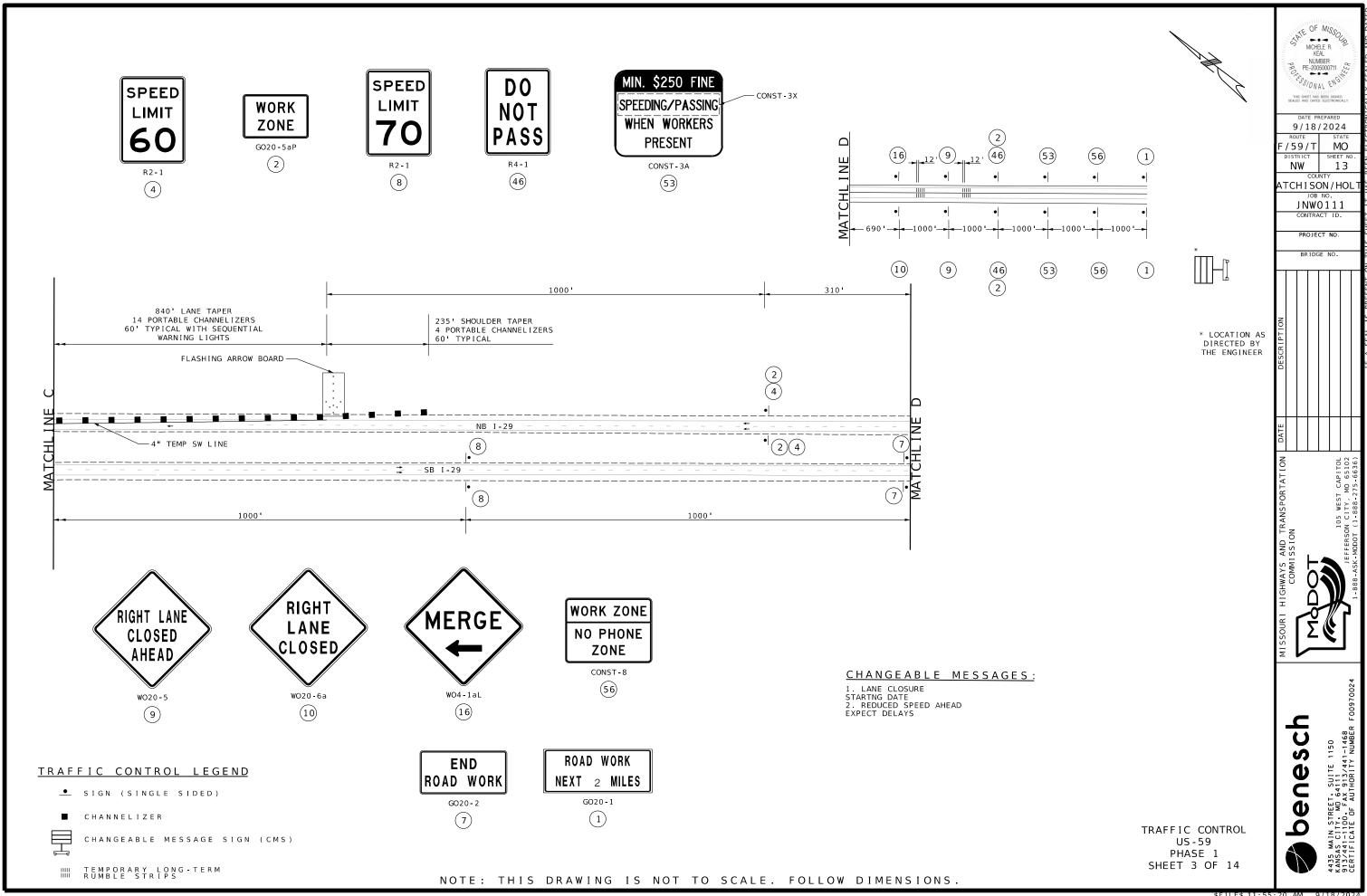


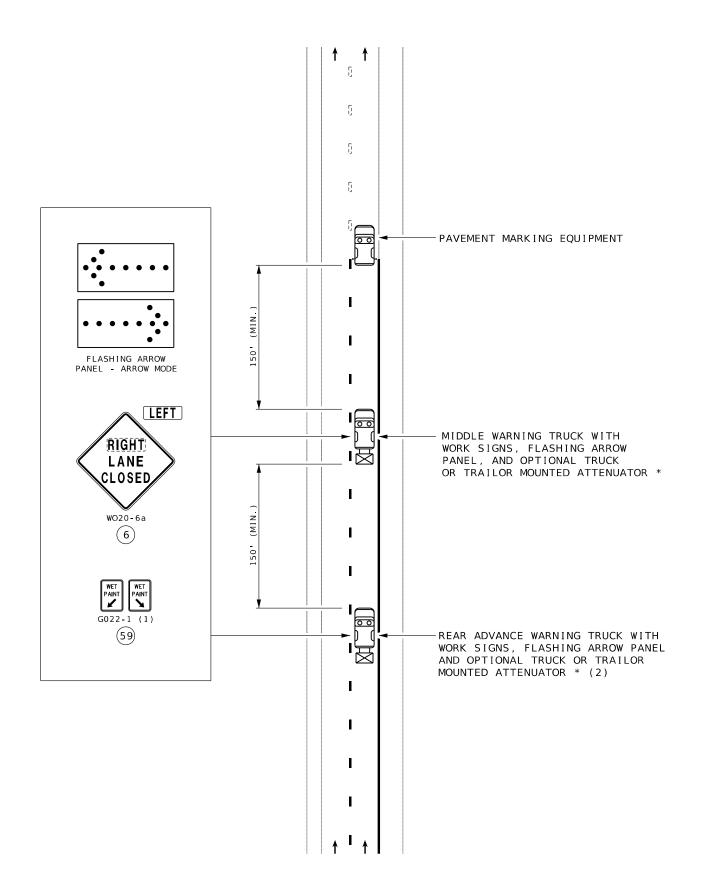
DATE PREPARED 7 / 11 / 2024 F / 5.9 / T MO
DISTRICT SHEET NO
NW 9 COUNTY ATCH I SON / HOL











#### NOTES:

UPON APPROVAL OF THE ENGINEER, THE CONTRACTOR MAY PROVIDE ADDITIONAL PROTECTIVE TRUCKS EQUIPPED WITH PROPER WARNING DEVICES.

PROTECTIVE TRUCK AND WORK VEHICLES SHALL DISPLAY HIGH-INTENSITY ROTATING, FLASHING, OSCILLATING OR STROBE LIGHTS.

VEHICLE HAZARD WARNING SIGNALS SHALL NOT BE USED INSTEAD OF THE VEHICLE'S HIGH-INTENSITY ROTATING, ROTATING, FLASHING, OSCILLATING OR STROBE LIGHTS.

FLASHING ARROW PANELS SHALL BE INCIDENTAL TO TRUCK MOUNTED ATTENUATORS, WHEREVER USED, NO ADDITIONAL PAYMENT WILL BE MADE.

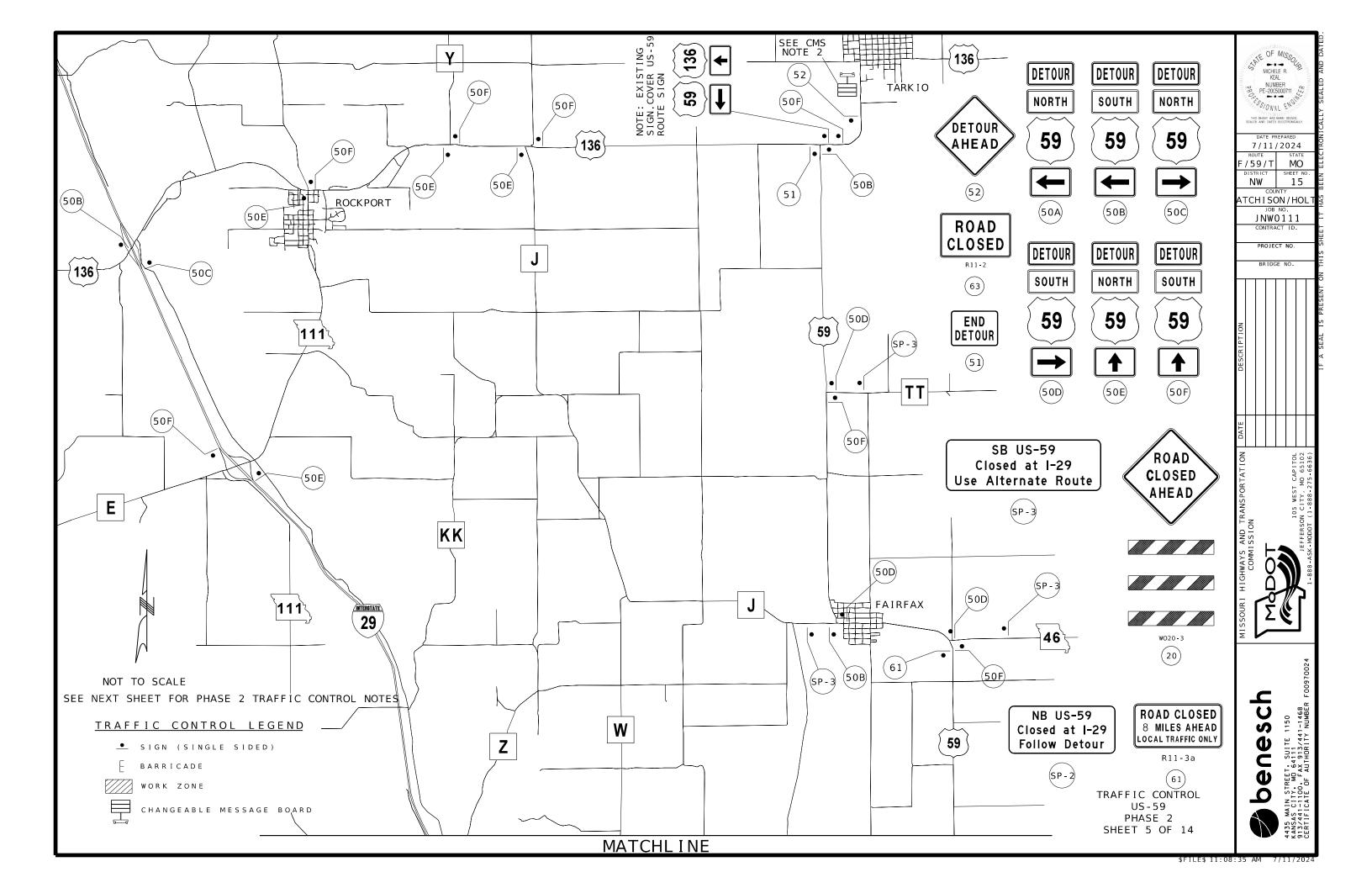
- (1) WET PAINT SIGNS ARE INSTALLED TO INDICATE THE SIDE TO WHICH THE PAVEMENT MARKING MATERIAL IS BEING APPLIED. AT THE CONTRACTOR'S OPTION, A FRONT FACING WET PAINT SIGN MAY BE INSTALLED ON THE LEFT SIDE OF THE PAVEMENT MARKING EQUIPMENT.
- (2) REAR ADVANCE WARNING TRUCK IS POSITIONED AT THE NO TRACK POINT OF THE PAVEMENT MARKING MATERIAL, OR VERTICAL OR HORIZONTAL CURVES THAT RESTRICT SIGHT DISTANCE, OR SPACING SHOWN.

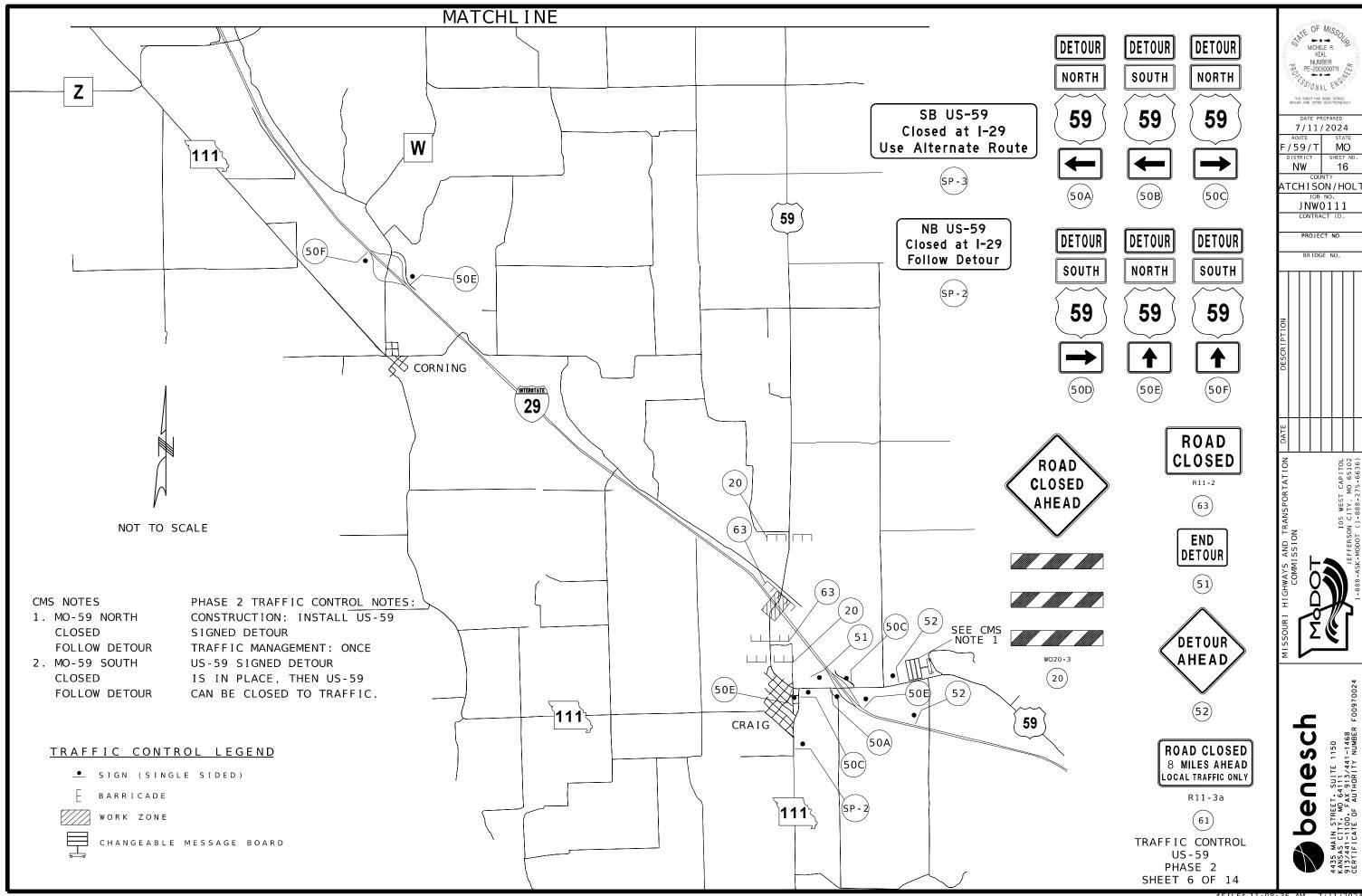
\* TMA COST IS INCIDENTAL - NO DIRECT PAY

MICHELE R. KEAL NUMBER TSSIONAL ENGI 7/11/2024 -/59/T MO NW 14 ATCH I SON / HOL JNW0111 PROJECT NO.

benesch 4435 MAIN STREET. SUITE 1150 KANSAS CITY. MO 64111 913/441-11100. FAX 913/441-11468

TRAFFIC CONTROL US-59 PHASE 1 SHEET 4 OF 14







←24.2→15.8 10 ← 39.8 → 24.2→

SP - 2

6.0" Radius, 1.3" Border, Black on Orange; "NB US-59", D; "Closed Ahead", D; "Follow Detour", D;

Table of letter and object lefts

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N 24.2	<b>B</b> 233.	<b>U</b> 350	. 0 5 9	- 9 . 0 6	7 . 6 7	4.5	<b>9</b> 83.	1
C 11.1	I 20.	<b>o</b> 6 24 .	8 8 32	<b>e</b> 2 . 2 3	9.7 4	7.2		
	<b>A</b> 62.	<b>h</b> 873.	<b>e</b> 481	. 6 8	<b>d</b> 9.197	7.3		
F 9.6	<b>o</b> l 6 . 8	I 25.2	1 2 30	<b>o</b> . 0 34	. 2 <b>w</b>	. 7		
	<b>D</b> 60.	<b>e</b> 469.	3 7 6	<b>o</b> 5 . 7 8	<b>u</b> 2.99	1.3	<b>r</b> 100	. 2

## SB US-59 Closed at I-29 Use Alternate Route

-48.2  $\rightarrow 15.8 \mid 10 \mid -39.8$   $\rightarrow 48.2$  $-31.3 - \cancel{*} -41.7 - \cancel{10} + \cancel{10} + \cancel{25.4} + \cancel{*} -31.3 - \cancel{*}$ 

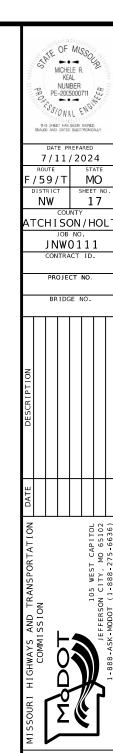
SP-3

6.0" Radius, 1.3" Border, Black on Orange; "SB US-59", D; "Closed at I-29", D; "Use Alternate Route", D;

Table of letter and object lefts

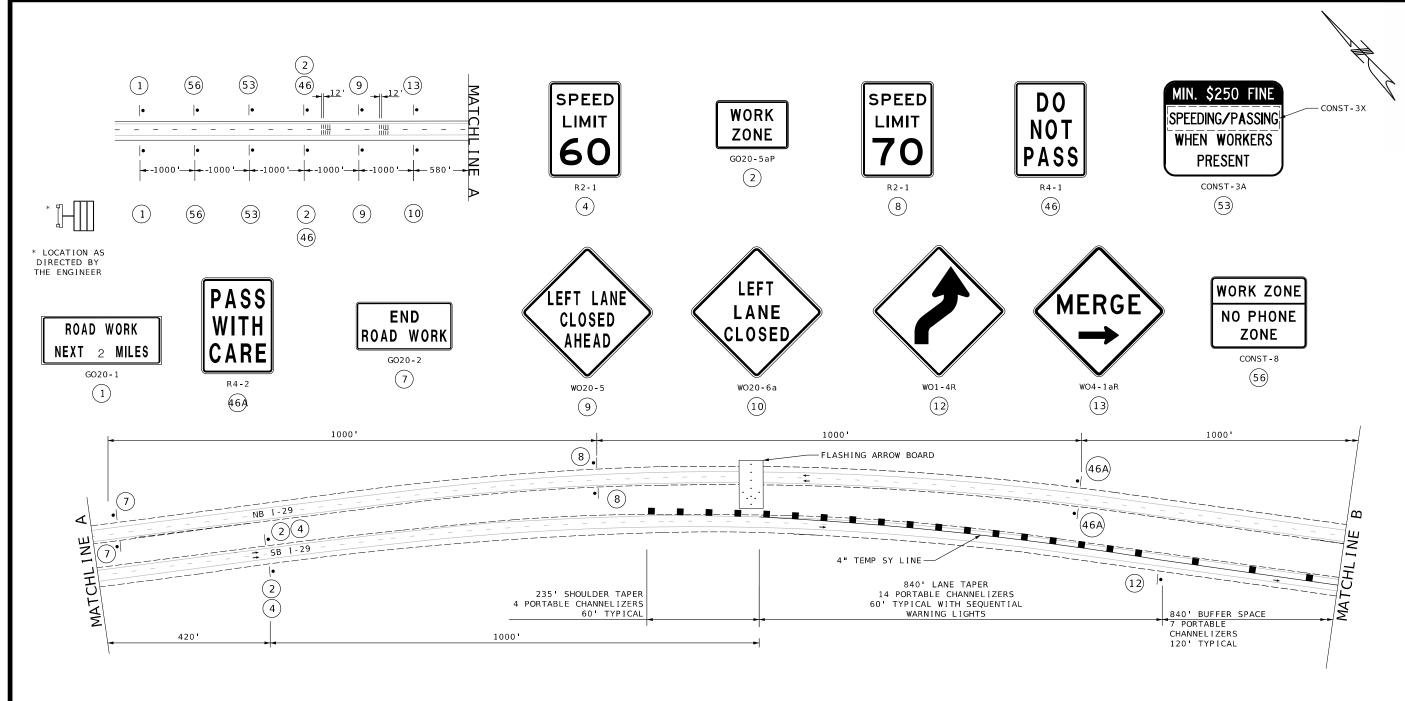
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48.	257.3	374.0	83.0	91.6	98.5	107.1	
<b>C</b>	I	<b>o</b>	<b>s</b>	<b>e</b>	<b>d</b>	<b>a</b>	<b>t</b>
31.	340.8	44.9	52.4	59.8	67.4	83.0	90.9
	I 105.	3 109	. 2 11!	9 5 . 4 1 <i>2</i>	24.0		
<b>U</b> 9.7	<b>s</b> 19.0	<b>e</b> 26.4					

A I t e r n a t e e 42.0 52.6 56.5 62.8 71.0 77.2 85.4 93.4 99.6 115.2124.1132.5140.5146.7



benesch

TRAFFIC CONTROL US - 59 PHASE 2 SHEET 7 OF 14



#### PHASE 3 TRAFFIC CONTROL NOTES:

CONSTRUCTION:

1. REMOVE BRIDGE DECK. CONTRACTOR SHOULD COMPLETE THE BRIDGE DECK REMOVAL FROM THE CENTER AND WORK TOWARDS APPROACH SLABS.
2. CONTRACTOR SHALL PROVIDE PROTECTON FROM DEBRIS FALLING INTO THE TRAVEL LANE.

TRAFFIC MANAGEMENT:

TRAFFIC MANAGEMENT:

1. CLOSE THE I-29 INSIDE TRAVEL LANE FOR BOTH NORTHBOUND AND SOUTHBOUND DIRECTION. OUTSIDE THRU LANE WILL SHIFT TO THE OUTSIDE SHOULDER.

2. CONTRACTOR SHALL PLACE PORTABLE CHANGEABLE MESSAGE SIGNS (CMS). CONTRACTOR SHALL RUN CMS MESSAGE NUMBER ONE FOR TWO WEEKS PRIOR TO INSTALLING TRAFFIC CONTROL TO REDUCE TO ONE LANE AND 60 MPH ZONE. ONCE LANE REDUCTION IS IN PLACE, CONTRACTOR SHALL CHANGE THE MESSAGE TO NUMBER 2.

3. I-29 SPEEDS SHALL BE REDUCED TO 60 MPH WITH THE WORK ZONE.

4. US-59 DETOUR SHALL BE INSTALLED PRIOR TO CLOSING US-59 TO TRAFFIC. SEE US-59 DETOUR PLANS FOR THE TEMPORARY SIGNING PLAN. TRAFFIC CONTROL DEVICES SHALL BE INSTALLED PER MODOT STANDARDS AND

CHANGEABLE MESSAGES:

CHANGEABLE MESSAGES: 1. LANE CLOSURE STARTING DATE

2. REDUCE SPEED AHEAD EXPECT DELAYS

US-59 PHASE 3 SHEET 8 OF 14

TRAFFIC CONTROL

TEMPORARY LONG-TERM RUMBLE STRIPS

CHANGEABLE MESSAGE SIGN (CMS)

TRAFFIC CONTROL LEGEND

SIGN (SINGLE SIDED)

CHANNELIZER

NOTE: THIS DRAWING IS NOT TO SCALE. FOLLOW DIMENSIONS.

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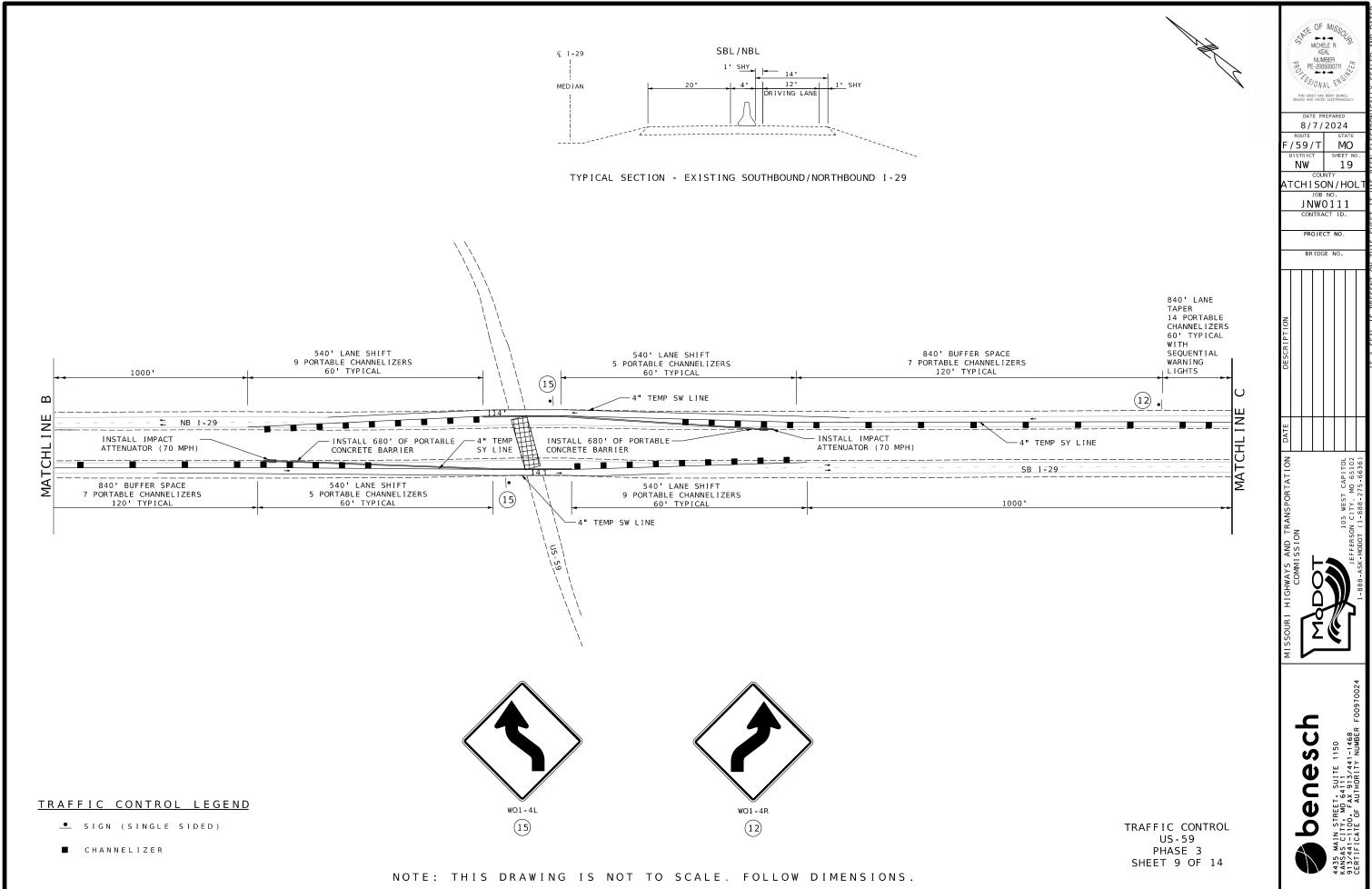
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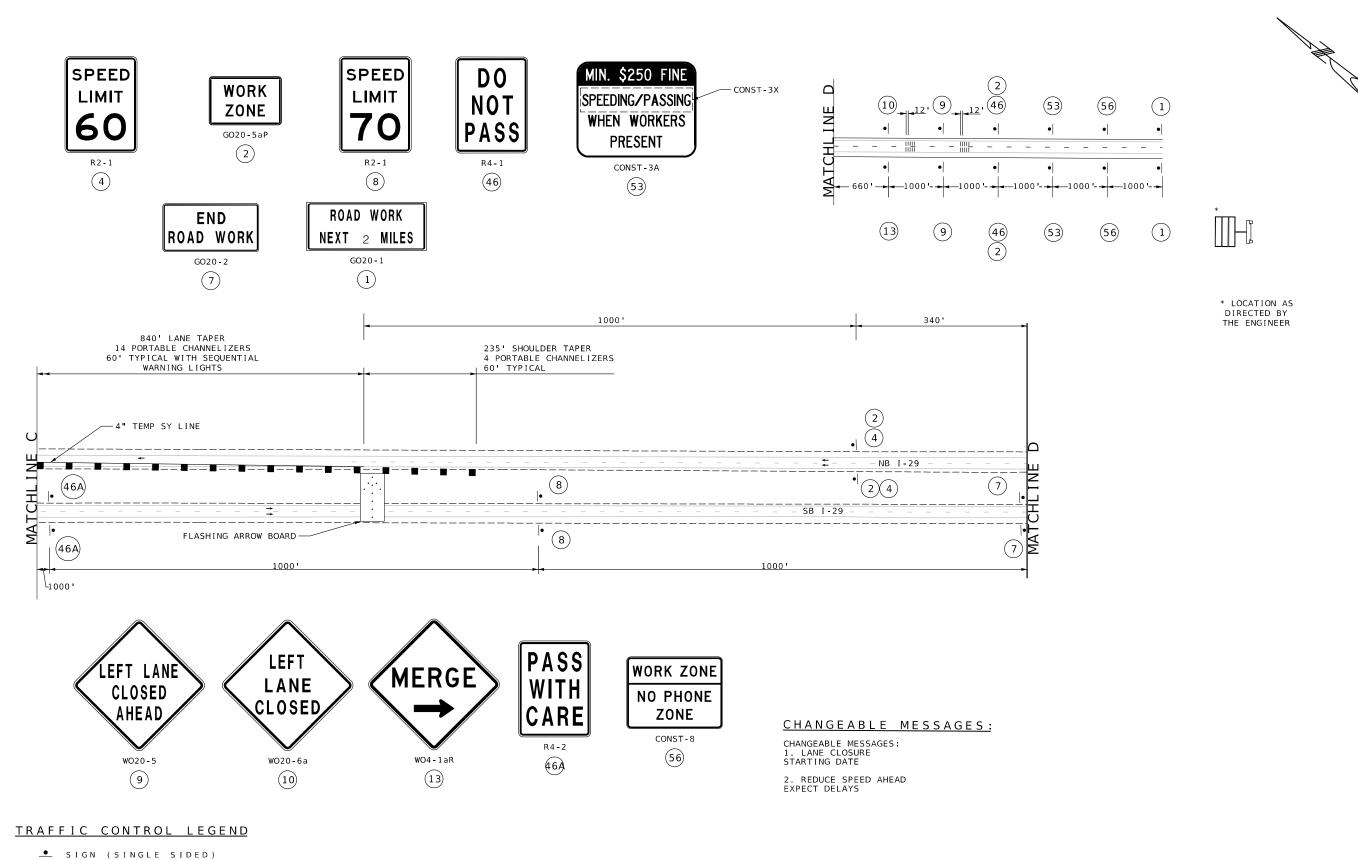
JNW0111

PROJECT NO

18

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■ CHANNELIZER

CHANGEABLE MESSAGE SIGN (CMS)

IIII TEMPORARY LONG-TERI

NOTE: THIS DRAWING IS NOT TO SCALE. FOLLOW DIMENSIONS.

TRAFFIC CONTROL US-59 PHASE 3 SHEET 10 OF 14 MICHELE R. KEAL NUMBER PE-2005000711

FOSIONAL ENGT

9/18/2024

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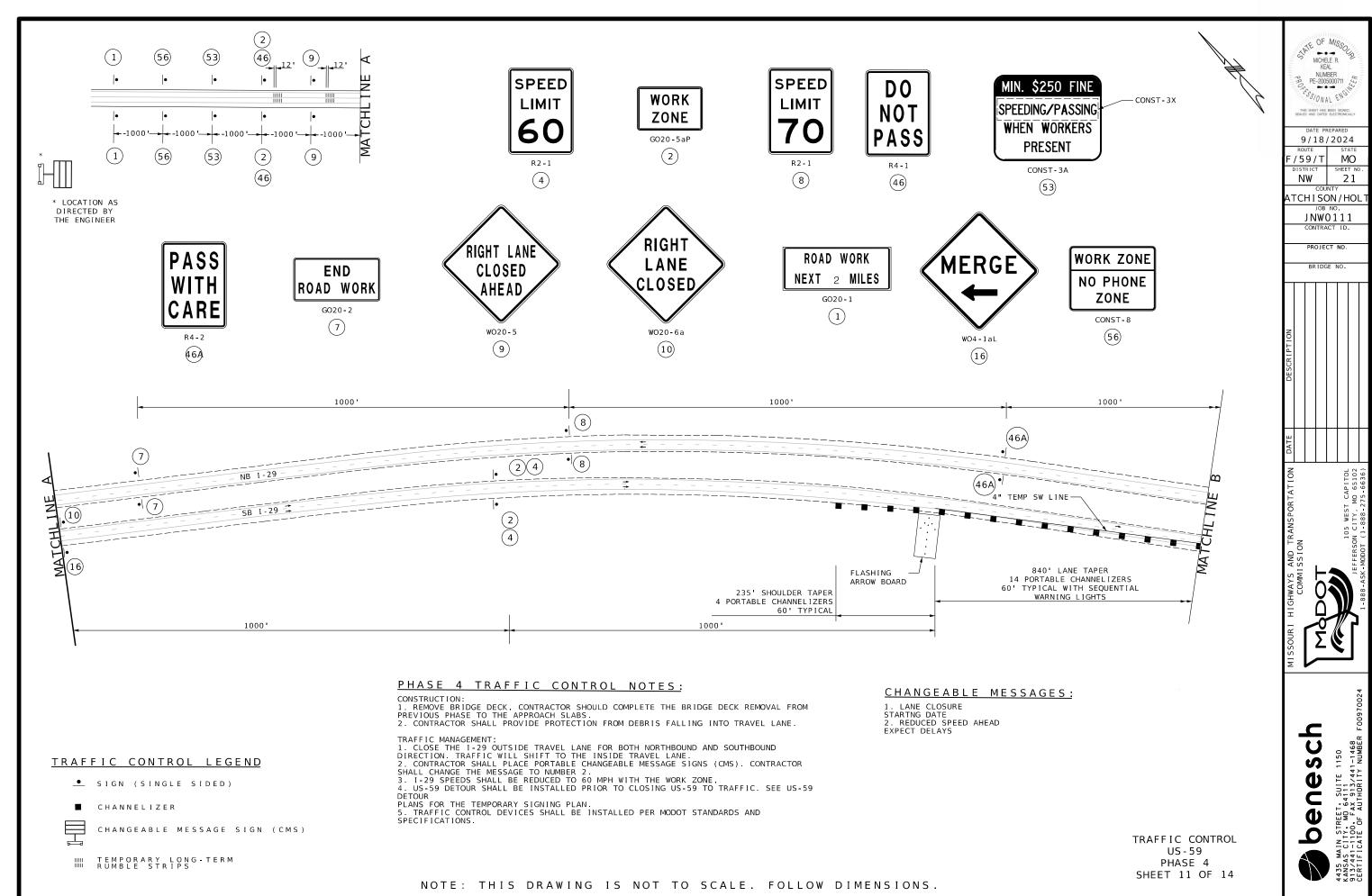
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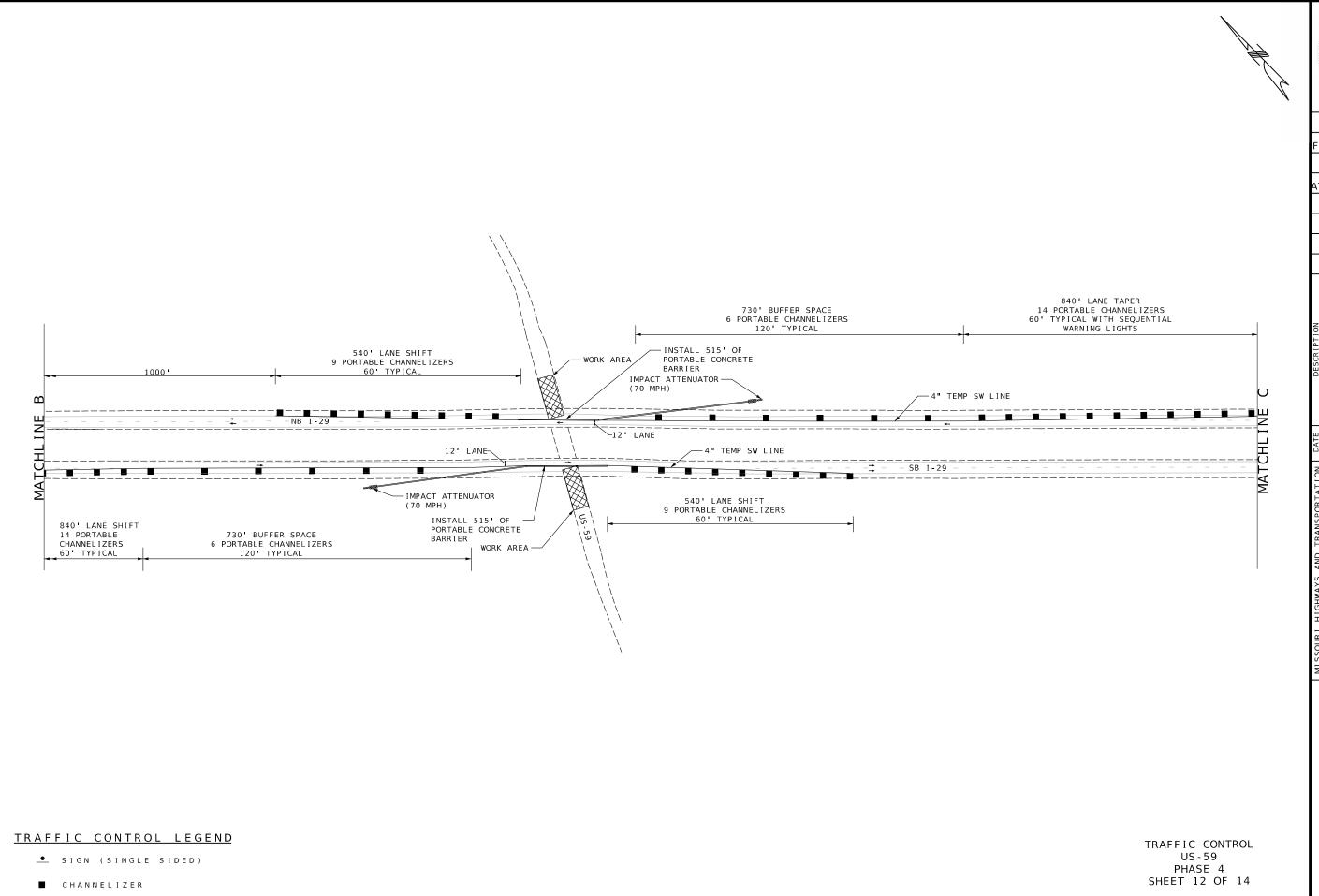
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MICHELE R.
KEAL
NUMBER
PE-2005000711
APPRIORETE BERTEN REPORT

BATE PREPARED

8/7/2024

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DISTRICT SHEET NO

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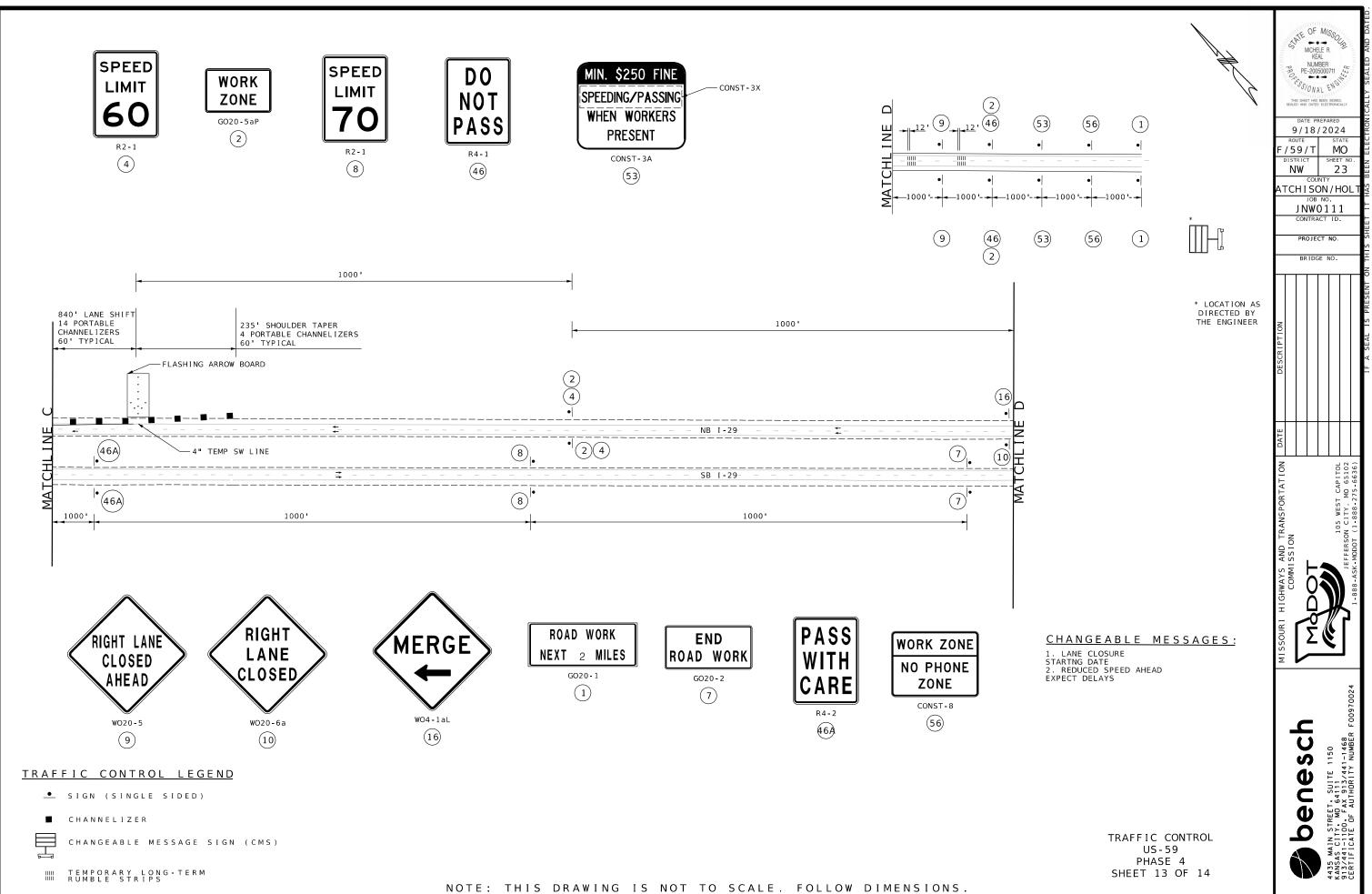
CONTRACT ID.

PROJECT NO.

NTE DESCRIPTION

OURT HIGHWAYS AND TRANSPORTATION DISCOMMISSION

benesch MAJS MAIN STRET. SUITE 1150 KANSAS CITY. MO 6411-1468



#### PHASE 5 TRAFFIC CONTROL NOTES:

CONSTRUCTION:
1. CONSTRUCT TEMPORARY SHORING ALONG I-29 SOUTHBOUND OUTSIDE SHOULDER FOR THE BRIDGE TO SUPPORT DURING REPLACEMENT OF GIRDER PIN.

- TRAFFIC MANAGEMENT:

  1. CLOSE THE I-29 SOUTHBOUND OUTSIDE TRAVEL LANE.

  1-29 SOUTHBOUND: CONTINUE TO USE TRAFFIC CONTROL FROM PHASE 4.

  1-29 NORTHBOUND: REMOVE ALL TRAFFIC CONTROL AND OPEN BOTH LANES TO TRAFFIC.

  2. TRAFFIC CONTROL DEVICES SHALL BE INSTALLED PER MODOT STANDARDS AND SPECIFICATIONS

SAFE OF MISSOURCE R.
MICHELE R.
KEAL
NUMBER
PE-2005000711
FS.SONAL ENSINE THIS SHEET HAS BEEN SIGNED. SEALED AND DATED ELECTRONICALLY.

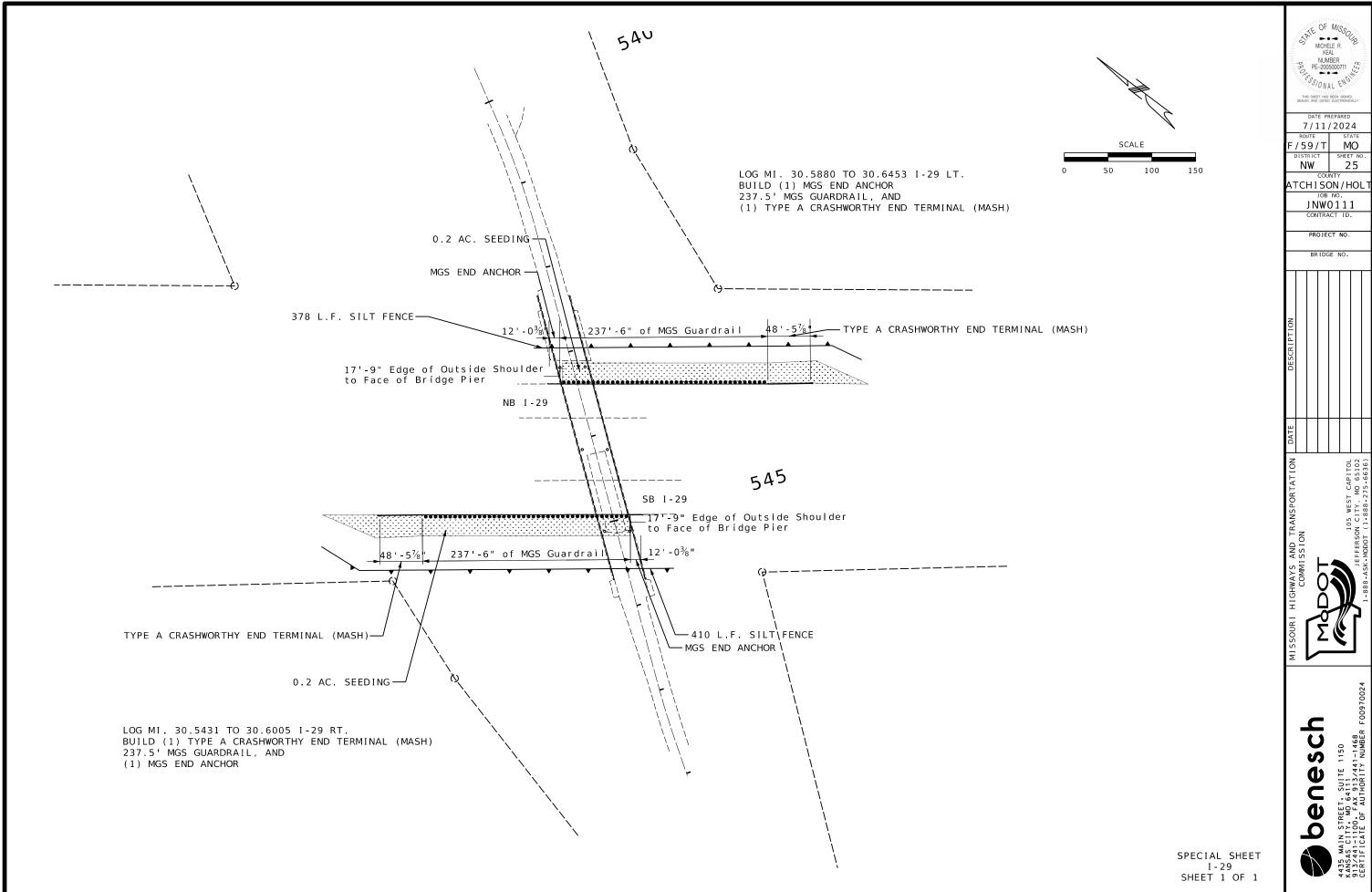
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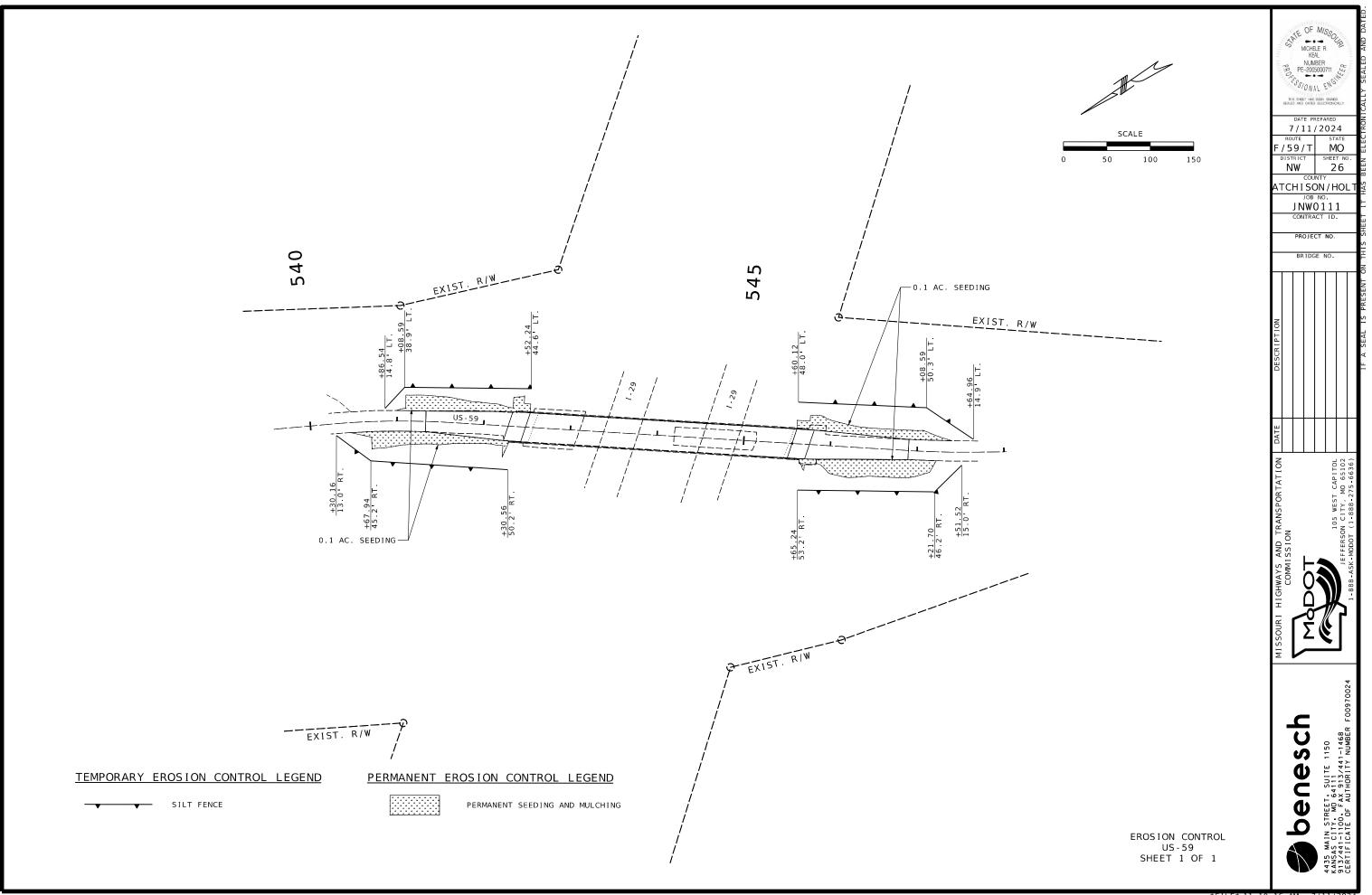
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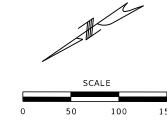
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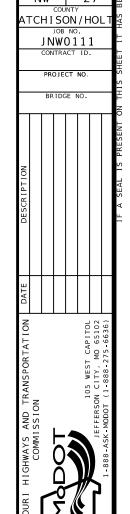
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TRAFFIC CONTROL US - 59 PHASE 5 SHEET 14 OF 14









MICHELE R.
KEAL
NUMBER
PE-2005000711

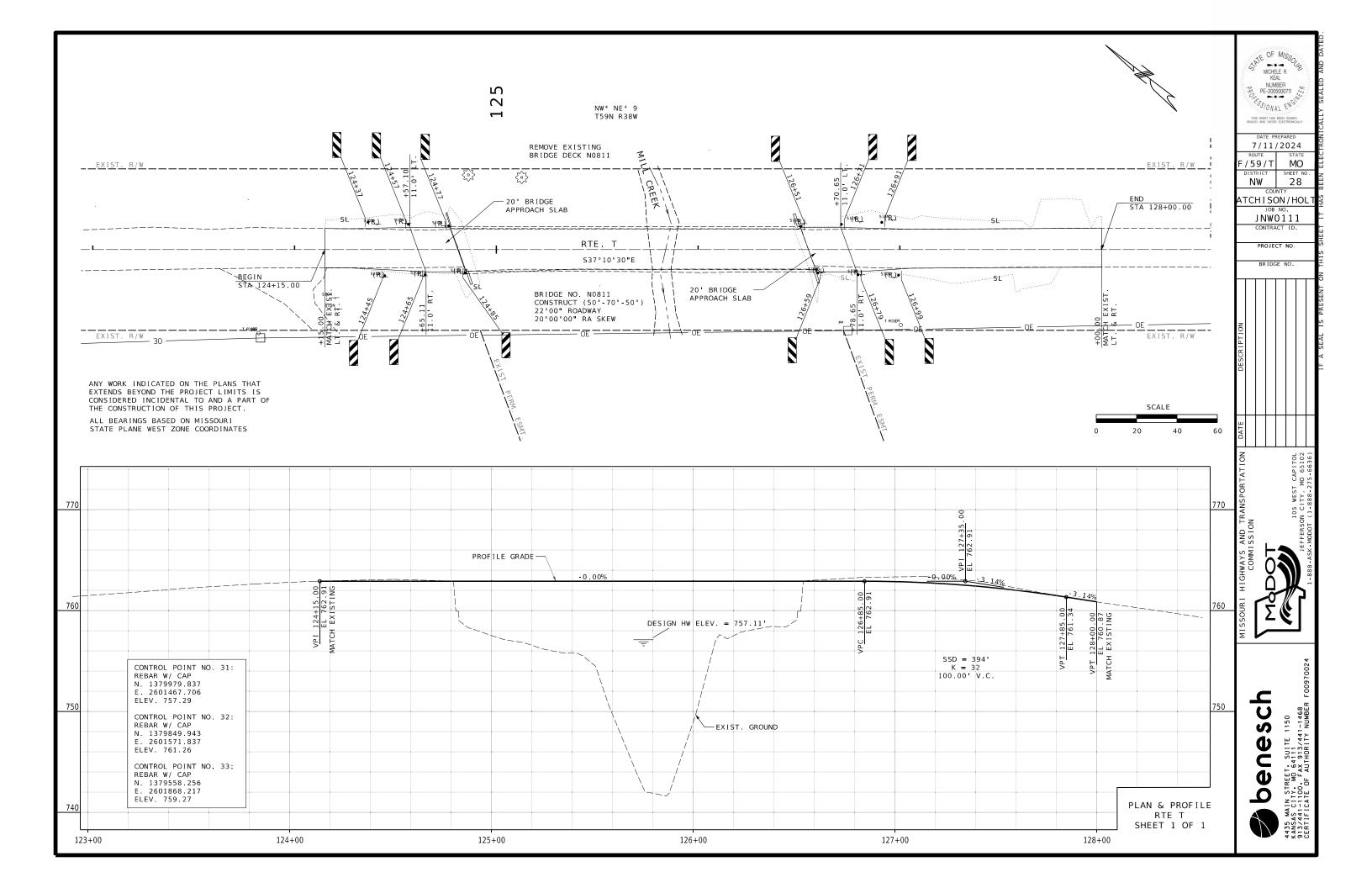
DATE PREPARED 7/11/2024 F/59/T MO

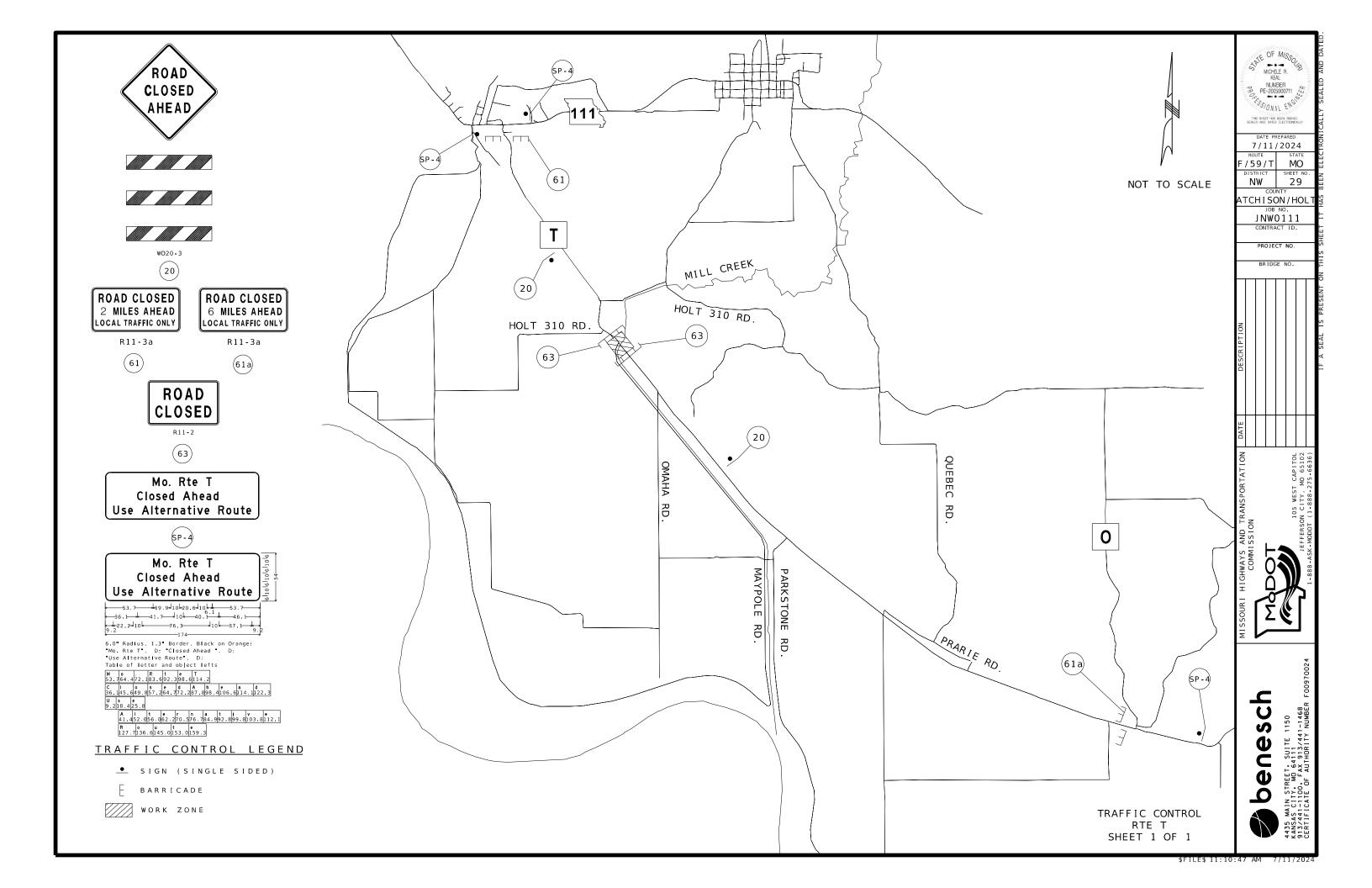
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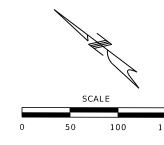
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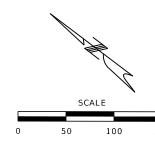
PAVEMENT MARKING MO-59 SHEET 1 OF 1

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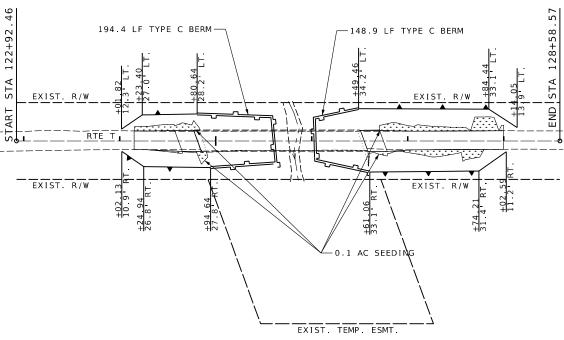








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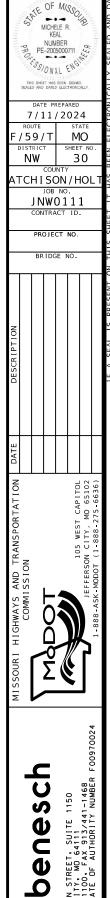
TEMPORARY EROSION CONTROL LEGEND

SILT FENCE TEMPORARY TYPE C BERM PERMANENT EROSION CONTROL LEGEND



PERMANENT SEEDING AND MULCHING

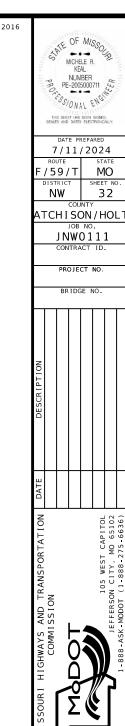
EROSION CONTROL RTE T SHEET 1 OF 1



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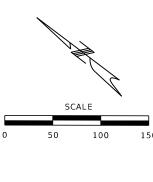






125

-4" D.Y. CENTERLINE



DATE PREPARED
7/11/2024
ROUTE STATE
F/59/T MO
DISTRICT SHEET NO.
NW 33 COUNTY
ATCHISON/HOL

JOB NO.
JNW0111

CONTRACT ID.

OF MISSON
MICHELE R.
KEAL
NUMBER
PE-2005000711
FS-5/ONAL ENGINE

benesch

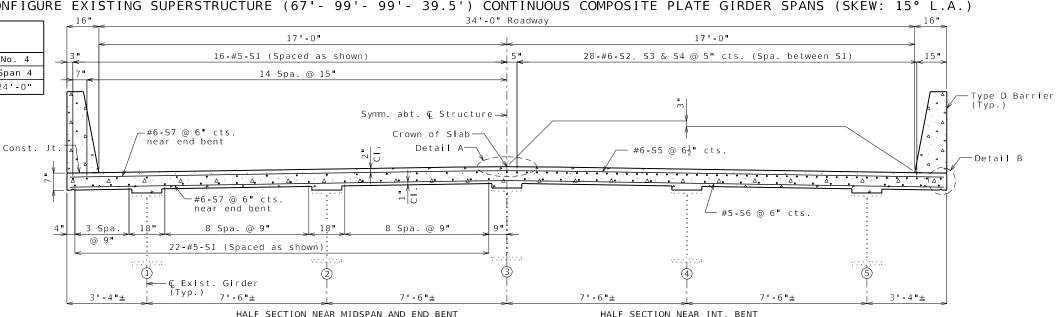
PAVEMENT MARKING RTE T SHEET 1 OF 1

## U.I.P., REDECK AND RECONFIGURE EXISTING SUPERSTRUCTURE (67'- 99'- 99'- 39.5') CONTINUOUS COMPOSITE PLATE GIRDER SPANS (SKEW: 15°

	S2, S		Showing Bar Le		
Int. Be	nt No. 2	Int. Be	nt No. 3	Int. Bei	nt No. 4
Span 1	Span 2	Span 2	Span 3	Span 3	Span 4
22'-9"	22'-6"	31'-6"	24'-9"	16'-3"	24'-0"

Required For Bar	Lap Length Splices **
Bar Size	Splice Length
4	2'-7"
5	3'-3"
6	3'-10"
7	4'-11"

\*\* Unless otherwise shown.



TYPICAL SECTION THRU SLAB

#### General Notes:

Design Specifications: 2002 AASHTO LFD (17th Ed.) Standard Specifications Seismic Performance Category A

Design Loading:

H15-44 (1969) (Existing) HS20-44 (New Construction)
35 lb/sf Future Wearing Surface
Earth - 120 lb/cf, Equivalent Fluid Pressure 45 lb/cf Fatigue Stress - Case III

Design Unit Stresses:

Class B-1 Concrete (Barrier) f'c = 4,000 psiClass B-2 Concrete (End Bents & Superstructure, except Barrier) f'c = 4,000 psiReinforcing Steel (Grade 60) fy = 60,000 psi

Joint Filler:

All joint filler shall be in accordance with Sec 1057 for preformed sponge rubber expansion and partition joint filler, except as noted.

Reinforcing Steel:

Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise shown.

Bars bonded in existing concrete not removed shall be cleanly stripped and embedded into new concrete where possible. If length is available, existing bars shall extend into new concrete at least 40 diameters for plain bars and 30 diameters for deformed

Roadway surfacing adjacent to bridge ends shall match new bridge slab súrface. (Róadwáy item)

Outline of existing work is indicated by light dashed lines. Heavy lines indicate new work.

Contractor shall verify all dimensions in field before finalizing the shop drawings.

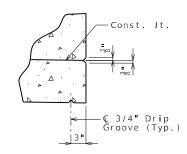
The area exposed by the removal of concrete and not covered with new concrete shall be coated with an approved qualified special mortar in accordance with Sec 704.

Rubblized concrete from the existing bridge deck that qualifies as clean fill may be placed on spill slopes at end bents (Roadway item).

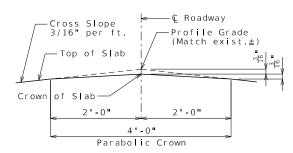
Traffic Handling:

Structure to be closed during construction. Traffic to be maintained on other routes during construction. See roadway plans for traffic control.

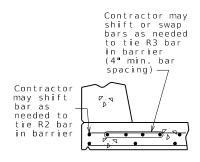
Vertical clearance for Interstate 29 traffic during construction shall be 15'-0" minimum over a 26'-0" wide horizontal opening of the roadway in each direction.



DETAIL B



#### DETAIL A



OPTIONAL SHIFTING TOP BARS AT BARRIER

Estimated Quantities	i	
I t em		Total
Removal of Miscellaneous ACM (Non-Friable)	sq. foot	30
Removal of Existing Bridge Deck	sq. foot	11,375
Bridge Approach Slab (Minor)	sq. yard	153
Slab on Steel	sq. yard	1,255
Type D Barrier	linear foot	672
Substructure Repair (Unformed)	sq. foot	40
Fabricated Structural Low Alloy Steel (Plate Girder) A709,	Grade 50 pound	53,490
Slab Drain	each	18
Intermediate Field Coat (System G)	sq. foot	2,900
Finish Field Coat (System G)	sq. foot	2,900
Reconfigure Existing Structural Steel	lump sum	1
Vertical Drain ar End Bents	each	2
Laminated Neoprene Bearing Pad (Tapered)	each	5
Laminated Neoprene Bearing Pad Assembly	each	5

Estimated Ovantities

Cost of any required excavation for bridge will be considered completely covered by the contract unit price for other items \*See Special Provisions.

Estimated Quantities for Slab on St	eel
I t em	Total
Class B-2 Concrete cu. yard	280
Reinforcing Steel (Epoxy Coated) pound	94,590

The table of Estimated Quantities for Slab on Steel represents the quantities used by the State in preparing the cost estimate for concrete slabs. The area of the concrete slab will be measured to the nearest square yard longitudinally from end of slab to end of slab and transversely from out to out of bridge slab (or with the horizontal dimensions as shown on the plan of slab). Payment for stay-in-place corrugated steel forms, conventional forms, all concrete and epoxy coated reinforcing steel will be considered completely covered by the contract unit price for the slab. Variations may be encountered in the estimated quantities but the variations cannot be used for an adjustment in the contract unit price.

Method of forming the slab shall be in accordance with Sec 703. All hardware for forming the slab to be left in place as a permanent part of the structure shall be coated in accordance with ASTM A123 or ASTM B633 with a thickness Class SC 4 and a finish Type I, II or III

Slab shall be cast-in-place with conventional forming or stay-in-place corrugated steel forms. Precast prestressed panels will not be permitted.

For Optional Stay-In-Place Form Details, see Sheet No. 2.

#### REPAIRS TO BRIDGE: ROUTE 59 OVER ROUTE I-29

ROUTE 59 FROM ROUTE 46 TO ROUTE I-29 ABOUT 8.9 MILES SOUTH OF ROUTE 46 BEGINNING STATION 542+48.00± (MATCH EXISTING)

MODOT 6219		MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION
19	_	MADOT
	6219	105 WEST CAPIT
	92	JEFFERSON CITY, MO 651

OL 02 63

NUMBER E-25019

7/3/2024

HOLT JNW0111

PROJECT NO

A19063

MO

SHEET NO

1

59

BR

U 0

DESIGNED BY: KLW JAN 2024 DETAILED BY: JTC JAN 2024 CHECKED BY: NSC MAR 2024 Remove bridge deck and barrier per Sec 216.

Remove top of backwall down to upper const. joint. Grind Surface smooth and level

TYPICAL END BENT SECTION

#### TYPICAL WING BARRIER REMOVAL

#### DETAILS OF CONCRETE REMOVAL

Prior to deck removal, profile grade along bridge shall be recorded at tenth points of each span.

The cost of concrete removal as shown will be considered completely covered by the contract unit price for Removal of Existing Bridge Deck.

(1) Vertical reinforcement to be cut off one inch below concrete removal surface and the resulting holes shall be filled with a qualified special mortar.

A smooth, level surface shall be provided at top of backwall removal lines.

#### General Notes:

All concrete above the existing bearing seat is included with the Superstructure Quantities.

#### Stay-In-Place Forms:

Corrugated steel forms, supports, closure elements and accessories shall be in accordance with grade requirement and coating designation G165 of ASTM A653. Complete shop drawings of the permanent steel deck forms shall be required in accordance with Sec 1080.

Corrugations of stay-in-place forms shall be filled with an expanded polystyrene material. The polystyrene material shall be placed in the forms with an adhesive in accordance with the manufacturer's recommendations.

Form sheets shall not rest directly on the top of girder flanges. Sheets shall be securely fastened to form supports with a minimum bearing length of one inch on each end. Form supports shall be placed in direct contact with the flange. Welding on or drilling holes in the girder flanges will not be permitted. All steel fabrication and construction shall be in accordance with Sec 1080 and 712. Certified field welders will not be required for welding of the form supports.

The design of stay-in-place corrugated steel forms is per manufacturer which shall be in accordance with Sec 703 for false work and forms. Maximum actual weight of corrugated steel forms allowed shall be 4 psf assumed for girder loading.

The contractor shall provide a method of preventing the direct contact of the stay-in-place forms and connection components with uncoated weathering steel members that is approved by the engineer.

#### Pouring and Finishing Slab:

The contractor shall provide bracing necessary for lateral and torsional stability of the girders during construction of the concrete slab and remove the bracing after the slab has attained 75% design strength. Contractor shall not weld on or drill holes in the girders. The cost for furnishing, installing, and removing bracing will be considered completely covered by the contract unit price for Slab on Steel.

Alternate pour sequences may be submitted to the engineer for approval. Keyed construction joints shall be provided between pours.

#### Haunching

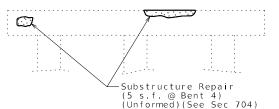
For adjusted girder deflection due to weight of new deck and barriers, see Sheet No. 9.

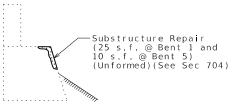
#### Structural Steel Protective Coatings:

Prime Coat (New Steel): The cost of the inorganic zinc prime coat will be considered completely covered by the contract unit price for the fabricated structural steel.

Field Coat(s): The color of the field coat(s) shall be Gray (Federal Standard #26373). The cost of the intermediate field coat will be considered completely covered by the contract unit price per sq. foot for Intermediate Field Coat (System G). The cost of the finish field coat will be considered completely covered by the contract unit price per sq. foot for Finish Field Coat (System G).

At the option of the contractor, the intermediate field coat and finish field coat may be applied in the shop. The contractor shall exercise extreme care during all phases of loading, hauling, handling, erection and pouring of the slab to minimize damage and shall be fully responsible for all repairs and cleaning of the coating systems as required by the engineer.





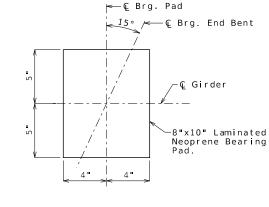
SUBSTRUCTURE REPAIR DETAILS

#### ·

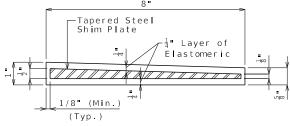
Fill corrugations

with foam (Typ.)

SECTION A-A



END BENT NO. 5 BEARING PAD LAYOUT



# BEARING PAD DETAIL END BENT NO. 5 General Note:

Neoprene bearing pads shall be 60 durometer and shall be in accordance with Sec 716.

#### OPTIONAL STAY-IN-PLACE FORM DETAILS

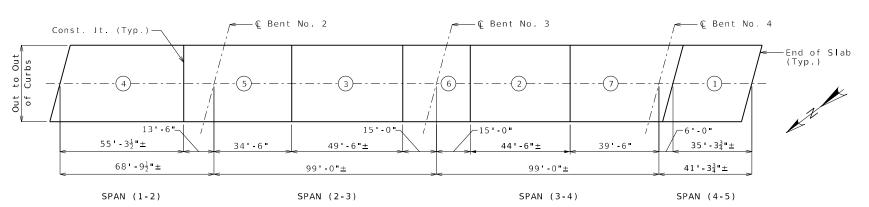
Steel corrugated A→

Form

support

Α-

bridge forms-



					e of Pour ds./Hr.				
		Direction With No Retarder Retarder							
Basic Sequence	1	1 2 3 4 5 6 7 Either Direction 25							
Alternate pours to the basic skip sequence are subject to the approval of the engineer in accordance with Sec 703.									
Alternate "A"	Alternate "A" 1 4 + 5 + 3 + 6 + 2 + 7								
Pours	End t	o 7		E	nd to 1			25	39

The contractor shall pour and satisfactorily finish the slab pours at the rate given. Retarder, if used, shall be an approved type and retard the set of concrete to 2.5 hours.

#### SLAB POURING SEQUENCE

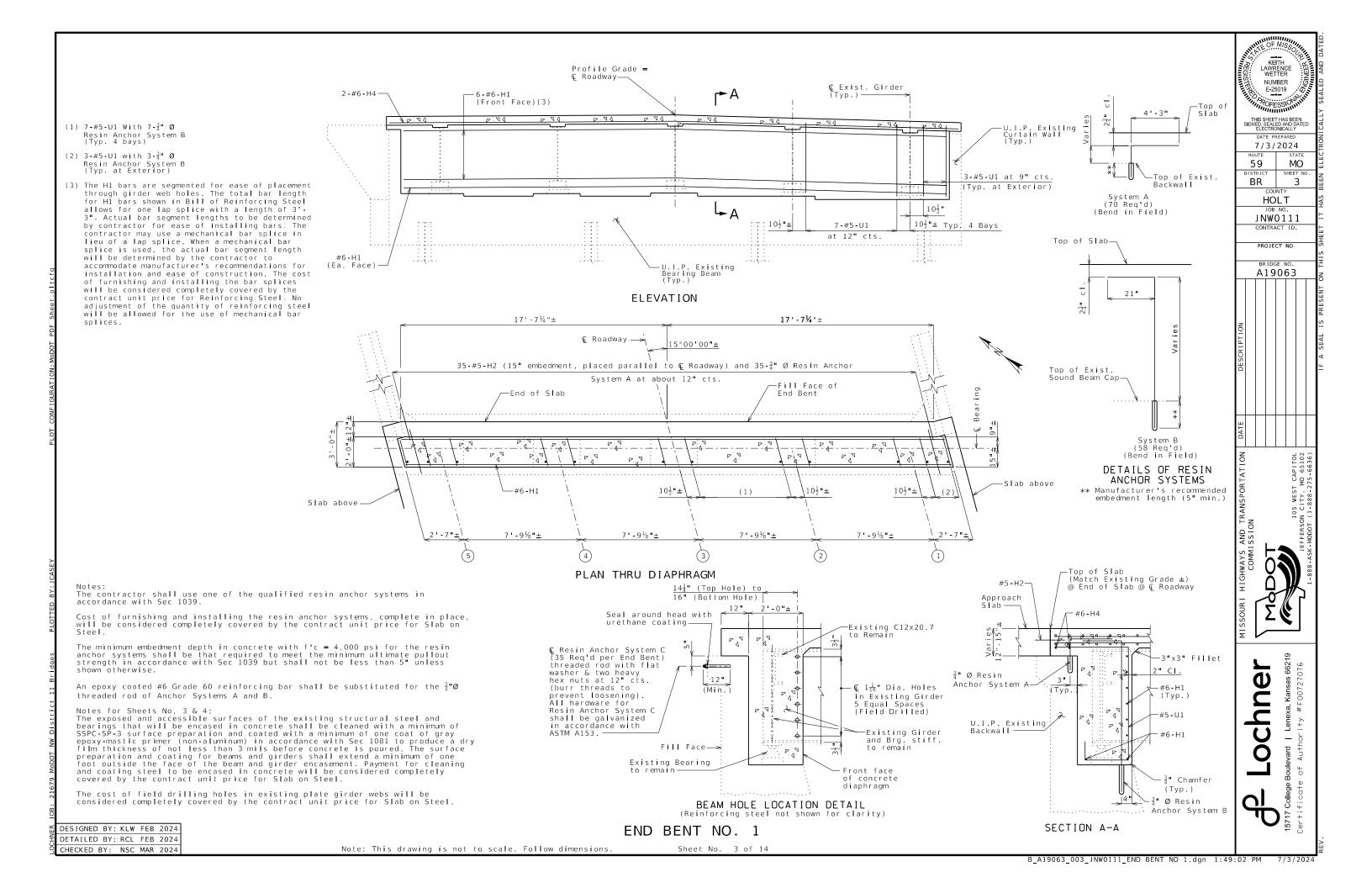
DESIGNED BY: KLW JAN 2024 DETAILED BY: JTC JAN 2024 CHECKED BY: NSC MAR 2024

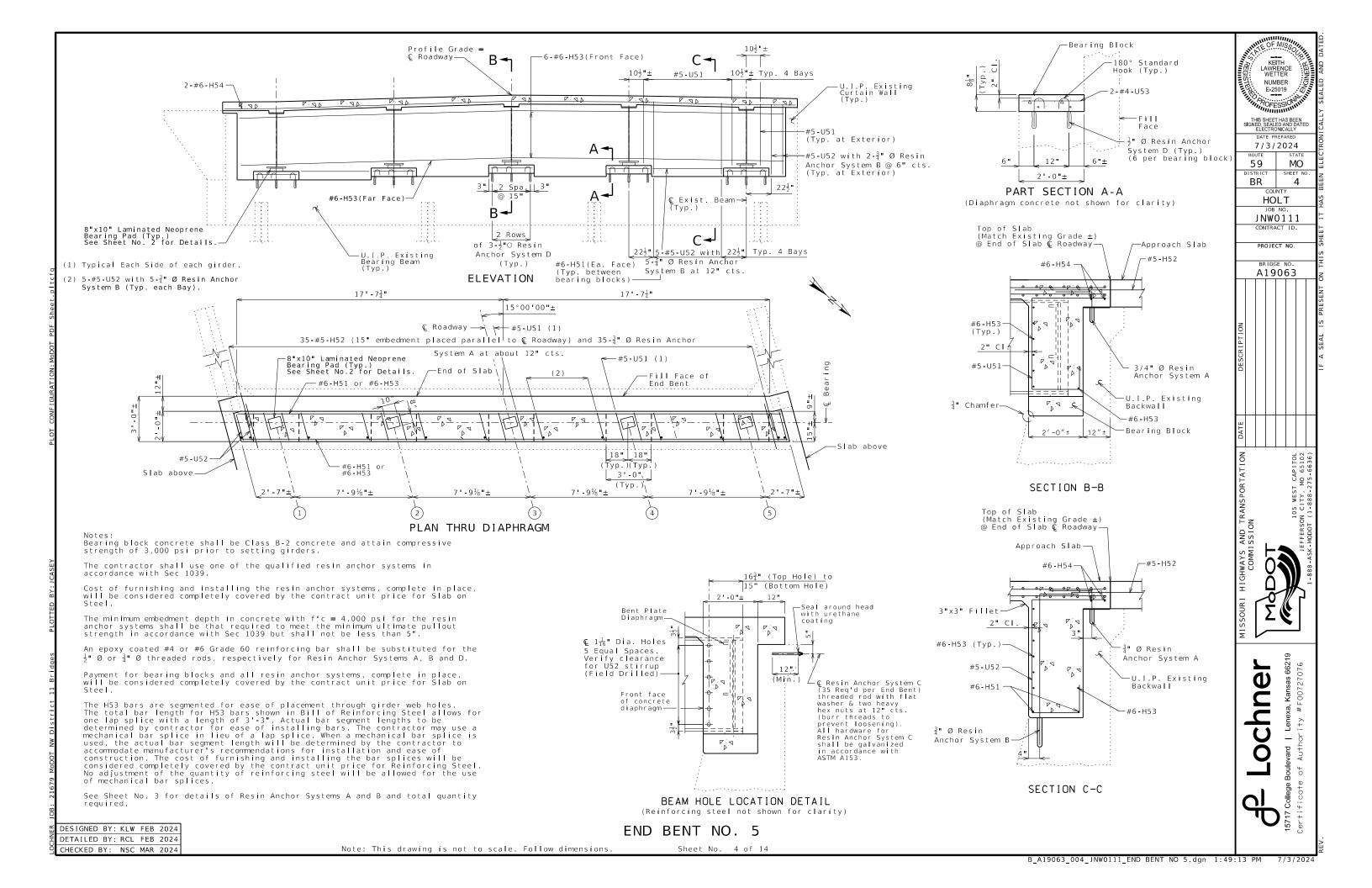


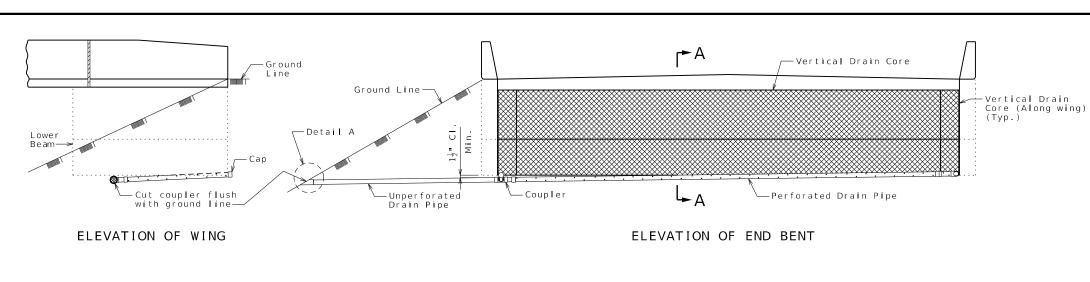
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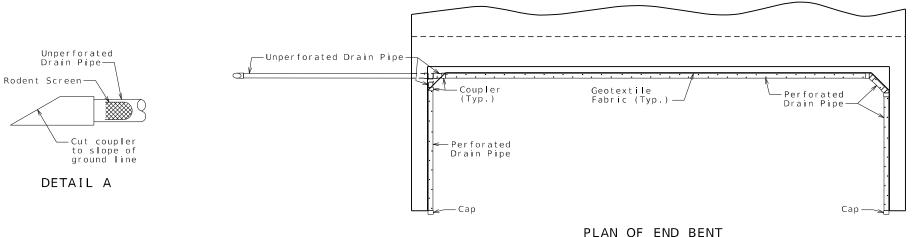
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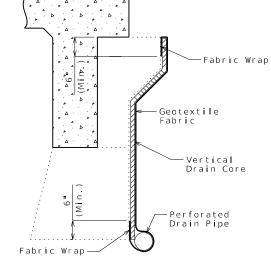








Note: This drawing is not to scale. Follow dimensions.



PART SECTION A-A (Section thru wing similar)



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NUMBER E-25019

THIS SHEET HAS BEEN SIGNED, SEALED AND DATE ELECTRONICALLY

7/3/2024

HOLT

JNW0111

PROJECT NO. BRIDGE NO A19063

MO SHEET NO

5

59

BR

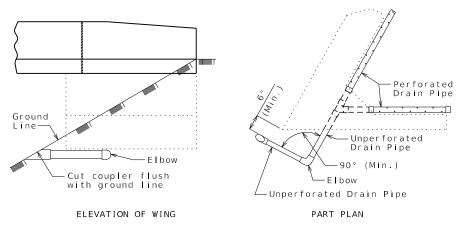
#### General Notes:

All drain pipe shall be sloped 1 to 2 percent

Drain pipe may be either 6-inch diameter corrugated metallic-coated steel pipe underdrain, 4-inch diameter corrugated polyvinyl chloride (PVC) drain pipe, or 4-inch diameter corrugated polyethylene (PE)

Drain pipe shall be placed at fill face of end bent and inside face of wings. The pipe shall slope to lowest grade of ground line, also missing the lower beam of end bent by a minimum of 1 1/2 inches.

Perforated pipe shall be placed at fill face side and inside face of wings at the bottom of end bent and plain pipe shall be used where the vertical drain ends to the exit at ground line.



#### OPTIONAL TURNED DRAIN

(Use only when straight drain is not practical.)

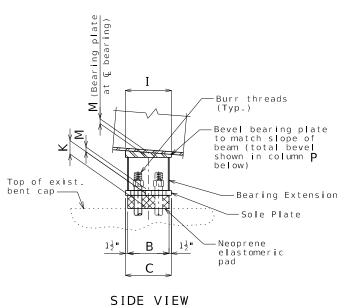
## VERTICAL DRAIN AT END BENTS

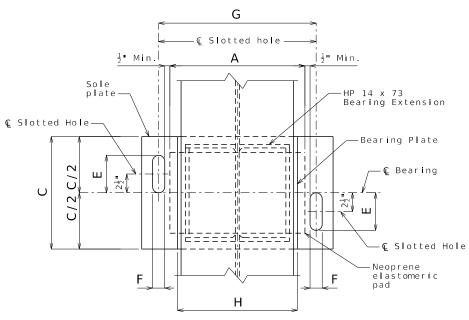
DESIGNED BY: KLW FEB 2024 DETAILED BY: JTC FEB 2024 CHECKED BY: BPW MAR 2024

(Squared end bent shown, skewed end bent similar)

Sheet No. 5 of 14

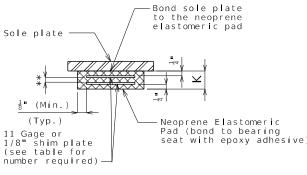
B\_A19063\_005\_JNW0111\_VERTICAL DRAINS AT END BENT dgn 1:49:22 PM 7/3/2024





	EXPANSION BEARINGS																					
BENT NO.	Α	В	C	D	Е	F	G	Н	I	J	Κ	L	Μ	Z	Р	Q	R	S	Τ	С	NUMBER OF SHIM PLATES *	NUMBER REQUIRED
4	18"	12"	15"	2 ' - 1 <sup>1</sup> / <sub>2</sub> "	5"	1 <del>5</del> "	21"	16"	15"	4 <sup>1</sup> / <sub>4</sub> "	2½ "	15"	1½ •	$3\frac{3}{16}$ "	3 H	2 <del>1</del> "	1 m	145"	13 <del>5</del>	6 <del>§</del> "	4	5
				he requ																	TOTAL BEARINGS	5

layers of elastomer and molded together to form an integral unit.



\*\* Layers of 1/2" elastomer alternating with 11 gage or 1/8" shim plate

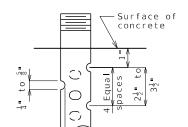
All structural steel for the anchor bolts and heavy hexagon nuts shall be coated with a minimum of two coats of organic zinc primer (5 mils minimum).

Neoprene Elastomeric Pads shall be 60 Durometer.

Structural steel for sole plate, bearing plate and bearing extension shall be ASTM A709 Grade 50 and shall be coated with a minimum of two coats of

Laminated Neoprene Bearing Pad Assembly shall be in accordance with Sec 716.

Cost of anchor bolts, drilling, grouting, bearing extension and any incidental labor or materials, complete in place, will be considered completely covered by the contract unit price for Laminated Neoprene Bearing Pad Assembly.



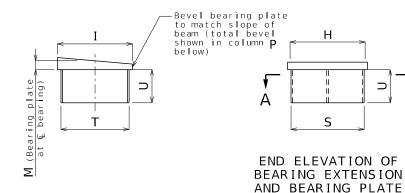
DETAIL OF 1 1/2"Ø ANCHOR BOLTS

 $\frac{1}{8}$ " to  $\frac{1}{4}$ "

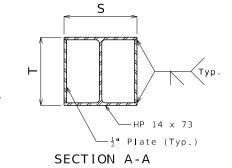
DESIGNED BY: KLW MAR 2024

DETAILED BY: JTC MAR 2024 CHECKED BY: NSC MAR 2024

SWEDGE ANCHOR BOLT DETAIL



SIDE ELEVATION OF BEARING EXTENSION AND BEARING PLATE



# LAMINATED NEOPRENE BEARING PAD ASSEMBLY

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 6 of 14



THIS SHEET HAS BEEN SIGNED, SEALED AND DATE ELECTRONICALLY 7/3/2024 59 MO SHEET NO BR 6 HOLT JNW0111 CONTRACT ID. PROJECT NO BRIDGE NO A19063 PART PLAN 3"⊘ Well (Min.) seat with epoxy adhesive) DETAIL OF ANCHOR BOLT WELL

NEOPRENE ELASTOMERIC PAD **GENERAL NOTES:** Existing anchor bolts shall be removed one inch below top of existing concrete beam and the resulting holes shall be filled with qualified special mortar Anchor bolts shall be 1 1/2"Ø ASTM F1554 Grade 55 swedged bolts and shall extend 18" into the cored anchor bolt wells with ASTM A563 Grade A Heavy Hex nuts. Actual manufacturer's certified mill test reports(chemical and mechanical) shall be provided. Swedging shall be 1" less than extension into the correct. into the concrete. Anchor bolt shall be at the centerline of slotted hole at 60°F. Bearing position shall be adjusted R for each 10° fall or rise in temperature at installation.

organic zinc primer (5 mils minimum).

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Kansas 6621 F00727076 hnei

NUMBER E-25019

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Longitudinal dimensions are taken parallel to grade.

Fabricated structural steel shall be ASTM A709, Grade 50.

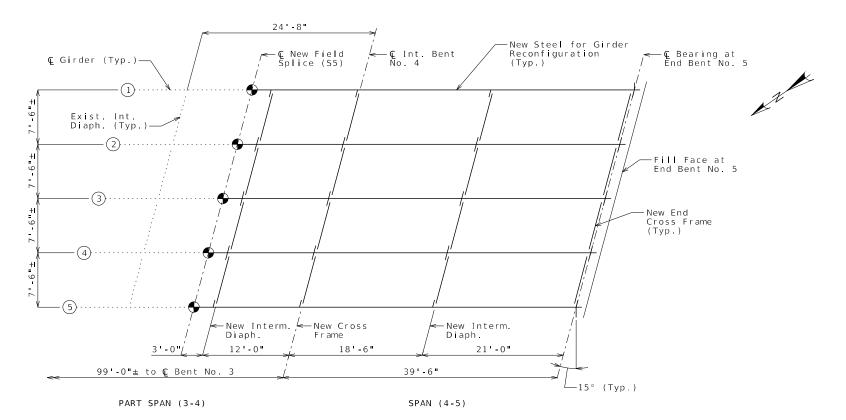
For details of reconfiguration to existing girders, see Sheet No. 8.

For details of stiffeners, see Sheet No. 8.

For details and spacing of shear connectors, see Sheet No. 8.

For details of bolted field splices, see Sheet No. 9.

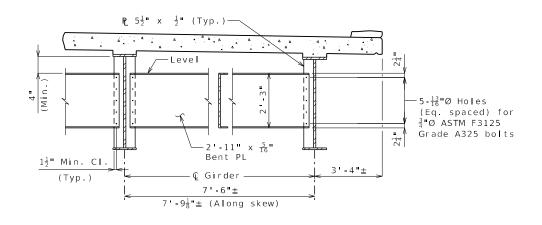
For location of slab drain attachment holes, see Sheet No. 10.



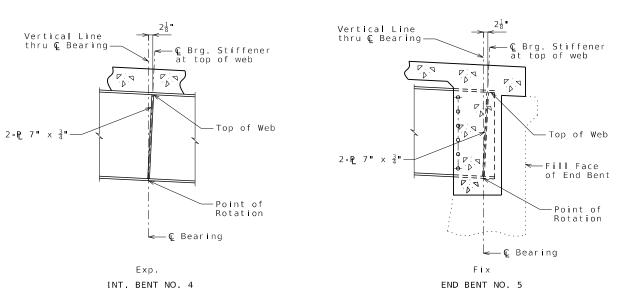
#### PART PLAN OF STRUCTURAL STEEL

#### Note:

At the contractor's option, holes in the diaphragm plate of non slab bearing diaphragms may be made 3/16" larger than the nominal diameter of the bolt. A hardened washer shall be used under the bolt head and nut when this option is used. Holes in the girder diaphragm connection plate or transverse web stiffener shall be standard size.



TYPICAL PART SECTION SHOWING CROSS FRAMES AND INTERMEDIATE DIAPHRAGMS



PART LONGITUDINAL SECTIONS

DESIGNED BY: KLW FEB 2024 DETAILED BY: JTC FEB 2024 CHECKED BY: NSC MAR 2024

RECONFIGURATION OF EXISTING GIRDERS

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 7 of 14

B\_A19063\_007\_JNW0111\_RECONFIG OF EXIST GIRDERS.dgn 1:49:41 PM 7/3/2024

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NUMBER E-25019

7/3/2024

HOLT

JNW0111

PROJECT NO.

BRIDGE NO A19063

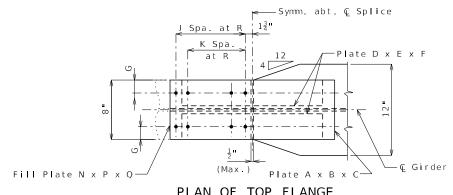
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SHEET NO

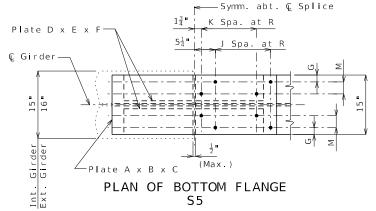
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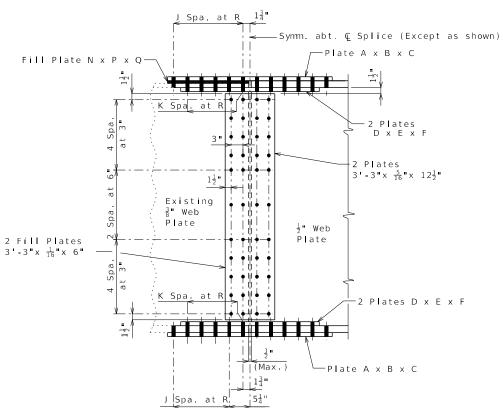
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#### PLAN OF TOP FLANGE \$5





#### DETAIL OF BOLTED FIELD SPLICE

DESIGNED BY: KLW FEB 2024
DETAILED BY: JTC FEB 2024
CHECKED BY: NSC MAR 2024

Bolts shall be 7/8-inch diameter ASTM F3125 Grade A325 Type 1 in 15/16-inch diameter holes.

Contact surfaces shall be in accordance with Sec 1081 for surface preparation.

The flange and splice plates shall be subject to notch toughness requirements.

	11 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		10 10 10
= = = = = 0 Exterior	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1   1   1   1   1   1   1   1   1   1	16 16 0
Parallel to Grade Bottom of Top Flange			
4 Equal Spaces > 67'-0"± >	10 Equal Spaces > 99'-0"± >	10 Equal Spaces > 99'-0"± >	4 Equal Spaces 39'-6"±
€	€ Bearing	€ Bearing	€ Bearing → Span (4-5)

#### DEAD LOAD DEFLECTION

Dead load deflection includes weight of concrete slab and barrier.

Negative Values indicates upward deflection.

#### Haunching:

Slab is to be considered a uniform thickness as shown on the plans. Haunching will vary. See front sheet for slab thickness. Adjust haunch over girders to match existing grade. Adjust haunch for concrete dead load deflection per detail this sheet.

TABLE OF DIMENSIONS - FIELD SPLICE														
LOCATION	Α	В	С	D	Е	F	G	J	K	М	N	Р	Q	R
S5 Top Flange	8"	S 00 ■	2'-0½"	3 <b>•</b>	യിധ =	18½"	1½"	3	2	-	8"	1 <b>1</b>	12"	ω"
S5 Bottom Flange	15"	<u>3</u> <b></b>	5'-9½"	6 <b>"</b>	<u>3</u> <b></b>	5 ' - 2½"	2"	4	4	2 <del>1</del> "	-	-	•	7"

#### Note:

Drill holes in existing girder flanges and webs using new splice plates as a template. See special provisions.

WILLIAM COLONIA	HILLS DESCRIPTION	ر ا : د	WET NUM E-2	TER IBER 5019	₹ ) ;		SINETY S		
THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY									
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		-	соц <b>1О</b>	L NO	Γ	9			
		COV	TRA	CT	1 D				
			1 9						
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DATE									
RANSPORTATION	7				105 WEST CAPITOL	ON CITY, MO 65102	12622 376 000 17		

MISSOURI HIGHWAYS AND TRANSPORTATIC COMMISSION

MADOT

105 WEST CAPITO

105 WEST CAPITO

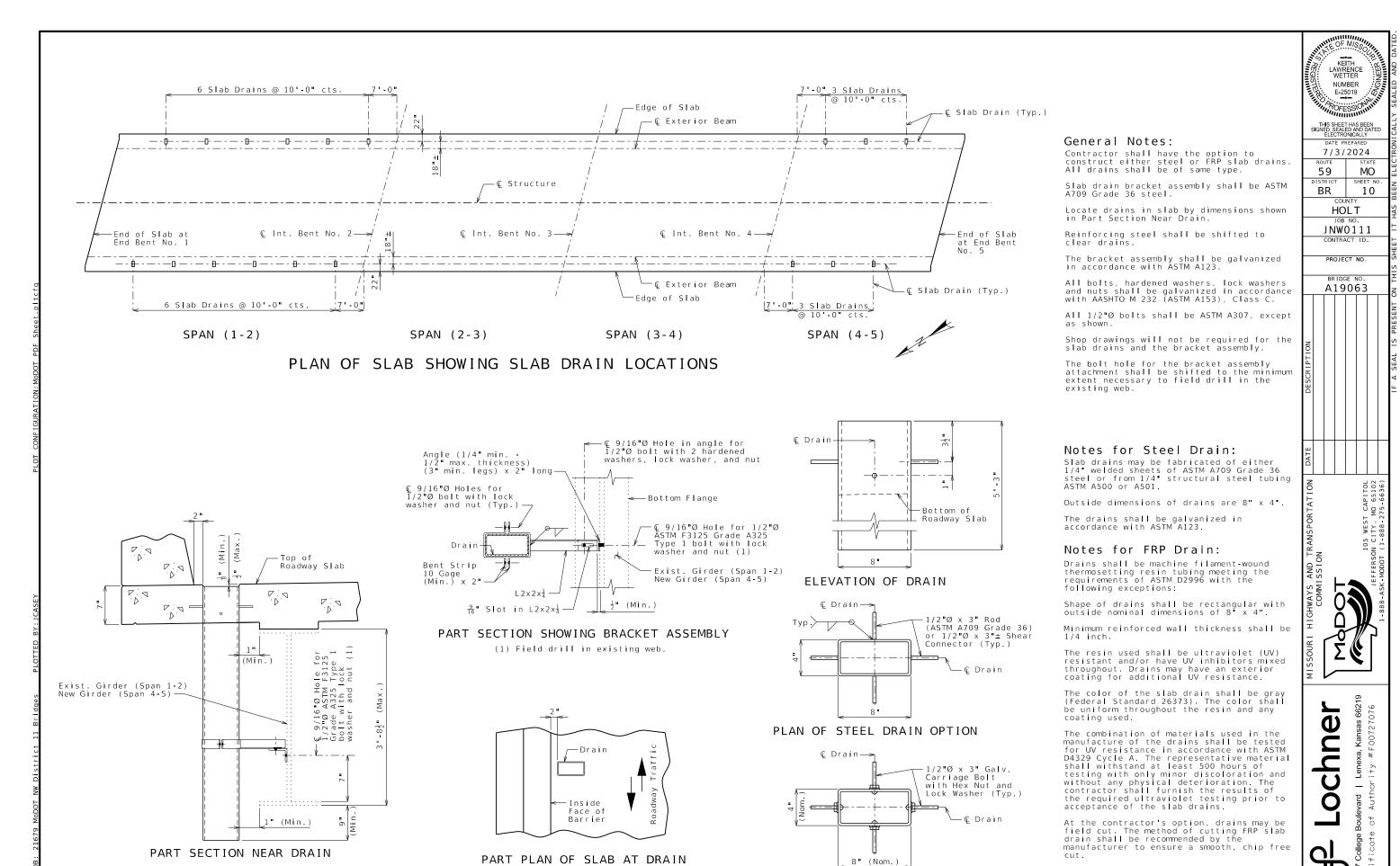
106 WEST CAPITO

107 WEST CAPITO

10

Lochner Amera, Kansas 6627

Note: This drawing is not to scale. Follow dimensions.



SLAB DRAINS

Sheet No. 10 of 14

Note: This drawing is not to scale. Follow dimensions.

PLAN OF FRP DRAIN OPTION

DESIGNED BY: KLW FEB 2024

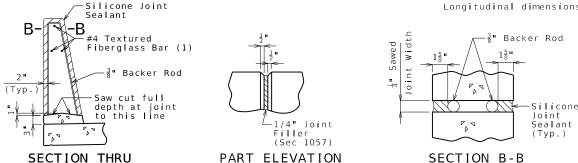
DETAILED BY: JTC FEB 2024 CHECKED BY: BPW MAR 2024

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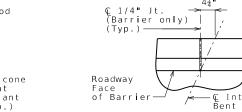
#### ELEVATION OF BARRIER

(Left barrier shown, right barrier similar)

Longitudinal dimensions are taken parallel to grade.

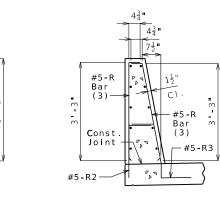


SPAN (3-4)



#### PART PLAN SHOWING JOINT LOCATION

SPAN (4-5)



# 4" Plastic Waterstop (Centered Joint on joint) Joint DO

#### R-BAR PERMISSIBLE ALTERNATE SHAPE

(3) The R1 bar may be separated into two bars as shown, at the contractor's option, only when slip forming is not used. (All dimensions are out to

#### WATERSTOP DETAIL Plastic waterstop shall be

placed in all formed joints, except structures with superelevation, use on lower joints only

Cost of plastic waterstop complete in place, will be considered completely covered by the contract unit price for Type D Barrier.

#### General Notes:

\* Slip-formed option only.

Conventional forming or slip forming may be used. Saw cut joints may be used with conventional forming.

Top of barrier shall be built parallel to grade and barrier joints (except at end bents) normal to grade.

All exposed edges of barrier shall have either a 1/2-inch radius or a 3/8-inch bevel, unless otherwise noted.

Payment for all concrete and reinforcement, complete in place, will be considered completely covered by the contract unit price for Type D Barrier per

Concrete in barrier shall be Class B-1.

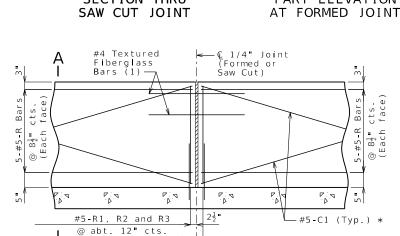
Measurement of barrier is to the nearest linear foot for each structure, measured along the outside top of slab from end of wing to end of wing.

Concrete traffic barrier delineators shall be placed on top of the barrier as shown on Missouri Standard Plan 617.10 and in accordance with Sec 617. Delineators on bridges with two-lane, two-way traffic shall have retroreflective sheeting on both sides. Concrete traffic barrier delineators will be considered completely covered by the contract unit price for Type D Barrier

Joint sealant and backer rods shall be in accordance with Sec 717 for silicone joint sealant for saw cut and formed joints.

For slip-formed option, both sides of barrier shall have a vertically broomed finish and the top shall have a transversely broomed finish.

Plastic waterstop shall not be used with saw cut joints.



#### PART ELEVATION OF BARRIER

#5-R1, R2 and R3

@ abt. 12" cts.

(1) Four feet long, centered on joint, slip-formed option only

DESIGNED BY: NSC MAR 2024

DETAILED BY: RCL MAR 2024 CHECKED BY: DMA MAR 2024 Use a minimum lap of 3'-1" for #5 horizontal barrier bars. The cross-sectional area above the slab is 3.52 square feet.

SECTION A-A

- #5 **-** R3

(2) To top of bar

#5-R1

#5-C1

#5-R2

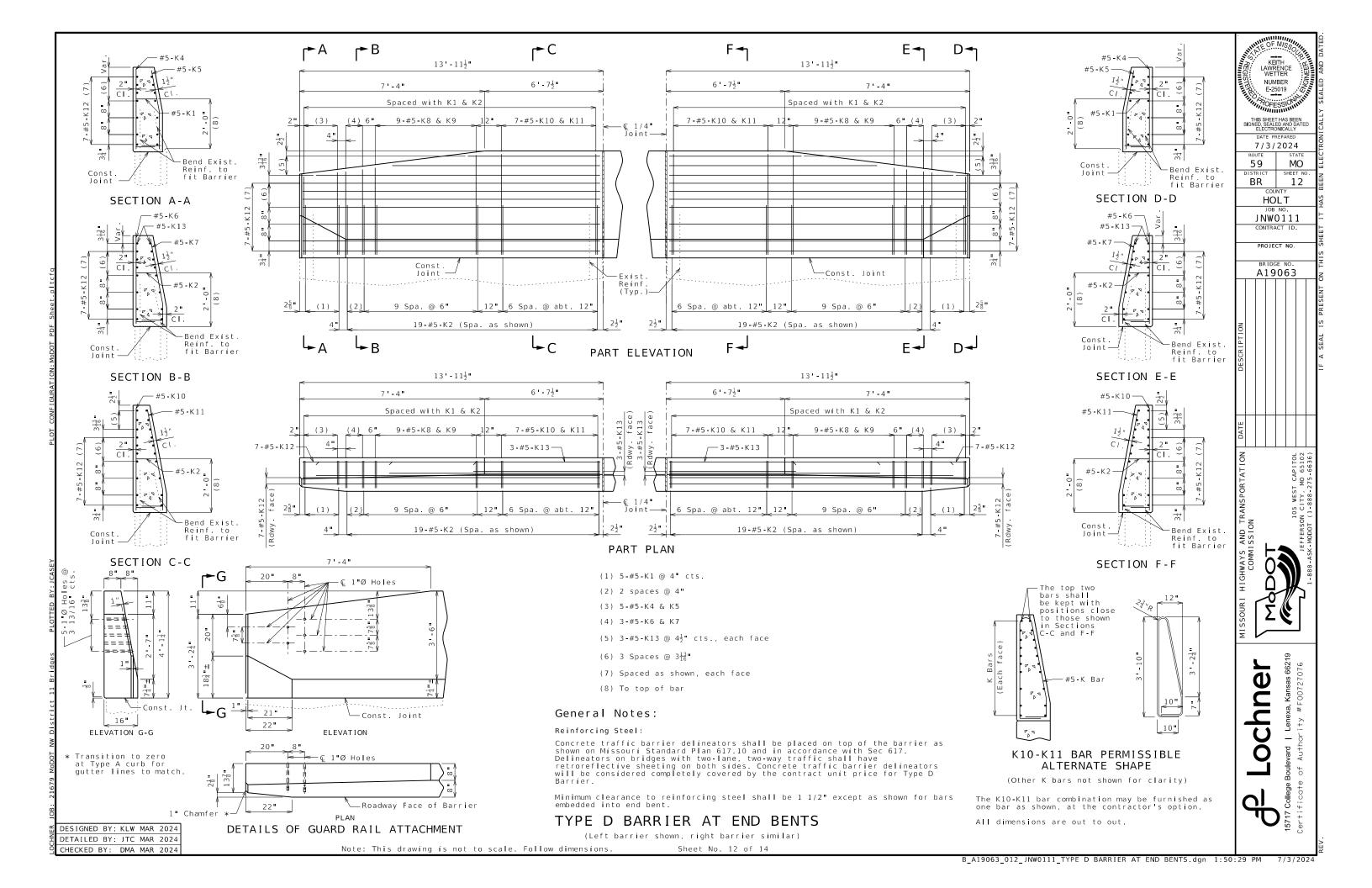
Const. Joint

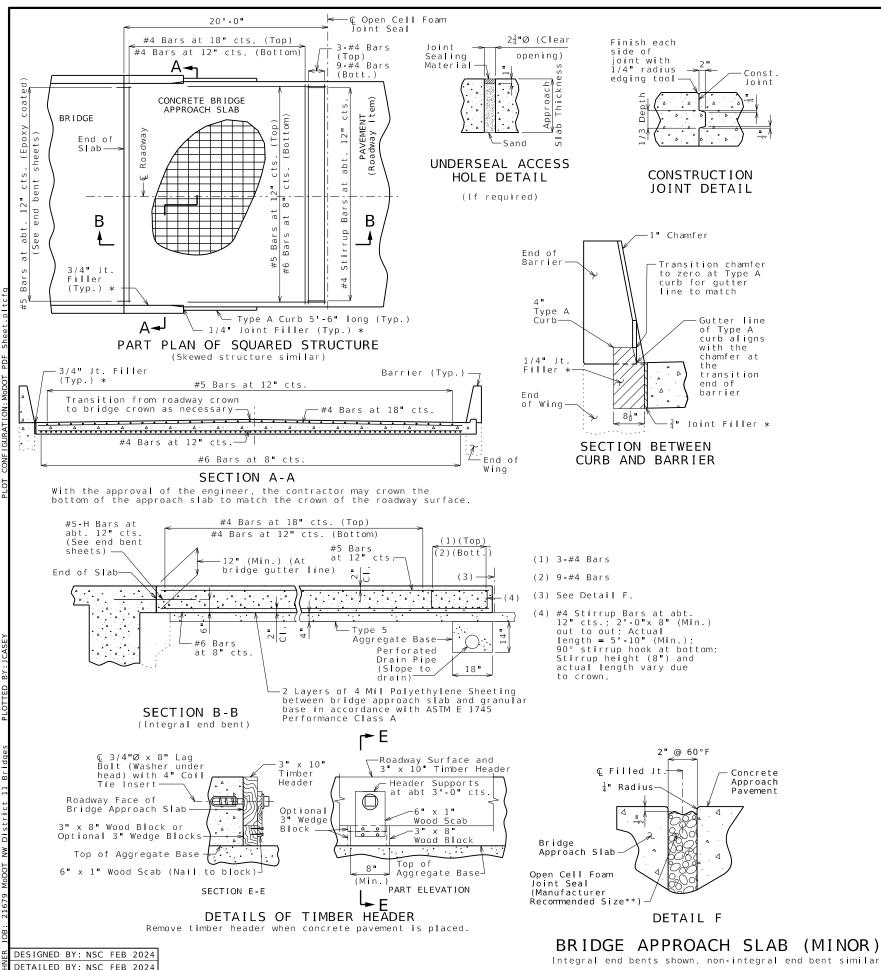
#### TYPE D BARRIER Sheet No. 11 of 14

NUMBER E-25019 7/3/2024 59 MO SHEET NO BR 11 HOLT JNW0111 PROJECT NO A19063 66219

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Note: This drawing is not to scale. Follow dimensions.





CHECKED BY: BPW MAR 2024

#### General Notes:

The contractor shall pour and satisfactorily finish the bridge slab before placing the bridge approach slab.

All concrete for the bridge approach slab shall be in accordance with Sec 503 (f'c = 4,000 psi).

The reinforcing steel in the bridge approach slab shall be epoxy coated Grade 60 with fy = 60,000 psi.

Longitudinal construction joints in bridge approach slab shall be aligned with longitudinal construction joints in bridge slab.

Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise show

The reinforcing steel in the bridge approach slab shall be continuous. The transverse reinforcing steel may be made continuous by providing a minimum lap splice of 23 inches for #4 bars, or by mechanical bar splice.

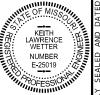
All joint filler shall be in accordance with Sec 1057 for preformed fiber expansion joint filler except as noted.

Payment for furnishing all materials, labor and excavation necessary to construct the concrete bridge approach slab, including the timber header underdrain, Type 5 aggregate base, joint filler underdrain, Type 5 aggregate base, joint filler and all other appurtenances and incidental work as shown on this sheet, complete in place, will be considered completely covered by the contract unit price for Bridge Approach Slab (Minor) per square yard

See Missouri Standard Plan 609.00 for details

Drain pipe may be either 6" diameter corrugated metallic-coated pipe underdrain, 4" diameter corrugated polyvinyl chloride (PVC) drain pipe, or 4" diameter corrugated polyethylene (PE)

- \* Seal joint between vertical face of approach slab and wing with sealant in accordance with Sec 717 for silicone joint sealant for saw cut and formed joints.
- \*\* Required Joint Size Range 1 1/16" Minimum to 3 15/16" Maximum



7/3/2024

59 MO SHEET NO BR 13

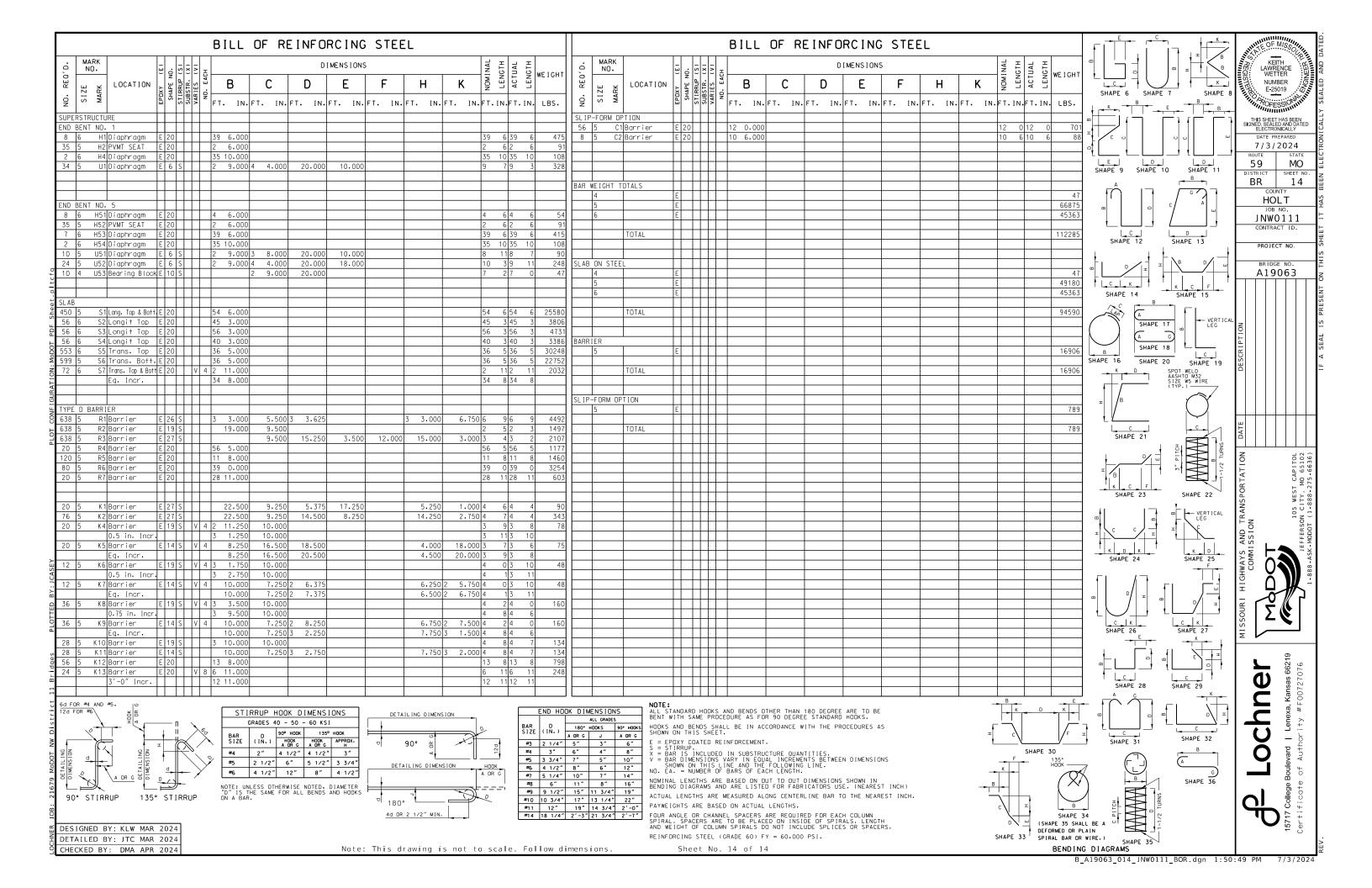
HOLT JNW0111 CONTRACT ID.

PROJECT NO

A19063

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BRIDGE APPROACH SLAB (MINOR)

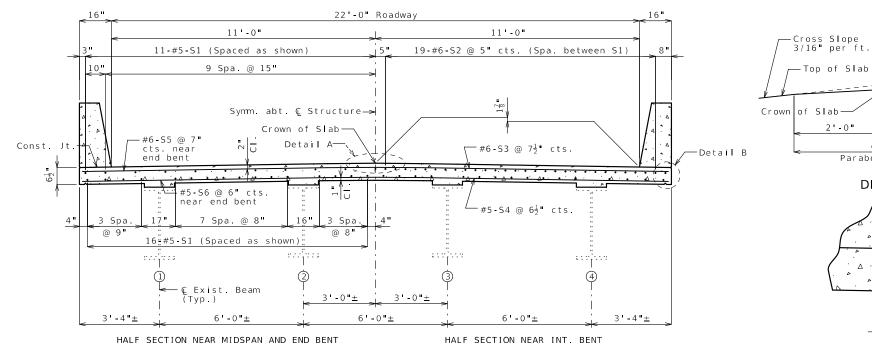


#### Table Showing S2 Bar Lengths Int. Bent No. 2 Int. Bent No. 3 Span 2 Span 3 Span 2 21'-0" 21 -0" 21'-0" 21'-0"

	Lap Length Splices **
Bar Size	Splice Length
4	2'-7"
5	3'-3"
6	3'-10"
7	4'-11"

\*\* Unless otherwise shown.

#### U.I.P., REDECK, AND MAKE COMPOSITE EXISTING (50'- 70'- 50') CONTINUOUS WIDE FLANGE BEAM SPANS (SKEW:20° R.A.)



TYPICAL SECTION THRU SLAB

Design Specifications: 2002 AASHTO LFD (17th Ed.) Standard Specifications Seismic Performance Category A

Design Loading:

General Notes:

H15-44 (I Lane) (1962) (Existing) HS20-44 (New Construction)
35 lb/sf Future Wearing Surface
Earth - 120 lb/cf, Equivalent Fluid Pressure 45 lb/cf Fatigue Stress - Case III

Design Unit Stresses

Class B-1 Concrete (Barrier) f'c = 4,000 psiClass B-2 Concrete (End Bents & Superstructure, except Barrier) f'c = 4,000 psiReinforcing Steel (Grade 60) fy = 60,000 psi

Joint Filler:

All joint filler shall be in accordance with Sec 1057 for preformed sponge rubber expansion and partition joint filler, except as noted.

Reinforcing Steel:

Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise shown.

Miscellaneous:

Bars bonded in existing concrete not removed shall be cleanly stripped and embedded into new concrete where possible. If length is available, existing bars shall extend into new concrete at least 40 diameters for plain bars and 30 diameters for deformed bars, unless otherwise noted

Roadway surfacing adjacent to bridge ends shall match new bridge slab surface (Roadway Item)

Outline of existing work is indicated by light dashed lines. Heavy lines indicate new work.

Contractor shall verify all dimensions in field before

The area exposed by the removal of concrete and not covered with new concrete shall be coated with an approved qualified special mortar in accordance with Sec 704.

Rubblized concrete from the existing bridge deck that qualifies as clean fill many be placed on spill slopes at end bents above ordinary high water line (Roadway item).

Traffic Handling:

Structure to be closed during construction. Traffic to be maintained on other routes during construction. See roadway plans for traffic control.

Structural Steel Protective Coating (Pile):

The exposed surfaces of the existing structural steel CIP pile shells, to limits shown on the bridge plans, shall be coated with one 6-mil thickness of aluminum gray epoxymastic primer applied over an SSPC-SP3 surface preparation in accordance with Sec 1081. The bituminous coating shall be applied one foot above and one foot below the existing ground line and in accordance with Sec 702. The cost of surface preparation will be considered completely covered by the contract lump sum price for Surface Preparation for Applying Epoxy-Mastic Primer. The cost of the aluminum epoxy-mastic primer and bituminous coating will be considered completely covered by the contract lump sum price for Aluminmum Epoxy-Mastic Primer.

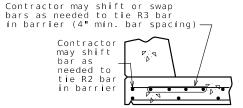
Structural Steel Protective Coating (Top Flange):

In accordance with Sec. 216.50 and 1081, the top, and additionally the sides, and bottom of the top flange shall be coated with not less than 3.0 mils of Gray Epoxy Mastic-Primer (non-alumimum) applied over an SSPC-SP3 surface preparation. Payment for coating steel will be considered completely covered by the contract sq. foot price for Removal of Existing Bridge Deck.

Method of forming the slab shall be in accordance with Sec 703. All hardware for forming the slab to be left in place as a permanent part of the structure shall be coated in accordance with ASTM A123 or ASTM B633 with a thickness Class SC 4 and a finish Type I, II, or III.

Slab shall be cast-in-place with conventional forming or stay-in-place corrugated steel forms. Precast prestressed panels will not be permitted.

For Optional Stay-In-Place Form Details, see Sheet No. 2.



OPTIONAL SHIFTING TOP BARS AT BARRIER

Estimated Quantities		
I t em		Total
Removal of Existing Bridge Deck	sq. foot	3,862
Bridge Approach Slab (Minor)	sq. yard	101
Slab on Steel	sq. yard	473
Type H Barrier	linear foot	348
Substructure Repair (Unformed)	sq. foot	10
Shear Connectors	each	1,848
Slab Drain	each	34
Surface Preparation for Applying Epoxy - Mastic Primer	lump sum	1
Aluminum Epoxy - Mastic Primer	lump sum	1
Non-Destructive Testing	linear foot	19
Vertical Drain at End Bents	each	2
Open Cell Foam Joint Seal	linear foot	44

SEC/SUR 4

TWP 59

- C Roadway

2'-0"

-Const. Jt.

¢ 3/4" Drip

Groove (Typ.)

4'-0"

Parabolic Crown

DETAIL A

DETAIL B

-Profile Grade

(Match exist ±)

RGE

38W

Cost of any required excavation for bridge will be considered completely covered by the contract unit price for other items

\* Concrete Bridge Approach Slab only. See Special Provisions.

Estimated Quantities for Slab on St	eel
I t em	Total
Class B-2 Concrete cu. yard	96
Reinforcing Steel (Epoxy Coated) pound	41,280

The table of Estimated Quantities for Slab on Steel represents the quantities used by the State in preparing the cost estimate for concrete slabs. The area of the concrete slab will be measured to the nearest square yard longitudinally from end of slab to end of slab and transversely from out to out of bridge slab (or with the horizontal dimensions as shown on the plan of slab). Payment for stay-in-place corrugated steel forms, conventional forms, all concrete and epoxy coated reinforcing steel will be considered completely covered by the contract unit price for the slab. Variations may be encountered in the estimated quantities but the variations cannot be used for an adjustment in the contract unit price

#### REPAIRS TO BRIDGE: ROUTE T OVER MILL CREEK

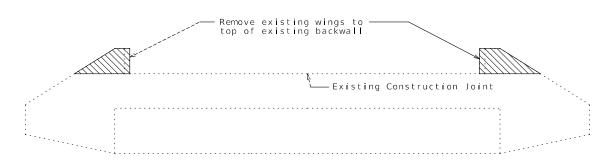
ROUTE T FROM ROUTE 111 TO ROUTE O ABOUT 2.3 MILES SOUTHEAST OF ROUTE 111 BEGINNING STATION 124+81.00± (MATCH EXISTING) 07/11/2024

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#### DETAILS OF CONCRETE REMOVAL AT END BENTS

The cost of concrete removal as shown will be considered completely covered by the contract unit price for Removal of Existing Bridge Deck. Vertical backwall and wingwall reinforcement to be cut off one inch below concrete removal surface and the resulting holes shall be filled with a qualified special mortar

A smooth, level surface shall be provided at Bents No. 1 & 4 removal lines.

#### General Notes:

#### Stay-In-Place Forms:

Corrugated steel forms, supports, closure elements, and accessories shall be in accordance with grade requirement and coating designation G165 of ASTM A653. Complete shop drawings of the permanent steel deck forms shall be required in accordance with Sec 1080.

Corrugations of stay-in-place forms shall be filled with an expanded polystyrene material. The polystyrene material shall be placed in the forms with an adhesive in accordance with the manufacturer's recommendations.

Form sheets shall not rest directly on the top of beam flanges. Sheets shall be securely fastened to form supports with a minimum bearing length of one inch on each end. Form supports shall be placed in direct contact with the flange. Welding on or drilling holes in the beam flanges will not be permitted. All steel fabrication and construction shall be in accordance with Sec 1080 and 712. Certified field welders will not be required for welding of the form supports.

The design of stay-in-place corrugated steel forms is per manufacturer which shall be in accordance with Sec 703 for false work and forms. Maximum actual weight of corrugated steel forms allowed shall be 4 psf assumed for beam loading.

The contractor shall provide a method of preventing the direct contact of the stay-in-place forms and connection components with uncoated weathering steel members that is approved by the engineer.

#### Pouring and Finishing Slab:

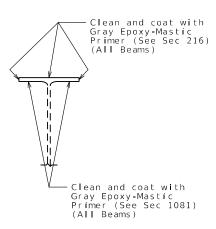
The contractor shall provide bracing necessary for lateral and torsional stability of the beams during construction of the concrete slab and remove the bracing after the slab has attained 75% design strength. Contractor shall not weld on or drill holes in the beams. The cost for furnishing, installing, and removing bracing will be considered completely covered by the contract unit price for Slab on Steel.

Slab shall be poured upgrade from end to end at a minimum rate of 25 cubic yards per hour.

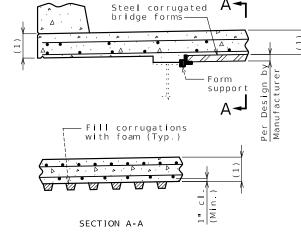
Alternate pour sequences may be submitted to the engineer for approval. Keyed construction joints shall be provided between pours.

#### Haunching:

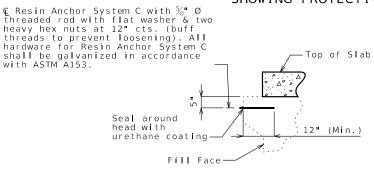
(1) Slab is to be considered a uniform thickness as shown on the plans. Haunching height will vary. See front sheet for slab thickness, for adjusted girder deflection due to weight of new deck and barriers, see Bridge Electronic Deliverables.



#### TYPICAL SECTION THRU BEAM SHOWING PROTECTIVE COATING

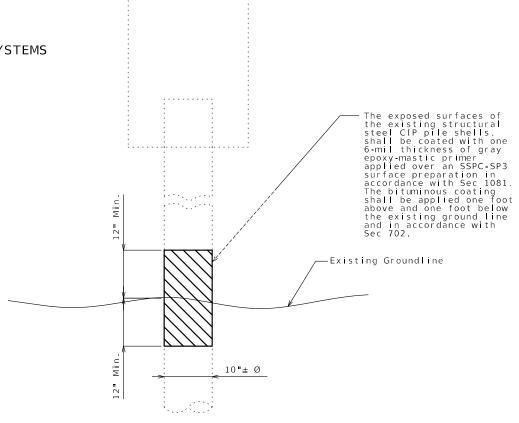


OPTIONAL STAY-IN-PLACE FORM DETAILS



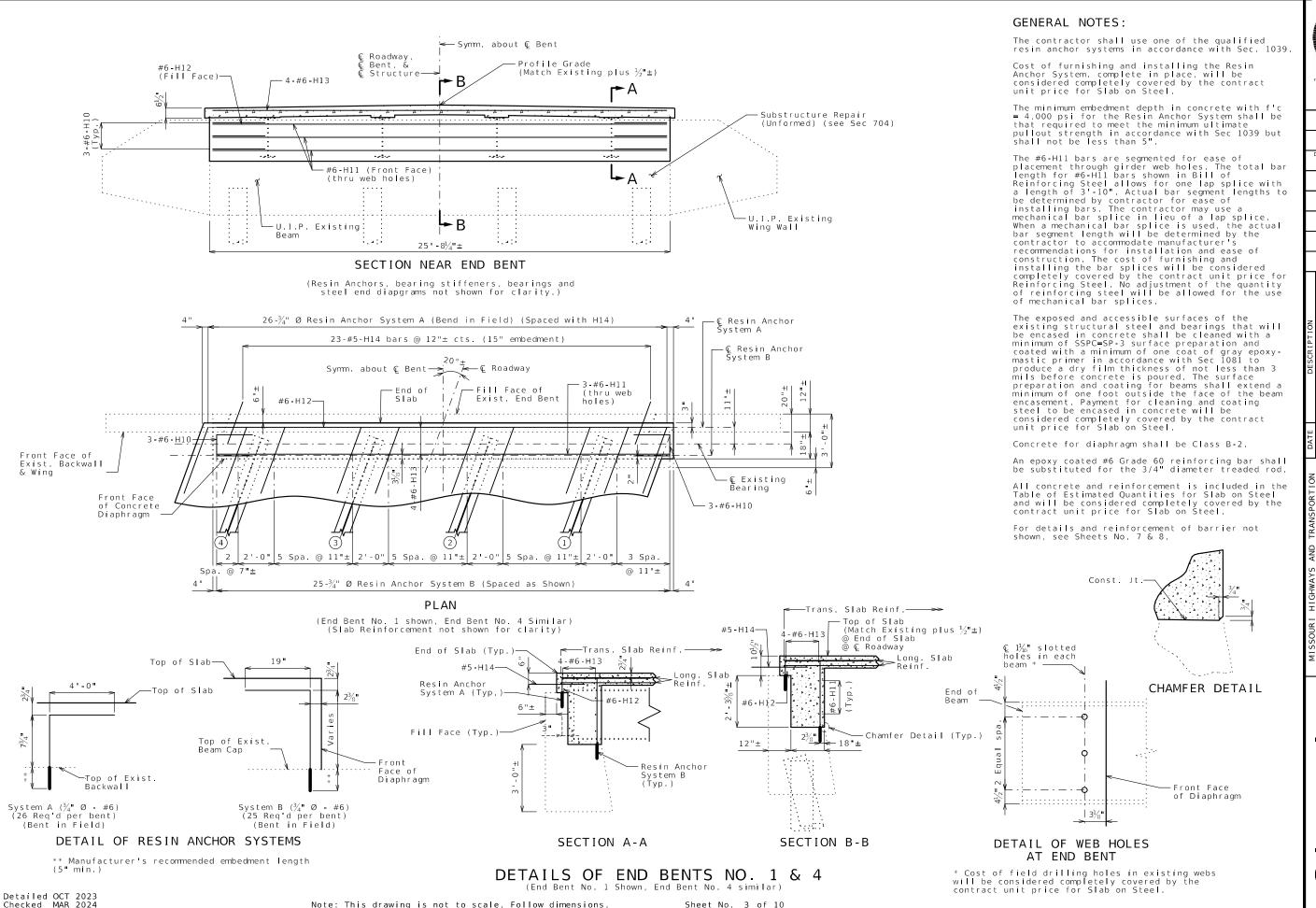
System C (23 Req'd per end bent)

#### DETAILS OF RESIN ANCHOR SYSTEMS



INT. BENT PROTECTIVE COATING DETAILS

#### REHAB DETAILS



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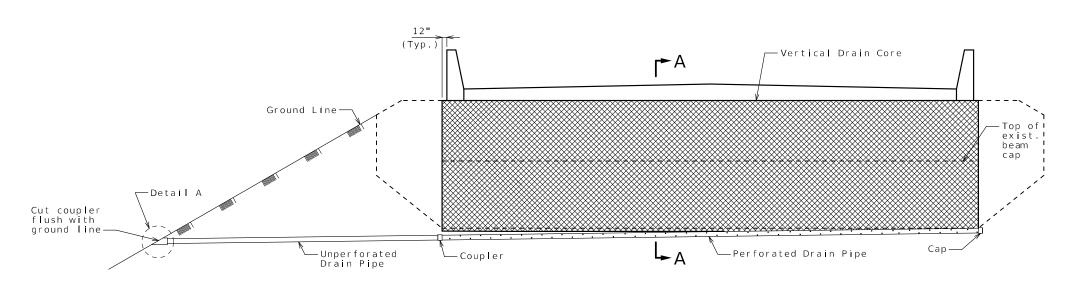
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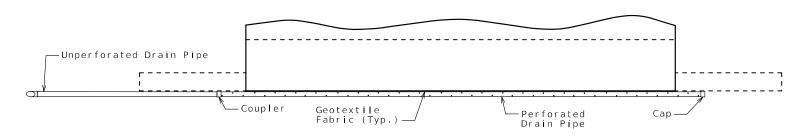
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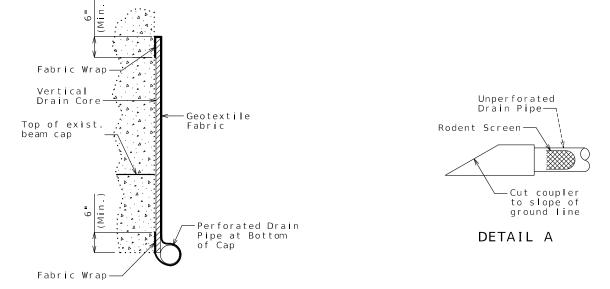
Main Plaza, 4435 Main St. Ste 1150
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ELEVATION OF END BENT



PLAN OF END BENT



PART SECTION A-A (Section thru wing similar)

#### VERTICAL DRAIN AT END BENTS

(Squared end bent shown, skewed end bent similar)

#### General Notes:

All drain pipe shall be sloped 1 to 2 percent.

Drain pipe may be either 6-inch diameter corrugated metallic-coated steel pipe underdrain, 4-inch diameter corrugated polyvinyl chloride (PVC) drain pipe, or 4inch diameter corrugated polyethylene (PE) drain pipe.

Drain pipe shall be placed at fill face of end bent and inside face of wings. The pipe shall slope to lowest grade of ground line, also missing the lower beam of end bent by a minimum of 1 1/2 inches.

Perforated pipe shall be placed at fill face side and inside face of wings at the bottom of end bent and plain pipe shall be used where the vertical drain ends to the exit at ground line.

All excavation necessary for installation of vertical drain will be considered completely covered by the contract unit price for Vertical Drain at End Bents.

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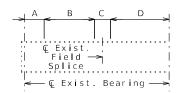
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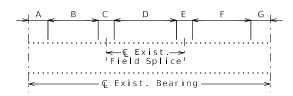
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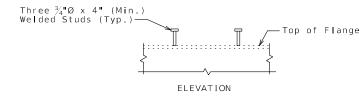


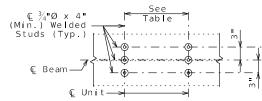
ELEVATION SHOWING SHEAR CONNECTOR SPACING FOR SPAN 1 & 3 BEAMS



ELEVATION SHOWING SHEAR CONNECTOR SPACING FOR SPAN 2 BEAMS

	TABLE SHOWING SHEAR CONNECTOR UNIT SPACING										
Beam S.C. per unit A B C D E F G											
Beam (Span 1-2)	ε	0"±	37 Units @ 12" cts.	2'-9"	10 Uni	ts @ 15" cts.					
Beam (Span 2-3)	3	15" <u>±</u>	13 Units @ 15"± cts.	2'-3"	34 Uni	ts @ 12 <b>"</b> ± cts.	2'-3"	13 Units @ 15"± cts.	15"±		
Beam (Span 4-3)	3	0"±	37 Units @ 12" cts.	2'-9"	10 Uni	ts @ 15" cts.		<del>-</del> -			
	Total shear connectors required per girder 462										





PLAN OF SHEAR CONN. (3 PER UNIT)



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benesch one Main Plaza, 4435 Main St., Ste 1150 Kansas City, Wo 64111 913/441-1100, FAX 913/441-1468

# Top of Slab \* Top of Slab \* Bottom of Slab \* Exist. Beam \* HALF SECTION THRU BEAM THRU COVER ©

SECTION THRU EXIST.
BEAM SHOWING
SHEAR CONNECTORS

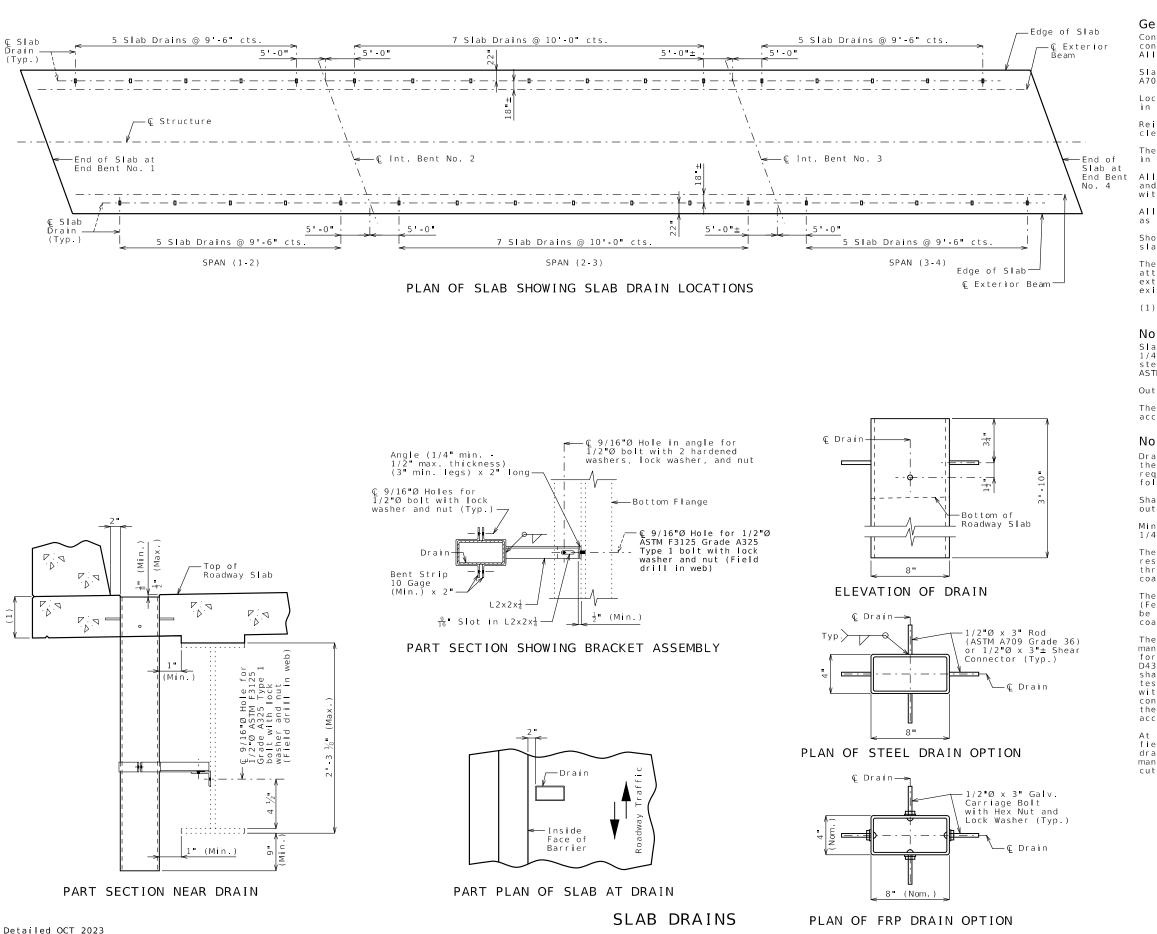
\* 3" Minimum

\*\* Min. Haunch = 0.00" Max. Haunch = 0.75" (Exterior Beams) Max. Haunch = 3.00" (Interior Beams)

#### General Notes:

The cost of supplying and installing shear connectors will be considered completely covered by the contract unit price for Shear Connectors.

Shear connectors shall be in accordance with Sec 712, 1037 & 1080.



#### General Notes:

Contractor shall have the option to construct either steel or FRP slab drains. All drains shall be of same type.

Slab drain bracket assembly shall be ASTM A709 Grade 36 steel

Locate drains in slab by dimensions shown in Part Section Near Drain.

Reinforcing steel shall be shifted to clear drains.

The bracket assembly shall be galvanized in accordance with ASTM A123.

All bolts, hardened washers, lock washers and nuts shall be galvanized in accordance with AASHTO M 232 (ASTM A153), Class C.

All 1/2"Ø bolts shall be ASTM A307, except

Shop drawings will not be required for the slab drains and the bracket assembly.

The bolt hole for the bracket assembly attachment shall be shifted to the minimum extent necessary to field drill in the existing web.

(1) See front sheet for slab thickness.

#### Notes for Steel Drain:

Slab drains may be fabricated of either 1/4" welded sheets of ASTM A709 Grade 36 steel or from 1/4" structural steel tubing ASTM A500 or A501.

Outside dimensions of drains are 8" x 4".

The drains shall be galvanized in accordance with ASTM A123.

#### Notes for FRP Drain:

Drains shall be machine filament-wound thermosetting resin tubing meeting the requirements of ASTM D2996 with the

Shape of drains shall be rectangular with outside nominal dimensions of  $8\,^{\circ}$  x  $4\,^{\circ}$ .

Minimum reinforced wall thickness shall be

The resin used shall be ultraviolet (UV) resistant and/or have UV inhibitors mixed throughout. Drains may have an exterior coating for additional UV resistance.

The color of the slab drain shall be gray (Federal Standard 26373). The color shall be uniform throughout the resin and any coating used.

The combination of materials used in the manufacture of the drains shall be tested for UV resistance in accordance with ASTM D4329 Cycle A. The representative material shall withstand at least 500 hours of testing with only minor discoloration and without any physical deterioration. The contractor shall furnish the results of the required ultraviolet testing prior to acceptance of the slab drains.

At the contractor's option, drains may be field cut. The method of cutting FRP slab drain shall be recommended by the manufacturer to ensure a smooth, chip free



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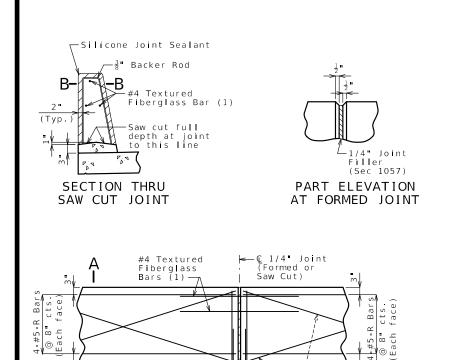
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Checked MAR 2024

#### ELEVATION OF BARRIER

(Left barrier shown, right barrier similar) Longitudinal dimensions are horizontal.



#### PART ELEVATION OF BARRIER

PV

21 "

0 0

#5-R1, R2 & R3

@ abt. 12" cts.

Detailed OCT 2023

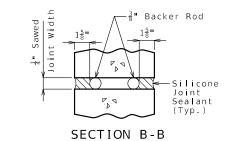
PV

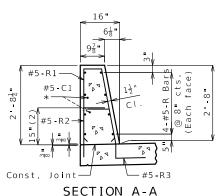
#5-R1, R2 & R3

@ abt 12" cts.

√— #5-C1 (Typ.) \*

(1) Four feet long, centered on joint slip-formed option only

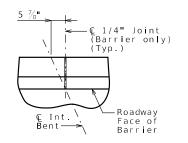




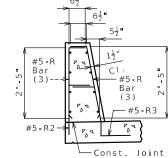
# Use a minimum lap of 3'-1" for #5 horizontal barrier bars.

The cross-sectional area above the slab is 2.89 square feet.

(2) To top of bar



# PART PLAN SHOWING JOINT LOCATION



#### R-BAR PERMISSIBLE ALTERNATE SHAPE

(3) The R1 bar may be separated into two bars as shown, at the contractor's option, only when slip forming is not used. (All dimensions are out to out.)

#### General Notes:

\* Slip-formed option only.

Conventional forming or slip forming may be used. Saw cut joints may be used with conventional forming.

Top of barrier shall be built parallel to grade and barrier joints normal to grade.

All exposed edges of barrier shall have either a 1/2-inch radius or a 3/8-inch bevel, unless otherwise noted.

Payment for all concrete and reinforcement, complete in place, will be considered completely covered by the contract unit price for Type H Barrier per linear foot.

Concrete in barrier shall be Class B-1.

Measurement of barrier is to the nearest linear foot for each structure, measured along the outside top of slab from end of slab to end of slab.

Concrete traffic barrier delineators shall be placed on top of the barrier as shown on Missouri Standard Plan 617.10 and in accordance with Sec 617. Delineators on bridges with two-lane, two-way traffic shall have retroreflective sheeting on both sides. Concrete traffic barrier delineators will be considered completely covered by the contract unit price for Type H Barrier.

Joint sealant and backer rods shall be in accordance with Sec 717 for silicone joint sealant for saw cut and formed joints.

For slip-formed option, both sides of barrier shall have a vertically broomed finish and the top shall have a transversely broomed finish. CHRISTOPHER
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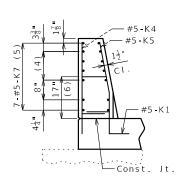
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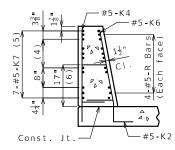
**benesch**Main Plaza, 4435 Main St. Ste 1150
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TYPE H BARRIER

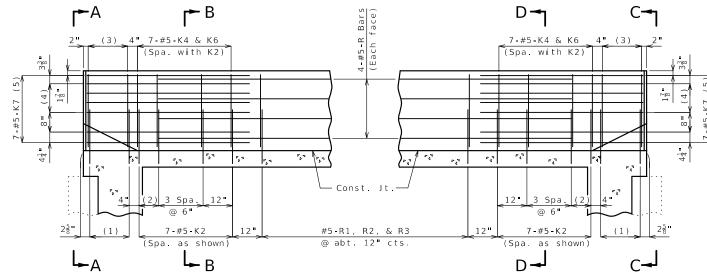
Sheet No. 7 of 10



ELEVATION A-A



SECTION B-B



#### PART ELEVATION

(1) 5-#5-K1 @ 4" cts.

(2) 2 Spaces @ 4"

(Spa. with K2)

(3) 4" 7-#5-K4 & K6

- (3) 5-#5-K4 and 5-#5-K5, spaced with K1
- (4) 3 Spaces @ 3<sup>13</sup><sub>16</sub>"
- (5) Spaced as shown, each face
- (6) To top of bar

7-#5-K4 & K6 4"

(Spa. with K2)

12" | 3 Spa. (2)

7-#5-K2

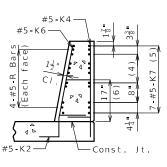
(Spa. as shown)



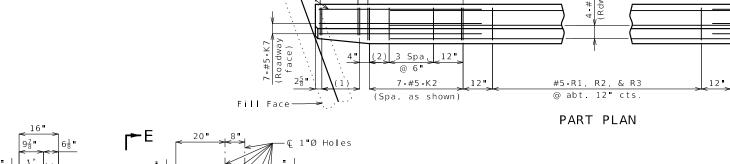
-End of Slab

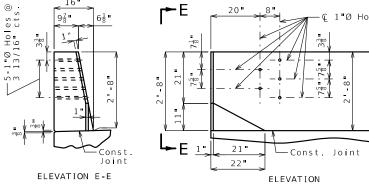
#5-K4 #5-K5 #5-K1 #5-K1 #5-K1 #5-K1 #5-K1

ELEVATION C-C



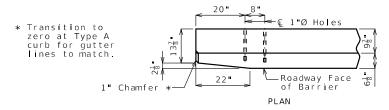
SECTION D-D





End of Slab

7-#5-K7



#### DETAILS OF GUARD RAIL ATTACHMENT

#### General Notes:

Concrete traffic barrier delineators shall be placed on top of the barrier as shown on Missouri Standard Plan 617.10 and in accordance with Sec 617. Delineators on bridges with two-lane, two-way traffic shall have retroreflective sheeting on both sides. Concrete traffic barrier delineators will be considered completely covered by the contract unit price for Type H Barrier.

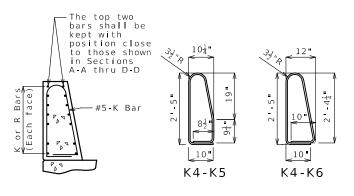
Reinforcing Steel:

Minimum clearance to reinforcing steel shall be 1 1/2".

Use a minimum lap of 3'-1" between K7 bars and R bars.

### TYPE H BARRIER AT END BENTS

(Left barrier shown, right barrier similar)



#### PERMISSIBLE ALTERNATE SHAPES

(Other K bars not shown for clarity)

The K4-K5 and K4-K6 bar combination may be furnished as one bar as shown, at the contractor's option.

All dimensions are out to out.

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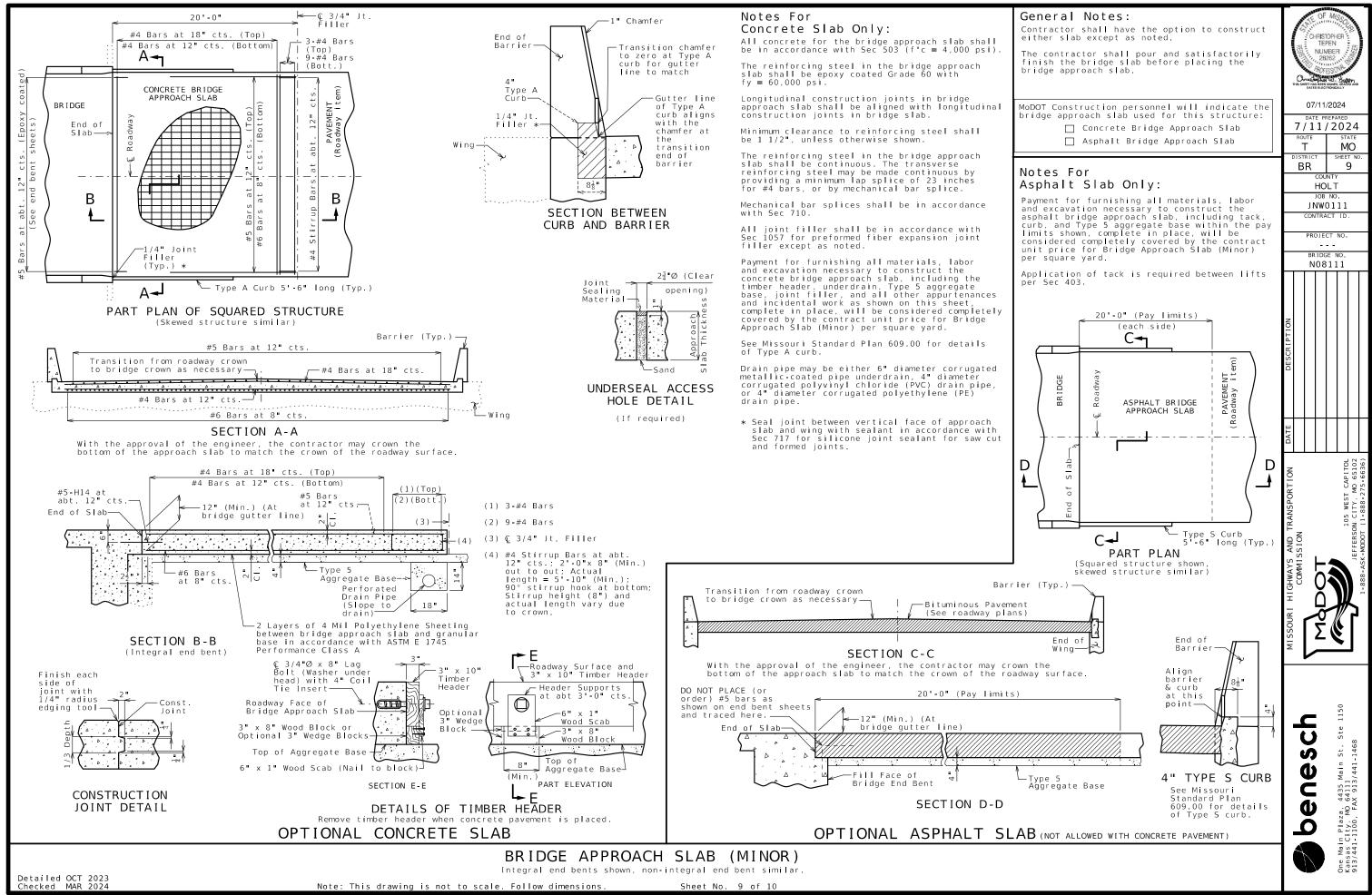
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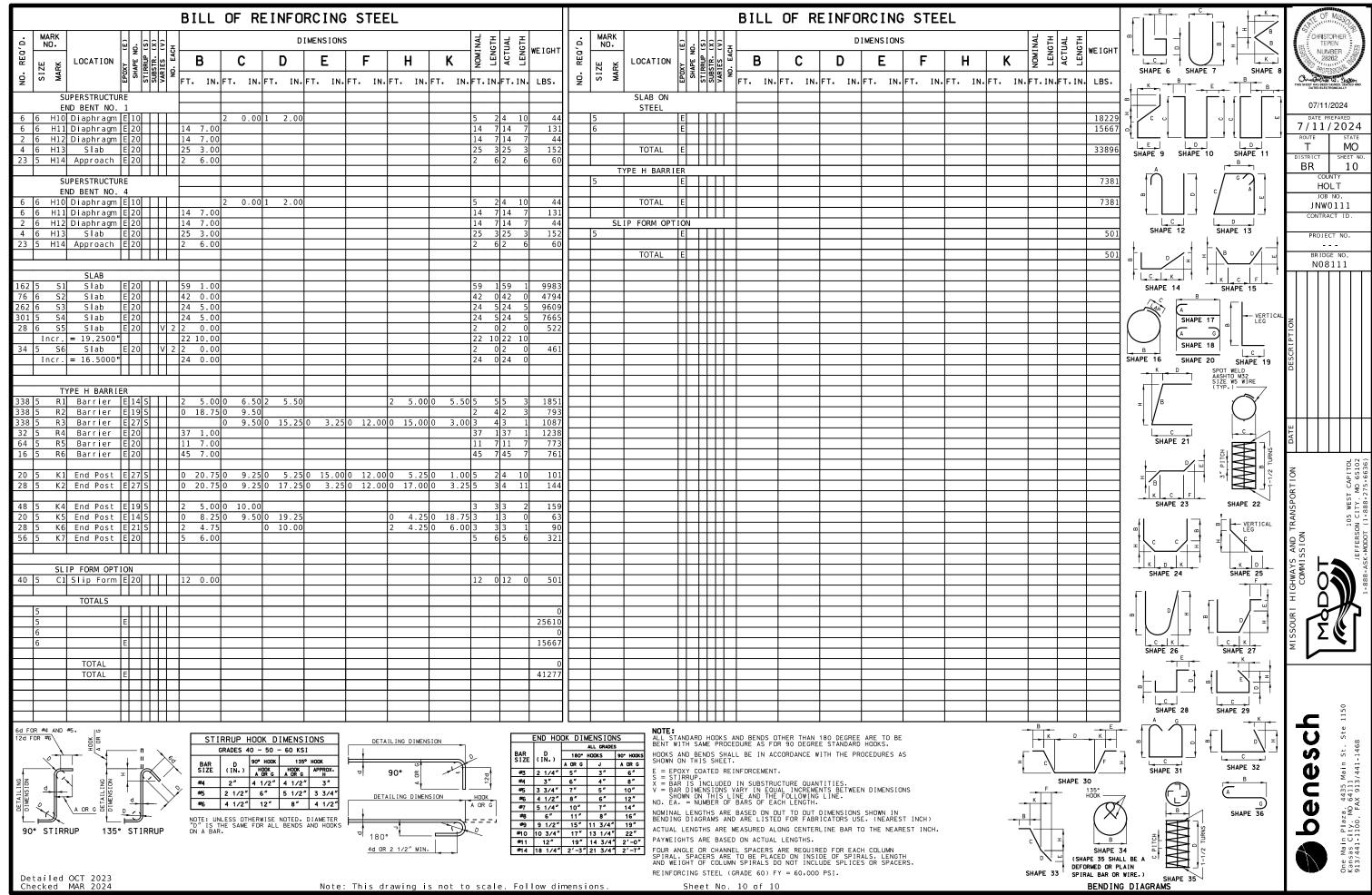


	Table Showing S2 Bar Lengths								
Int. Ber	Int. Bent No. 2 Int. Bent No. 3								
Span 1	Span 1 Span 2 Span 2 Span 3								
16'-9"	16'-9"	16'-9"	16'-9"						

	Lap Length Splices **		
Bar Size	Splice Length		
4	2'-7"		
5	3'-3"		
6	3'-10"		
7	4'-11"		

\*\* Unless otherwise shown

#### General Notes:

Design Specifications: 2002 AASHTO LFD (17th Ed.) Standard Specifications Seismic Performance Category A

Design Loading:

H15-44-(1 Lane) (1961) (Existing) H520-44 (New Construction) No Future Wearing Surface Earth - 120 lb/cf, Equivalent Fluid Pressure 45 lb/cf Fatigue Stress - Case III

Design Unit Stresses:

Class B-1 Concrete (Barrier) f'c = 4,000 psi
Class B-2 Concrete (End Bents & Superstructure,
except Barrier) f'c = 4,000 psi
Reinforcing Steel (Grade 60) f'c = 60,000 psi

Joint Filler:

All joint filler shall be in accordance with Sec 1057 for preformed sponge rubber expansion and partition joint filler, except as noted.

Reinforcing Steel:

Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise shown.

Miscellaneous

Protective coating for concrete bents and piers (Epoxy) shall be applied as shown on the bridge plans and in accordance with Sec 711.

Bars bonded in existing concrete not removed shall be cleanly stripped and embedded into new concrete where possible. If length is available, existing bars shall extend into new concrete at least 40 diameters for plain bars and 30 diameters for deformed bars, unless otherwise noted.

Roadway surfacing adjacent to bridge ends shall match new bridge slab surface. (Roadway item)

Outline of existing work is indicated by light dashed lines. Heavy lines indicate new work.

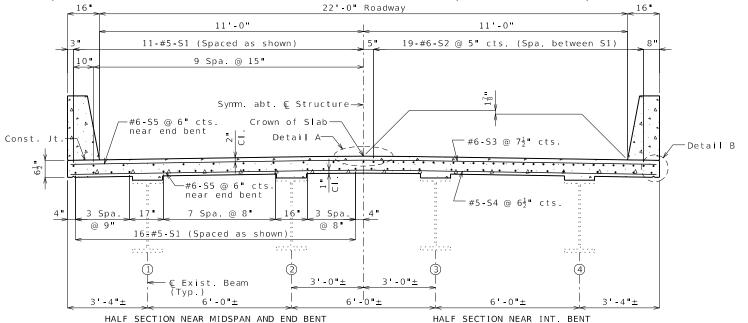
Contractor shall verify all dimensions in field before ordering materials.

The area exposed by the removal of concrete and not covered with new concrete shall be coated with an approved qualified special mortar in accordance with Sec 704.

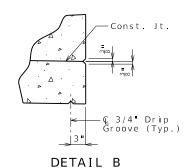
Rubblized concrete from the existing bridge deck that qualifies as clean fill may be placed on spill slopes at end bents above ordinary high water line (Roadway item).

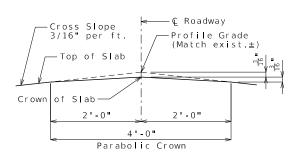
Traffic Handling:

Structure to be closed during construction. Traffic to be maintained on other routes during construction. See roadway plans for traffic control.

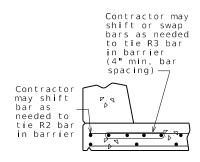


TYPICAL SECTION THRU SLAB





DETAIL A



OPTIONAL SHIFTING TOP BARS AT BARRIER

I t em		Total
Removal of Existing Bridge Deck	sq. foot	3,437
Bridge Approach Slab (Minor)	sq. yard	101
Slab on Steel	sq. yard	423
Type H Barrier	linear foot	311
Substructure Repair (Unformed)	sq. foot	20
Protective Coating - Concrete Bents and Piers (Epoxy)	lump sum	1
Fabricated Structural Carbon Steel (Misc.)	pound	980
Cleaning and Coating Existing Bearings	each	8
Slab Drain	each	28
Non-Destructive Testing	linear foot	42
Vertical Drain at End Bents	each	2

Cost of any required excavation for bridge will be considered completely covered by the contract unit price for other items.

Estimated Quantities	for	Slab on St	eel
I t em			Total
Class B-2 Concrete		cu. yard	81
Reinforcing Steel (Epoxy Coated)		pound	30,020

The table of Estimated Quantities for Slab on Steel represents the quantities used by the State in preparing the cost estimate for concrete slabs. The area of the concrete slab will be measured to the nearest square yard longitudinally from end of slab to end of slab and transversely from out to out of bridge slab (or with the horizontal dimensions as shown on the plan of slab). Payment for stay-in-place corrugated steel forms, conventional forms, all concrete and epoxy coated reinforcing steel will be considered completely covered by the contract unit price for the slab. Variations may be encountered in the estimated quantities but the variations cannot be used for an adjustment in the contract unit price.

Method of forming the slab shall be in accordance with Sec 703. All hardware for forming the slab to be left in place as a permanent part of the structure shall be coated in accordance with ASTM A123 or ASTM B633 with a thickness Class SC 4 and a finish Type I, II or III.

Slab shall be cast-in-place with conventional forming or stay-in-place corrugated steel forms. Precast prestressed panels will not be permitted.

Bridge deck surface may be finished with a vibratory screed.

For Optional Stay-In-Place Form Details, see Sheet No. 2.

#### REPAIRS TO BRIDGE: ROUTE F OVER ROCK CREEK

ROUTE F FROM IOWA STATE LINE TO ROUTE B
ABOUT 2.5 MILES SOUTH OF IOWA STATE LINE
BEGINNING STATION 132+15.70± (MATCH EXISTING)

NUMBER E-25019 7/3/2024 F MO SHEET NO BR 1 **ATCHISON** JNW0111 PROJECT NO R02741

Lochner S717 College Boulevard | Lenexa, Kansas 66219 ertificate of Authority #F00727076

DESIGNED BY: KLW JAN 2024 DETAILED BY: JTC JAN 2024 CHECKED BY: NSC FEB 2024 The cost of concrete removal as shown will be considered completely covered by the contract unit price for Removal of Existing Bridge Deck. Vertical backwall and wingwall reinforcement to be cut off one inch below concrete removal surface and the resulting holes shall be filled with a qualified special mortar.

A smooth, level surface shall be provided at Bent Nos. 1 and 4 removal lines.

#### General Notes:

All concrete for the backwall widening to form pavement seat is included with the Superstructure Quantities.

The contractor shall use one of the qualified resin anchor systems in accordance with Sec 1039.

Cost of furnishing and installing the resin anchor systems, complete in place, will be considered completely covered by the contract unit price for Slab on Steel.

Corrugated steel forms, supports, closure elements and accessories shall be in accordance with grade requirement and coating designation G165 of ASTM A653. Complete shop drawings of the permanent steel deck forms shall be required in accordance with Sec 1080.

Corrugations of stay-in-place forms shall be filled with an expanded polystyrene material. The polystyrene material shall be placed in the forms with an adhesive in accordance with the manufacturer's recommendations.

Form sheets shall not rest directly on the top of beam flanges. Sheets shall be securely fastened to form supports with a minimum bearing length of one inch on each end. Form with a minimum bearing rength of one fitch on each end. Form supports shall be placed in direct contact with the flange. Welding on or drilling holes in the beam flanges will not be permitted. All steel fabrication and construction shall be in accordance with Sec 1080 and 712. Certified field welders will not be required for welding of the form

The design of stay in-place corrugated steel forms is per manufacturer which shall be in accordance with Sec 703 for false work and forms. Maximum actual weight of corrugated steel forms allowed shall be 4 psf assumed for beam loading.

The contractor shall provide a method of preventing the direct contact of the stay-in-place forms and connection components with uncoated weathering steel members that is approved by the engineer.

#### Pouring and Finishing Slab:

The contractor shall provide bracing necessary for lateral and torsional stability of the beams during construction of the concrete slab and remove the bracing after the slab has attained 75% design strength. Contractor shall not weld on or drill holes in the beams. The cost for furnishing, installing, and removing bracing will be considered completely covered by the contract unit price for Slab on

Slab shall be poured upgrade from end to end at a minimum rate of 25 cubic yards per hour.

Alternate pour sequences may be submitted to the engineer for approval. Keyed construction joints shall be provided

(1) Slab is to be considered a uniform thickness as shown on the plans. Haunching will vary. See front sheet for slab thickness For adjusted girder deflection due to weight of new deck and barriers, see Bridge Electronic

#### Structural Steel Protective Coating:

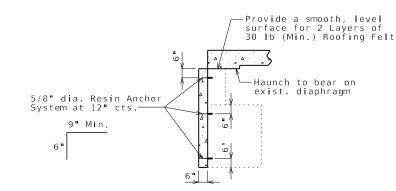
Protective Coating: System G in accordance with Sec 1081. All existing end bent bearings shall be recoated with System G.

Surface Preparation: Surface preparation of the existing steel shall be in accordance with Sec 1081 for Recoating of Structural Steel (System G) with organic zinc primer. The cost of surface preparation will be considered completely covered by the contract unit price for Cleaning and Coating Existing Bearings.

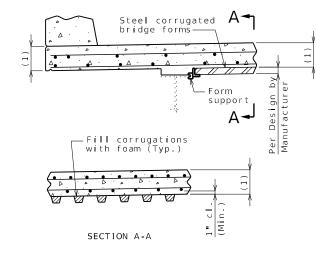
Prime Coat: The cost of the prime coat will be considered completely covered by the contract unit price for Cleaning and Coating Existing Bearings. Tint of the prime coat for System G shall be similar to the color of the field coat to be used.

Field Coat: The color of the finish field coat shall be Gray (Federal Standard #26373). The cost of the intermediate field coat will be considered completely covered by the contract unit price for Cleaning and Coating Existing Bearings. The cost of the finish field coat will be considered completely covered by the contract unit price for Cleaning and Coating Existing Bearings.

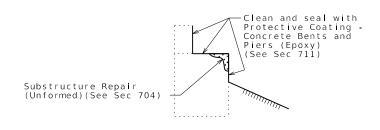
Sec 1081.10.4.6 shall be ignored for the cleaning and recoating work to the bearings



#### PART SECTION THRU SLAB AT END BENTS



OPTIONAL STAY-IN-PLACE FORM DETAILS



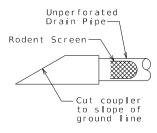
TYPICAL SECTION THRU END BENTS NO. 1 & 4 SHOWING PROTECTIVE COATING



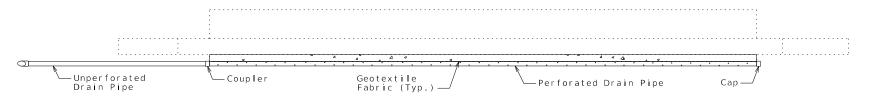
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DESIGNED BY: KLW JAN 2024 DETAILED BY: JTC JAN 2024 CHECKED BY: NSC FEB 2024

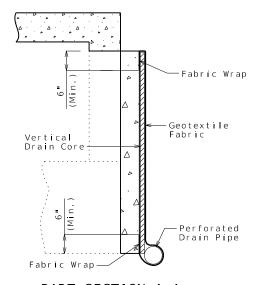
ELEVATION OF END BENT



DETAIL A



PLAN OF END BENT BELOW APPROACH SEAT



PART SECTION A-A (Section thru wing similar)

#### General Notes:

All drain pipe shall be sloped 1 to 2 percent.

Drain pipe may be either 6-inch diameter corrugated metallic-coated steel pipe underdrain, 4-inch diameter corrugated polyvinyl chloride (PVC) drain pipe, or 4-inch diameter corrugated polyethylene (PE) drain pipe.

Drain pipe shall be placed at fill face of end bent. The pipe shall slope to lowest grade of ground line.

Perforated pipe shall be placed at fill face side and inside face of wings at the bottom of end bent and plain pipe shall be used where the vertical drain ends to the exit at ground line.

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NUMBER E-25019

7/3/2024

**ATCHISON** 

JNW0111

PROJECT NO.

R02741

MO

SHEET NO 3

F

BR

VERTICAL DRAIN AT END BENTS

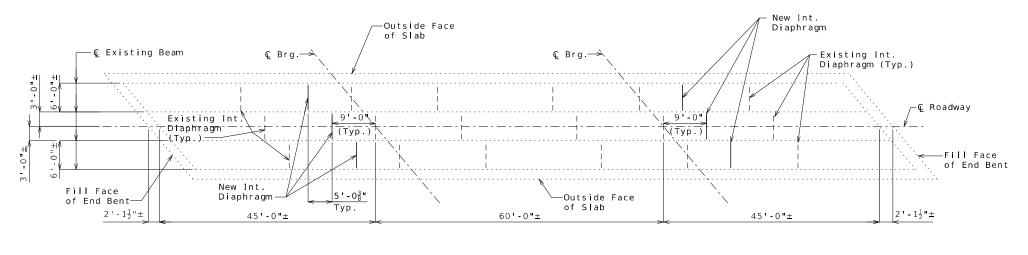
Sheet No. 3 of 9

(Squared end bent shown, skewed end bent similar)

Note: This drawing is not to scale. Follow dimensions.

DESIGNED BY: KLW FEB 2024 DETAILED BY: JTC FEB 2024 CHECKED BY: BPW MAR 2024

B\_R02741\_003\_JNW0111\_VERTICAL DRAINS AT END BENT.dgn 1:51:19 PM 7/3/2024

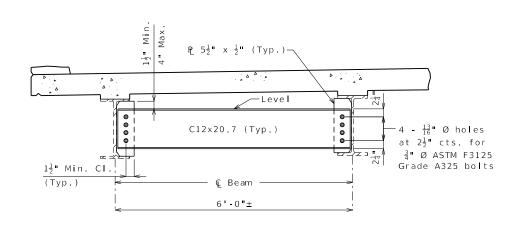


SPAN (1-2)

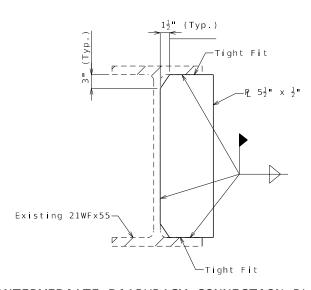
SPAN (2-3)

SPAN (3-4)

PLAN OF STRUCTURAL STEEL



TYPICAL PART SECTION SHOWING INTERMEDIATE DIAPHRAGMS (6 Locations)



INTERMEDIATE DIAPHRAGM CONNECTION PLATE

#### STEEL DIAPHRAGM NOTES:

Fabricated structural steel shall be ASTM A709 Grade 36 except as noted.

Payment for furnishing and installing steel intermediate diaphragms will be considered completely covered by the contract unit price for Fabricated Structural Carbon Steel (Misc.).

Structural Steel Protective Coating: Protective Coating: System G in accordance with Sec 1081.

 $\hbox{Prime Coat (New Steel): The cost of the inorganic zinc prime coat will be considered completely covered by the contact unit price for the fabricated structural steel. } \\$ 

Field Coats: The color of the field coats shall be Gray (Federal Standard #26373). The cost of the intermediate system field coat will be considered completely covered by the contract unit price for the fabricated structural steel.

At the option of the contactor, the intermediate field coat may be applied in the shop. The contractor shall exercise extreme care during all phases of loading, hauling, erection, and pouring of the slab to minimize damage and shall be fully responsible for all repairs and cleaning of the coating system as required by the engineer.

E DESIGNED BY: NSC FEB 2024
DETAILED BY: RCL FEB 2024
CHECKED BY: DMA MAR 2024

STEEL DIAPHRAGMS

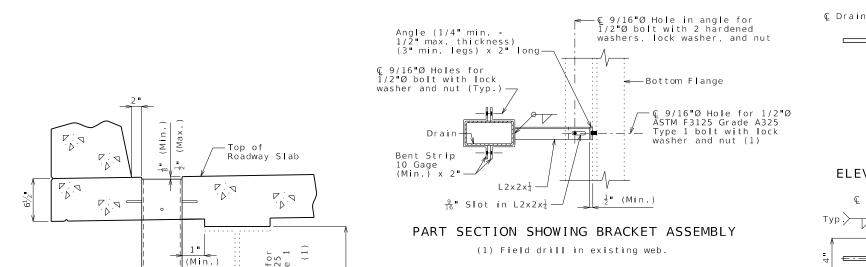
Note: This drawing is not to scale. Follow dimensions.

Sheet No. 4 of 9

NUMBER E-25019 7/3/2024 F MO SHEET NO BR 4 **ATCHISON** JNW0111 PROJECT NO. R02741

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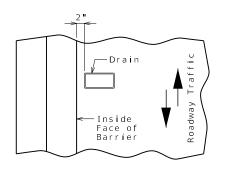
#### PLAN OF SLAB SHOWING SLAB DRAIN LOCATIONS



9/16"Ø Hol /2"Ø ASTM F rade A325 T olt with lo

1" (Min.)

PART SECTION NEAR DRAIN



PART PLAN OF SLAB AT DRAIN

Note: This drawing is not to scale. Follow dimensions.

SLAB DRAINS

Sheet No. 5 of 9

PLAN OF FRP DRAIN OPTION

8" (Nom.)

8 •

ELEVATION OF DRAIN

PLAN OF STEEL DRAIN OPTION

© Drain—

© Drain→

===

#### General Notes:

Contractor shall have the option to construct either steel or FRP slab drains. All drains shall be of same type.

Slab drain bracket assembly shall be ASTM A709 Grade 36 steel.

Locate drains in slab by dimensions shown in Part Section Near Drain.

Reinforcing steel shall be shifted to clear drains.

The bracket assembly shall be galvanized in accordance with ASTM A123.

All bolts, hardened washers, lock washers and nuts shall be galvanized in accordance with AASHTO M 232 (ASTM A153), Class C.

All 1/2"Ø bolts shall be ASTM A307, except

Shop drawings will not be required for the slab drains and the bracket assembly.

The bolt hole for the bracket assembly attachment shall be shifted to the minimum extent necessary to field drill in the existing web.

#### Notes for Steel Drain:

Slab drains may be fabricated of either 1/4 welded sheets of ASTM A709 Grade 36 steel or from 1/4 structural steel tubing ASTM A500 or A501.

Outside dimensions of drains are 8" x 4".

The drains shall be galvanized in accordance with ASTM A123.

#### Notes for FRP Drain:

Drains shall be machine filament-wound thermosetting resin tubing meeting the requirements of ASTM D2996 with the following exceptions:

Shape of drains shall be rectangular with outside nominal dimensions of 8  $^{\circ}$  x 4  $^{\circ}$  .

Minimum reinforced wall thickness shall be 1/4 inch.

The resin used shall be ultraviolet (UV) resistant and/or have UV inhibitors mixed throughout. Drains may have an exterior coating for additional UV resistance.

The color of the slab drain shall be gray (Federal Standard 26373). The color shall be uniform throughout the resin and any coating used.

The combination of materials used in the manufacture of the drains shall be tested for UV resistance in accordance with ASTM D4329 Cycle A. The representative material shall withstand at least 500 hours of testing with only minor discoloration and without any physical deterioration. The contractor shall furnish the results of the required ultraviolet testing prior to account and the stable drains. acceptance of the slab drains.

At the contractor's option, drains may be field cut. The method of cutting FRP slab drain shall be recommended by the manufacturer to ensure a smooth, chip free





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-Bottom of

Roadway Slab

·1/2"Ø x 3" Rod (ASTM A709 Grade 36)

−¢ Drain

Connector (Typ.)

1/2**"**Ø x 3**"** Galv.

Carriage Bolt with Hex Nut and

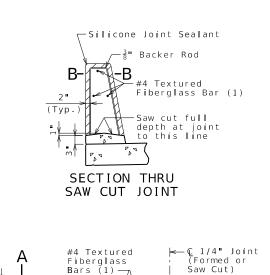
Lock Washer (Typ.)

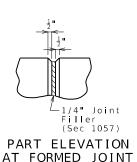
C Drain

1/2"Ø x 3"± Shear

#### ELEVATION OF BARRIER

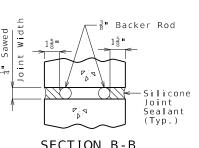
(Left barrier shown, right barrier similar) Longitudinal dimensions are horizontal.

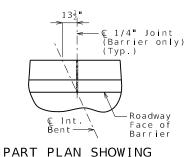


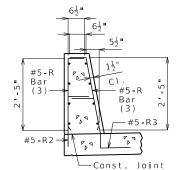


-#5-C1 (Typ.) \*

#5-R1, R2 & R3







JOINT LOCATION



#### SECTION A-A

 $6\frac{1}{8}$ 

Use a minimum lap of 3'-1" for #5 horizontal barrier bars.

The cross-sectional area above the slab is 2.89 square feet.

└─ #5 **-** R3

(2) To top of bar

#### R-BAR PERMISSIBLE ALTERNATE SHAPE

(3) The R1 bar may be separated into two bars as shown, at the contractor's option, only when slip forming is not used. (All dimensions are out to out.)

#### General Notes:

\* Slip-formed option only.

Conventional forming or slip forming may be used. Saw cut joints may be used with conventional forming.

Top of barrier shall be built parallel to grade and barrier joints normal to grade.

All exposed edges of barrier shall have either a 1/2-inch radius or a 3/8-inch bevel, unless otherwise noted.

Payment for all concrete and reinforcement, complete in place, will be considered completely covered by the contract unit price for Type H Barrier per linear foot

Concrete in barrier shall be Class B-1.

Measurement of barrier is to the nearest linear foot for each structure, measured along the outside top of slab from end of slab to end of slab.

Concrete traffic barrier delineators shall be placed on top of the barrier as shown on Missouri Standard Plan 617.10 and in accordance with Sec 617. Delineators on bridges with two-lane, two-way traffic shall have retroreflective sheeting on both sides. Concrete traffic barrier delineators will be considered completely covered by the contract unit price for Type H Barrier.

Joint sealant and backer rods shall be in accordance with Sec 717 for silicone joint sealant for saw cut and formed joints.

For slip-formed option, both sides of barrier shall have a vertically broomed finish and the top shall have a transversely broomed finish.

NUMBER E-25019 7/3/2024 F MO SHEET NO BR 6 **ATCHISON** JNW0111 CONTRACT ID. PROJECT NO. R02741

KEITH LAWRENCE WETTER

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# @ abt. 12" cts. PART ELEVATION OF BARRIER

0.4

#5-R1, R2 & R3

@ abt. 12" cts.

DESIGNED BY: NSC FEB 2024

DETAILED BY: RCL FEB 2024 CHECKED BY: KLW MAR 2024

(1) Four feet long, centered on joint, slip-formed option only

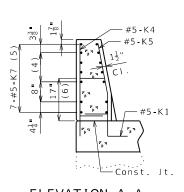
#### TYPE H BARRIER

Sheet No. 6 of 9

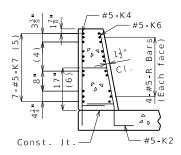
#5-R1-

#5-C1

Const. Joint —



**ELEVATION A-A** 



SECTION B-B

21"

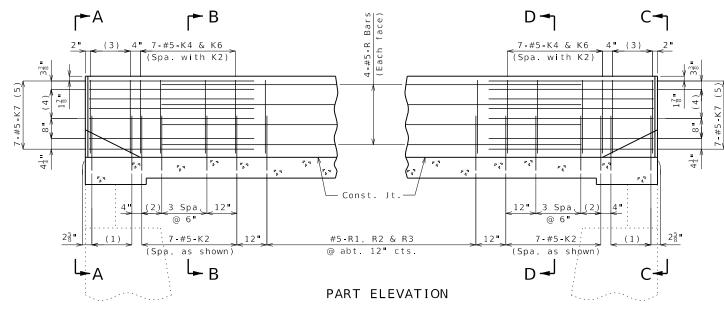
22"

— Const. Joint

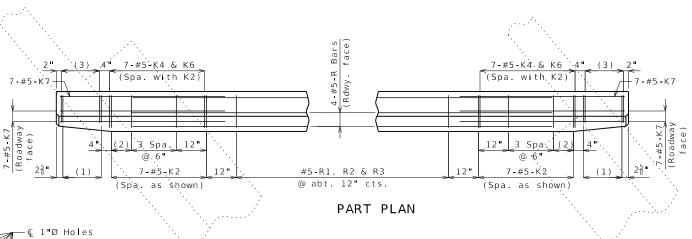
-Ç 1"Ø Holes

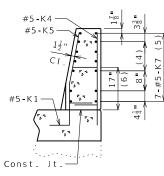
Roadway Face of Barrier

ELEVATION

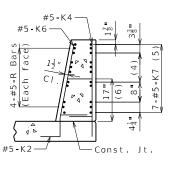


- (1) 5-#5-K1 @ 4" cts.
- (2) 2 Spaces @ 4"
- (3) 5-#5-K4 and 5-#5-K5, spaced with K1
- (4) 3 Spaces @ 3<sup>13</sup><sub>16</sub>"
- (5) Spaced as shown, each face
- (6) To top of bar





**ELEVATION C-C** 



SECTION D-D



NUMBER E-25019

7/3/2024

**ATCHISON** 

JNW0111

PROJECT NO.

R02741

MO 7

F

BR

# The top two kept with position close $10\frac{1}{4}$ " to those shown in Sections A-A thru D-D 10" \_10" K4-K5

# General Notes:

Concrete traffic barrier delineators shall be placed on top of the barrier as shown on Missouri Standard Plan 617.10 and in accordance with Sec 617. Delineators on bridges with two-lane, two-way traffic shall have retroreflective sheeting on both sides. Concrete traffic barrier delineators will be considered completely covered by the contract unit price for Type H Barrier.

#### Reinforcing Steel:

Minimum clearance to reinforcing steel shall be 1 1/2".

Use a minimum lap of 3'-1" between K7 bars and R bars.

#### TYPE H BARRIER AT END BENTS

(Left barrier shown, right barrier similar)

### PERMISSIBLE ALTERNATE SHAPES

(Other K bars not shown for clarity)

The K4-K5 and K4-K6 bar combination may be furnished as one bar as shown, at the contractor's option.

All dimensions are out to out.

# K4-K6

hnei C 0

DESIGNED BY: NSC FEB 2024 DETAILED BY: JTC FEB 2024 CHECKED BY: KLW MAR 2024

ELEVATION E-E

\* Transition to zero at Type A curb for gutter

lines to match

1" Chamfer \*

DETAILS OF GUARD RAIL ATTACHMENT

