DESIGN DESIGNATION

US - 169

A.A.D.T. 2022 = 871 A.A.D.T. - 2042 = 1,176

D.H.V. = 10%

T = 17%

V = 60 M.P.H.

D = 50%/50%

FUNCTIONAL CLASSIFICATION- MINOR ARTERIAL

RTE M

A.A.D.T. - 2022 = 272 A.A.D.T. - 2042 = 354

V = 55 M.P.H.

D = 50%/50%

FUNCTIONAL CLASSIFICATION- MAJOR COLLECTOR

RTE B

A.A.D.T. - 2022 = 446

A.A.D.T. - 2042 = 580

V = 55 M.P.H

D = 50%/50%

FUNCTIONAL CLASSIFICATION- MAJOR COLLECTOR

NO RIGHT OF WAY ACQUISITION

CONVENTIONAL SYMBOLS

BUILDINGS AND STRUCTURES ニュニコ GUARD RAIL • • • • GUARD CARLE CONCRETE RIGHT-OF-WAY MARKER STEEL RIGHT-OF-WAY MARKER LOCATION SURVEY MARKER \circ UTILITIES FIBER OPTICS -FO--F0- OVERHEAD CABLE TV -OTV--OTV- UNDERGROUND CABLE TV OVERHEAD TELEPHONE -UTV-- OT --UTV-- UT -- OE -- UE -- SS -UT OE UNDERGROUND TELEPHONE OVERHEAD POWER UNDERGROUND POWFR —UE— —S— SANITARY SEWER -ss-WATER SAN MANHOLE TYD Q FIRE HYDRANT *` ⊕ • WATER VALVE WATER METER DROP INLET DITCH BLOCK =

Δ

 \boxtimes

NOTE: DASHED OR OPEN SYMBOLS INDICATE EXISTING FEATURES

GROUND MOUNTED SIGN

TELEPHONE PEDESTAL

LIGHT POLE H-FRAME POWER POLE

CHAIN LINK WOVEN WIRE

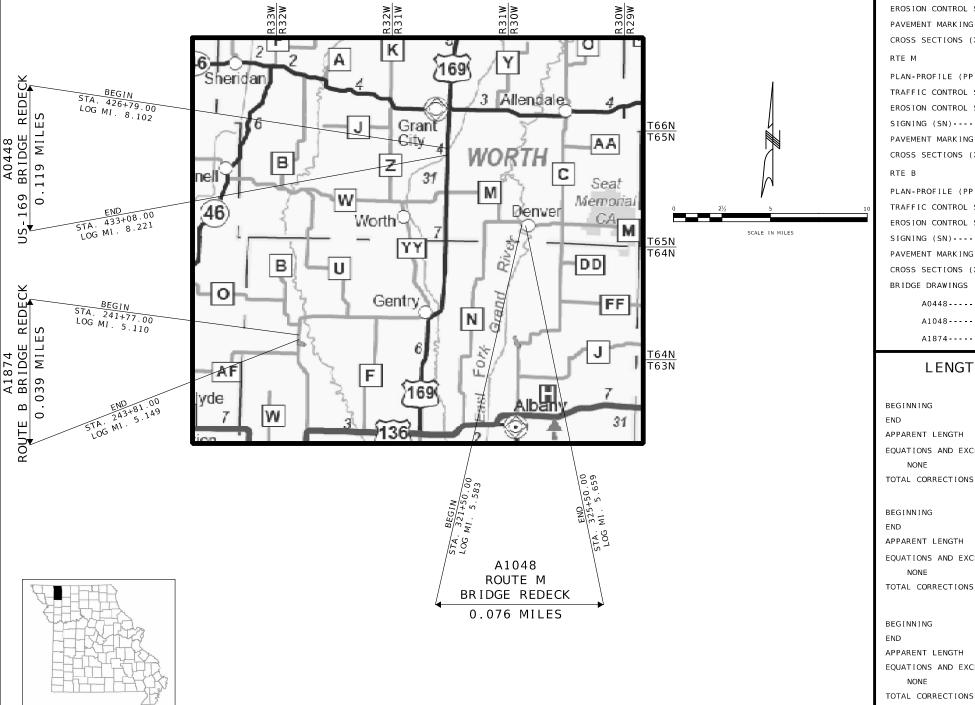
GATE POST

BENCHMARK

FENCE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

PLANS FOR PROPOSED STATE HIGHWAY WORTH & GENTRY COUNTY



THE EXISTENCE AND APPROXIMATE LOCATION OF UTILITY FACILITIES KNOWN TO EXIST, AS SHOWN ON THE PLANS, ARE BASED ON THE BEST INFORMATION AVAILABLE TO THE COMMISSION AT THIS TIME. THIS INFORMATION IS PROVIDED BY THE COMMISSION "AS-IS" AND THE COMMISSION EXPRESSLY DISCLAIMS ANY REPRESENTATION OR WARRANTY AS TO THE COMPLETENESS, ACCURACY, OR SUITABILITY OF THE INFORMATION FOR ANY USE. RELIANCE UPON THIS INFORMATION IS DONE AT THE RISK AND PERIL OF THE USER, AND THE COMMISSION SHALL NOT BE LIABLE FOR ANY DAMAGES THAT MAY ARISE FROM ANY ERROR IN THE INFORMATION. IT IS, THEREFORE,

THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY THE EXISTENCE, LOCATION AND STATUS OF ANY FACILITY. SUCH VERIFICATION INCLUDES DIRECT CONTACT WITH THE LISTED UTILITIES.

INDEX OF SHEETS

DESCRIPTION	SHEET NUMBER
TITLE SHEET	1
TYPICAL SECTIONS (TS) (2 SHEETS)	2
QUANTITIES (QU) (3 SHEETS)	3
RTE 169	
PLAN-PROFILE (PP)	4
TRAFFIC CONTROL SHEETS (TC)	5 - 6
EROSION CONTROL SHEETS (EC)	7
PAVEMENT MARKING (PM)	8
CROSS SECTIONS (XS)	1 - 8
RTE M	
PLAN-PROFILE (PP)	9
TRAFFIC CONTROL SHEETS (TC)	10-11
EROSION CONTROL SHEETS (EC)	12
SIGNING (SN)	13-14
PAVEMENT MARKING (PM)	15
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RTE B	
PLAN-PROFILE (PP)	16
TRAFFIC CONTROL SHEETS (TC)	17-18
EROSION CONTROL SHEETS (EC)	19
SIGNING (SN)	20-21
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CROSS SECTIONS (XS)	1 - 6
BRIDGE DRAWINGS (B)	
A0448	1 - 14
A1048	1-12
A1874	1-12

LENGTH OF PROJECT

US - 1	69		
BEGINNING	STA.	426+79.00	
END	STA.	433+08.00	
APPARENT LENGTH		629	FEET
EQUATIONS AND EXCEPTIONS:			
NONE			
TOTAL CORRECTIONS		0.00	FEE
RTE	М		
BEGINNING	STA.	321+50.00	
END	STA.	325+50.00	
APPARENT LENGTH		400	FEET
EQUATIONS AND EXCEPTIONS:			
NONE			
TOTAL CORRECTIONS		0.00	FEE
RTE	В		
BEGINNING	STA.	241+77.00	
END	STA.	243+81.00	
APPARENT LENGTH		204	FEET
EQUATIONS AND EXCEPTIONS:			
NONE			

NET LENGTH OF PROJECT

FOR INFORMATION ONLY ESTIMATED DISTURBED ACRES

STATE LENGTH



NW GENTRY & WORT JNW0009 PROJECT NO



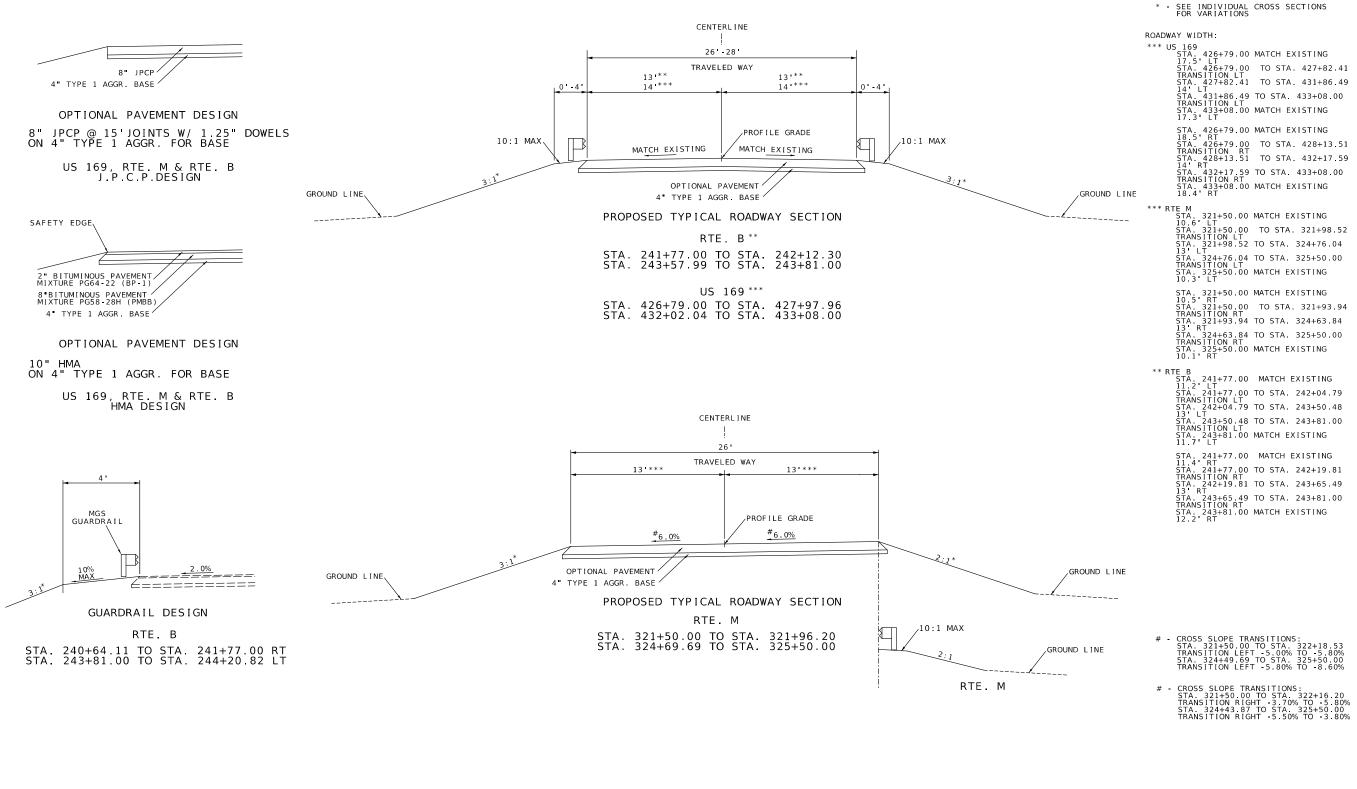
C S 0 Ф 2

0.00 FEET

1,233 FEET

0.234 MILES

0.98 ACRES



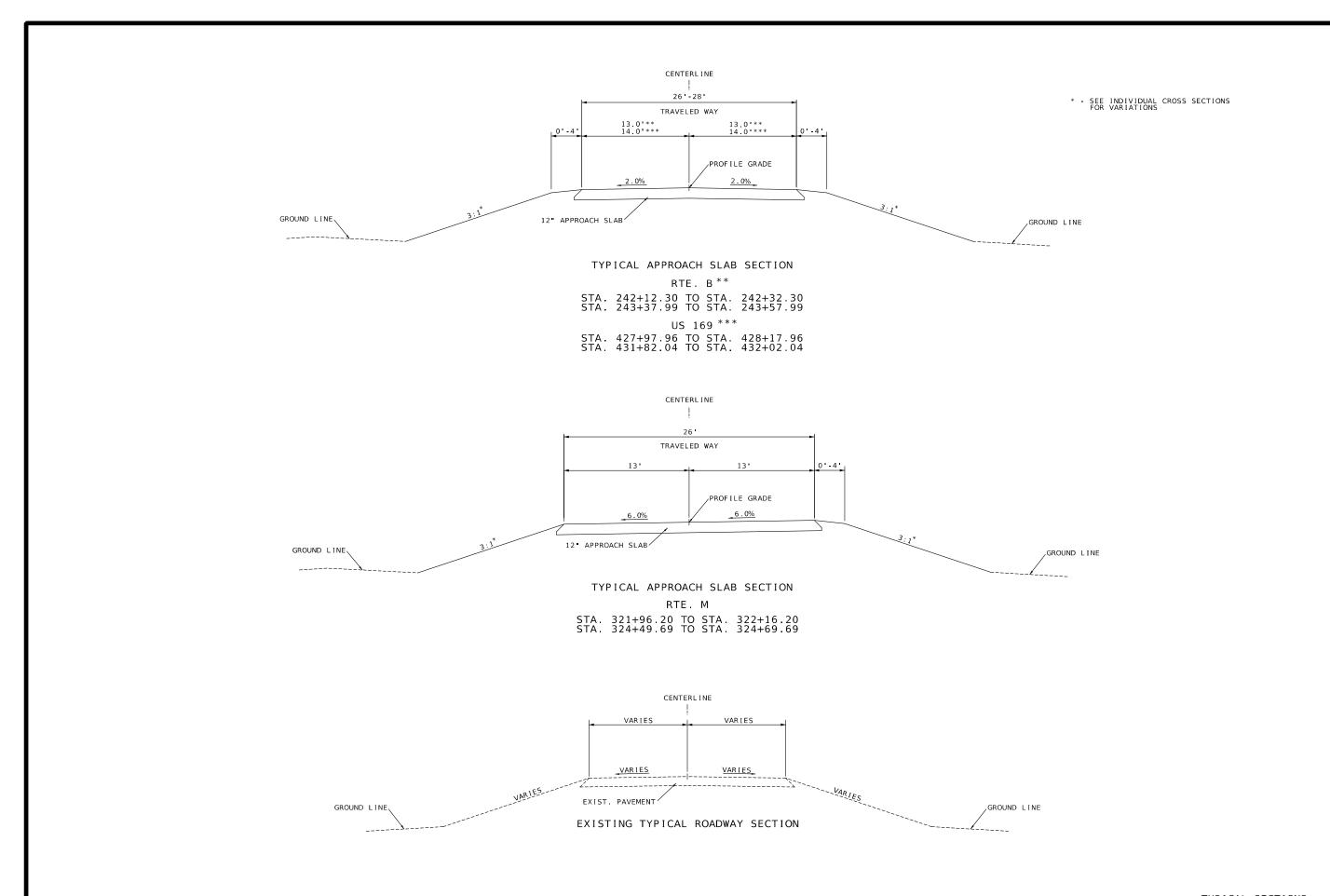
MICHELE R. KEAL NUMBER CE-20C5000711 8/7/2024 US 169/ MO M/B DISTRICT SHEET NO 2 NW GENTRY & WORT JNW0009 PROJECT NO.

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TYPICAL SECTIONS

SHEET 1 OF 2



MICHELE R.
KEAL
NUMBER
PE-2005000711 8/7/2024 US 169 / MO
M/B
DISTRICT SHEET NO SHEET NO NW GENTRY & WORT JNW0009 PROJECT NO.

SECTION 1999435 MAIN STRET, SUITE 1150
ANSAS CITY, MO 6411-1468

STATION	STATION	SIDE	MPROVEMENTS DESCRIPTION	QUANTITY	UNITS			
RTE 169	J STATION	SIDE	DESCRIPTION	QUANTITI	UNITS			
426+49.99	175.5	LF						
426+84.23								
426+79.00	431+97.96	CL	PAVEMENT	175.9 494.0	LF SY			
427+99.05	401.07.00	LT	SIGN	1	EA			
428+35,58		RT	SIGN	1	EA			
431+40.83	433+16,74	LT	GUARDRAIL	175.9	LF			
431+74.20	433+50.04	RT	GUARDRAIL	175.8	LF			
431+68.49	100.00101	T LT	SIGN	1	EA			
431+68.24		 [SIGN	1	EA			
432+02.12		RT	SIGN	1	EA			
432+02.04	433+08.00	CL	PAVEMENT	428.6	SY			
402.02.04	1 400.00.00	1 02 1	SUBTOTAL	1	LS			
RTE M		-	008101712	'				
321+50.00	321+96.20	T CL T	PAVEMENT	156.7	SY			
321+96.82	021100.20	T LT	SIGN	1	EA			
322+10.58								
322+13.25								
322+17.65		LT	SIGN	1	EA EA			
324+44.22		RT	SIGN	1	EA			
324+45.26	324+93,51	RT	GUARDRAIL	48.3	LF			
324+59.45	021100.01	 	SIGN	1	EA			
324+58.68		† <u>LT</u> †	SIGN	1	EA			
324+64.25		RT	SIGN	1	EA			
324+69.69	325+50.00	CL	PAVEMENT	232.3	SY			
324+73,27	020.00.00	LT	SIGN	1	EA			
324+83.62		RT	SIGN	1	EA			
324+88.49		T LT	SIGN	1	EA			
021100.10		1 - 1	SUBTOTAL	1	LS			
RTE B			332.37.12	' '				
241+77.00	242+12.30	CL	PAVEMENT	140.7	SY			
242+23.31		LT	SIGN	1	EA			
242+40.35		RT	SIGN	1	EA			
243+31.02		 LT 	SIGN	1	EA			
243+47.45		RT	SIGN	1	EA			
243+57.99	243+81.00	CL	PAVEMENT	118.4	SY			
		1 1	SUBTOTAL	1	LS			
			PAY TOTAL	1	LS			

CLEARING AND GRUBBING										
BEGIN END										
STATION	STATION	(AC)								
RTE 169										
426+74.10	433+38.96	0.3								
	SUBTOTAL	0.3								
RTE M										
321+50.0	325+72.96	0.1								
	SUBTOTAL	0.1								
RTE B										
240+34.11	244+24.39	0.1								
	SUBTOTAL	0.1								
	PAY TOTAL	1								

MOBILIZATION	
1 LUMP SUM	

CONTRACTOR FURNISHED SURVEYING & STAKING 1 LUMP SUM

EARTHWORK										
BEGIN END CLASS A COMPACTING EMBANKMENT										
STATION	STATION	EXCAVATION	EMBANKMENT	IN PLACE						
		(CY)	(CY)	(CY)						
RTE 169										
426+74.10	433+38.96	85	68	216						
	SUBTOTALS	85	216							
RTE M										
321+50.00	325+72.77	41	33	73						
	SUBTOTALS	41	73							
RTE B										
240+34.11	244+00.00	19	15	5						
	SUBTOTALS	19	15	5						
	PAY TOTALS	145	116	295						

OPTIONAL PAVEMENT											
TYPE 1											
BEGIN	END	OPTIONAL	AGGREGATE								
STATION	STATION	PAVEMENT	BASE (4")								
		(SY)	(SY)								
RTE 169											
426+79.00	427+97.96	423.8	423.8								
432+02.04	433+08.00	373.9	373.9								
	SUBTOTALS	797.7	797.7								
RTE M											
321+50.00	321+98.99	120,8	120.8								
324+63.54	325+50.00	207.0	207.0								
	SUBTOTALS	327.8	327.8								
RTE B			•								
241+77.00	242+19.81	95.4	95.4								
243+50.77	243+81.00	63.7	63.7								
	SUBTOTALS	159.1	159.1								
	PAY TOTALS	1285	1285								



US 169 / MO M/B
DISTRICT SHEET NO.
NW 3
COUNTY
GENTRY & WORTH

JOB NO.

JNW0009

CONTRACT ID.

PROJECT NO.

DESCRIPTION A STORY OF THE STOR

JRI HIGHWAYS AND TRANSPORTATION
COMMISSION
105 WEST CAPITOL
HEFFERSON CITY, MO 65102

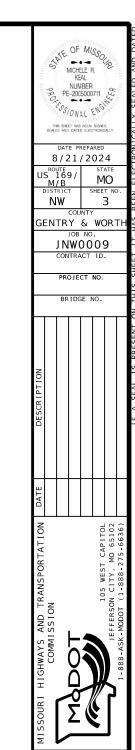
benesch35 MAIN STRET. SUITE 1150
ANSAS CITY. MO 64111-1468

	GUARDRAIL											
BEGIN END STATION STATION SIDE		MGS GUARDRAIL (LF)	RDRAIL GUARDRAIL (MASH)		END ANCHOR (EA)	MGS BRIDGE APPROACH TRANSITION (EA)	BRIDGE ANCHOR SECTION (SAFETY BARRIER CURB) (EA)					
RTE 169			7	. ,		/	. ,	1 /				
427+07.51	428+33.51	RT	37.5		1		1					
427+13.86	428+02.41	LT			1		1					
431+66.49	433+05.05	LT	37.5		1		1					
431+97.59	432+86.14	RT			1		1					
	SI	JBTOTALS	75.0		4		4					
ROUTE M												
324+43.87	325+43.01	RT	12.5		1		1					
	Sl	JBTOTALS	12.5		1		1					
ROUTE B												
240+64.11 241+39.81 RT		87.5		1		1						
243+30.48	244+20.82	LT		62.5		1		1				
	Sl	JBTOTALS	87.5	62.5	1	1	1	1				
	PA	Y TOTALS	175	63	6	1	6	1				

		PERM	MANENT PAVE	MENT MARKING)
			MARKIN	NE PAVEMENT NG PAINT P BEADS	
BEGIN	END		4"	4"	
STATION	STATION	SIDE	WHITE (LF)	YELLOW (LF)	REMARKS
RTE 169		'	(=: /	(=:)	
426+79.00	433+08.00	RT	629.0		EDGE LINE
426+79.00	433+08.00	LT	629.0		EDGE LINE
426+79.00	433+08.00	CL		157.3	INTERMITTENT CENTERLINE
	SUB	TOTALS	1,258	157	
RTE M					
321+50.00	325+50.00	CL		800.0	DOUBLE YELLOW CENTERLINE
	SUB	TOTALS	0	800	
RTE B					
241+77.00	243+81.00	RT	204.0		EDGE LINE
241+77.00	243+81.00	LT	204.0		EDGE LINE
241+77.00	243+81.00	CL		51.0	INTERMITTENT CENTERLINE
	SUB	TOTALS	408	51	
	PAY	TOTALS	1,666	1,008	

SEEDING AND MULCHING											
BEGIN END COOL SEASON											
STATION	STATION	MIXTURES	MULCHING								
		(AC)	(AC)								
RTE 169											
426+74.10	426+74.10 433+38.96 0.3 0.3										
	SUBTOTALS	0.3	0.3								
RTE M											
321+50.0	325+72.96	0.10	0.10								
	SUBTOTALS	0.10	0.10								
RTE B											
240+34.11	244+24.39	0.10	0.10								
	SUBTOTALS	0.10	0.10								
	PAY TOTALS	0.5	0.5								
	FAT TOTALS	0.0	J 0.5								

		TEMPORARY	'EROSION C	ONTROL		
				TYPE 3B		
BEGIN	END		SILT	EROSION	TYPE C	SEDIMENT
STATION			FENCE	CONTROL	BERM	REMOVAL
				BLANKET		
			(LF)	(SY)	(LF)	(CY)
RTE 169						
426+68,85	428+42.71	RT	173.86			2
426+71.15	427+97.45	LT	126.3			2
427+97.45	428+79.27	CL			226.2	2
431+07.15	432+04.30	CL			267.1	3
431+65.31	433+47.94	LT	182.63			2
432+04.30	433+20.55	RT	116.25			2
		SUBTOTALS	599.0		493.3	13
RTE M						
321+49.06	322+21.46	LT	72.4			1
321+49.59	322+13.87	RT	64.28			1
321+90.00	322+13.94	RT		9.5		
321+96.91	322+18.26	LT		13.9		
322+13.87	322+19.00	CL			106.1	1
323+98.79	324+54.01	CL			194.2	2
324+43.87	325+72.77	RT		392.6		
324+57.45	324+64.83	LT		10.1		
324+37.93	325+76.20	RT	138.27			2
324+54.01	325+50.00	LT	95.99			1
325+90.00	325+13.00	LT	11.5			1
		SUBTOTALS	382.44	426.1	300.3	9
RTE B						
240+33.50	242+39.81	RT	206.31			2
240+46.00	241+05.00	RT		76.5		
241+30.00	241+60.00	RT		49.9		
241+76.12	242+23.87	LT	47.75			1
242+13.84	242+39.81	RT		29.1		
242+23.87	242+58.54	CL			115.8	1
243+16.00	243+49.07	CL			114.5	1
243+29.20	244+21.17	LT	91.97			1
243+38.00	243+45.00	LT		11.7		
243+49.07	243+81.44	RT	32.37			1
244+08.06	244+24.39	LT		36		
		SUBTOTALS	378.4	203.2	230.3	7
		PAY TOTALS	1360	629	1024	29



														EFFECTIVE: 04-01-20
		TOTAL	1 '	TOTAL SIGN				1 1 1	QTY TO					
		1 ' 1		RELOC NUM.		C I CN	1 1	QTY TOTAL R			M .			
SIGN	IN. SQ.FT	EACH SQ FT		SQ.FT.	DESCRIPTION	SIGN	IN. SQ.FI	GUIDE SIGNS		FI.	DESCRIPTION	I TEM NUMBER	l	DESCRIPTION
WO1 - 1	L 48X48 16.00		I		TURN (SYMBOL LEFT ARROW)	E05-1	36X48 12.00		<u> </u>		GORE EXIT	6122008	QII	IMPACT ATTENUATOR 40 MPH (SAN
WO1-1					TURN (SYMBOL RIGHT ARROW)	E05-2	48X36 12.00				EXIT OPEN	6122009		IMPACT ATTENUATOR 45 MPH (SAN
WO1-2					CURVE (SYMBOL LEFT ARROW)		48X36 12.00				EXIT CLOSED	6122010		IMPACT ATTENUATOR 50 MPH (SAN
WO1 - 21					CURVE (SYMBOL RIGHT ARROW)		60X24 10.00				ROAD WORK NEXT XX MILES	6122012		IMPACT ATTENUATOR 55 MPH (SAN
WO1 - 3					REVERSE TURN (SYMBOL LEFT ARROW) REVERSE TURN (SYMBOL RIGHT ARROW)		48X24 8.00 36X18 4.50				END ROAD WORK PILOT CAR FOLLOW ME	6122014 6122017		IMPACT ATTENUATOR 60 MPH (SANI IMPACT ATTENUATOR 65 MPH (SANI
WO1 - 4					REVERSE CURVE (SYMBOL LEFT ARROW)		42X30 8 75				PILOT CAR IN USE WAIT & FOLLOW	6122019		IMPACT ATTENUATOR 70 MPH (SAN
WO1-4	R 48X48 16.00				REVERSE CURVE (SYMBOL RIGHT ARROW)	GO20-4a	18X12 1.50				PILOT CAR IN USE WAIT & FOLLOW	6122020		REPLACEMENT SAND BARREL
WO1-4					DOUBLE ARROW REVERSE CURVE (SYMBOL LT ARROWS)		36X24 6.00				WORK ZONE (PLAQUE)	6122030		IMPACT ATTENUATOR (RELOCATION
	bR 48X48 16.00				DOUBLE ARROW REVERSE CURVE (SYMBOL RT ARROWS)	MO4-8a	24X18 3.00			51		6123000A		TRUCK OR TRAILER MOUNTED ATTE
WO1 - 4	cL 48X48 16.00 cR 48X48 16.00				TRIPLE ARROW REVERSE CURVE (SYMBOL LT ARROWS) TRIPLE ARROW REVERSE CURVE (SYMBOL RT ARROWS)	MO4 - 9L MO4 - 9R	48X36 12.00				DETOUR (LEFT ARROW) DETOUR (RIGHT ARROW)	6161008 6161012	6	ADVANCED WARNING RAIL SYSTEM BUOYS (BOATS KEEP OUT)
WO1 - 6					HORIZONTAL ARROW (SYMBOL)	MO4 - 9P	48X12 4 00				STREET NAME (PLAQUE)	6161013		BUOYS (NO WAKE)
WO1-6	a 72X36 18.00				HORIZ. ARROW (SYMBOL ON PERMANENT BARRICADE)	MO4-10L	48X18 6.00				DETOUR (ARROW LEFT)	6161014		SPECIAL SIGN ASSEMBLY (BOATS
WO1-7					DOUBLE HEAD HORIZONTAL ARROW (SYMBOL)	MO4-10R	48X18 6.00				DETOUR (ARROW RIGHT)	6161025		CHANNELIZER (TRIM LINE)
WO1 - 7					DOUBLE HEAD HORIZ. ARROW (SYMBOL ON PERM. BARR.)	D1 1	140740 12 25	REGULATORY	SIGNS		CTOR	6161030	32	TYPE III MOVEABLE BARRICADE
WO1 - 8					CHEVRON (SYMBOL) CHEVRON (SYMBOL FOR DIVIDED HIGHWAYS)	R1-1 R1-2	48X48 13 25 48TRI 6 93				STOP YIELD	6161033 6161040		DIRECTION INDICATOR BARRICADE FLASHING ARROW PANEL
WO3 - 1					STOP AHEAD (SYMBOL)	R1-2a	36X36 9.00				TO ONCOMING TRAFFIC (PLAQUE)	6161047		TYPE III OBJECT MARKER
WO3 - 2					YIELD AHEAD (SYMBOL)	R1-3P	30X12 2.50				ALL WAY (PLAQUE)	6161055		SEQUENTIAL FLASHING WARNING L
WO3 - 3					SIGNAL AHEAD (SYMBOL)	R2-1	36X48 12.00				SPEED LIMIT XX	6161070		TUBULAR MARKER
WO3 - 4					BE PREPARED TO STOP	R3-1	48X48 16.00				NO RIGHT TURN (SYMBOL)	6161095		RADAR SPEED ADVISORY SYSTEM
WO3 - 5 WO4 - 1					SPEED LIMIT AHEAD MERGE (SYMBOL FROM LEFT)	R3-2 R3-3	48X48 16.00 36X36 9.00				NO LEFT TURN (SYMBOL) NO TURNS	6161096		CHANGEABLE MESSAGE SIGN, COMMISSION FURNISHED/RETAINED
WO4-1					MERGE (SYMBOL FROM RIGHT)	R3 - 4	48X48 16 00				NO U-TURN (SYMBOL)	0101090		CHANGEABLE MESSAGE SIGN W/O CO
WO4 - 1					MERGE (ARROW SYMBOL)	R3 - 7L	30X30 6.25				LEFT LANE MUST TURN LEFT	6161098A	1**	INTERFACE - CONTRACTOR FURNIS
WO4 - 1	aR 48X48 16.00				MERGE (ARROW SYMBOL)	R3-7R	30X30 6.25				RIGHT LANE MUST TURN RIGHT			CHANGEABLE MESSAGE SIGN WITH
WO5 - 1					ROAD/BRIDGE/RAMP NARROWS	R4-1	36X48 12.00				DO NOT PASS	6161099		INTERFACE - CONTRACTOR FURNIS
WO5 - 3					ONE LANE BRIDGE	R4-2 R4-8a	36X48 12.00				PASS WITH CARE	6162000A		WORK ZONE TRAFFIC SIGNAL SYST
WO5 - 5 WO6 - 1					NARROW LANES DIVIDED HIGHWAY (SYMBOL)	R4-8a R4-7a	36X48 12.00 36X48 12.00				KEEP LEFT (HORIZONTAL ARROW) KEEP RIGHT (HORIZONTAL ARROW)	6162002		TEMPORARY LONG-TERM RUMBLE STI TEMPORARY TRAFFIC BARRIER
WO6 - 2					DIVIDED HIGHWAY END (SYMBOL)	R5 - 1	30X30 6.25				DO NOT ENTER	61736000		CONTRACTOR FURNISHED/RETAINED
WO6-3	48X48 16.00				TWO WAY TRAFFIC (SYMBOL)	R5-1a	36X24 6.00				WRONG WAY			TEMPORARY TRAFFIC BARRIER
WO7 - 3					NEXT XX MILES (PLAQUE)	R6-1L	54X18 6.75				ONE WAY ARROW (LEFT)	6173602E		CONTRACTOR FURNISHED/COMMISSI
WO8 - 1					BUMP	R6-1R	54X18 6.75				ONE WAY ARROW (RIGHT)	6174000A		TEMP. TRAFFIC BARRIER HEIGHT
WO8 - 2 WO8 - 3					DIP PAVEMENT ENDS	R6-2L R6-2R	24X30 5.00 24X30 5.00				ONE WAY (LEFT) ONE WAY (RIGHT)	6175010A		RELOCATING TEMPORARY TRAFFIC TEMPORARY TRAFFIC BARRIER
WO8 - 4					SOFT SHOULDER	R9 - 9	24X12 2 00				SIDEWALK CLOSED	6176000B		COMMISSION FURNISHED/RETAINED
WO8 - 5	48X48 16.00				SLIPPERY WHEN WET (SYMBOL)						SIDEWALK CLOSED AHEAD,			TEMP. TRAFFIC BARRIER HEIGHT
WO8 - 6					TRUCK CROSSING (WITH FLAGS)	R9-11L	24X18 3.00				(ARROW LEFT) CROSS HERE	6177000E		COMMISSION FURNISHED/RETAINED
WO8 - 6					TRUCK ENTRANCE	DO 110	24X18 3.00				SIDEWALK CLOSED AHEAD,	6208064A		TEMPORARY RAISED PAVEMENT MARI
WO8 - 7	36X36 9.00 a 36X36 9.00				LOOSE GRAVEL FRESH OIL/LOOSE GRAVEL	R9-11R R10-6	24X18 3.00 24X36 6.00				(ARROW RIGHT) CROSS HERE STOP HERE ON RED (45^ ARROW)	9029400		TEMPORARY TRAFFIC SIGNALS TEMPORARY TRAFFIC SIGNALS AND
	48X48 16.00				LOW SHOULDER			6 60.00		63	3 ROAD CLOSED	3023101		TEM STORY THOU TE STORYES THE
	1 48X48 16.00				UNEVEN LANES						ROAD CLOSED XX MILES AHEAD			
	2 48X48 16.00				NO CENTER LINE		60X30 12.50				1 LOCAL TRAFFIC ONLY			
	5 48X48 16.00	+ + +			GROOVED PAVEMENT		60X30 12.50 60X48 20.00	3 37.50		62	2 ROAD CLOSED TO THRU TRAFFIC			
	5P 30X24 5.00 7 48X48 16.00				MOTORCYCLE (PLAQUE) SHOULDER DROP-OFF (SYMBOL)		56X12 4.67				FINE SIGN SPEEDING/PASSING (PLATE)			
	7P 30X24 5.00				SHOULDER DROP-OFF (PLAQUE)	001101	100,122	MISCELLANE	OUS SIG	NS	or zza mo, mosmo (v zmz)			
W10-1	42RND. 9.62				RAILROAD CROSSING	CONST-5	48X36 12.00				POINT OF PRESENCE			
	1 24X24 4.00				DOUBLE DOWN ARROW (SYMBOL)		96X48 32.00				POINT OF PRESENCE			
	2 48X48 16.00 2X 24X18 3.00				LOW CLEARANCE (SYMBOL)		48X24 8.00 72X36 18.00				RATE OUR WORK ZONE RATE OUR WORK ZONE		616 0	O O1 MISC LIMAD CLIMAT
	2x 24x18 3.00 2a 84x24 14.00			+ +	LOW CLEARANCE (PLAQUE) OVERHEAD LOW CLEARANCE (FEET AND INCHES)		48X36 12.00				WORK ZONE NO PHONE ZONE			9.01 - MISC. LUMP SUM ORARY TRAFFIC CONTROL
	4 120X60 50.00				LOW CLEARANCE XX FT XX IN XX MILES AHEAD	SP - 1	90X48 30.00				US-169 CLOSED AHEAD			LUMP SUM = 1
	5 120X60 50.00				WIDTH RESTRICTION XX FT XX IN XX MILES AHEAD	SP - 2	144X48 48.00				MO. RTE M CLOSED AHEAD		**NOT I	NCLUDED IN TEMPORARY TRAFFIC
	1 30X30 6.25	+ + +			ADVISORY SPEED (PLAQUE)	SP - 3	72X30 15.00				MO. RTE B FOLLOW DETOUR			- 1 LUMP SUM QUANTITY
	2 30X24 5.00 3 30X24 5.00				XXX FEET (PLAQUE) X MILE (PLAQUE)		36X78 19.50 36X78 19.50				0 DETOUR ASSEMBLY NORTH/STRAIGHT			LL TEMPORARY TRAFFIC CONTROL I
	1 48X48 16.00				ROAD/BRIDGE/RAMP WORK AHEAD		36X78 19.50				Da DETOUR ASSEMBLY NORTH/LEFT Db DETOUR ASSEMBLY NORTH/RIGHT			LUDED IN PAY ITEM 616-99.01, M
	2 48X48 16.00			51	DETOUR AHEAD		36X78 19.50				DC DETOUR ASSEMBLY SOUTH/STRAIGHT		DTHERWI	M TEMPORARY TRAFFIC CONTROL, U SE NOTED. QUANTITIES SHOWN HAV
WO20-	3 48X48 16.00	5 80.00		20	ROAD CLOSED AHEAD	SPECIAL	36X78 19.50	1 19.50		50	d DETOUR ASSEMBLY SOUTH/LEFT			E FOR ALL 3 BRIDGE CLOSURES AN TO CHANGE BASED ON FIELD COND
WO20-					ONE LANE ROAD AHEAD		36X78 19.50				e DETOUR ASSEMBLY SOUTH/RIGHT		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	STANGE BASED ON TILED COND
WO20-			-		RIGHT/CENTER/LEFT LANES CLOSED AHEAD		36X78 19.50				DETOUR ASSEMBLY NORTH/STRAIGHT			
	5a 48X48 16.00 6a 48X48 16.00				2 RIGHT/CENTER/LEFT LANES CLOSED AHEAD RIGHT/CENTER/LEFT LANE CLOSED		36X78 19.50 36X78 19.50				DETOUR ASSEMBLY NORTH/LEFT DETOUR ASSEMBLY NORTH/RIGHT			
	7a 48X48 16.00				FLAGGER (SYMBOL, WITH FLAGS)		36X78 19.50				DI DETOUR ASSEMBLY SOUTH/STRAIGHT			
	2 36X36 9.00				FRESH OIL		36X78 19.50				j DETOUR ASSEMBLY SOUTH/LEFT			
	5 48X48 16.00				SHOULDER WORK AHEAD		36X78 19.50			50	k DETOUR ASSEMBLY SOUTH/RIGHT			
	1 48X48 16.00				BLASTING ZONE AHEAD	616-10		CNC						
	2 42X36 10.50 3 42X36 10.50				TURN OFF 2-WAY RADIO AND PHONE END BLASTING ZONE	616-10		GNS 1187.50	ТОТ	ΤΔΙ				SUMMARY OF Q
	1 48X48 16.00				DOUBLE REVERSE CURVE		TED SIGNS		10					SHEET 3

OTE: ALL TEMPORARY TRAFFIC CONTROL ITEMS ARE INCLUDED IN PAY ITEM 616-99.01, MISC. LUMP SUM TEMPORARY TRAFFIC CONTROL, UNLESS OTHERWISE NOTED. QUANTITIES SHOWN HAVE BEEN ESTIMATE FOR ALL 3 BRIDGE CLOSURES AND ARE UBJECT TO CHANGE BASED ON FIELD CONDITIONS.

> SUMMARY OF QUANTITIES SHEET 3 OF 3



EFFECTIVE: 04-01-2023

IMPACT ATTENUATOR 40 MPH (SAND BARRELS) IMPACT ATTENUATOR 45 MPH (SAND BARRELS) IMPACT ATTENUATOR 50 MPH (SAND BARRELS) IMPACT ATTENUATOR 55 MPH (SAND BARRELS) IMPACT ATTENUATOR 60 MPH (SAND BARRELS)

IMPACT ATTENUATOR 65 MPH (SAND BARRELS)

IMPACT ATTENUATOR 70 MPH (SAND BARRELS)

TRUCK OR TRAILER MOUNTED ATTENUATOR (TMA)

SPECIAL SIGN ASSEMBLY (BOATS KEEP OUT)

RADAR SPEED ADVISORY SYSTEM CHANGEABLE MESSAGE SIGN, COMMISSION FURNISHED/RETAINED CHANGEABLE MESSAGE SIGN W/O COMM. 1** INTERFACE - CONTRACTOR FURNISHED/RETAINED CHANGEABLE MESSAGE SIGN WITH COMM. INTERFACE - CONTRACTOR FURNISHED/RETAINED

SEQUENTIAL FLASHING WARNING LIGHT

WORK ZONE TRAFFIC SIGNAL SYSTEM TEMPORARY LONG-TERM RUMBLE STRIPS

CONTRACTOR FURNISHED/COMMISSION RETAINED TEMP. TRAFFIC BARRIER HEIGHT TRANSITION RELOCATING TEMPORARY TRAFFIC BARRIER

TEMP. TRAFFIC BARRIER HEIGHT TRANSITION

TEMPORARY TRAFFIC SIGNALS AND LIGHTING

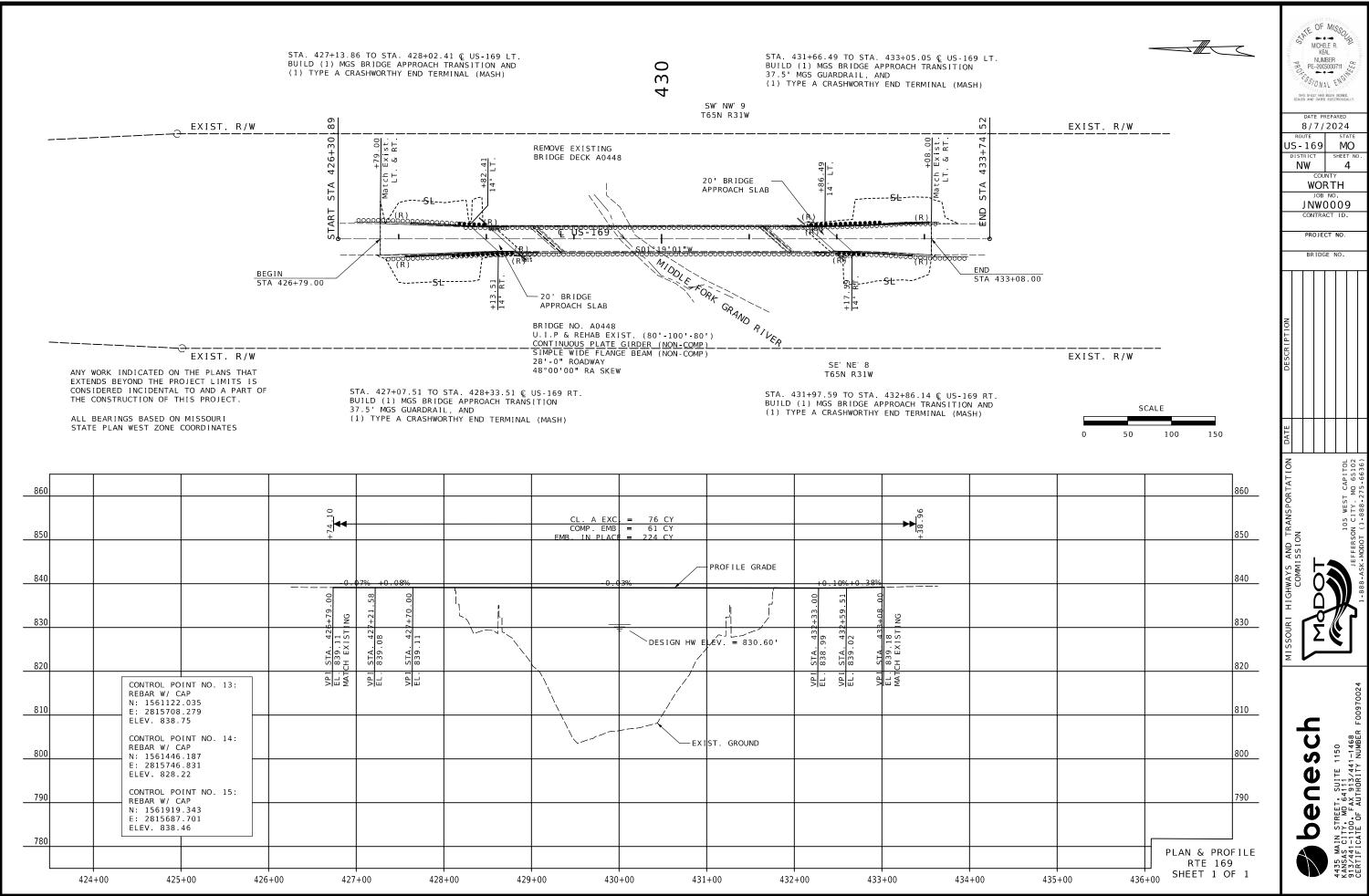
COMMISSION FURNISHED/RETAINED TEMPORARY RAISED PAVEMENT MARKER TEMPORARY TRAFFIC SIGNALS

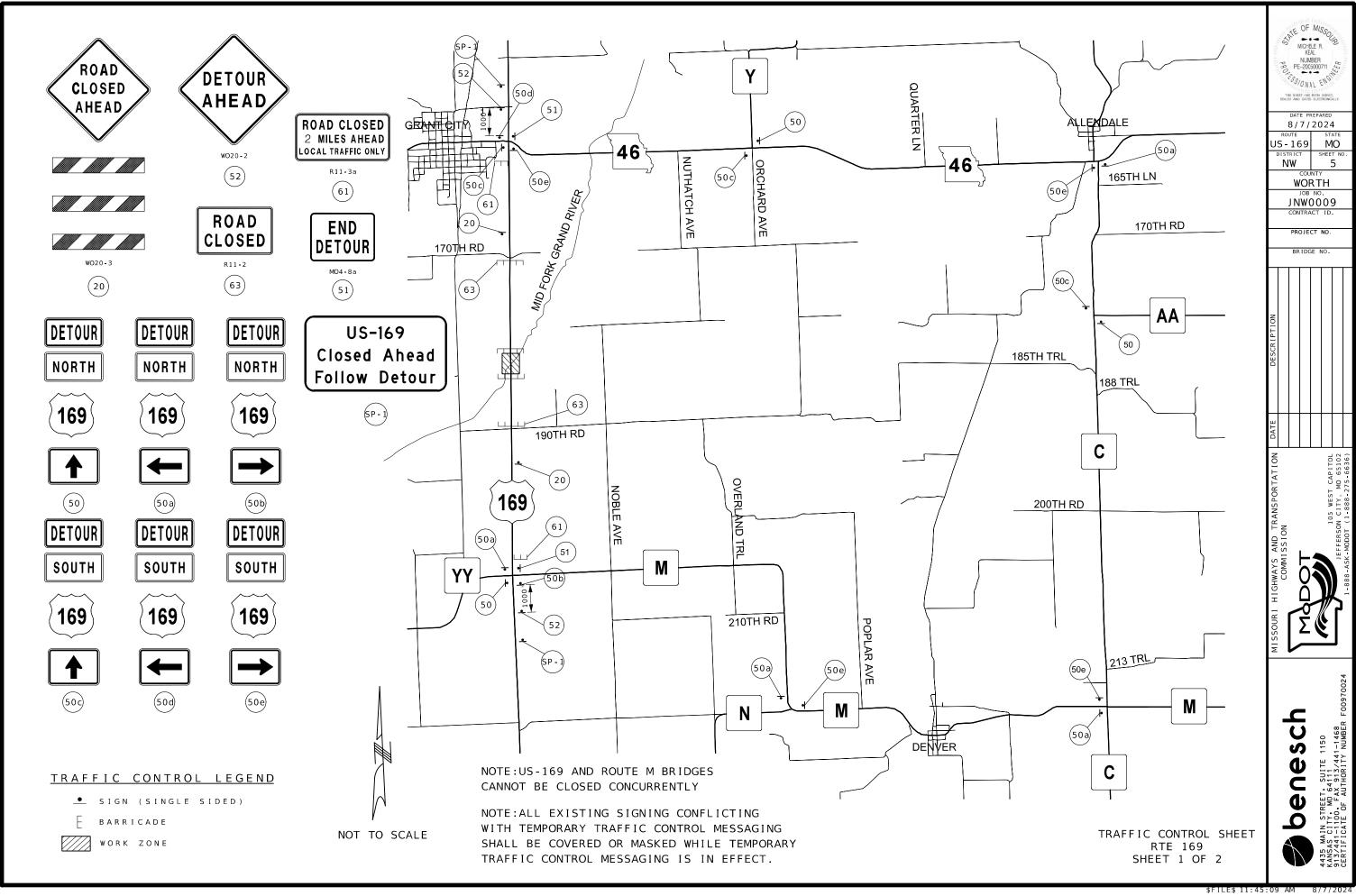
8/21/2024 US 169/ M/B DISTRICT MO SHEET NO 3 N₩

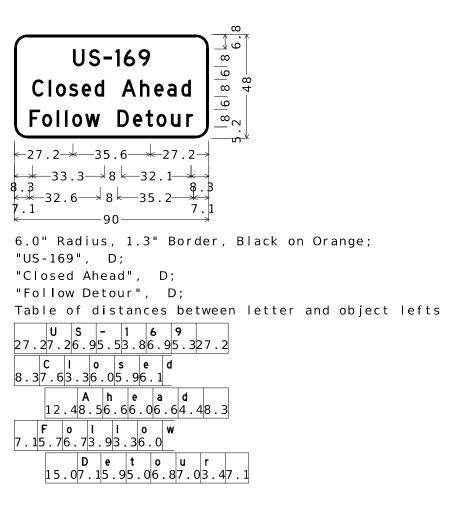
GENTRY & WORT JNW0009

> PROJECT NO. BRIDGE NO.

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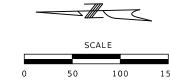


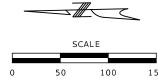


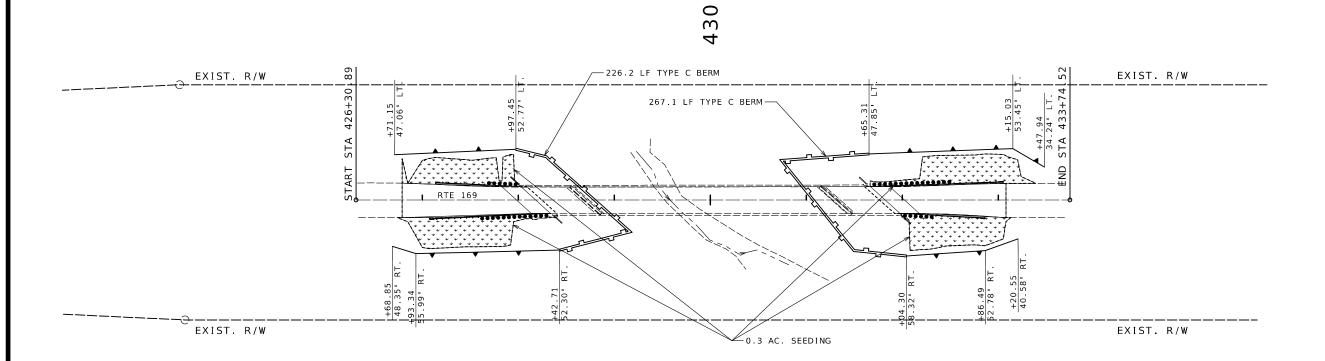


MICHELE R.
KEAL
NUMBER
PE-2005000711
Tission NAL ENGT 8/7/2024 US-169 MO DISTRICT NW WORTH CONTRACT ID. PROJECT NO.

benesch 4435 MAIN STREET, SUITE 1150 KANSAS CITY, WG 64111 411468 913.441-1100, FAX 913.441-1468 GERTIFICEE OF AUTHORITY NUMBER F00970024







TEMPORARY EROSION CONTROL LEGEND

SILT FENCE TEMPORARY BERM TYPE C PERMANENT EROSION CONTROL LEGEND

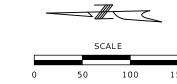
SEEDING AND MULCHING

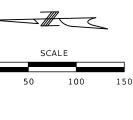
EROSION CONTROL RTE 169 SHEET 1 OF 1

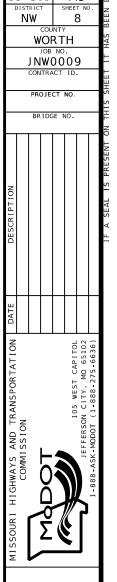




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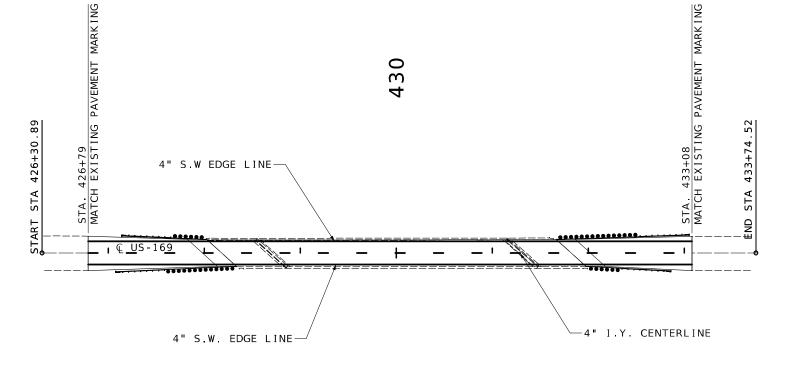






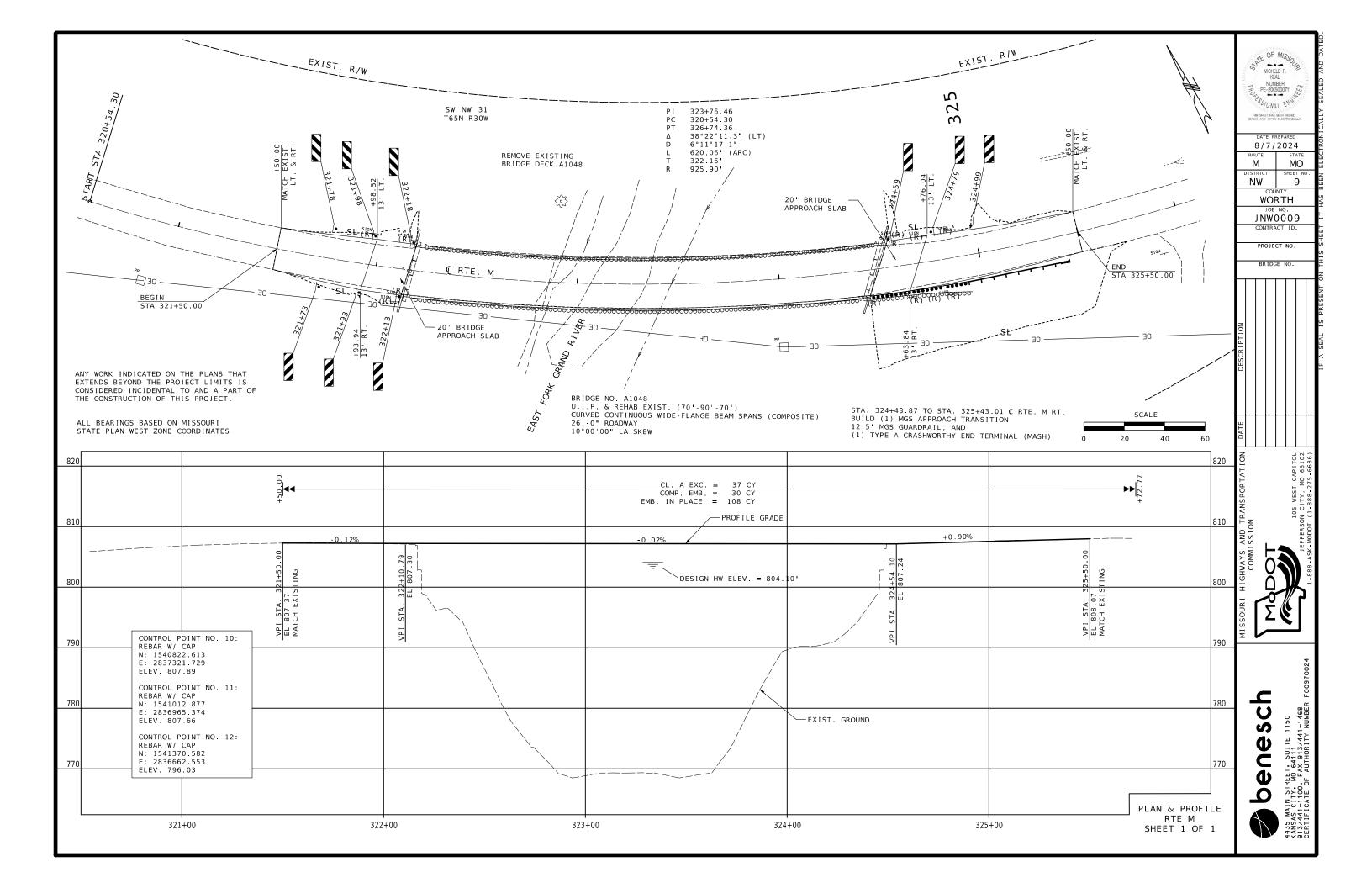
SAFE OF MISSON
MICHELE R.
KEAL
NUMBER
PE-2005000711

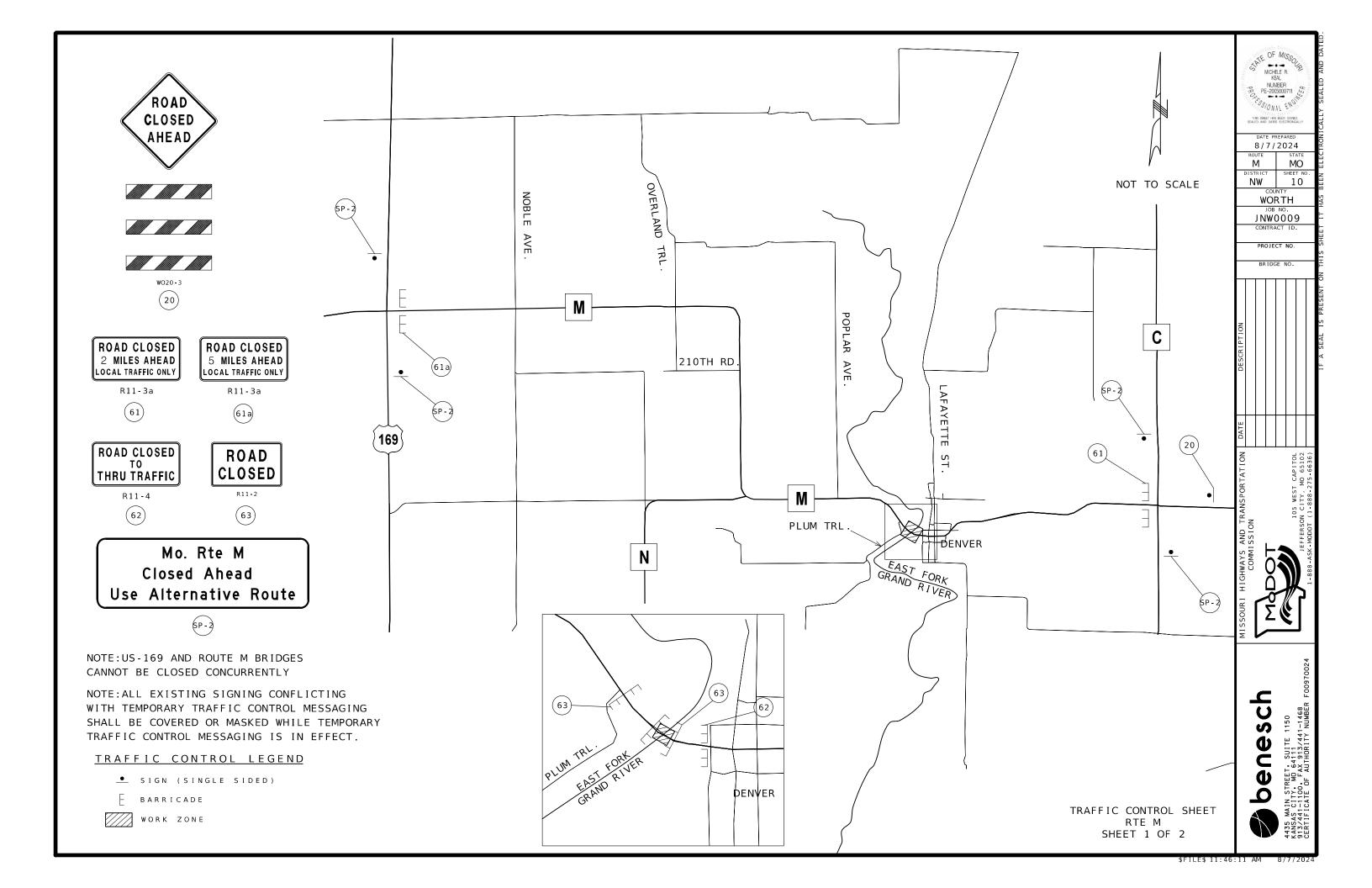
8/7/2024 US - 169 MO

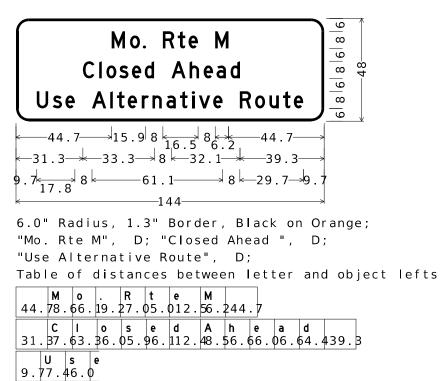


PAVEMENT MARKING RTE 169 SHEET 1 OF 1

benesch

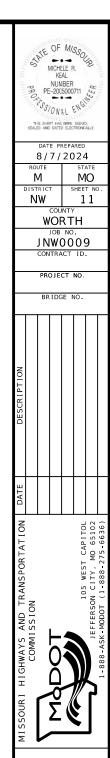




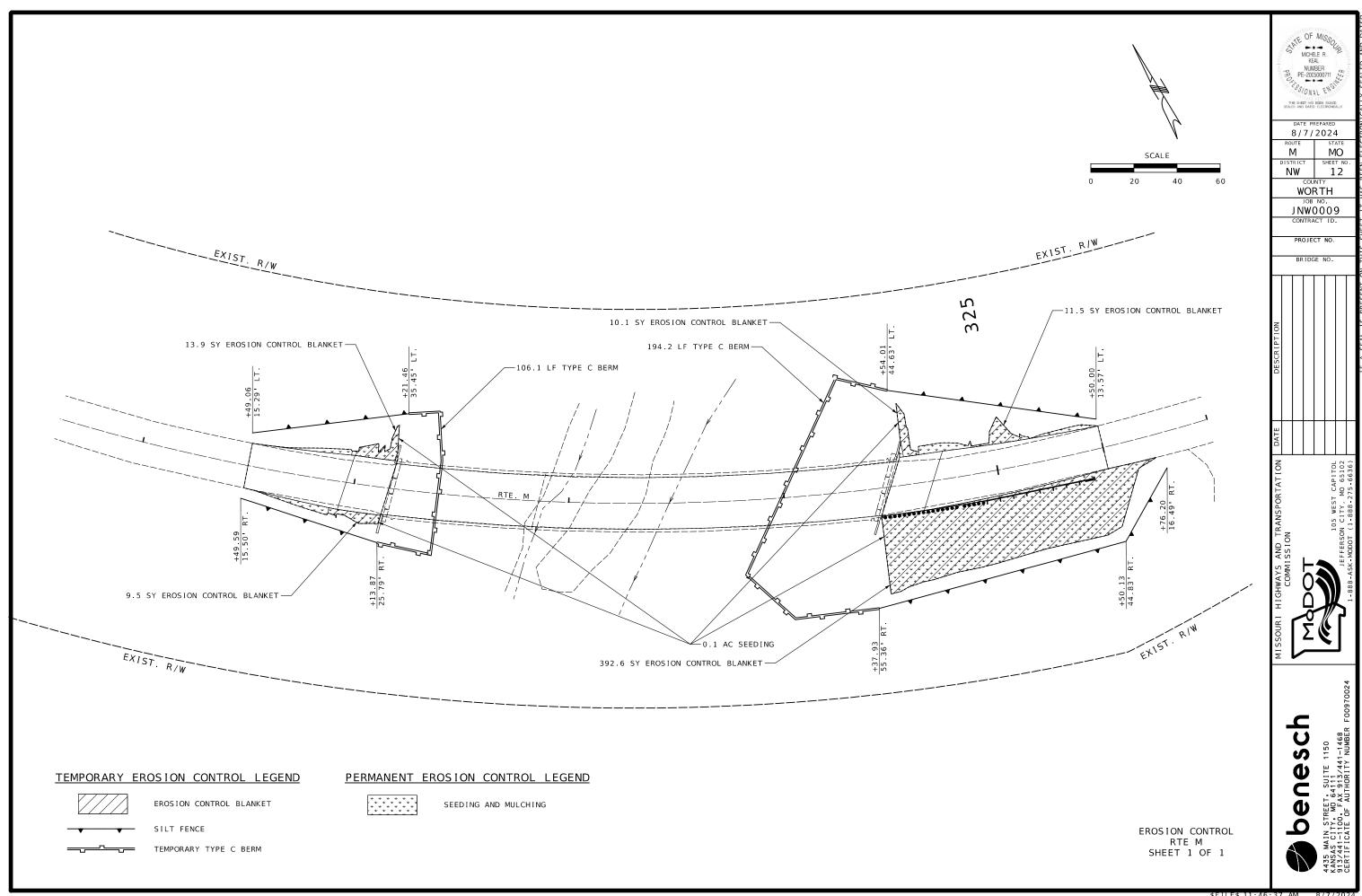


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R o u t e 12.57.16.76.45.04.59.7



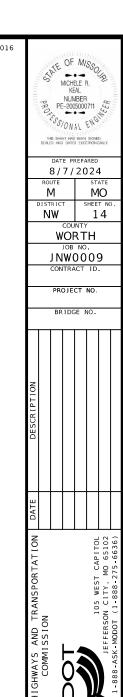
benesch 4435 MAIN STRET. SUITE 1150 KANSAS CITY, MO 64111 913.744-1100. FAS 913.44-1-468 1917-1100. FAS 913.44-1-468



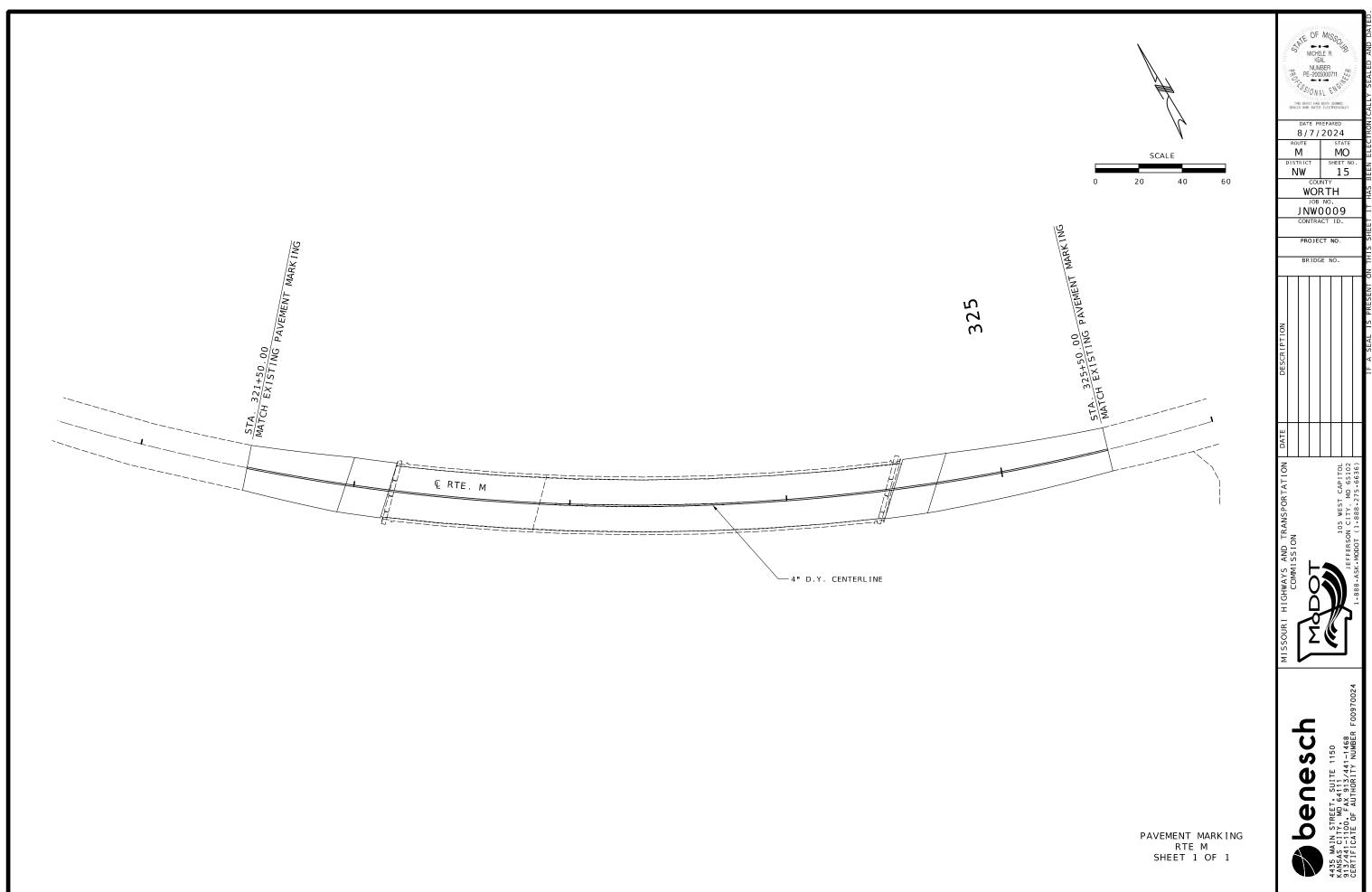
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S I G NO	I SIGN	STATION		ATION	SIGN DTL SHT NO	ITEM NO. 9031010 CY	POST DES NO.	POST P NO.1 N		ST LB	S TOTA R ITEM N 903121	10		OST IO.2	LBS TOTA PER ITEM FT 90312	20 I NO.	2.55 L	BARS BS PER I	ГОТАІ	ITEM NO. 9031250A LF	POST NO.1	POST 1 NO.2	TOTAL ITEM NO. 9031270A	12-GA	N DRIVEN 7-GA	CONCRETOR TO THE TOTAL TEM NO	POST POST NO 1 NO 2	ITEM NO. 9031280	INSERT (6 FT) ITEM NO 9031272A EA	DRIVEN 7-GA ITEM NO		ASSEMBLY ITEM NO. 9031241 EA	OTHER REQUIR		PE-2005000711 FE-2005000711 FE-200500071 FE-20050071 FE-200500071 FE-200500071
	12"X36" 12"X36"	321+73 321+78		14 69 RT 15 12 LT				-													9.5		9.5	1 1											DATE PREPARED 8/7/2024
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					-		.58	0.63	5	3½ 3½	12 18		4~6" 5~6"		. 13				5 6	W10 W12	26 35		2.17	5 0 5 6	36 36	5 6	1 31 5 3 1 44 5 9	1.37	5 5	1.43	5 -9" 6 -3"	1.52		D-29	

		S	ΓΑΝ	DAF	D S	IGN ASSEMBLIES				SIGN SUMMARY										
						TYPE		1							SIZE, TYPE & SQUA	RE FEET				
STATI					CION	DESCRIPTION SIZES S MIN	4D E D	05 5	ACH		STANDARD SIGN OR	SIGN DETAIL SHEET	NO.	FLAT_SHEET	FLAT SHEET FLUORESCENT	STRUCTURAL	STRUCTURAL FLUORESCENT			
STATI	ION	LOCATION	OM3 - L 12 X36		SIGN	DESCRIPTION, SIZES & NUM	/IBEK	OF E	ACH		SPECIAL SIGN NUMBER	NO.	LACII	SIZE SH ITEM NO. 9035004A	SHF * ITEM NO. 9035069A	ST ITEM NO. 9035011A	STF * ITEM NO. 9035071A			
											OM3-L - TYPE 3 OBJECT MARKER, LEFT		3	12"X36"	9.00	9033011A	90350/1A			
			OM3 L 12 X36	OM3 R 12 X36							OM3-R - TYPE 3 OBJECT MARKER, RIGHT		6	12"X36"	18.00					
321+7	73	RTE M, 14 69 RT		1																
321+7	78	RTE M, 15.12 LT	1	1																
321+9	00	RTE M, 14.21 RT RTE M, 14.77 LT	1	1																
322+1	13	RTE M, 13.00 RT	1	1																
322+1	18	RTE M, 13.00' LT	1																	
324+5	59	RTE M, 13.00' LT		1																
324+7	79	RTE M, 14.27' LT		1																
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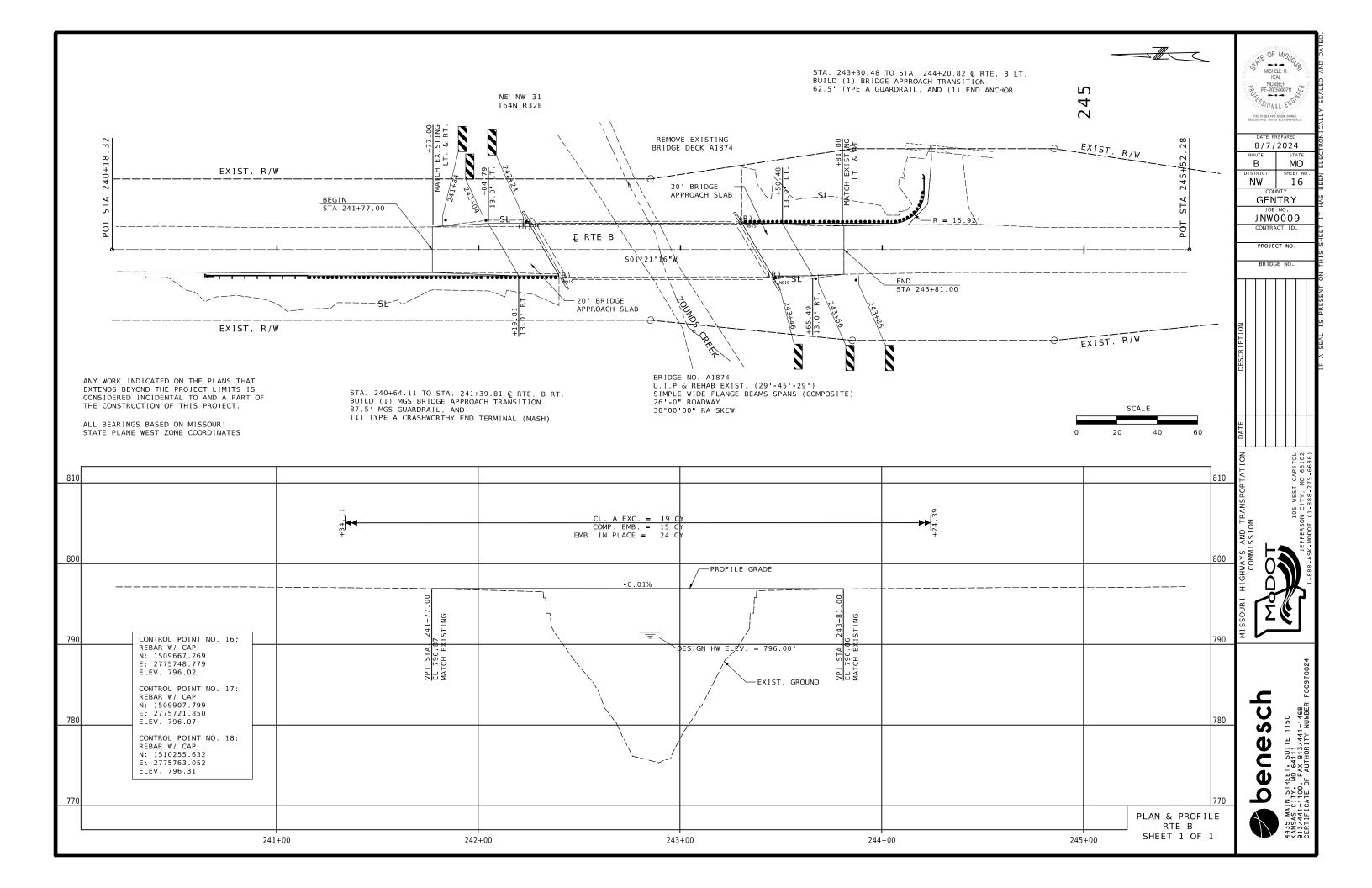
* ORANGE, YELLOW & YELLOW/GREEN

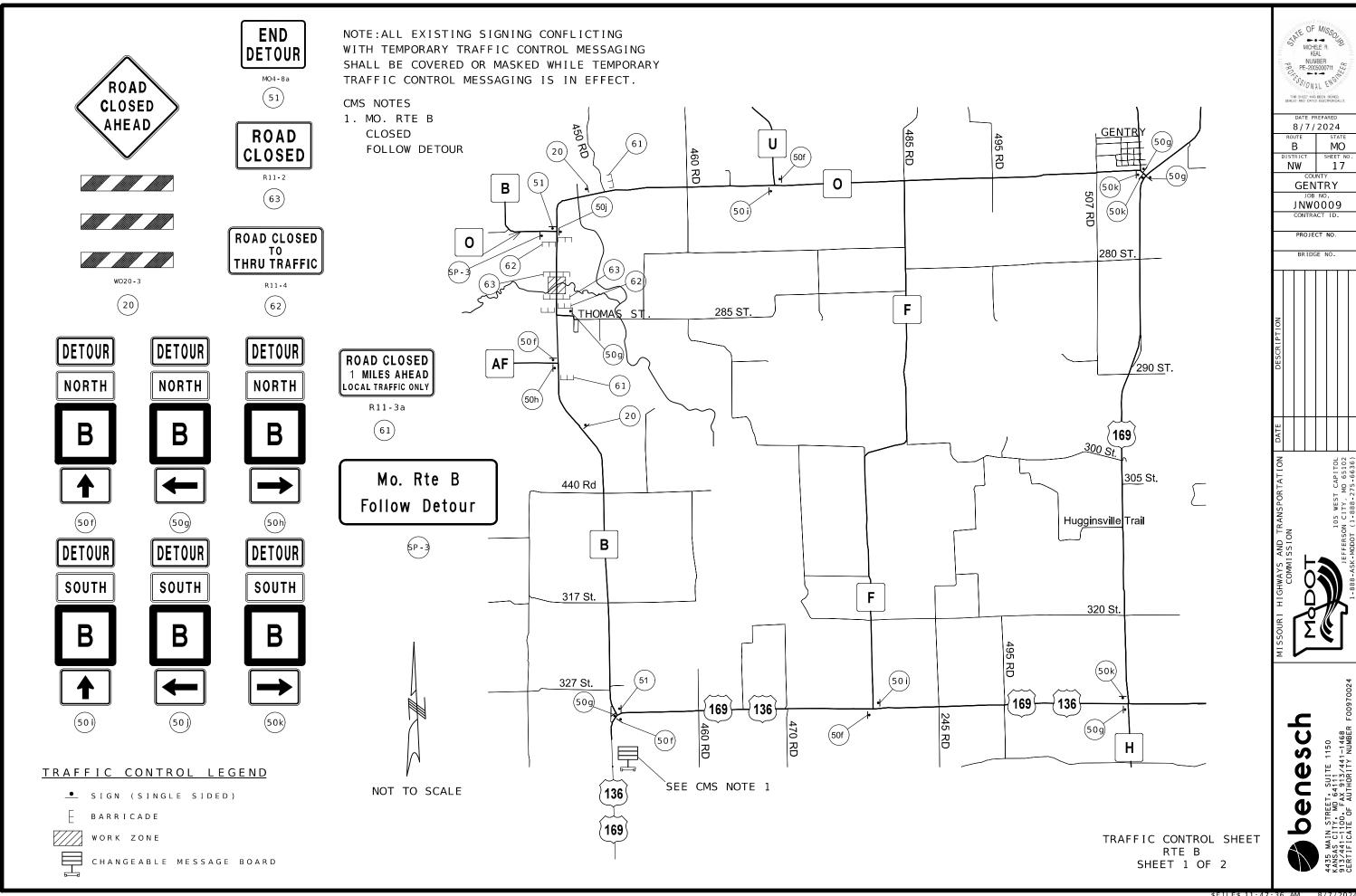


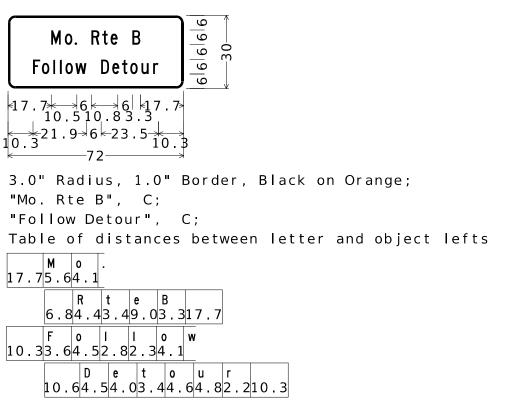


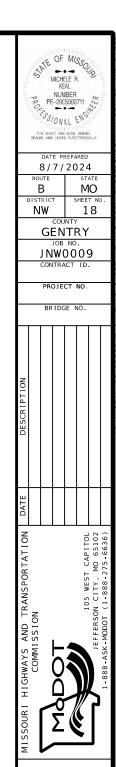


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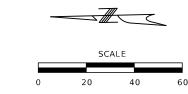


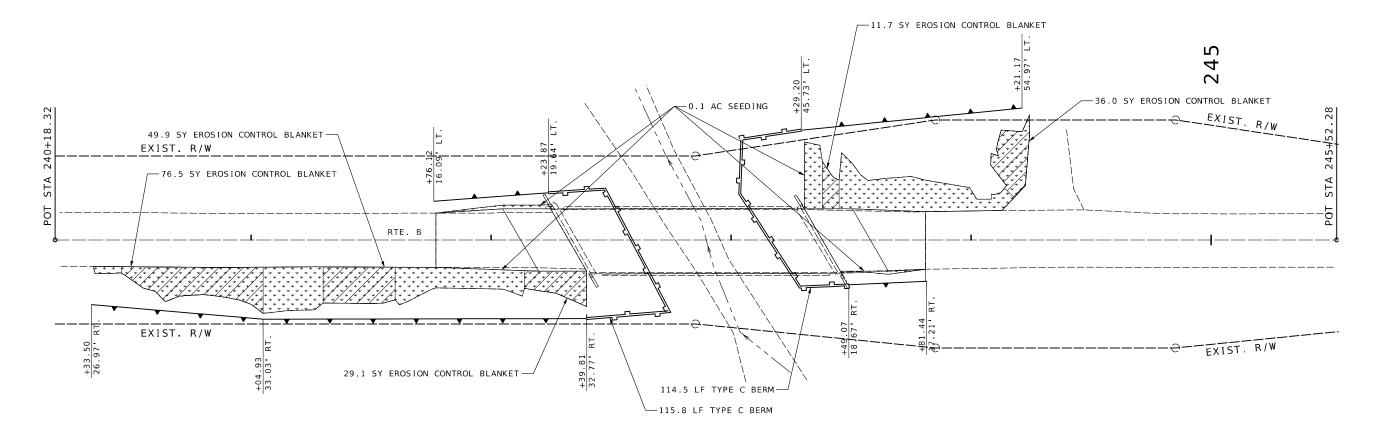






benesch 435 MAIN STRET, SUITE 1150 KANSAS CITY, WD 64111 913/441-1100, FAX 913/441-1468 618/111100, FAX 913/441-1468





TEMPORARY EROSION CONTROL LEGEND

EROSION CONTROL BLANKET

SILT FENCE

TEMPORARY BERM TYPE C

PERMANENT EROSION CONTROL LEGEND



SEEDING AND MULCHING

EROSION CONTROL RTE B SHEET 1 OF 1



DATE PE	REPARED
8/7/	2024
ROUTE	STATE
В	MO
ISTRICT	SHEET NO.
NW	19
COL	NTY

GENTRY
JOB NO.
JNW0009
CONTRACT ID.

PROJECT NO.

BRIDGE NO.

NSPORTATION DATE DESCRIPTION

WEST CAPITOL

SITY, MO 65102

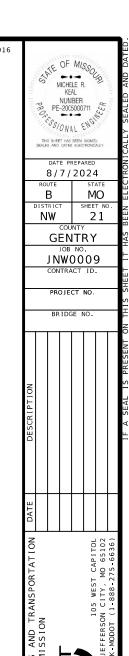


benesch 435 Main Street, Suite 1150 KANSAS CITY, MO 64111 913.441-1100, FAX 913.441-1468 CERTIFICATE OF AUTHRRITY WINMER FORGYOOD

																																				EFFECTIVE 07-01-2022	galatica
		SI	GNS			- 11	CONCRETE FOOT INGS	S	TRU	CTUF PO	RAL STS	STE	EL		PIP	E PC	OSTS	*	B B	ACK BARS	I NG **		U- HANNEL						RATED	SQUAR	E STEE						SATE OF MISSON
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GN SIG	SN S	STATION		LOCATIO	N G	SIGN OTL SHT NO	ITEM NO. 9031010 CY	POST DES NO.	POST NO.1	POST NO.2	POST NO.3	LBS PER FT	TOTAL ITEM NO 903121 LBS	L PIP O. SIZ	PE POS	T POST 1 NO.2	LBS PER FT	TOTAL ITEM NO 9031220 LBS	NO LEACH	GTH T	BARS PER FT OTAL TO	TAL LBS	ITEM NO. 9031250A LF	POST NO.1		TOTAL ITEM NO. 9031270A LF	12-GA	DRIVEN 7-GA. ITEM NO. 9031273A	7 - GA	POST POS NO 1 NO	2 ITEM NO. 9031280	(6 FT)	ITEM NO.	7 GA ITEM NO	ASSEMBLY ITEM NO. 9031241 EA	OTHER REQUIRED ITEMS	MICHELE R. KEAL NUMBER PE-2005000711 Fig. 5/O NAL EN
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							ROUND			DST .											PC	NO.		LBS/		.BS/IN	STUB LENGTH		DEPTH	C.Y. DE	1 GRADE	. DEPTI	H C.Y.	3:1 OR 2 DEPTH	C.Y.		Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z
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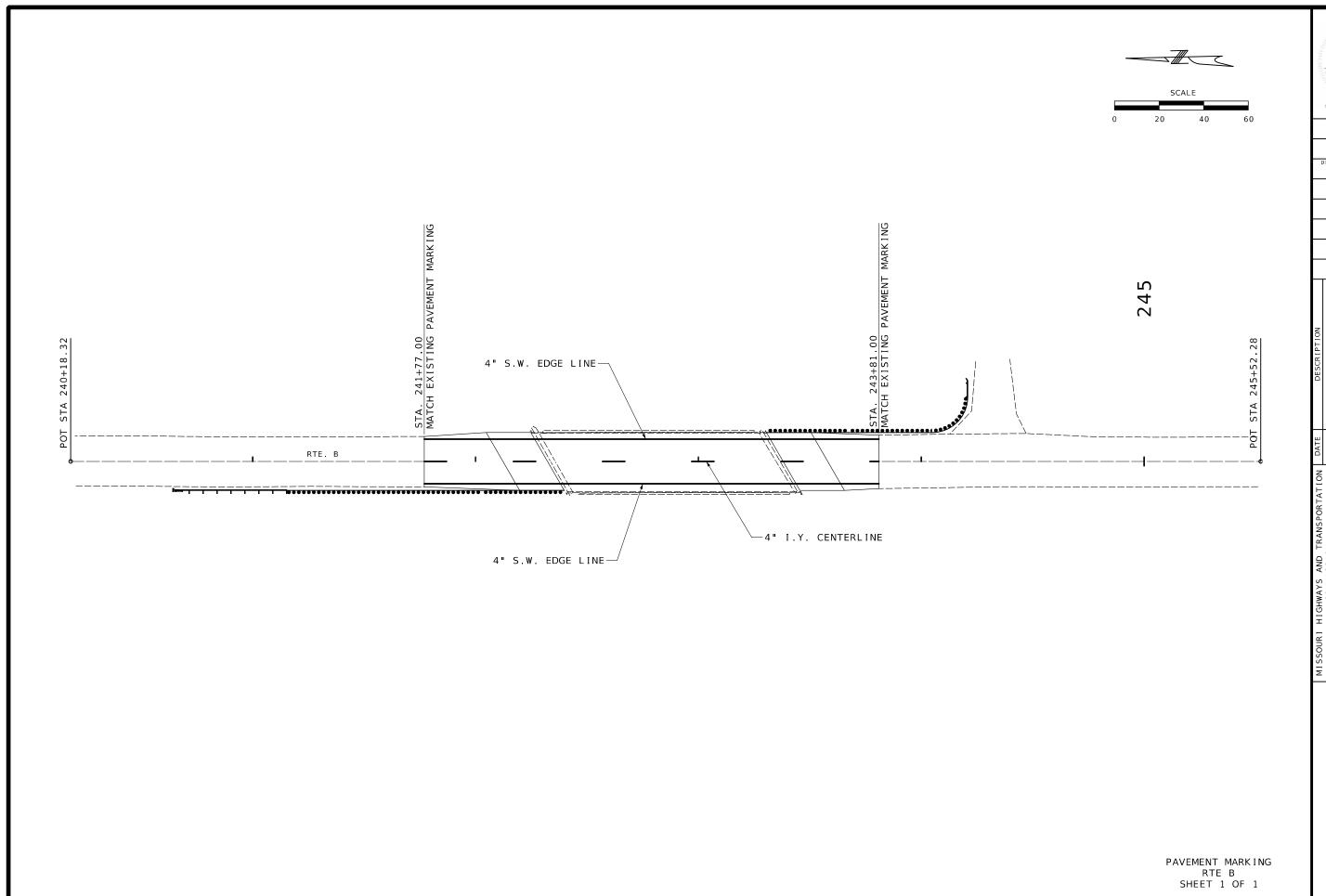
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* ORANGE, YELLOW & YELLOW/GREEN





benesch Sa Main Street, Suite 1150 NAS CITY, MO 6411 3.741-1100, FAX 913.441-1468 PITE INTERIOR OF ANTHORITY NINGER FOOGTOON



MICHELE R. KEAL NUMBER PE-2005000711 NUMBER PE-2005000711 E 8/7/2024 ROUTE B STATE MO DISTRICT SHEET NO COUNTY GENTRY JOB NO.
JNW0009
CONTRACT ID. PROJECT NO.

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Table Showing S3 Bar Lengths Int. Bent No. 3 Int. Bent No. 4 Span 2 Span 3 Span 3 Span 4 26'-5" 33'-0" 33'-0" 26'-5"

	Lap Length Splices **
Bar Size	Splice Length
4	2'-7"
5	3'-3"
6	3'-10"
7	4'-11"

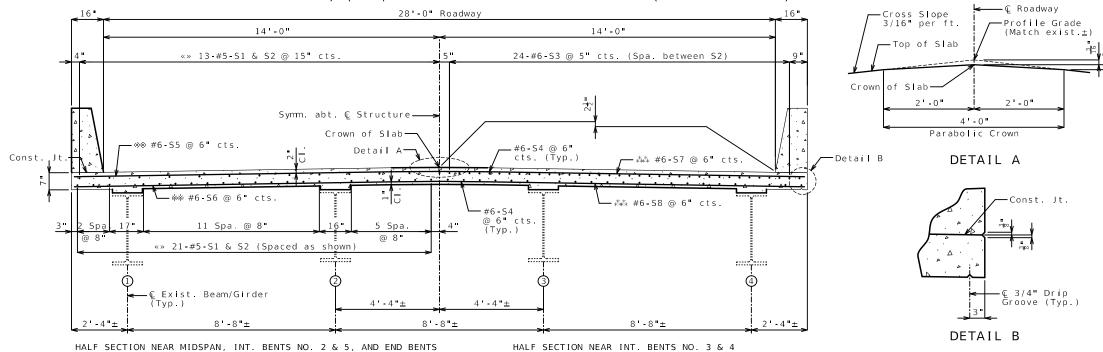
** Unless otherwise shown.

«» S1 bars located in Span 1-2 & Span 5-6 and S2 bars located in Span 2-3, Span 3-4, & Span 4-5

 $*\!*\!*$ S5 and S6 bars located in Span 1-2 & Span 5-6 near End Bents No. 1 & 6 & Int. Bents No. 2 & 5

*** S7 and S8 bars located in Span 2-3 & Span 4-5 near Int. Bents No. 2 & 5

U.I.P., REDECK, AND MAKE COMPOSITE EXISTING (49') SIMPLE WIDE FLANGE BEAM SPAN, (80'- 100'- 80') CONTINUOUS PLATE GIRDER SPANS, (49') SIMPLE WIDE FLANGE BEAM SPAN (SKEW: 48° R.A.)



General Notes:

Design Specifications: 2002 AASHTO LFD (17th Ed.) Standard Specifications Seismic Performance Category A

Design Loading:

H20-44 (1957) (Existing) H520-44 (New Construction) 35 lb/sf Future Wearing Surface Earth - 120 lb/cf, Equivalent Fluid Pressure 45 lb/cf Fatigue Stress - Case III

Design Unit Stresses

Class B-1 Concrete (Barrier)

Class B-2 Concrete (End Bents & Superstructure,
except Barrier)

Class B Concrete (Intermediate Bent)

Reinforcing Steel (Grade 60)

f'c = 4,000 psi
f'c = 4,000 psi
f'c = 4,000 psi
fy = 60,000 psi

Joint Filler

All joint filler shall be in accordance with Sec 1057 for preformed sponge rubber expansion and partition joint filler, except as noted.

Reinforcing Steel:

Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise shown.

Miscellaneous

Bars bonded in existing concrete not removed shall be cleanly stripped and embedded into new concrete where possible. If length is available, existing bars shall extend into new concrete at least 40 diameters for plain bars and 30 diameters for deformed bars, unless otherwise noted.

Roadway surfacing adjacent to bridge ends shall match new bridge slab surface. (Roadway Item)

Outline of existing work is indicated by light dashed lines. Heavy lines indicate new work.

Contractor shall verify all dimensions in field before ordering new material.

The area exposed by the removal of concrete and not covered with new concrete shall be coated with an approved qualified special mortar in accordance with Sec 704.

Rubblized concrete from the existing bridge deck that qualifies as clean fill may be placed on spill slopes at end bents above ordinary high water line (Roadway item).

Traffic Handling:

Structure to be closed during construction. Traffic to be maintained on other routes during construction. See roadway plans for traffic control.

TYPICAL SECTION THRU SLAB

Structural Steel Protective Coating (Top Flange):

In accordance with Sec. 216.50 and 1081, the top, and additionally the sides, and bottom of the top flange shall be coated with not less than 3.0 mils of Gray Epoxy Mastic-Primer (non-aluminum) applied over an SSPC-SP3 surface preparation. Payment for coating steel will be considered completely covered by the contract sq. foot price for Removal of Existing Bridge Deck.

Structural Steel Protective Coating

Protective Coating: System G in accordance with Sec $1081,\ 10$ feet on each side of the centerline of Int. Bents No. 2 and 5 shall be coated.

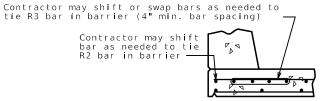
Surface Preparation: Surface preparation of the existing steel shall be in accordance with Sec 1081 for Overcoating of Structural Steel. The cost of surface preparation will be considered completely covered by the contract unit price per sq. foot for Surface Preparation for Overcoating Structural Steel (System G).

Field Coat(s): The color of the field overcoat shall be Brown (Federal Standard #30045) and shall be applied in accordance with Sec 1081.10.3.4. The cost of the intermediate field coat will be considered completely covered by the contract unit price per sq. foot for Intermediate Field Coat (System G). The cost of the finish field coat will be considered completely covered by the contract unit price per sq. foot for Finish Field Coat (System G).

Method of forming the slab shall be in accordance with Sec 703 All hardware for forming the slab to be left in place as a permanent part of the structure shall be coated in accordance with ASTM A123 or ASTM B633 with a thickness Class SC 4 and a finish Type I, II, or III.

Slab shall be cast-in-place with conventional forming or stay-in-place corrugated steel forms. Precast prestressed panels will not be permitted.

For Optional Stay-In-Place Form Details, see Sheet No. 2.



OPTIONAL SHIFTING TOP BARS AT BARRIER

Estimated Quanti	Lies	
I t em		Total
Temporary Shoring	lump sum	1
Removal of Existing Bridge Deck	sq. foot	11,499
Partial Removal of Substructure Concrete	lump sum	1
Bridge Approach Slab (Minor)	sq. yard	127
Slab on Steel	sq. yard	1,241
Type H Barrier	linear foot	728
Substructure Repair (Formed)	sq. foot	28
Substructure Repair (Unformed)	sq. foot	59
Reinforcing Steel (Bridges)	pound	110
Shear Connectors	each	3,444
Anchor Bolt Replacement	each	2
Cleaning and Coating Existing Bearings	each	1
Slab Drain	each	66
Surface Preparation for Overcoating Structural Stee	el (System G) sq. foot	1,700
Intermediate Field Coat (System G)	sq. foot	1,700
Finish Field Coat (System G)	sq. foot	500
Vertical Drain at End Bents	each	2
Strip Seal Expansion Joint System	linear foot	89
Open Cell Foam Joint Seal	linear foot	56

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Cost of any required excavation for bridge will be considered completely covered by the contract unit price for other items.

* Concrete Bridge Approach Slab only. See Special Provisions.

Estimated Quantities for Slab on S	teel
I t em	Total
Class B-2 Concrete cu. yar	d 279
Reinforcing Steel (Epoxy Coated) poun	101,970

The table of Estimated Quantities for Slab on Steel represents the quantities used by the State in preparing the cost estimate for concrete slabs. The area of the concrete slab will be measured to the nearest square yard longitudinally from end of slab to end of slab and transversely from out to out of bridge slab (or with the horizontal dimensions as shown on the plan of slab). Payment for stay-in-place corrugated steel forms, conventional forms, all concrete and epoxy coated reinforcing steel will be considered completely covered by the contract unit price for the slab. Variations may be encountered in the estimated quantities but the variations cannot be used for an adjustment in the contract unit price.

REPAIRS TO BRIDGE: US 169 OVER MIDDLE FORK OF GRAND RIVER

ROUTE 169 FROM ROUTE 46 TO ROUTE M
ABOUT 2.3 MILES SOUTH OF ROUTE 46
BEGINNING STATION 428+17.96± (MATCH EXISTING)

one Main Plaza, 4435 Main St., Suite 1150, Kansas City, Mo 64111 816/221, 4222, FAX 913/441-1468 CERTIFICATE OF AUTHORITY NUMBER F00970024

Designed MAR. 2024 Detailed APR. 2024 Checked JUL. 2024

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 1 of 14

General Notes:

Stay-In-Place Forms:

Corrugated steel forms, supports, closure elements and accessories shall be in accordance with grade requirement and coating designation G165 of ASTM A653. Complete shop drawings of the permanent steel deck forms shall be required in accordance with Sec 1080.

Corrugations of stay-in-place forms shall be filled with an expanded polystyrene material. The polystyrene material shall be placed in the forms with an adhesive in accordance with the manufacturer's recommendations.

Form sheets shall not rest directly on the top of beam flanges. Sheets shall be securely fastened to form supports with a minimum bearing length of one inch on each end. Form supports shall be placed in direct contact with the flange. Welding on or drilling holes in the beam flanges will not be permitted. All steel fabrication and construction shall be in accordance with Sec 1080 and 712. Certified field welders will not be required for welding of the form supports

The design of stay-in-place corrugated steel forms is per manufacturer which shall be in accordance with Sec 703 for false work and forms. Maximum actual weight of corrugated steel forms allowed shall be 4 psf assumed for beam loading.

The contractor shall provide a method of preventing the direct contact of the stay-in-place forms and connection components with uncoated weathering steel members that is approved by the engineer

Pouring and Finishing Slab:

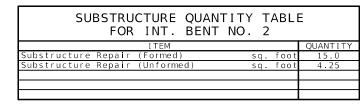
The contractor shall provide bracing necessary for lateral and torsional stability of the beams during construction of the concrete slab and remove the bracing after the slab has attained 75% design strength. Contractor shall not weld on or drill holes in the beams. The cost for furnishing, installing, and removing bracing will be considered completely covered by the contract unit price for Slab on Steel. The contractor shall provide bracing necessary for lateral

Slab shall be poured upgrade from end to end at a minimum rate of 25 cubic yards per hour.

Alternate pour sequences may be submitted to the engineer for approval. Keyed construction joints shall be provided between pours

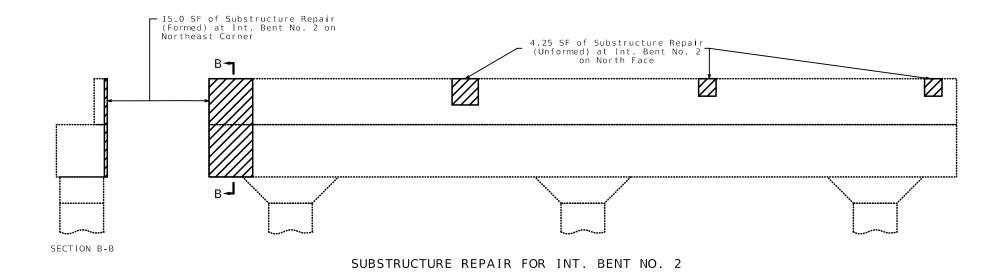
(1) Slab is to be considered a uniform thickness as shown on the plans. Haunching will vary. See front sheet for slab thickness. For adjusted girder deflection due to weight of new deck and barriers, see Bridge Electronic Deliverables.





Gray Epoxy-Mastic Primer (See Sec 216) (All Beams)

Notes: These quantities are included in the Estimated Quantities Table on Sheet No. 1.



Designed MAR. 2024 Detailed APR. 2024

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- Fill corrugations with foam (Typ.)

SECTION A-A

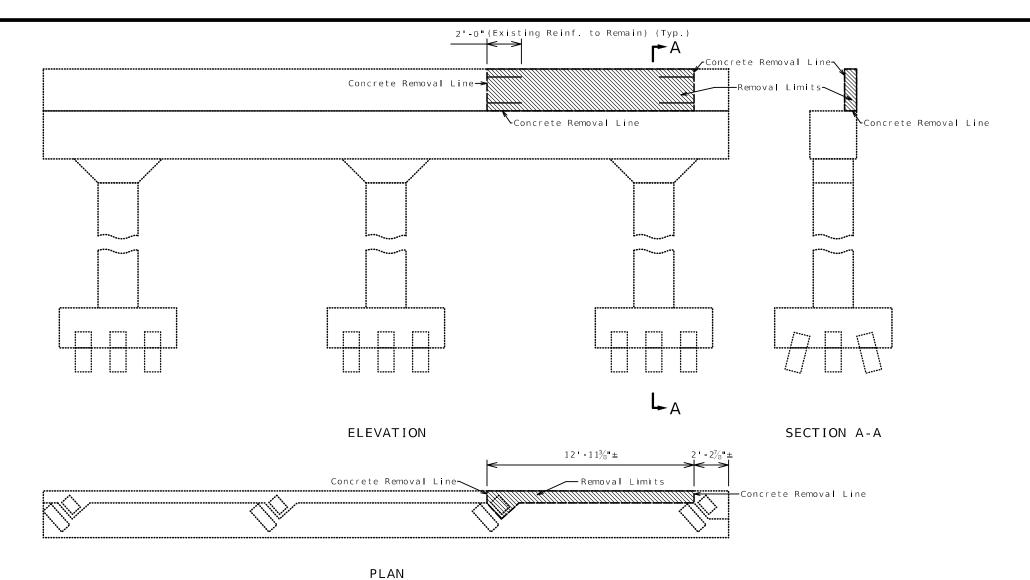
OPTIONAL STAY-IN-PLACE FORM DETAILS

NUMBER 10/01/2024 10/1/2024 US 169 MO SHEET NO BR 2 WORTH JNW0009

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Notes:

The cost of concrete removal as shown will be considered completely covered by the contract unit price for Partial Removal of Substructure Concrete.

A smooth, level surface shall be provided at Bent No. 5 removal lines.

REPAIRS TO INTERMEDIATE BENT NO. 5

Designed MAR. 2024 Detailed APR. 2024 Checked JUL. 2024

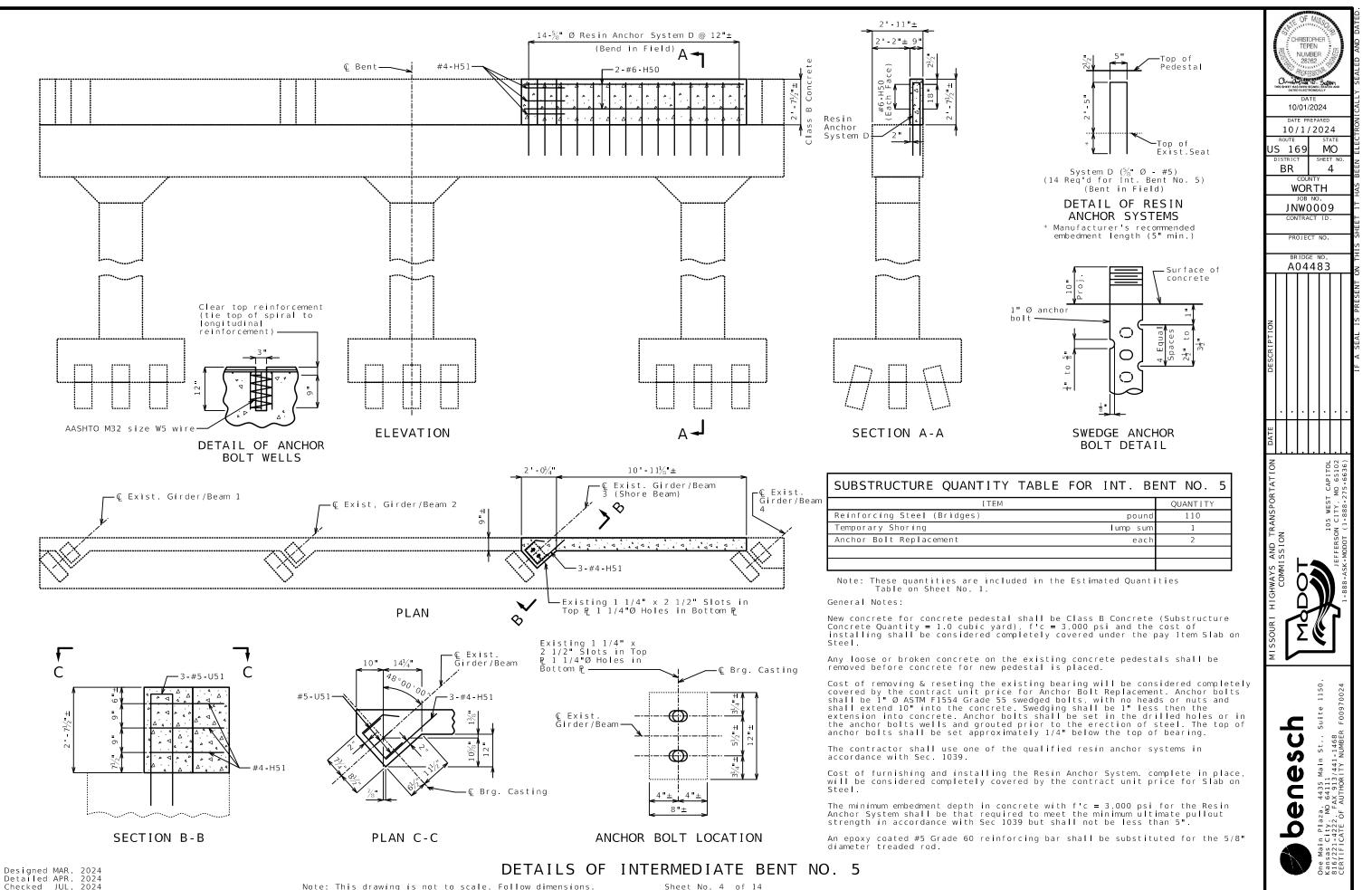
Note: This drawing is not to scale. Follow dimensions.

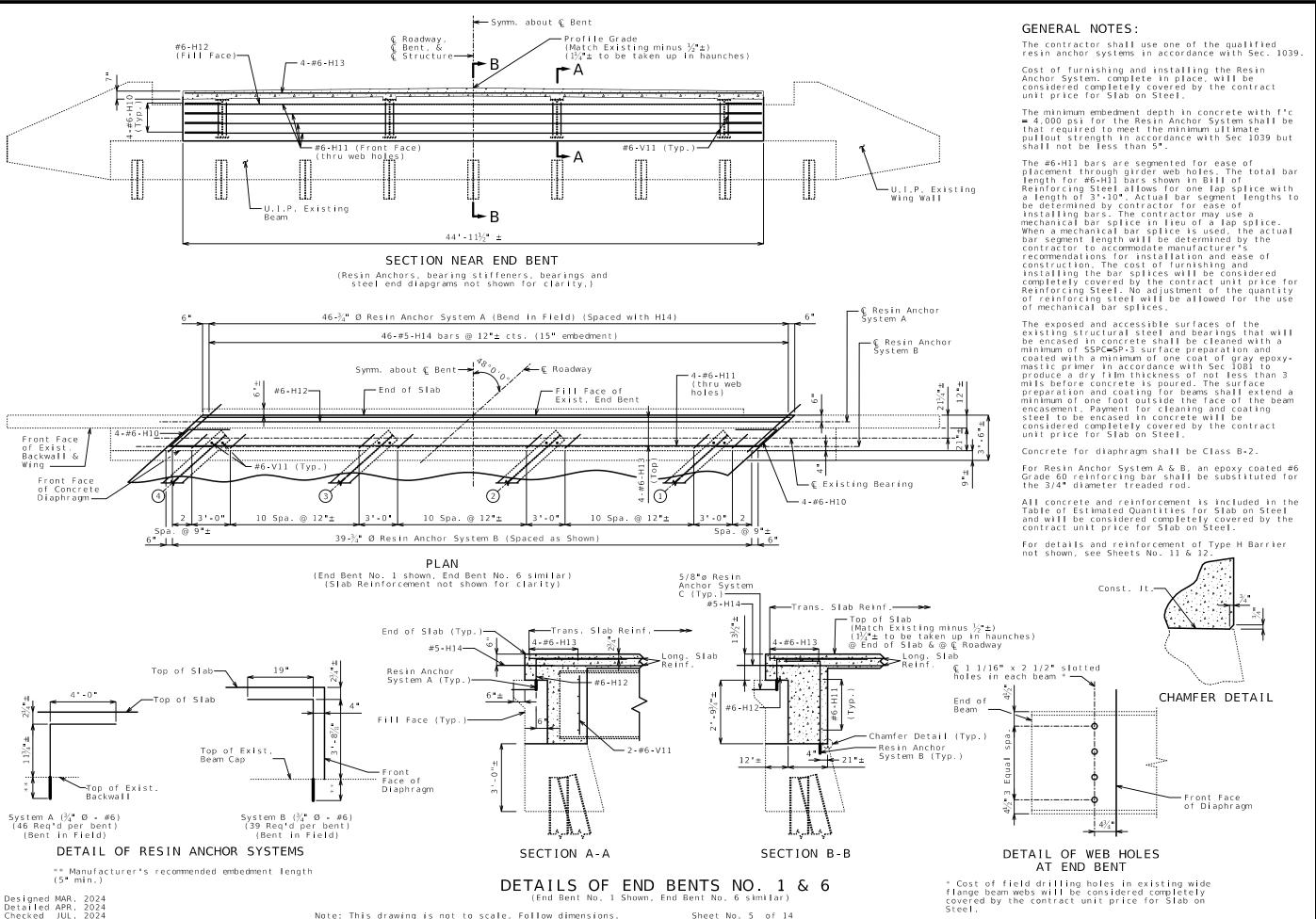
Sheet No. 3 of 14

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US 169 MO SHEET NO BR 5

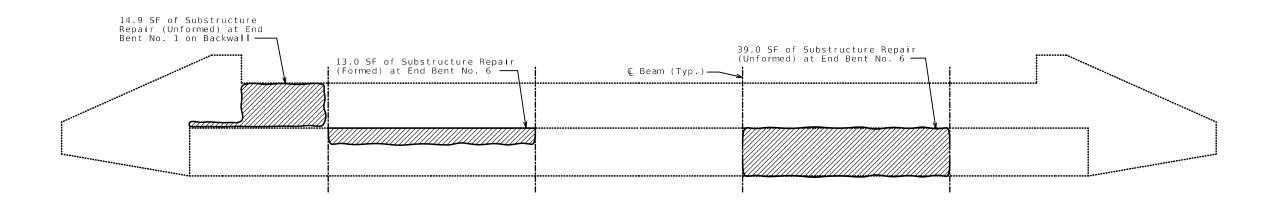
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SUBSTRUCTURE REPAIR

C Resin Anchor System C with 5/8" Ø threaded rod with flat washer & two heavy hex nuts at 12" cts. (buff threads to prevent loosening). All hardware for Resin Anchor System C shall be galvanized in accordance with ASTM A153.

Seal around head with urethane coating

Fill Face

System C (46 Req'd per end bent)

DETAILS OF RESIN ANCHOR SYSTEMS

SUBSTRUCTURE QUANTITY TABLE FOR END BE	NTS NO.	1 & 6
ITEM	Bent No. 1 Total	Bent No. 6 Total
Substructure Repair (Formed) sq. foo	t	13.0
Substructure Repair (Unformed) sq. foo	t 14.9	39.0

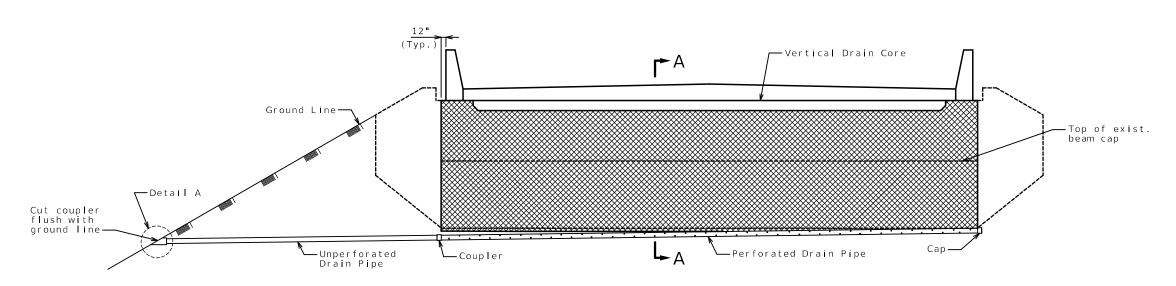
Notes: These quantities are included in the Estimated Quantities Table on Sheet No. 1.

10/01/2024 10/1/2024 US 169 MO BR 6 WORTH JNW0009 A04483

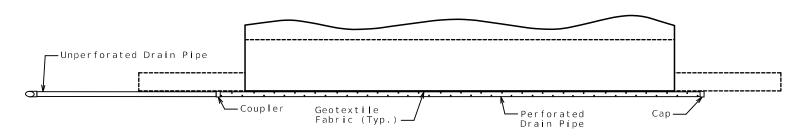
SenesceMain Plaza, 4435 Main St., Suite 115
7221-4222, FAX 913/441-1468

DETAILS OF END BENTS NO. 1 & 6

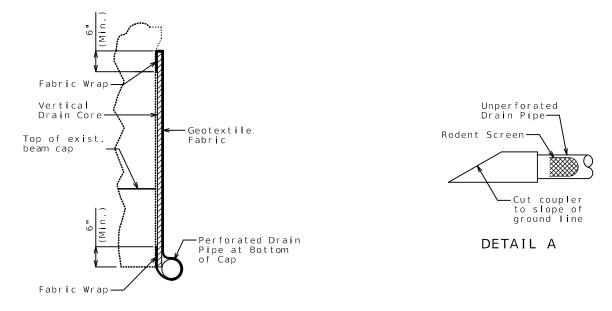
Designed MAR. 2024 Detailed APR. 2024 Checked JUL. 2024



ELEVATION OF END BENT



PLAN OF END BENT



PART SECTION A-A (Section thru wing similar)

Designed MAR. 2024 Detailed APR. 2024 Checked JUL. 2024

VERTICAL DRAIN AT END BENTS

(Squared end bent shown, skewed end bent similar)

Note: This drawing is not to scale. Follow dimensions. Sheet No. 7 of 14



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drain pipe.

General Notes:

All drain pipe shall be sloped 1 to 2

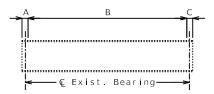
Drain pipe may be either 6-inch diameter corrugated metallic-coated steel pipe underdrain, 4-inch diameter corrugated polyvinyl chloride (PVC) drain pipe, or 4-inch diameter corrugated polyethylene (PE)

Drain pipe shall be placed at fill face of end bent and inside face of wings. The pipe shall slope to lowest grade of ground line, also missing the lower beam of end bent by a minimum of 1 1/2 inches.

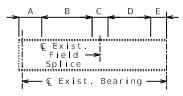
Perforated pipe shall be placed at fill face side and inside face of wings at the bottom of end bent and plain pipe shall be

used where the vertical drain ends to the exit at ground line.

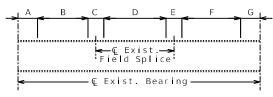
All excavation necessary for installation of vertical drain will be considered completely covered by the contract unit price for Vertical Drain at End Bents.



ELEVATION SHOWING SHEAR CONNECTOR SPACING FOR SPANS 1 & 5 BEAMS

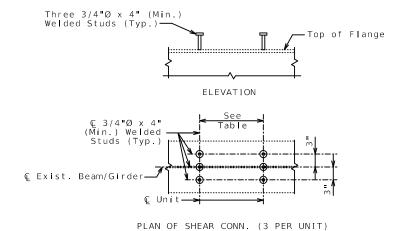


ELEVATION SHOWING SHEAR CONNECTOR SPACING FOR SPANS 2 & 4 GIRDERS



ELEVATION SHOWING SHEAR CONNECTOR SPACING FOR SPAN 3 GIRDERS

		TAB	LE SH	OWING SHEAR CON	INECTO	R UNIT SP	AC I NG			
	Beam/Girder	S.C. per unit	Α	В	С	D		Е	F	G
ſ	Beam (Spans 1-2 & 5-6)	3	4" <u>±</u>	49 Units @ 12"± cts.	4"±					
ſ	Girder (Spans 2-3 & 4-5)	3	6"±	44 Units @ 16" cts.	3'-9"	15 Units @ 1	6" cts.	3"		
Ī	Girder (Span 3-4)	3	13"±	14 Units @ 16" cts.	3'-7"	43 Units @ 1	6" cts.	3'-7"	14 Units @ 16" cts.	13"±
		•	•	_		Total	shear c	onnector	s required per girder	861





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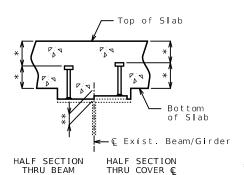
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US 169 MO

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SECTION THRU EXIST. BEAM/GIRDER SHOWING SHEAR CONNECTORS

Designed MAR. 2024 Detailed APR. 2024 Checked JUL. 2024 * 3" Minimum

** Min. Haunch = 0.00" Max. Haunch = 1.75" (Exterior Beams/Girders) Max. Haunch = 3.00" (Interior Beams/Girders)

The cost of suppl

The cost of supplying and installing shear connectors will be considered completely covered by the contract unit price for Shear Connectors.

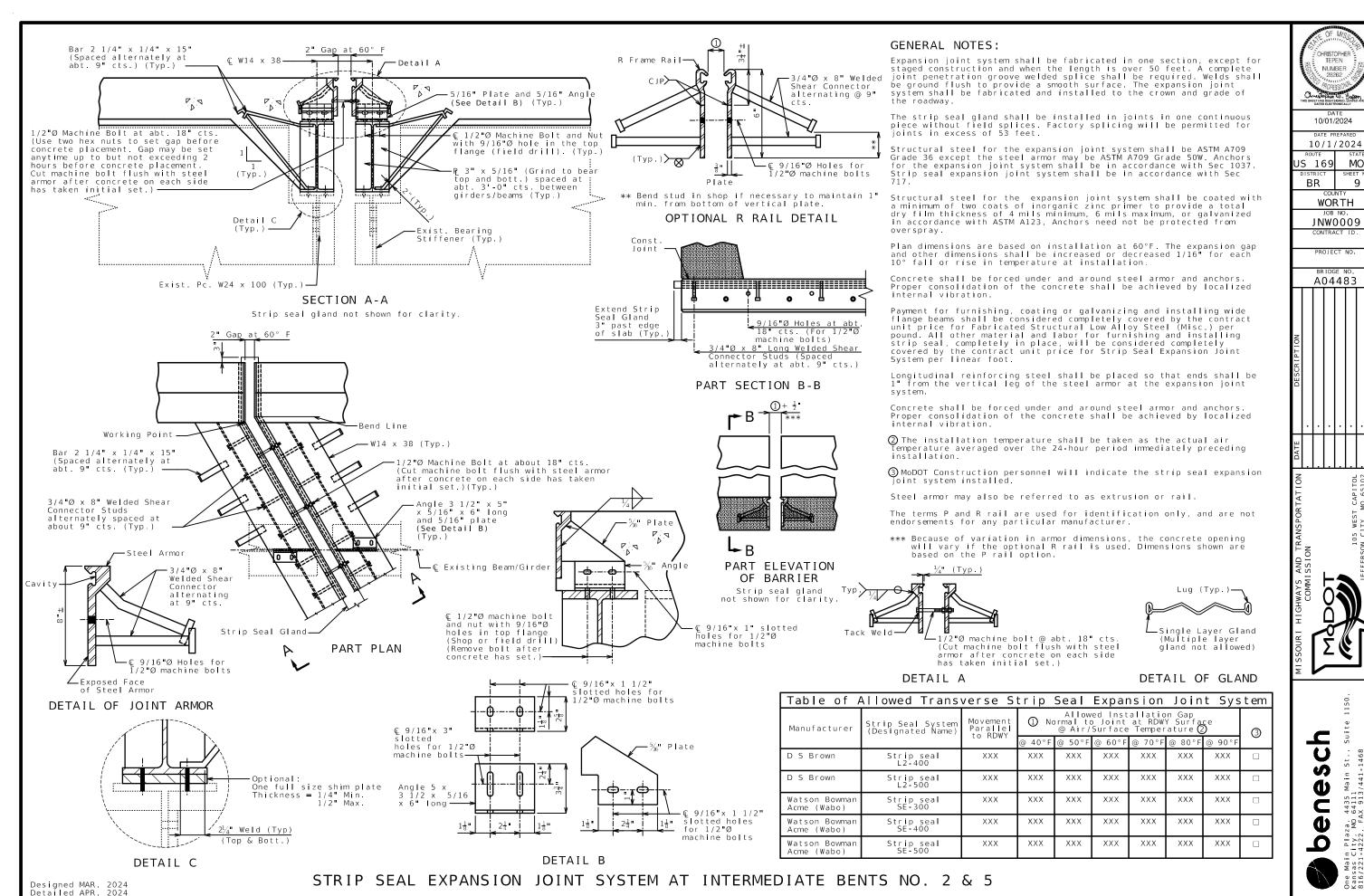
General Notes:

Shear connectors shall be in accordance with Sec 712, 1037 & 1080.

DETAILS OF SHEAR CONNECTORS

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 8 of 14



Note: This drawing is not to scale. Follow dimensions.

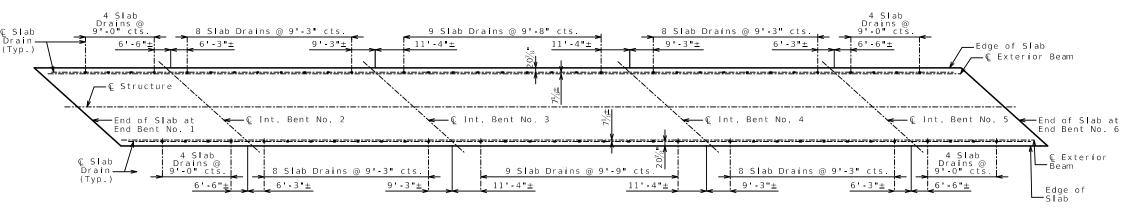
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Sheet No. 9 of 14

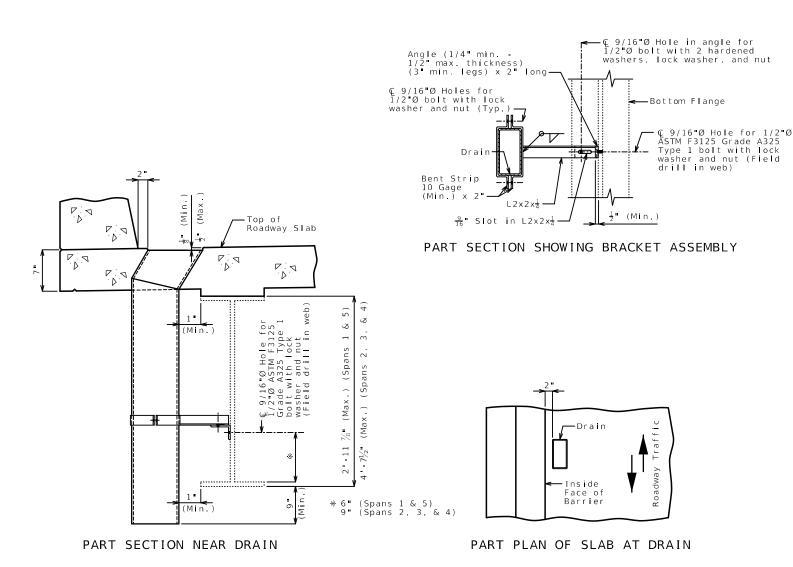
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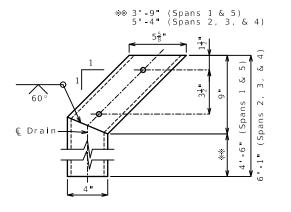


PLAN OF SLAB SHOWING SLAB DRAIN LOCATIONS

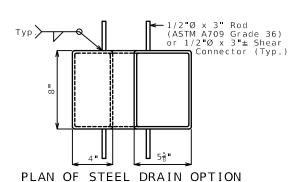


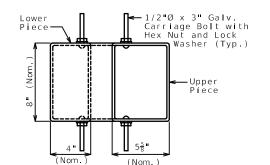
Note: This drawing is not to scale. Follow dimensions.

SLAB DRAINS



ELEVATION OF DRAIN





PLAN OF FRP DRAIN OPTION

General Notes:

Contractor shall have the option to construct either steel or FRP slab drains. All drains shall be of same type.

Slab drain bracket assembly shall be ASTM A709 Grade 36 steel.

Locate drains in slab by dimensions shown in Part Section Near Drain.

Reinforcing steel shall be shifted to clear drains.

The bracket assembly shall be galvanized in accordance with ASTM A123.

All bolts, hardened washers, lock washers and nuts shall be galvanized in accordance with AASHTO M 232 (ASTM A153), Class C.

All 1/2"Ø bolts shall be ASTM A307, except as shown.

Shop drawings will not be required for the slab drains and the bracket assembly.

The bolt hole for the bracket assembly attachment shall be shifted to the minimum extent necessary to field drill in the existing web.

Notes for Steel Drain:

Slab drains may be fabricated of either 1/4" welded sheets of ASTM A709 Grade 36 steel or from 1/4" structural steel tubing ASTM A500 or A501.

Outside dimensions of drains are 8" x 4".

The drains shall be galvanized in accordance with ASTM A123.

Notes for FRP Drain:

Drains shall be machine filament-wound thermosetting resin tubing meeting the requirements of ASTM D2996 with the following exceptions:

Shape of drains shall be rectangular with outside nominal dimensions of 8 " \times 4".

Minimum reinforced wall thickness shall be 1/4 ".

The resin used shall be ultraviolet (UV) resistant and/or have UV inhibitors mixed throughout. Drains may have an exterior coating for additional UV resistance.

The color of the slab drain shall be gray (Federal Standard 26373). The color shall be uniform throughout the resin and any coating used.

The combination of materials used in the manufacture of the drains shall be tested for UV resistance in accordance with ASTM D4329 Cycle A. The representative material shall withstand at least 500 hours of testing with only minor discoloration and without any physical deterioration. The contractor shall furnish the results of the required ultraviolet testing prior to acceptance of the slab drains.

At the contractor's option, drains may be field cut. The method of cutting FRP slab drain shall be recommended by the manufacturer to ensure a smooth, chip free cut.

Both upper and lower drain pieces shall be rigidly connected to each other. Drain flow shall not be obstructed. Approval of the engineer is required.



DATE 10/01/2024

10/1/2024

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US 169 MO
DISTRICT SHEET N
BR 10

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JOB NO.

JNW0009 CONTRACT ID.

PROJECT NO.

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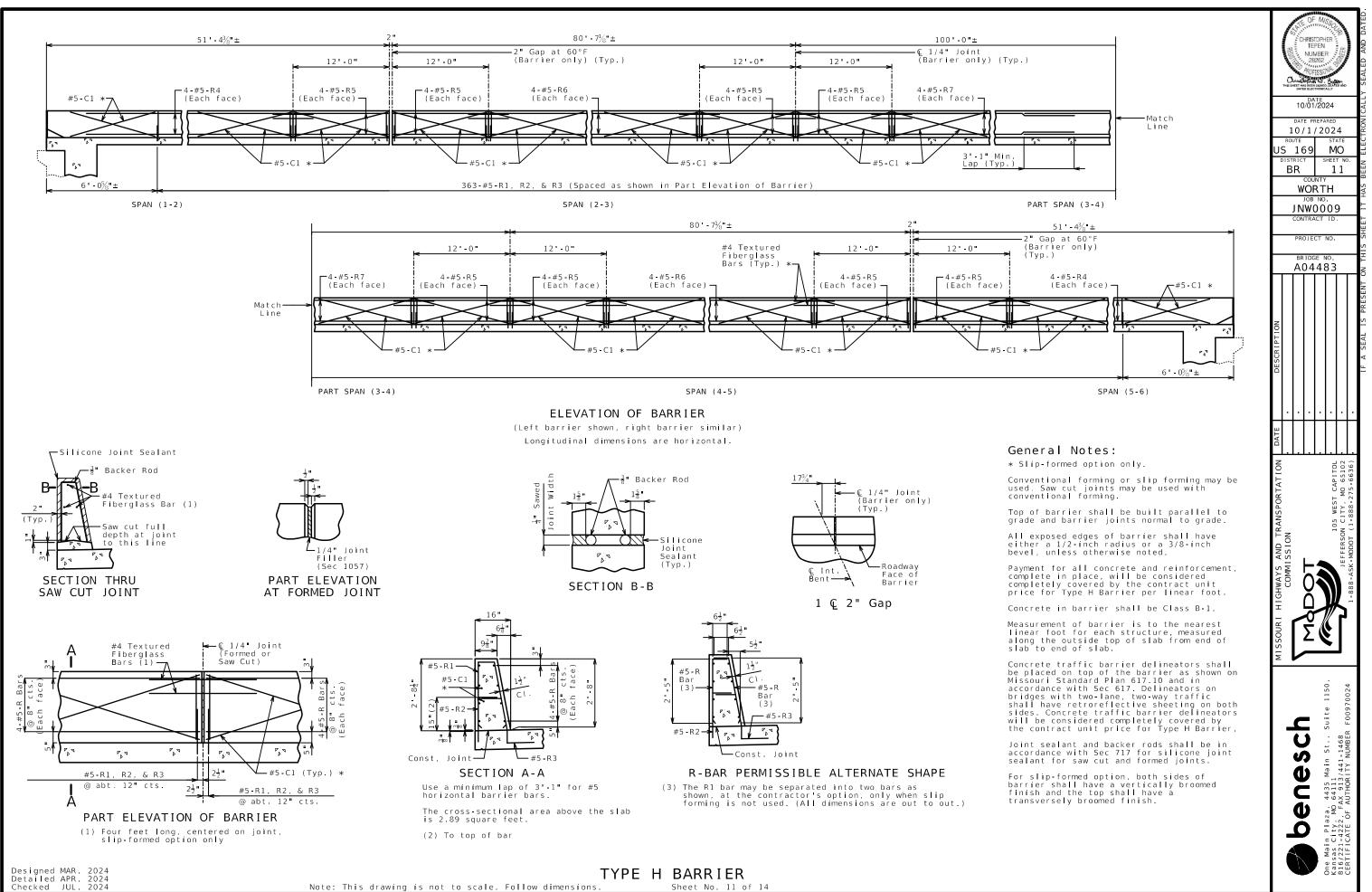
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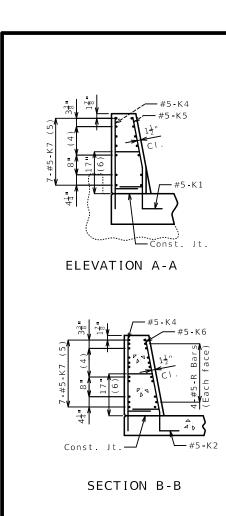
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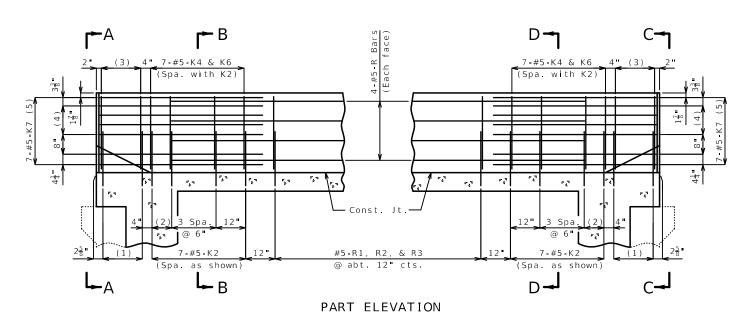
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Sheet No. 10 of 14

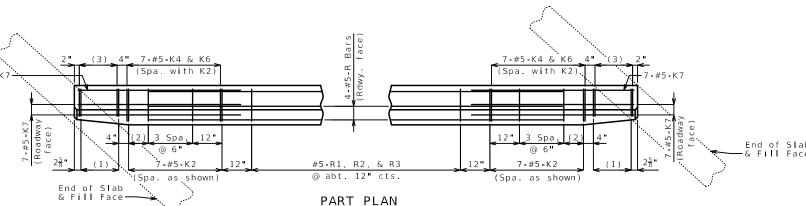


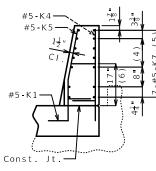
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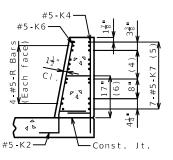


- (1) 5-#5-K1 @ 4" cts.
- (2) 2 Spaces @ 4"
- (3) 5-#5-K4 and 5-#5-K5, spaced with K1
- (4) 3 Spaces @ 3¹³₁₆"
- (5) Spaced as shown, each face
- (6) To top of bar

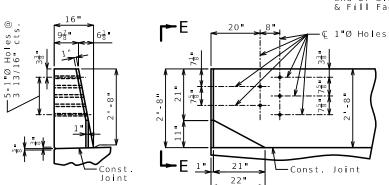


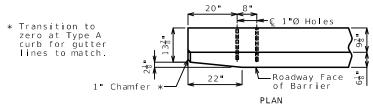


ELEVATION C-C



SECTION D-D





ELEVATION

DETAILS OF GUARD RAIL ATTACHMENT

General Notes:

Concrete traffic barrier delineators shall be placed on top of the barrier as shown on Missouri Standard Plan 617.10 and in accordance with Sec 617. Delineators on bridges with two-lane, two-way traffic shall have retroreflective sheeting on both sides. Concrete traffic barrier delineators will be considered completely covered by the contract unit price for Type H Barrier.

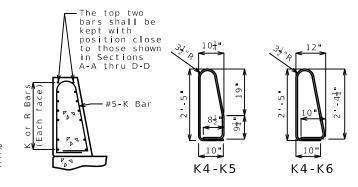
Reinforcing Steel:

Minimum clearance to reinforcing steel shall be 1 1/2".

Use a minimum lap of 3'-1" between K7 bars and R bars.

TYPE H BARRIER AT END BENTS

(Left barrier shown, right barrier similar)



PERMISSIBLE ALTERNATE SHAPES

(Other K bars not shown for clarity)

The K4-K5 and K4-K6 bar combination may be furnished as one bar as shown, at the contractor's option.

All dimensions are out to out.

MISSOURI HIGHWAYS AND TRANSPORTATION DATE

COMMISSION

1150,

105 WEST CAPITOL

1 S88 - ASK - WOODT (1-888-275-6636)

10/01/2024

DATE PREPARED

10/1/2024

US 169 MO

WORTH

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A04483

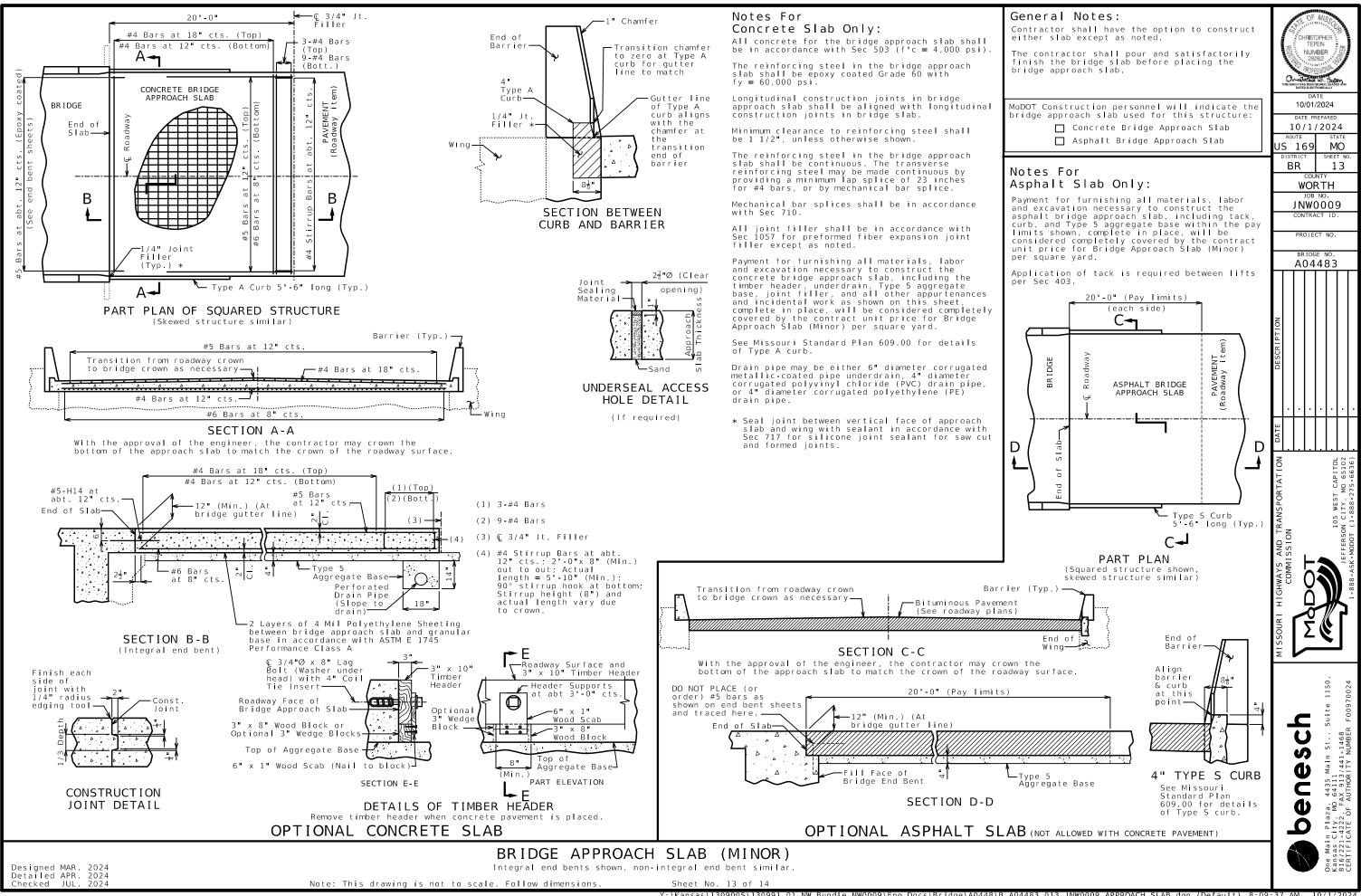
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BR

benesch ne Main Plaza, 4435 Main St., Suit ansas City, Mo 64111, 1468

Designed MAR. 2024 Detailed APR. 2024 Checked JUL. 2024

ELEVATION E-E



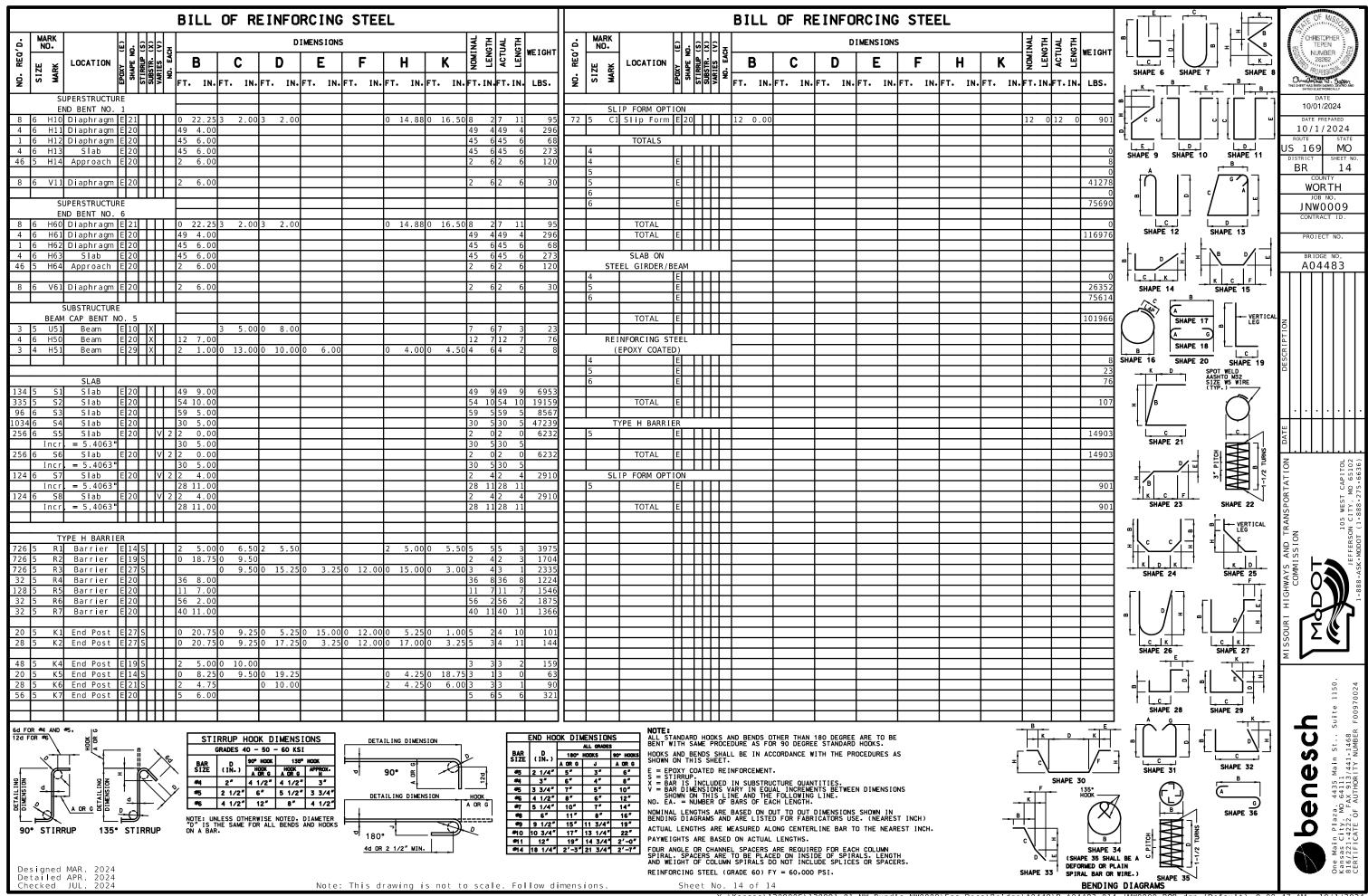


Table Showing S2 Bar Lengths Int. Bent No. 2 Int. Bent No. 3 Span 1 Span 2 Span 2 Span 3 21'-0" 27'-0" 27'-0" 21'-0"

Required Lap Length For Bar Splices ** Bar Size Splice Length 2'-7" 3'-3" 3'-10" 4'-11"

** Unless otherwise shown

General Notes:

Design Loading:

Joint Filler:

Reinforcing Steel:

Miscellaneous:

new material

Design Specifications:

Design Unit Stresses:

H15-44 (1961) (Existing) HS20-44 (New Construction)

Fatigue Stress - Case III

Class B-1 Concrete (Barrier)

Reinforcing Steel (Grade 60)

slab surface. (Roadway item)

mortar in accordance with Sec 704.

Structural Steel Protective Coating (Top Flange):

high water line (Roadway item)

lines indicate new work.

35 lb/sf Future Wearing Surface Earth - 120 lb/cf, Equivalent Fluid Pressure 45 lb/cf

except Barrier)

All joint filler shall be in accordance with Sec 1057 fo

preformed sponge rubber expansion and partition joint filler, except as noted.

Minimum clearance to reinforcing steel shall be 1 1/2", unless

Bars bonded in existing concrete not removed shall be cleanly stripped and embedded into new concrete where possible. If length is available, existing bars shall extend into new concrete at least 40 diameters for plain bars and 30 diameters for deformed bars, unless otherwise noted.

Roadway surfacing adjacent to bridge ends shall match new bridge

Contractor shall verify all dimensions in field before ordering

The area exposed by the removal of concrete and not covered with new concrete shall be coated with an approved qualified special

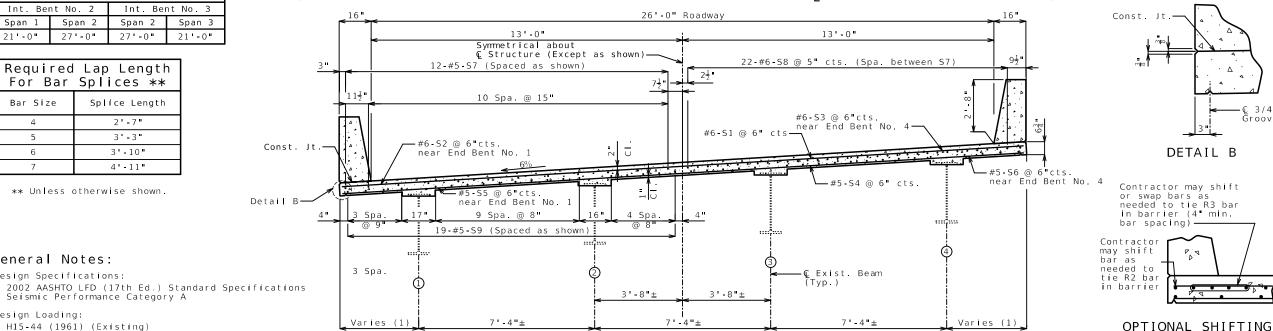
Rubblized concrete from the existing bridge deck that qualifies as clean fill may be placed on spill slopes at end bents above ordinary

For adjusted beam deflection due to the weight of the new deck and barriers, see Bridge Electronic Deliverables.

Outline of existing work is indicated by light dashed lines. Heavy

Class B-2 Concrete (End Bents & Superstructure,

U.I.P. & REDECK EXISTING (70'- 90'- 70') CONTINUOUS COMPOSITE WIDE FLANGE BEAM SPANS (SKEW: 7°59'31" L.A. TO PERPENDICULAR LINE FROM TANGENT TO ¢ ROADWAY AT STA. 322+16.20)



TYPICAL SECTION THRU SLAB

		(1) Overhang Widths Measured Perpendicular to ♀ Beam and Distances Along ♀ Beam						
Ī	Span	Location	Beam 1*	Beam 1**	Beam 4*	Beam 4**		
[© Brg @ EB1	0'-0"	3'-3"±	0'-0"	3'-4"±		
Π		0.1	7'-0 1 6"±	3'-1 5 "±	7'-0"±	3'-5 15 "±		
I		0.2	$14 - 0\frac{3}{16} \pm$	3 - 0 ¹ / ₄ ±	$14' - 0\frac{1}{16}" \pm$	3 - 7 - 5 " ±		
I	—	0.3	$21' - 0\frac{1}{4}" \pm$	$2' - 11\frac{13}{16}" \pm$	21'-0 1 6"±	3 -8 1 ±		
I		0.4	$28' - 0\frac{5}{16}" \pm$	3'-0"±	28 - 0 1 ±	3'-8 3 "±		
I	Z	0.5	35 - 0 8 ±	3'-0 13 "±	35'-0 1 "±	3'-7 11 "±		
I	SPAN	0.6	42 - $0\frac{1}{2}$ ±	3'-2 5 "±	42'-0 1 8"±	3'-6 8 "±		
I	SF	0.7	49'-0 9 "±	3 - 4 8 ±	$49' - 0\frac{3}{16}" \pm$	3 ' - 4 15 "±		
I		S1	50'-0"±	$3 - 4\frac{3}{4} \pm$	50'-0"±	3'-4 8 "±		
I		0.8	$56' - 0\frac{5}{8}" \pm$	3 - 3 ¹ / ₄ "±	$56 - 0\frac{1}{8}$ ±	$3 - 5\frac{11}{16}$ ±		
Į.		0.9	63 -0 11 ±	3'-2 16 "±	$63 - 0\frac{3}{16}$ ±	$3 - 6\frac{5}{16}$ ±		
ļ		© Brg @ IB2	0 0	3 - 1½" ±	0'-0"	$3 - 6\frac{3}{8} \pm$		
I		0.1	9 $-0\frac{1}{8}$ ±	$3 - 1\frac{11}{16} \pm$	8 -11 15 ±	3'-5½"±		
I		0.2	$18 - 0\frac{1}{4}$ ±	3'-2 ⁷ / ₈ "±	17'-11 \f g"±	3'-3 5 "±		
		S2	19 - 11 ³ / ₄ "±	3 - 3 ¹ / ₄ "±	$19 - 11\frac{3}{16} \pm$	3'-3½"±		
	7	0.3	$27 - 0\frac{5}{16}$ ±	3'-1½"±	26'-11 8 "±	$3 - 5\frac{1}{4}$ " ±		
I	Z	0.4	$36 \cdot 0\frac{7}{16}$ ±	$3 - 0\frac{5}{16} \pm$	$35 \cdot 11\frac{3}{4}$ ±	$3 - 7\frac{1}{16} \pm \frac{1}{16}$		
I	SPAN	0.5	45' - 0 ⁹ / ₁₆ " ±	3 - 0 = ±	$44 - 11\frac{11}{16} \pm$	$3 - 7\frac{7}{8} \pm$		
I	у.	0.6	54 - 0 ¹¹ / ₁₆ "±	3 -1 ±	53 - 11 5 ±	3 - 7 11 ±		
	0,	0.7 S3	$63 - 0\frac{13}{16} \pm 69 - 11\frac{3}{4} \pm$	$3' - 2\frac{7}{8}" \pm$	$62' - 11\frac{9}{16}" \pm$	$3' - 6\frac{1}{2}" \pm 3' - 4\frac{15}{16}" \pm$		
I		0.8	$72 - 0\frac{15}{16} \pm$	3'-5 1 6"±	$69' - 11\frac{3}{16}" \pm 71' - 11\frac{1}{2}" \pm$			
I		0.8	$81 - 1\frac{1}{16} \pm$	$3' - 4\frac{7}{16}" \pm 3' - 2\frac{3}{8}" \pm$	$80 - 11\frac{7}{16} \pm$	$3' - 5\frac{5}{16}" \pm 3' - 6\frac{1}{4}" \pm$		
ŀ		C Brg @ IB3	0'-0"	3 - 1 ³ / ₈ "±	0'-0"	$3 - 6\frac{1}{4} \pm$		
ŀ		0.1	7'-0"±	3'-1 5 "±	6 - 11½"±	3'-5 16 "±		
I		0.2	14'-0"±	3 - 1 = ±	$13 - 11\frac{3}{4}$ ±	$3 - 4\frac{1}{4} \pm$		
I		S4	19 11 7 ±	3'-2 13 "±	19 - 10 ⁵ / ₈ ±	3'-2 5 "±		
I	Ω	0.3	20'-11 15 "±	3'-2 1 6"±	20'-11 \f 8"±	$3 - 3\frac{1}{16} = \pm$		
I	z	0.4	27'-11 15 "±	3'-1"±	27 - 11½"±	3'-5 1 "±		
I	SPAN	0.5	34'-11 ¹⁵ / ₁₆ "±	3 ' - 0 1 ±	34'-11 5 "±	$3 - 6\frac{13}{16} \pm$		
	P.	0.6	$41 - 11\frac{15}{16} \pm$	$2 - 11\frac{13}{16} \pm$	$41' - 11\frac{3}{16}" \pm$	3 ' - 7 13 "±		
	0,	0.7	48'-0"±	3'-0 8 "±	$48' - 11\frac{1}{16}" \pm$	3'-8 3 "±		
I		0.8	55'-0"±	3 - 1 1 ±	55 - 10 15 ±	3'-8"±		

Note: * Length Along © Beam from © Bearings ** Overhang Widths Measured Perpendicular to © Beam

Estimated Quantities Total emoval of Existing Bridge Deck sa. foo 6,173 Bridge Approach Slab (Minor) sq. yar 118 Flowable Backfill cu. yar 740 Slab on Steel sq. yard Type H Barrier 493 inear foo Substructure Repair (Formed) sa foo 12 Substructure Repair (Unformed) sq. foo 14 Slab Drain 20 eac Non-Destructive Testing linear foo 58 Vertical Drain at End Bents eac Open Cell Foam Joint Seal linear foo 53

SEC/SUR 31

DETAIL B

TOP BARS AT BARRIER

TWP 65 N

3/4" Drip

Groove (Typ.)

RGE 30 W

Cost of any required excavation for bridge will be considered completely covered by the contract unit price for other items

*Concrete Bridge Approach Slab only. See Special Provisions.

(1) See Table Below

HALF SECTION NEAR INT. BENT

Estimated Quantities for Slab on S	teel
I t em	Total
Class B-2 Concrete cu. yar	18
Reinforcing Steel (Epoxy Coated) poun	59,180

The table of Estimated Quantities for Slab on Steel represents the quantities used by the State in preparing the cost estimate for concrete slabs. The area of the concrete slab will be measured to the nearest square yard longitudinally from end of slab to end of slab and transversely from out to out of bridge slab (or with the horizontal dimensions as shown on the plan of slab). Payment for stay-in-place corrugated steel forms, conventional forms, all concrete and epoxy coated reinforcing steel will be considered completely covered by the contract unit price for the slab. Variations may be encountered in the estimated quantities but the variations cannot be used for an adjustment in the

Method of forming the slab shall be in accordance with Sec 703. All hardware for forming the slab to be left in place as a permanent part of the structure shall be coated in accordance with ASTM A123 or ASTM B633 with a thickness Class SC 4 and a finish Type I, II, or III.

Slab shall be cast-in-place with conventional forming or stay-in-place corrugated steel forms. Precast prestressed panels will not be permitted.

For Optional Stay-In-Place Form Details, see Sheet No. 2.

REPAIRS TO BRIDGE: ROUTE M OVER EAST FORK OF GRAND RIVER

ROUTE M FROM ROUTE N TO ROUTE C ABOUT 1.4 MILES EAST OF ROUTE N BEGINNING STATION 322+16.20± (MATCH EXISTING)

(1) Overhang Widths Measured Perpendicular to © Beam and Distances Along © Beam							
Span Location		Beam 1*	Beam 1**	Beam 4*	Beam 4**		
© Brg @ EB1		0'-0"	3'-3"±	0'-0"	3'-4"±		
	0.1	7'-0 1 6"±	3 - 1 - 1 - ±	7'-0"±	3'-5 15 "±		
	0.2	$14 - 0\frac{3}{16}$ ±	3'-0 1 "±	14'-0 1 6"±	3'-7 5 "±		
١.	0.3	21'-0 1 "±	2'-11 13 "±	21 - 016 ±	3'-8 1 0"±		
-	0.4	28' - 0 5 "±	3'-0"±	28'-0 ¹ / ₈ "±	3'-8 3 "±		
SPAN	0.5	35'-0 3 "±	3'-0 13 "±	35'-0 ¹ / ₈ "±	3 - 7 11 ±		
∢	0.6	42'-0 1 "±	3 ' - 2 5 " ±	42'-0 1 8"±	3'-6 8 "±		
I %	0.7	49'-0 9 "±	3'-4 3 "±	49'-0 3 "±	3 ' - 4 15 " ±		
"	S1	50'-0"±	3'-4 3 "±	50'-0"±	3 ' - 4 5 " ±		
	0.8	56'-0 8 "±	3 ' - 3 1 " ±	56'-0 8 "±	3'-5 11 "±		
	0.9	63'-0 11 "±	3'-2 1 6"±	63'-0 3 "±	3'-6 5 "±		
	© Brg @ IB2	0'-0"	3 1½ ±	0'-0"	3 - 6 3 ±		
	0.1	9 - 0 ¹ / ₈ "±	3 ' - 1 11 ' ±	8 ' - 11 15 "±	$3' - 5\frac{1}{2}" \pm$		
	0.2	$18 - 0\frac{1}{4}$ ±	$3 - 2\frac{7}{8}$ ±	$17 - 11\frac{7}{8}$ ±	3 - 3 5 ±		
	S2	$19 - 11\frac{3}{4}$ " ±	3'-3 ¹ / ₄ "±	$19 - 11\frac{3}{16} \pm$	3'-3 ⁸ "±		
7	0.3	$27 - 0\frac{5}{16}$ ±	$3' - 1\frac{1}{2}" \pm$	$26 - 11\frac{7}{8}$ ±	$3' - 5\frac{1}{4}" \pm$		
-	0.4	36'-0 7 "±	3 -0 5 ±	$35 - 11\frac{3}{4}$ "±	$3 - 7\frac{1}{16} \pm$		
SPAN	0.5	45 0 9 ±	3 - 0 = ±	$44' - 11\frac{11}{16}" \pm$	$3 - 7\frac{7}{8}$ ±		
ا م	0.6	$54 - 0\frac{11}{16}$ ±	3'-1"±	53 11 8 ±	3'-7 11 "±		
١٥	0.7	63 013 ±	3 - 2 ⁷ / ₈ ±	62'-11 9 "±	$3' - 6\frac{1}{2}" \pm$		
	S3	69 - 11 3 "±	$3 - 5\frac{1}{16} \pm$	$69 - 11\frac{3}{16}$ ±	3 4 15 ±		
	0.8	72 $0\frac{15}{16}$ ±	3 - 4 ⁷ / ₁₆ ±	$71 - 11\frac{1}{2} \pm $	$3 - 5\frac{5}{16}$ ±		
	0.9	81 -1 ¹ / ₁₆ ±	3 - 2 3 ±	$80 - 11\frac{7}{16} \pm$	$3 - 6\frac{1}{4} \pm$		
	© Brg ⊚ IB3	0'-0"	$3 - 1\frac{3}{8} \pm$	0'-0"	$3 - 6\frac{1}{4} \pm$		
	0.1	7'-0"± 14'-0"±	3 - 1 - 5 = ±	6'-11 ⁷ / ₈ "±	3'-5 9 "±		
	54	$14 - 0 \pm 19 - 11\frac{7}{8} \pm$	$3 - 1\frac{7}{8} \pm$	$13 - 11\frac{3}{4}$ "± $19 - 10\frac{5}{8}$ "±	$3 - 4\frac{1}{4} \pm 3 - 2\frac{5}{8} \pm$		
m	0.3		3'-2 13 "±	$19 - 10\frac{1}{8} \pm 20 - 11\frac{1}{8} \pm 10$			
-	0.3	$20' - 11\frac{15}{16}" \pm 27' - 11\frac{15}{16}" \pm$	3'-2 ⁹ / ₁₆ "± 3'-1"±	$20 \cdot 11\frac{1}{8} \pm 27 \cdot 11\frac{1}{7} \pm$	$3' - 3\frac{1}{16}" \pm 3' - 5\frac{1}{4}" \pm$		
₹	0.5	$34 - 11\frac{16}{16} \pm$	3 0 1 ±	$34 - 11\frac{5}{16} \pm$	$3 - 6\frac{13}{16} \pm$		
SPAN	0.6	$41 - 11\frac{15}{16} \pm$	$2 - 11\frac{13}{16} \pm$	$41 - 11\frac{3}{16} \pm$	3 7 13 ±		
l s	0.7	48 -0 ±	$3 - 0\frac{1}{8} \pm$	$48 - 11\frac{1}{16} \pm$	$3' - 8\frac{3}{16}" \pm$		
	0.8	55'-0"±	$3 - 1\frac{1}{16}$ ±	$\frac{-6}{55} - 10\frac{15}{16} \pm$	3'-8"±		
	0.9	62'-0"±	3 - 2 5 ±	$62' - 10\frac{13}{6}" \pm$	3 7 13 ±		
	C Brg @ EB4	0'-0"	3'-4 8 "±	0'-0"	$3' - 5\frac{3}{4}" \pm$		
	1 1 - 3 3 2 2 3 1 0 0 1 3 3 4 -						

HALF SECTION NEAR MIDSPAN AND END BENT

Structure to be closed during construction. Traffic to be maintained on other routes during construction. See roadway plans for traffic control.

In accordance with Sec. 216.50 and 1081, the top, and additionally the sides, and bottom of the top flange shall be coated with not less than 3.0 mils of Gray Epoxy Mastic-Primer (non-aluminum) applied over an SSPC-SP3 surface preparation. Payment for coating steel will be considered completely covered by the contract sq. foot price for Removal of Existing Bridge Deck.

Detailed OCT. 2023 Checked APR 2024

Traffic Handling:

Note: This drawing is not to scale. Follow dimensions

f'c = 4,000 psi

f'c = 4,000 psi

fv = 60,000 psi

Sheet No. 1 of 12

PROJECT NO A10481

NUMBER

10/01/2024

10/1/2024

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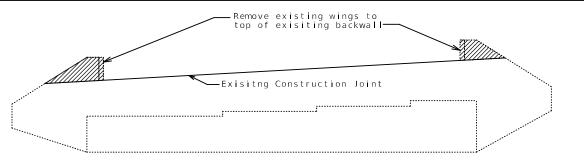
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DETAILS OF CONCRETE REMOVAL AT END BENT NO. 1

DETAILS OF CONCRETE REMOVAL AT END BENT NO. 4

The cost of concrete removal as shown will be considered completely covered by the contract unit price for Removal of Existing Bridge Deck. Vertical backwall and wingwall reinforcement to be cut off one inch below concrete removal surface and the resulting holes shall be filled with a qualified special mortar.

A smooth, level surface shall be provided at End Bents No. 1 & 4 removal lines.

General Notes:

Stay-In-Place Forms:

Corrugated steel forms, supports, closure elements and accessories shall be in accordance with grade requirement and coating designation G165 of ASTM A653. Complete shop drawings of the permanent steel deck forms shall be required in accordance with Sec 1080.

Corrugations of stay-in-place forms shall be filled with an expanded polystyrene material. The polystyrene material shall be placed in the forms with an adhesive in accordance with the manufacturer's recommendations.

Form sheets shall not rest directly on the top of beam flanges. Sheets shall be securely fastened to form supports with a minimum bearing length of one inch on each end. Form supports shall be placed in direct contact with the flange. Welding on or drilling holes in the beam flanges will not be permitted. All steel fabrication and construction shall be in accordance with Sec 1080 and 712. Certified field welders will not be required for welding of the form

The design of stay-in-place corrugated steel forms is per manufacturer which shall be in accordance with Sec 703 for false work and forms. Maximum actual weight of corrugated steel forms allowed shall be 4 psf assumed for beam loading.

The contractor shall provide a method of preventing the direct contact of the stay in-place forms and connection components with uncoated weathering steel members that is approved by the engineer.

Pouring and Finishing Slab:

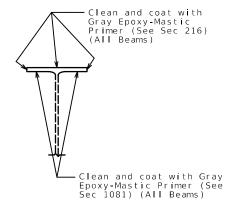
The contractor shall provide bracing necessary for lateral and torsional stability of the beams during construction of the concrete slab and remove the bracing after the slab has attained 75% design strength. Contractor shall not weld on or drill holes in the beams. The cost for furnishing, installing, and removing bracing will be considered completely covered by the contract unit price for Slab on

Slab shall be poured upgrade from end to end at a minimum rate of 25 cubic yards per hour.

Alternate pour sequences may be submitted to the engineer for approval. Keyed construction joints shall be provided between pours.

Haunching:

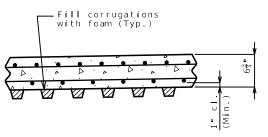
(1) Slab is to be considered a uniform thickness as shown on the plans. Haunching will vary. See front sheet for slab thickness.



TYPICAL SECTION THRU BEAM SHOWING PROTECTIVE COATING



- Remove the Connection Plate attachment (Typ.)



Steel corrugated bridge form

support

SECTION A-A

OPTIONAL STAY-IN-PLACE FORM DETAILS

BENT NO. 4 CONNECTION PLATE REMOVAL

Removal of connection plate to be considered completely covered by contract unit price for Removal of Existing Bridge Deck.

REHAB DETAILS

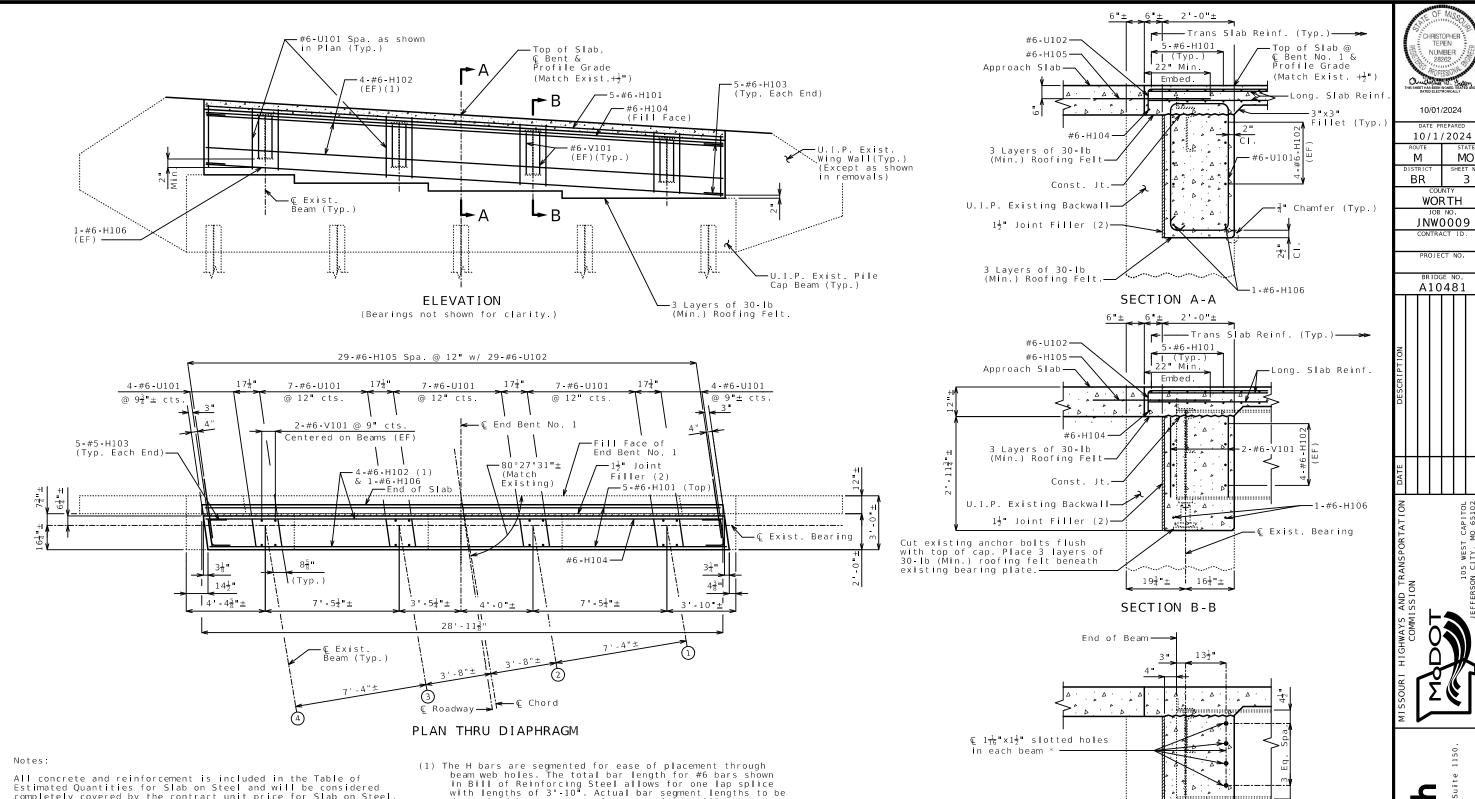
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PROJECT NO

A10481

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All concrete and reinforcement is included in the Table of Estimated Quantities for Slab on Steel and will be considered completely covered by the contract unit price for Slab on Steel

All concrete in diaphragm shall be Class B-2.

For details and reinforcement of Type H Barrier not shown, see Sheet No. 11.

EF = Each Face

* Cost of field drilling holes in existing wide flange beam webs will be considered completely covered by the contract unit price

Note: This drawing is not to scale. Follow dimensions.

- beam web holes. The total bar length for #6 bars shown in Bill of Reinforcing Steel allows for one lap splice with lengths of 3'-10". Actual bar segment lengths to be determined by contractor for ease of installing bars.

 The contractor may use a mechanical bar splice in lieu of a lap splice. When a mechanical bar splice is used, the actual bar segment length will be determined by the contractor to accommodate manufacturer's recommendations for installation and ease of construction. The cost of furnishing and installing the bar splices will be considered completely covered by the contract unit price for Reinforcing Steel. No adjustment of the quantity of reinforcing steel will be allowed for the use of mechanical bar splices
- (2) Preformed Flexible Foam Expansion Joint Filler, See Special Provisions

END BENT NO. 1 DETAILS

Detailed OCT. 2023 Checked APR. 2024

 $1\frac{1}{2}$ " Joint Filler (2)-

3 Layers of 30-1b (Min.) Roofing Felt.

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Front Face of

· © Exist Bearing

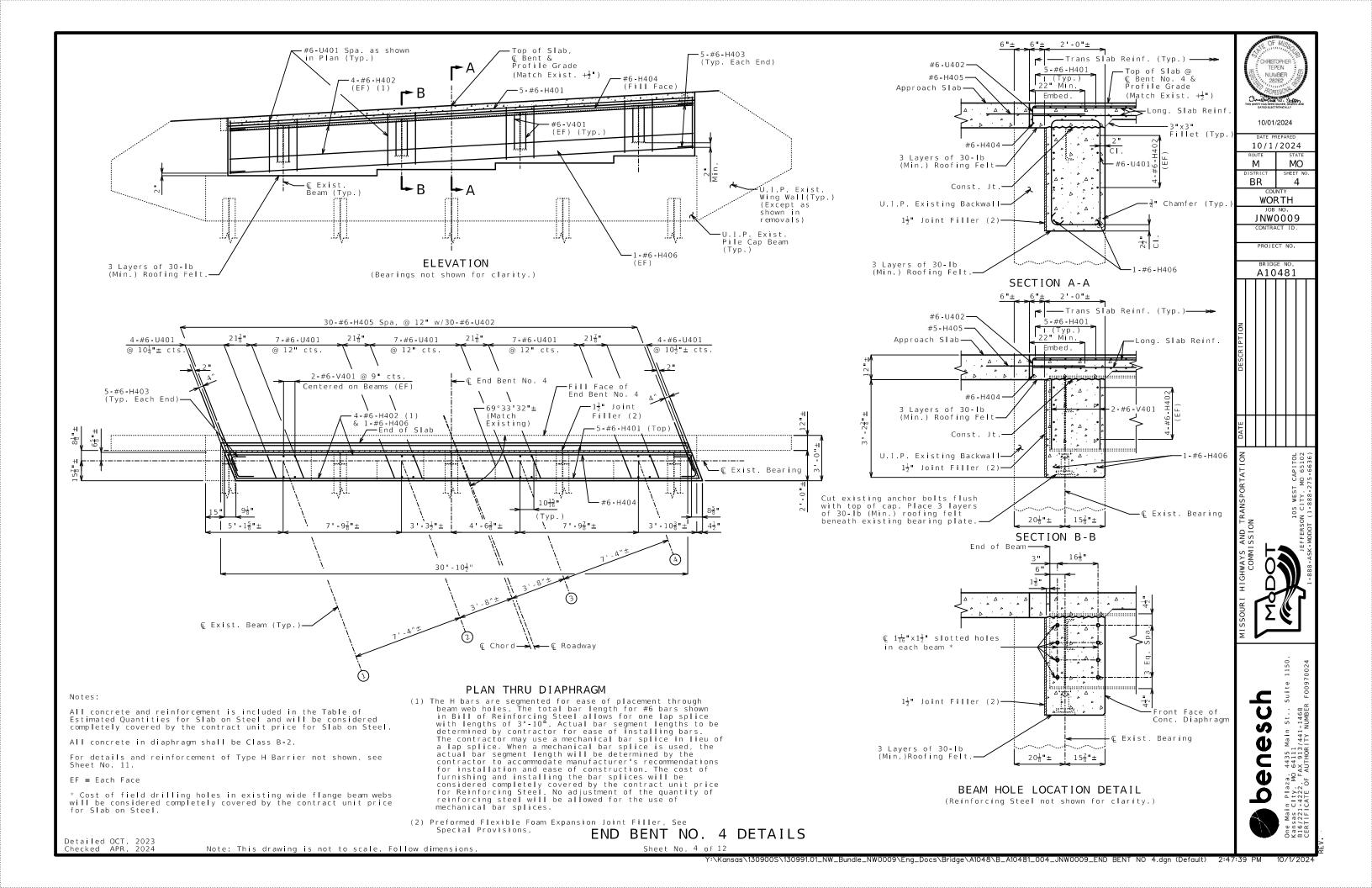
16¹/₄"±

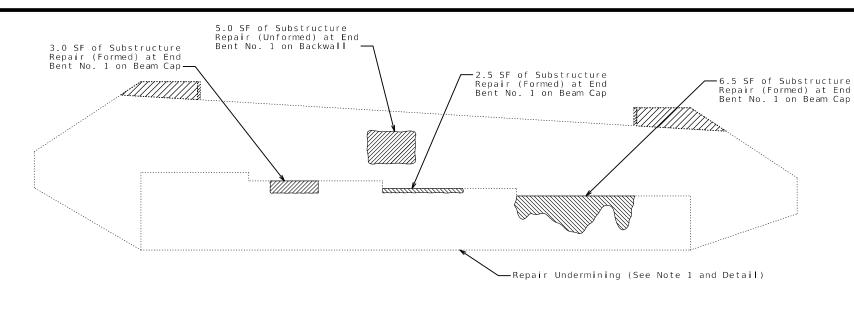
 $19\frac{3}{4}$ " ±

BEAM HOLE LOCATION DETAIL

(Reinforcing Steel not shown for clarity.)

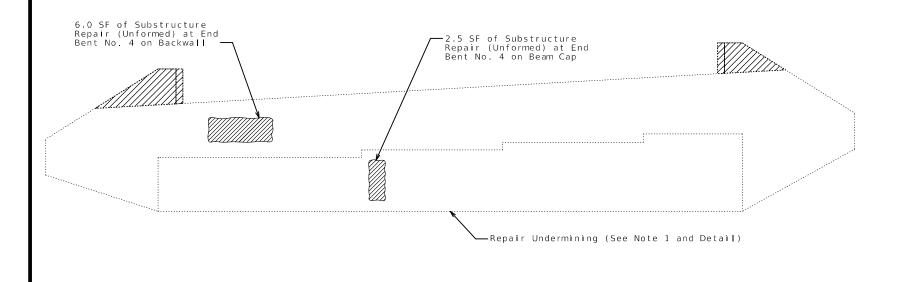
Conc. Diaphragm







END BENT NO. 1 SUBSTRUCTURE REPAIR





END BENT NO. 4 SUBSTRUCTURE REPAIR

REPAIR DETAILS OF END BENTS NO. 1 & 4

Detailed OCT. 2023 Checked APR. 2024 Note: This drawing is not to scale. Follow dimensions.

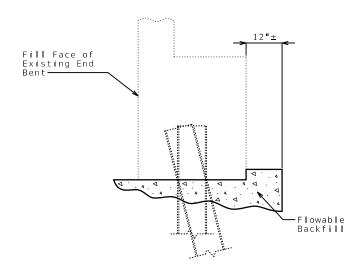
Sheet No. 5 of 12

SUBSTRUCTURE QUANTITY TABLE FOR END BENTS NO. 1 & 4

I t em	End Bent No.1 Total	End Bent No.4 Total	
Substructure Repair (Formed)	sq. foot	12.0	
Substructure Repair (Unformed)	sq. foot	5.0	8.5
Flowable Backfill	cu. yard	1.5	1.5

Notes:

- Repair undermining below pile cap using grout or flowable fill.
 Cost for all labor and materials to be paid for as
 Flowable Backfill.
- 2. These quantities are included in the Estimated Quantities Table on Sheet No. 1.
- All repairs to beam cap and backwall shall be completed prior to diaphragms being installed at end bents.



UNDERMINING REPAIR DETAIL



10/01/2024

DATE PREPARED 10/1/2024

ROUTE STATE

M MO

DISTRICT SHEET NO.

BR 5

COUNTY WORTH

JNW0009

PROJECT NO.

BRIDGE NO.
A10481

TRANSPORTATION DATE

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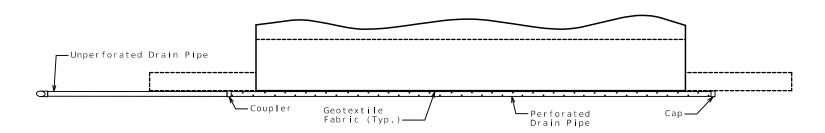
105 WEST CAPITOL

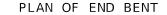
SSOURT HIGHWAYS AND TRANSPOON COMMISSION

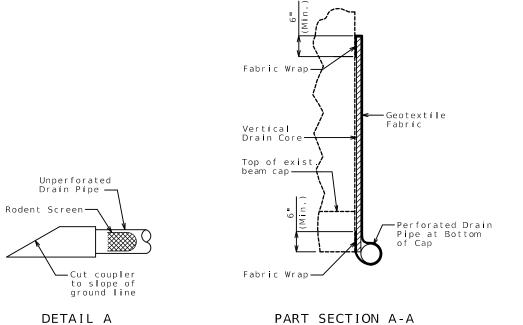
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Main Plaza, 4435 Main St., Suite 115

ELEVATION OF END BENT







PART SECTION A-A (Section thru wing similar)

General Notes:

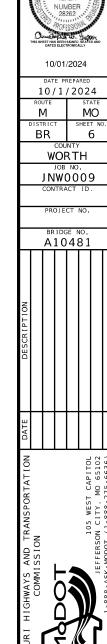
All drain pipe shall be sloped 1 to 2 percent.

Drain pipe may be either 6-inch diameter corrugated metallic-coated steel pipe underdrain, 4-inch diameter corrugated polyvinyl chloride (PVC) drain pipe, or 4-inch diameter corrugated polyethylene (PE) drain pipe.

Drain pipe shall be placed at fill face of end bent and inside face of wings. The pipe shall slope to lowest grade of ground line, also missing the lower beam of end bent by a minimum of 1 1/2 inches.

Perforated pipe shall be placed at fill reriorated pipe shall be placed at fill face side and inside face of wings at the bottom of end bent and plain pipe shall be used where the vertical drain ends to the exit at ground line.

All excavation necessary for installation of vertical drain will be considered completely covered by the contract unit price for Vertical Drain at End Bents.



Suite C S U C Ф 2

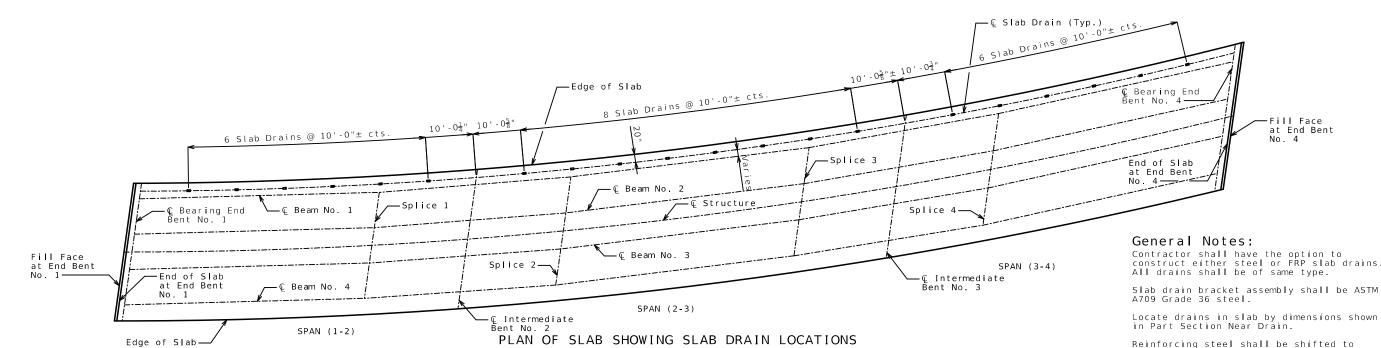
VERTICAL DRAIN AT END BENTS

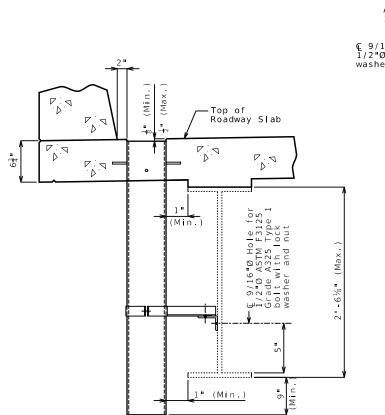
(Squared end bent shown, skewed end bent similar)

Detailed OCT. 2023 Checked APR. 2024

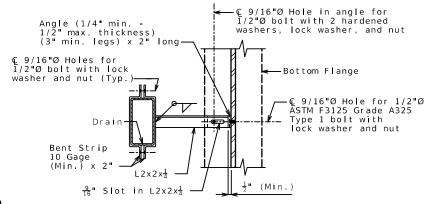
Note: This drawing is not to scale. Follow dimensions.

Sheet No. 6 of 12

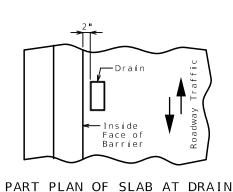




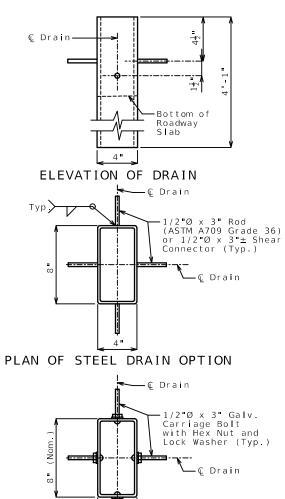
PART SECTION NEAR DRAIN

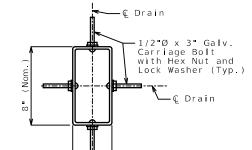


PART SECTION SHOWING BRACKET ASSEMBLY



SLAB DRAINS





PLAN OF FRP DRAIN OPTION

construct either steel or FRP slab drains.

Locate drains in slab by dimensions shown

clear drains.

The bracket assembly shall be galvanized in accordance with ASTM A123.

All bolts, hardened washers, lock washers and nuts shall be galvanized in accordance with AASHTO M 232 (ASTM A153), Class C.

All 1/2-inch diameter bolts shall be ASTM A307, except as shown.

Shop drawings will not be required for the slab drains and the bracket assembly.

Notes for Steel Drain:

Slab drains may be fabricated of either 1/4-inch welded sheets of ASTM A709 Grade 36 steel or from 1/4-inch structural steel tubing ASTM A500 or A501.

Outside dimensions of drains are 8" \times 4"

The drains shall be galvanized in accordance with ASTM A123.

Notes for FRP Drain:

Drains shall be machine filament-wound thermosetting resin tubing meeting the requirements of ASTM D2996 with the following exceptions:

Shape of drains shall be rectangular with outside nominal dimensions of $8\,{}^{\rm w}$ x $4\,{}^{\rm w}$.

Minimum reinforced wall thickness shall be

The resin used shall be ultraviolet (UV) resistant and/or have UV inhibitors mixed throughout. Drains may have an exterior coating for additional UV resistance.

The color of the slab drain shall be gray (Federal Standard 26373). The color shall be uniform throughout the resin and any coating used.

The combination of materials used in the manufacture of the drains shall be tested for UV resistance in accordance with ASTM D4329 Cycle A. The representative material bases Cycle A. The representative material shall withstand at least 500 hours of testing with only minor discoloration and without any physical deterioration. The contractor shall furnish the results of the required ultraviolet testing prior to acceptance of the slab drains

At the contractor's option, drains may be field cut. The method of cutting FRP slab drain shall be as recommended by the manufacturer to ensure a smooth, chip free



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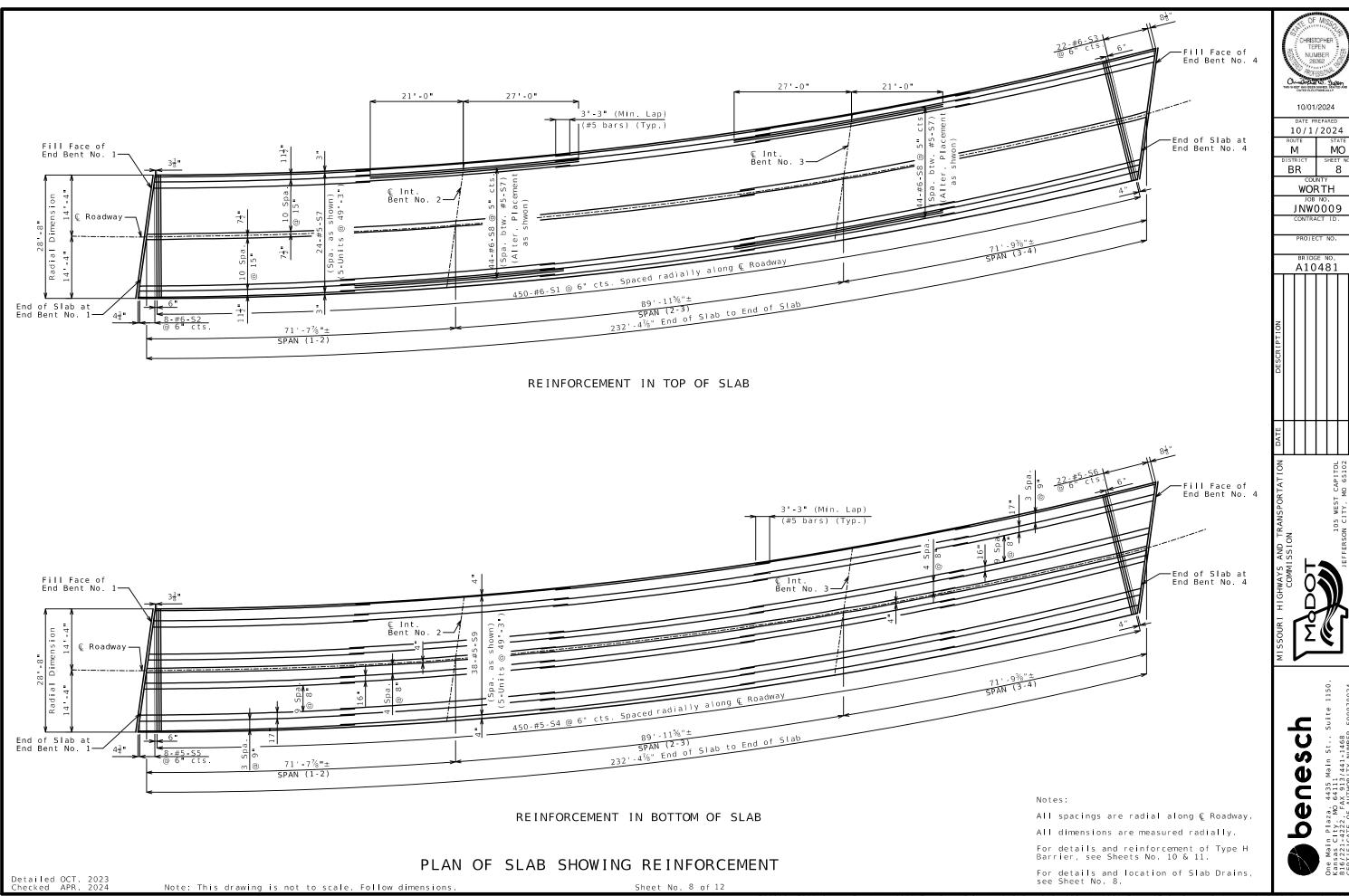
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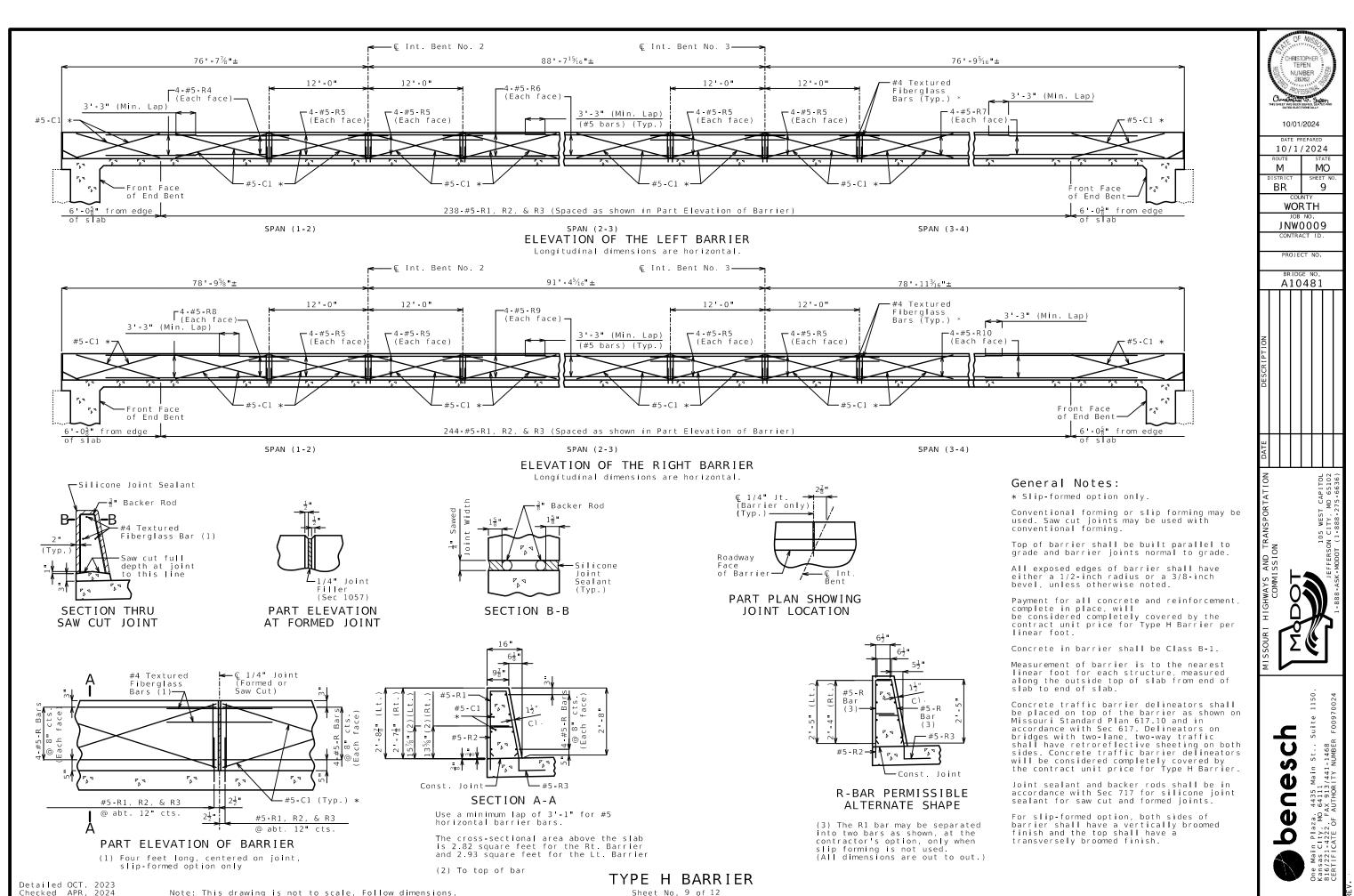
U S O C O 2

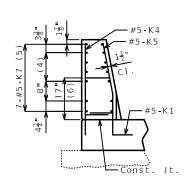
Detailed OCT. 2023 Checked APR. 2024

Note: This drawing is not to scale. Follow dimensions

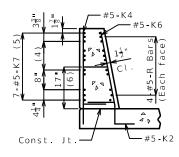
Sheet No. 7 of 12



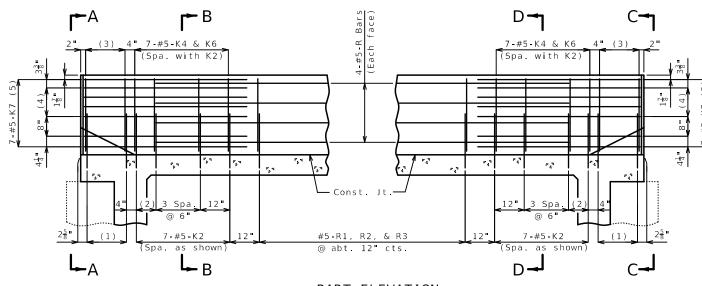




ELEVATION A-A



SECTION B-B



PART ELEVATION

- (1) 5-#5-K1 @ 4" cts.
- (2) 2 Spaces @ 4"

7-#5-K4 & K6

(Spawith K2

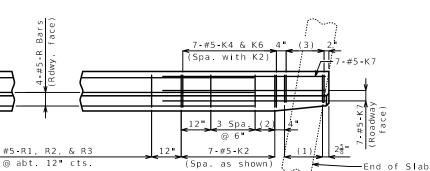
7-#5-K2

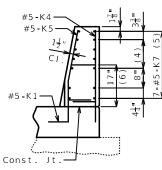
(Spa. as shown)

(3) 5-#5-K4 and 5-#5-K5, spaced with K1

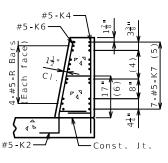
PART PLAN

- (4) 3 Spaces @ 3¹³"
- (5) Spaced as shown, each face
- (6) To top of bar

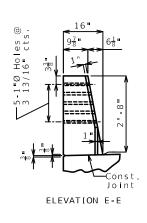


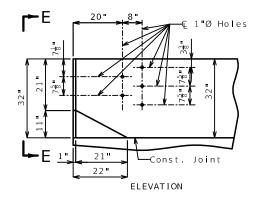


ELEVATION C-C



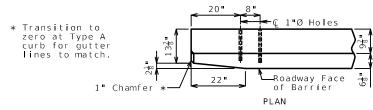
SECTION D-D





End of Slab-

7-#5-K7



DETAILS OF GUARD RAIL ATTACHMENT

General Notes:

Concrete traffic barrier delineators shall be placed on top of the barrier as shown on Missouri Standard Plan 617.10 and in accordance with Sec 617. Delineators on bridges with two-lane, two-way traffic shall have retroreflective sheeting on both sides. Concrete traffic barrier delineators will be considered completely covered by the contract unit price for Type H Barrier.

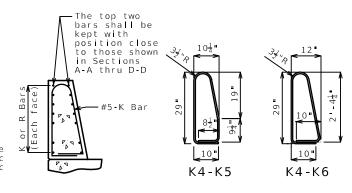
Reinforcing Steel:

Minimum clearance to reinforcing steel shall be 1 1/2".

Use a minimum lap of 3'-1" between K7 bars and R bars.

TYPE H BARRIER AT END BENTS

(Left barrier shown, right barrier similar)



PERMISSIBLE ALTERNATE SHAPES

(Other K bars not shown for clarity)

The K4-K5 and K4-K6 bar combination may be furnished as one bar as shown, at the contractor's option.

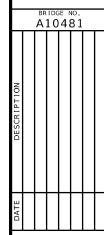
All dimensions are out to out.

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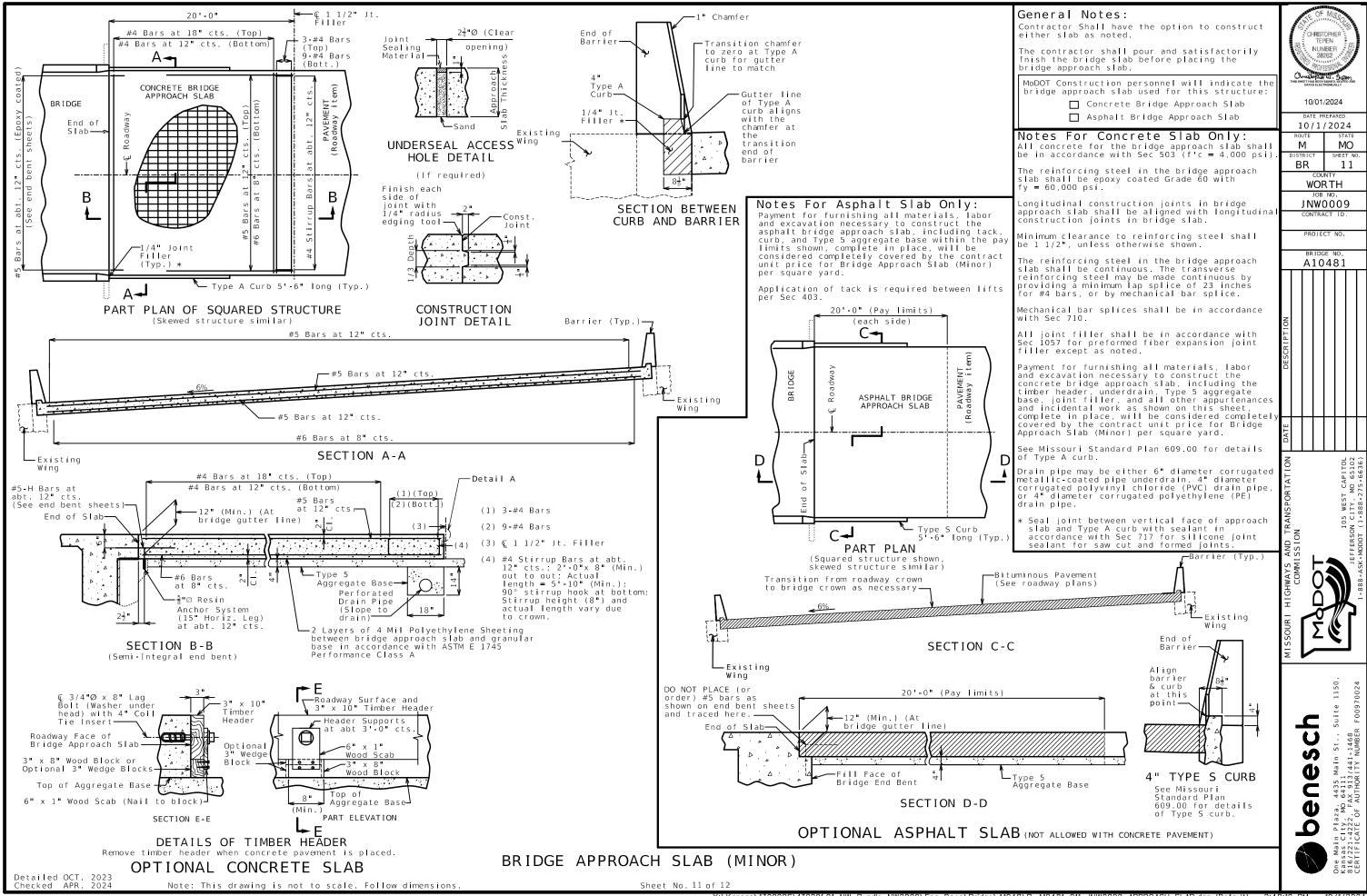
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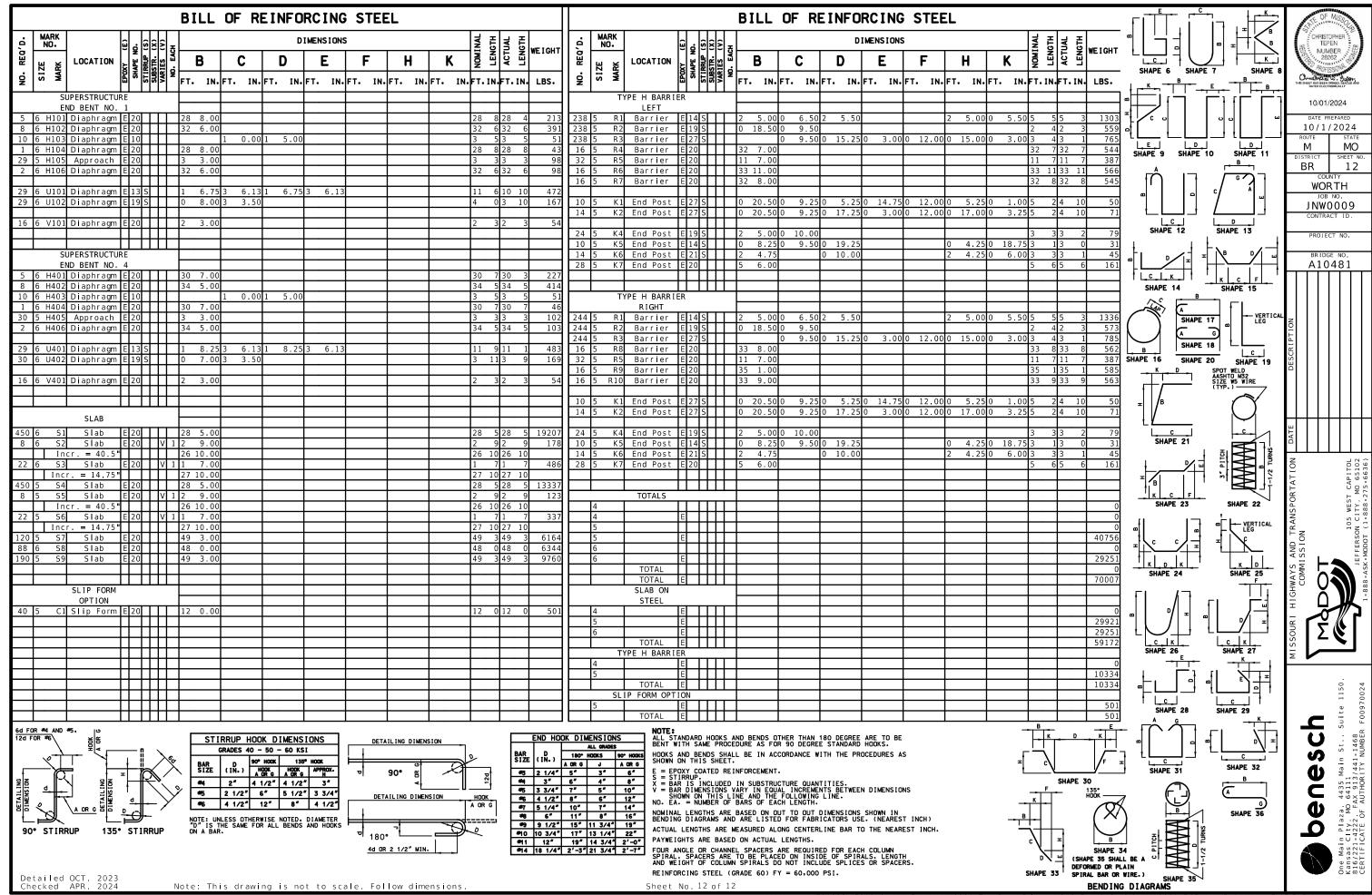


Table Showing S3 Bar Lengths Int. Bent No. 2 Int. Bent No. 3

4'-11"

Span 2

15'-0"

Span 3

10'-0"

	Lap Length Splices **
Bar Size	Splice Length
4	2'-7"
5	3'-3"
6	3'-10"

Span 2

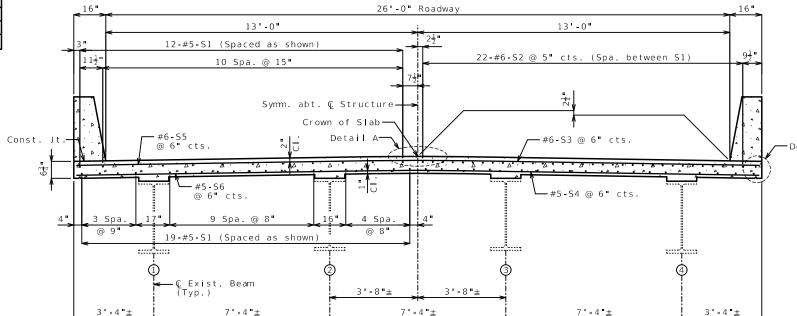
15'-0"

Span 1

10'-0"

** Unless otherwise shown

U.I.P., REDECK, AND COMPOSITE EXISTING (29')(45' COMPOSITE)(29') SIMPLE WIDE FLANGE BEAM SPANS (COMPOSITE) (SKEW: 30° R.A.)



HALF SECTION NEAR INT. BENT NO. 2

-Cross Slope 3/16" per ft. −Profile Grade (Match exist ±) Top of Slab Crow 2'-0" 4'-0" Parabolic Crown DETAIL A

TWP 64N

RGE

32W

SEC/SUR 31

-Const. Jt. C 3/4" Drip Ğroove (Typ.)

DETAIL B

General Notes:

Design Specifications

2002 AASHTO LFD (17th Ed.) Standard Specifications Seismic Performance Category A

Design Loading:

H15-44 (1961) (Existing) HS20-44 (New Construction) 15 lb/sf Future Wearing Surface Earth - 120 lb/cf, Equivalent Fluid Pressure 45 lb/cf Fatigue Stress - Case III

Design Unit Stresses:

Class B-1 Concrete (Barrier) Class B-2 Concrete (End Bents & Superstructure, f'c = 4,000 psif'c = 4,000 psi except Barrier) Reinforcing Steel (Grade 60) fy = 60,000 psi

All joint filler shall be in accordance with Sec 1057 for preformed sponge rubber expansion and partition joint filler, except as noted.

Reinforcing Steel:

Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise shown.

Bars bonded in existing concrete not removed shall be cleanly stripped and embedded into new concrete where possible. If length is available, existing bars shall extend into new concrete at least 40 diameters for plain bars and 30 diameters for deformed bars, unless otherwise noted.

Roadway surfacing adjacent to bridge ends shall match new bridge slab surface. (Roadway item)

Outline of existing work is indicated by light dashed lines. Heavy lines indicate new work.

Contractor shall verify all dimensions in field before ordering new material.

The area exposed by the removal of concrete and not covered with new concrete shall be coated with an approved qualified special mortar in accordance with Sec 704.

Rubblized concrete from the existing bridge deck that qualifies as clean fill may be placed on spill slopes at end bents above ordinary high water line (Roadway item).

Structure to be closed during construction. Traffic to be maintained on other routes during construction. See roadway plans for traffic control.

TYPICAL SECTION THRU SLAB

Structural Steel Protective Coating (Pile):

HALF SECTION NEAR MIDSPAN, INT. BENT NO. 3, AND END BENTS

The exposed surfaces of the existing structural steel CIP pile shells, to limits shown on the bridge plans, shall be coated with one 6-mil thickness of aluminum gray epoxy-mastic primer applied over an SSPC-SP3 surface preparation in accordance with Sec 1081. The bituminous coating shall be applied one foot above and one foot below the existing ground line and in accordance with Sec 702. The cost of surface preparation will be considered completely covered by the contract lump sum price for Surface Preparation for Applying Epoxy-Mastic Primer. The cost of the aluminum epoxy-mastic primer and bituminous coating will be considered completely covered by the contract lump sum price for Aluminum Epoxy-Mastic Primer.

Structural Steel Protective Coating (Top Flange):

In accordance with Sec. 216.50 and 1081, the top, and additionally, the sides and bottom, of the Top Flange shall be coated with not less than 3.0 mils of Gray Epoxy Mastic-Primer (non-aluminum) applied over an SSPC-SP3 surface preparation. Payment for coating steel will be considered completely covered by the contract sq. foot price for Removal of Existing Bridge Deck.

Estimated Quantities Total Removal of Existing Bridge Deck sq. foo 3.039 Bridge Approach Slab (Minor) 118 sq. yar 337 Slab on Steel sq. yar Type H Barrier linear foo 212 Substructure Repair (Formed) sq. foc 12 Substructure Repair (Unformed sq. foo 744 Shear Connectors eacl Slab Drain 22 eacl Surface Preparation for Applying Epoxy - Mastic Primer lump sun Aluminum Epoxy Mastic Primer lump sur Vertical Drain at End Bents eac Open Cell Foam Joint Seal linear foo 52

Cost of any required excavation for bridge will be considered completely covered by the contract unit price for other items

* Concrete Bridge Approach Slab only. See Special Provision

Estimated Quantities for Slab on St	eel
I t em	Total
Class B-2 Concrete cu. yard	90
Reinforcing Steel (Epoxy Coated) pound	26,380

The table of Estimated Quantities for Slab on Steel represents the quantities used by the State in preparing the cost estimate for concrete slabs. The area of the concrete slab will be measured to the nearest square yard longitudinally from end of slab to end of slab and transversely from out to out of bridge slab (or with the horizontal dimensions as shown on the plan of slab). Payment for stay-in-place corrugated steel forms, conventional forms, all concrete and epoxy coated reinforcing steel will be considered completely covered by the contract unit price for the slab. Variations may be encountered in the estimated quantities but the variations cannot be used for an adjustment in the

Method of forming the slab shall be in accordance with Sec 703. All hardware for forming the slab to be left in place as a permanent part of the structure shall be coated in accordance with ASTM A123 or ASTM B633 with a thickness Class SC 4 and a finish Type I, II, or III.

Slab shall be cast-in-place with conventional forming or stay-in-place corrugated steel forms. Precast prestressed panels will not be permitted.

For Optional Stay-In-Place Form Details, see Sheet No. 2.

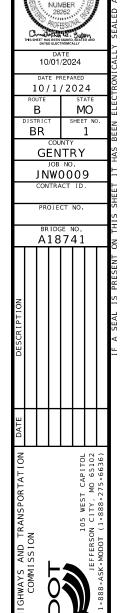
Bridge deck surface may be finished with a vibratory screed.

ROUTE B FROM ROUTE O TO ROUTE AF ABOUT 0.6 MILE S OF ROUTE O BEGINNING STATION 242+32.30± (MATCH EXISTING)

Contractor may shift or swap bars as needed to tie R3 bar in barrier (4" min. bar spacing) Contractor may shif bar as needed to tie R2 bar in barrier OPTIONAL SHIFTING

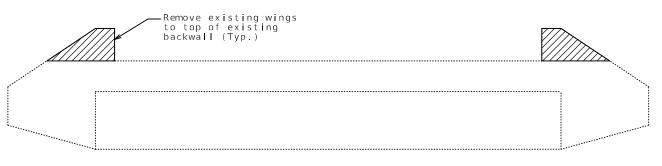
TOP BARS AT BARRIER

REPAIRS TO BRIDGE: ROUTE B OVER ZOUNDS CREEK



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Designed MAR. 2024 Detailed APR. 2024



DETAILS OF CONCRETE REMOVAL AT END BENTS

The cost of concrete removal as shown will be considered completely covered by the contract unit price for Removal of Existing Bridge Deck. Vertical backwall and wingwall reinforcement to be cut off one inch below concrete removal surface and the resulting holes shall be filled with a qualified special mortar.

A smooth, level surface shall be provided at Bents No. 1 & 4 removal lines.

General Notes:

Stay-In-Place Forms:

Corrugated steel forms, supports, closure elements and accessories shall be in accordance with grade requirement and coating designation G165 of ASTM A653. Complete shop drawings of the permanent steel deck forms shall be required in accordance with Sec 1080.

Corrugations of stay-in-place forms shall be filled with an expanded polystyrene material. The polystyrene material shall be placed in the forms with an adhesive in accordance with the manufacturer's recommendations.

Form sheets shall not rest directly on the top of beam flanges. Sheets shall be securely fastened to form supports with a minimum bearing length of one inch on each end. Form supports shall be placed in direct contact with the flange. Welding on or drilling holes in the beam flanges will not be permitted. All steel fabrication and construction shall be in accordance with Sec 1080 and 712. Certified field welders will not be required for welding of the form supports.

The design of stay-in-place corrugated steel forms is per manufacturer which shall be in accordance with Sec 703 for false work and forms. Maximum actual weight of corrugated steel forms allowed shall be 4 psf assumed for beam loading.

The contractor shall provide a method of preventing the direct contact of the stay-in-place forms and connection components with uncoated weathering steel members that is approved by the engineer.

Pouring and Finishing Slab:

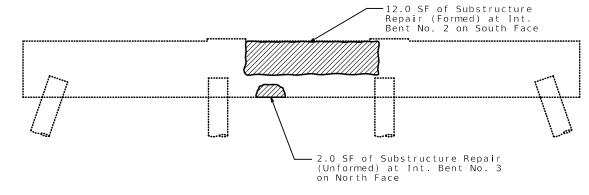
The contractor shall provide bracing necessary for lateral and torsional stability of the beams during construction of the concrete slab and remove the bracing after the slab has attained 75% design strength. Contractor shall not weld on or drill holes in the beams. The cost for furnishing, installing, and removing bracing will be considered completely covered by the contract unit price for Slab on Steel.

Slab shall be poured upgrade from end to end at a minimum rate of 25 cubic yards per hour.

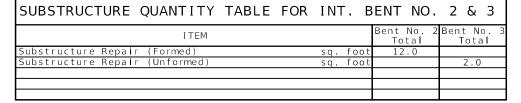
Alternate pour sequences may be submitted to the engineer for approval. Keyed construction joints shall be provided between pours.

Haunching

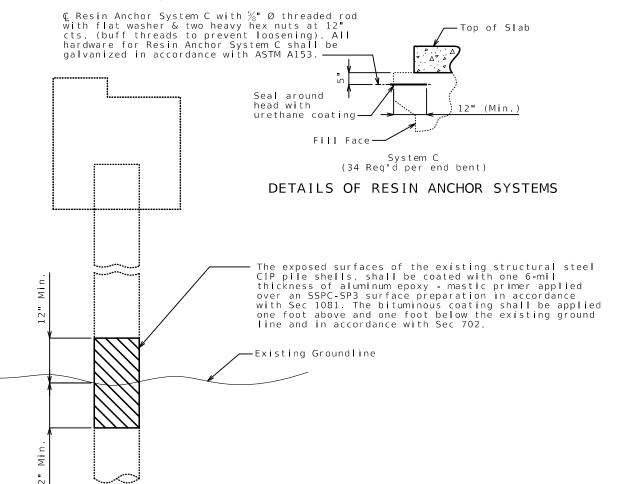
(1) Slab is to be considered a uniform thickness as shown on the plans. Haunching will vary. See front sheet for slab thickness. For adjusted girder deflection due to weight of new deck and barriers, see Bridge Electronic Deliverables.



SUBSTRUCTURE REPAIR FOR INT. BENT NO. 2 & 3



Notes: These quantities are included in the Estimated Quantities Table on Sheet No. 1.



INT. BENT PROTECTIVE COATING DETAILS

Designed MAR. 2024 Detailed APR. 2024 Checked IIII 2024 Steel corrugated A^{ullet}

Form support

A-

bridge forms-

- Fill corrugations

with foam (Typ.)

SECTION A-A

OPTIONAL STAY-IN-PLACE

FORM DETAILS

Clean and coat with Gray Epoxy-Mastic

Primer (See Sec 216)

(AII Beams)

Clean and coat with Gray Epoxy-Mastic Primer (See Sec 1081)

(All Beams)

TYPICAL SECTION THRU BEAM

SHOWING PROTECTIVE COATING

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HRISTOPHI TEPEN NUMBEF 28262

10/01/2024 DATE PREPARED 10/1/2024

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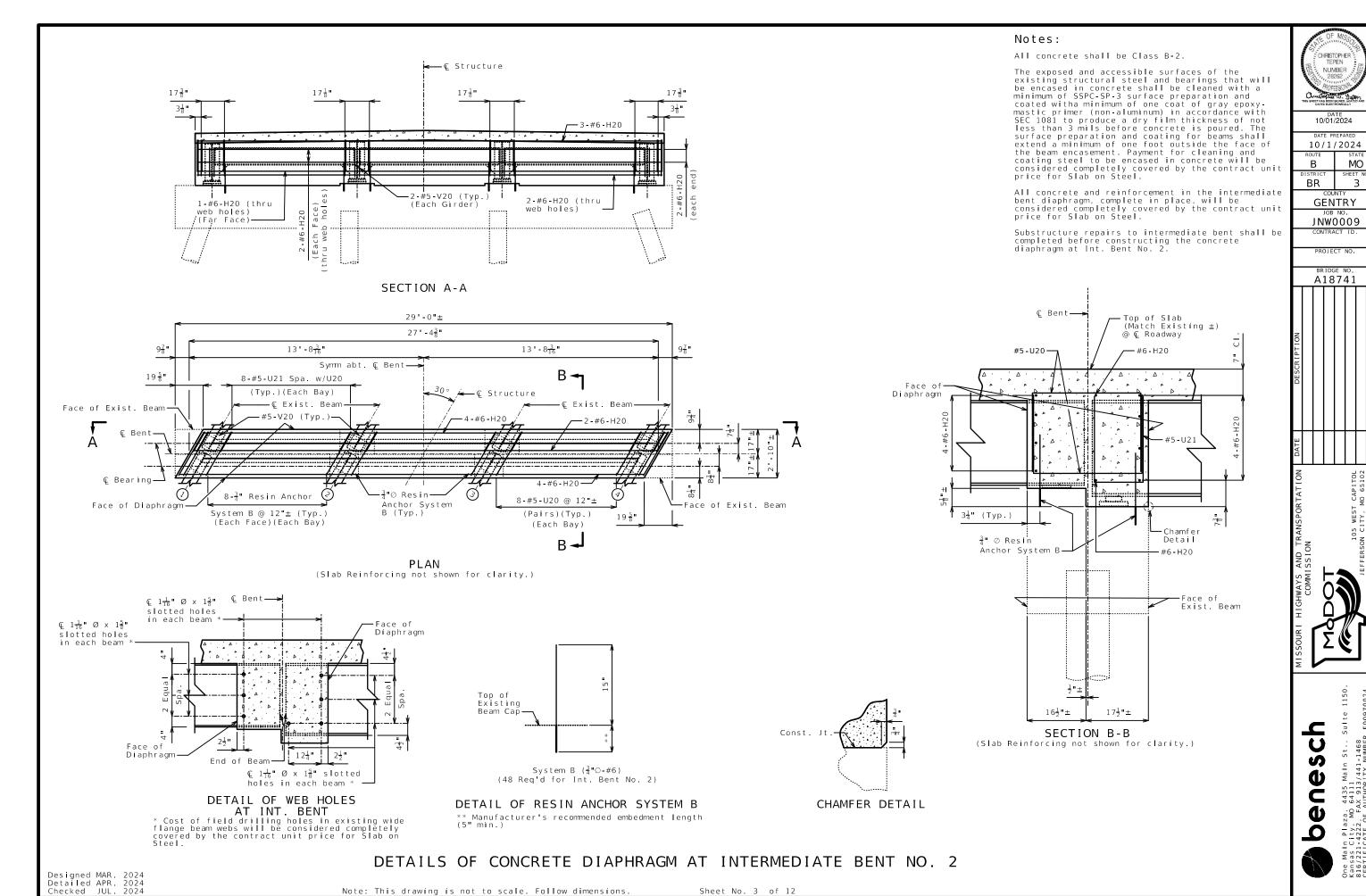
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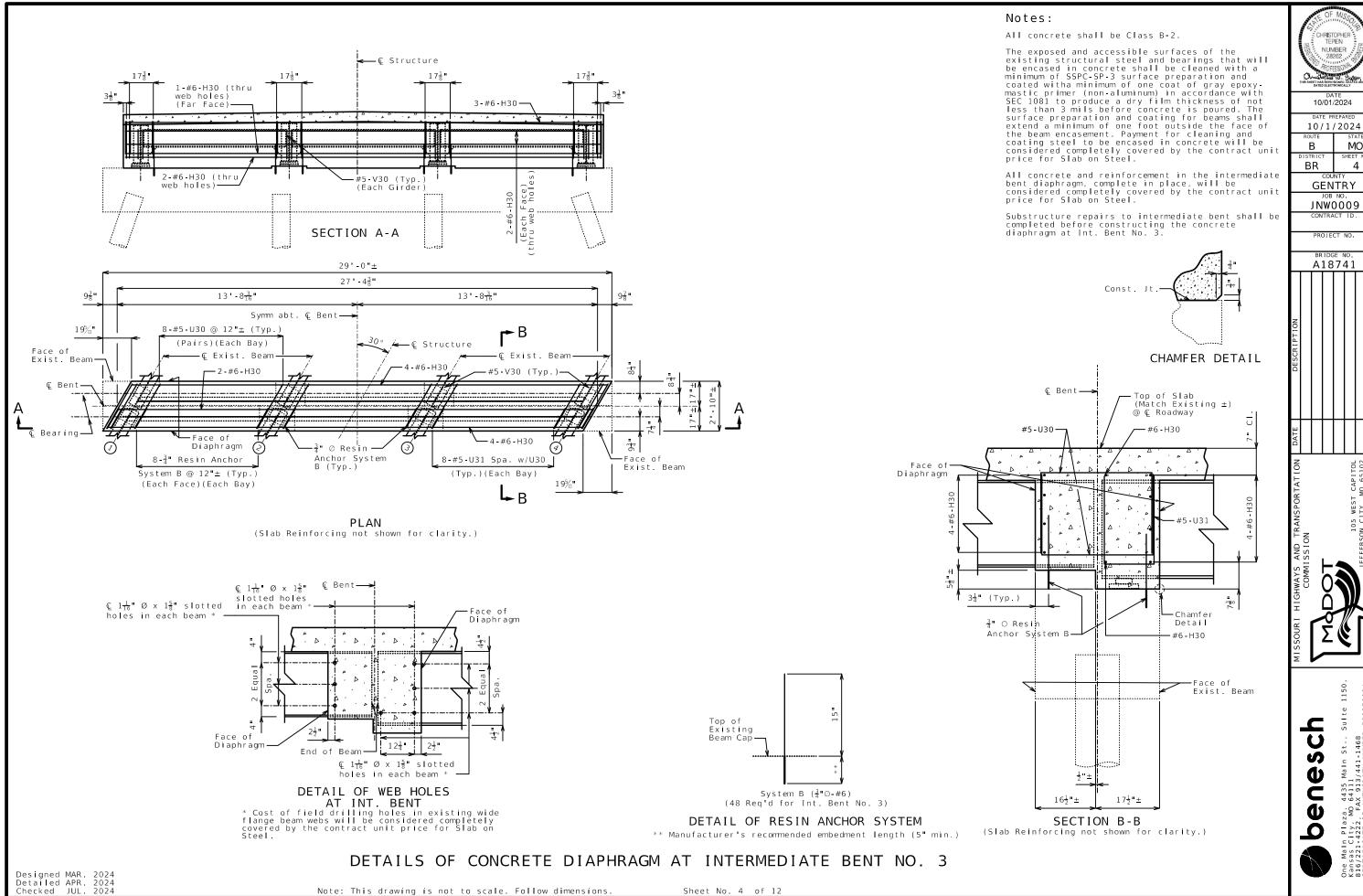
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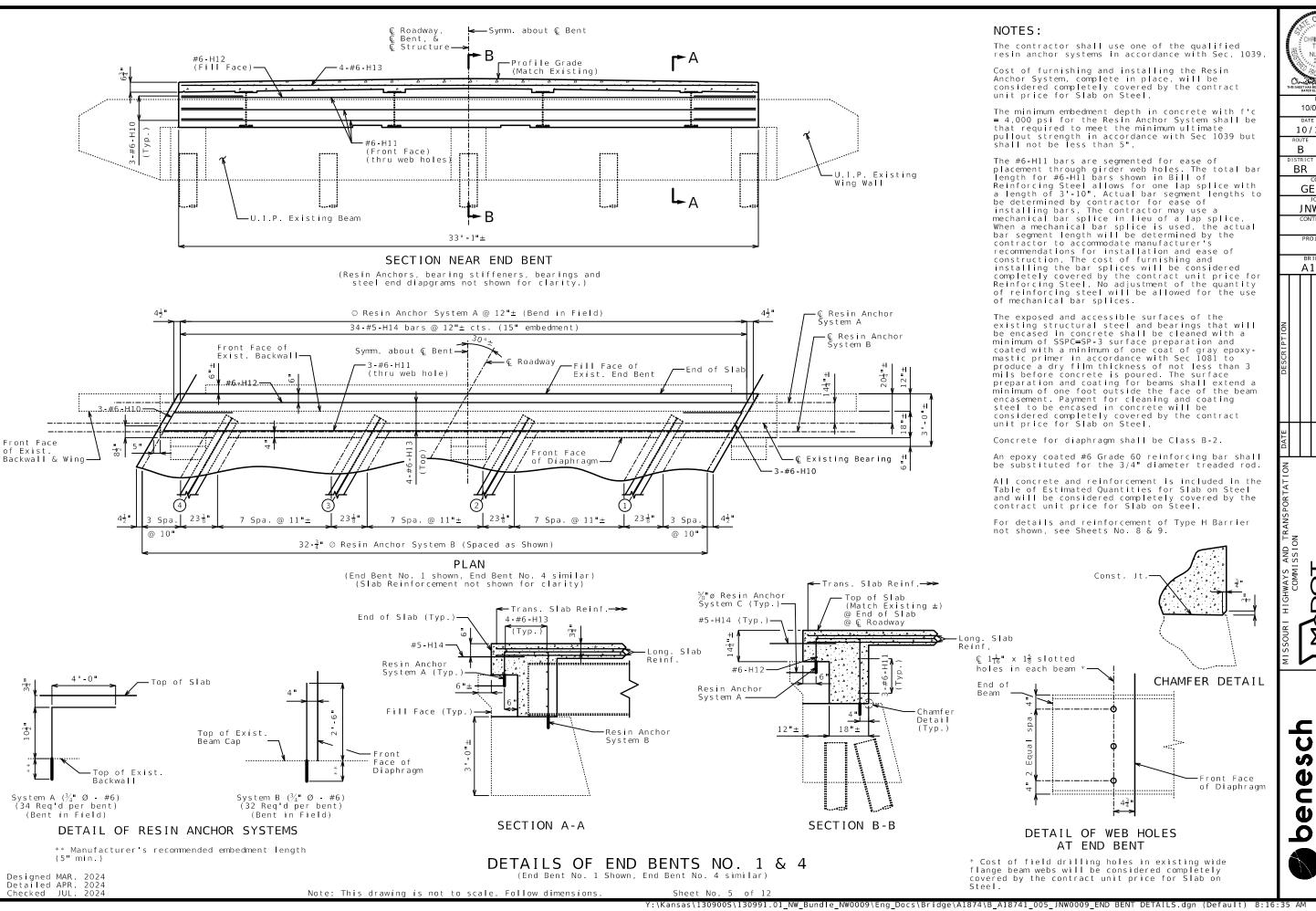
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SHEET NO

4



NUMBER , 28262

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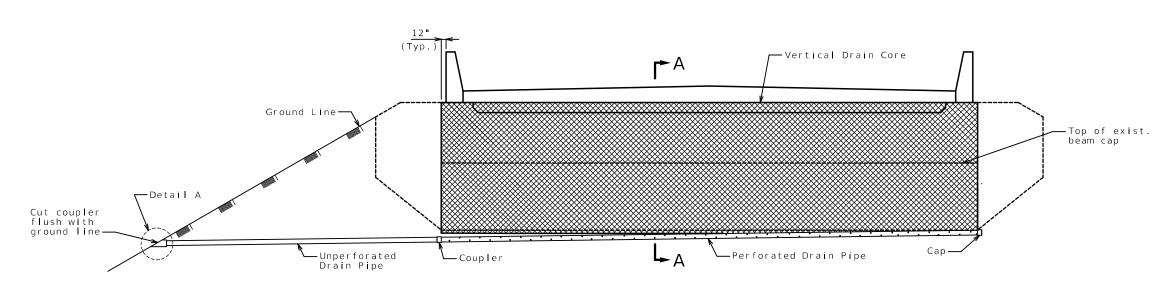
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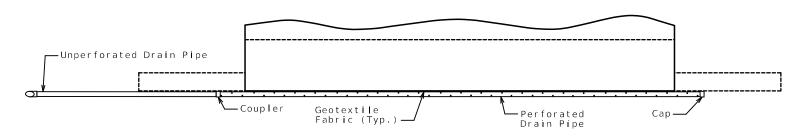
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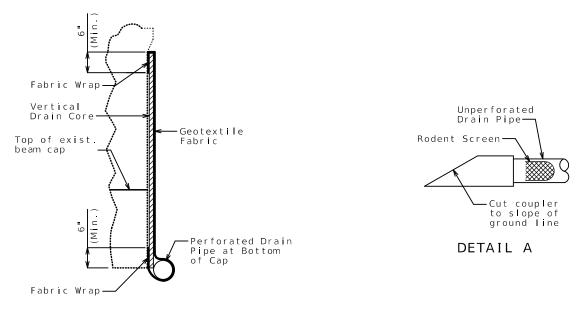
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ELEVATION OF END BENT



PLAN OF END BENT



PART SECTION A-A (Section thru wing similar)

VERTICAL DRAIN AT END BENTS

(Squared end bent shown, skewed end bent similar)

General Notes:

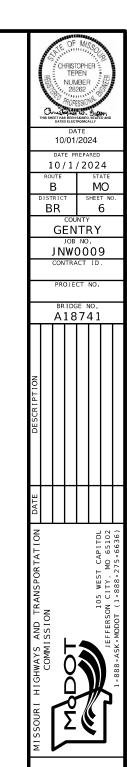
All drain pipe shall be sloped 1 to 2

Drain pipe may be either 6-inch diameter corrugated metallic-coated steel pipe underdrain, 4-inch diameter corrugated polyvinyl chloride (PVC) drain pipe, or 4-inch diameter corrugated polyethylene (PE) drain pipe.

Drain pipe shall be placed at fill face of end bent and inside face of wings. The pipe shall slope to lowest grade of ground line, also missing the lower beam of end bent by a minimum of $1\frac{1}{2}$ inches.

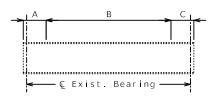
Perforated pipe shall be placed at fill face side and inside face of wings at the bottom of end bent and plain pipe shall be used where the vertical drain ends to the exit at ground line.

All excavation necessary for installation of vertical drain will be considered completely covered by the contract unit price for Vertical Drain at End Bents.



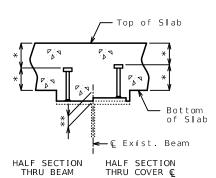
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Designed MAR. 2024 Detailed APR. 2024 Checked JUL. 2024



ELEVATION SHOWING SHEAR CONNECTOR SPACING FOR SPANS 1 & 3 BEAMS

TABLE SHOWING SHEAR CONNECTOR UNIT SPACING						
B e am		S.C. per unit	А	В	C	
Beam (Spans 1-2 & 4-3)	α	6½"±	31 Units @ 11"± cts.	10½"±		
Total shear connectors required per beam						

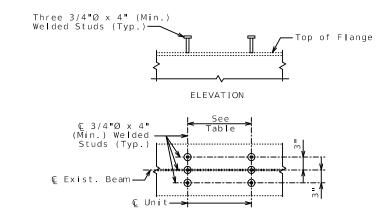


SECTION THRU EXIST. BEAM SHOWING SHEAR CONNECTORS

Designed MAR. 2024 Detailed APR. 2024 Checked JUL. 2024

* 3" Minimum

** Min. Haunch = 0.000" Max. Haunch = 0.375"



PLAN OF SHEAR CONN. (3 PER UNIT)

Notes:

The cost of supplying and installing shear connectors will be considered completely covered by the contract unit price for Shear Connectors.

Shear connectors shall be in accordance with Sec 712, 1037, & 1080.



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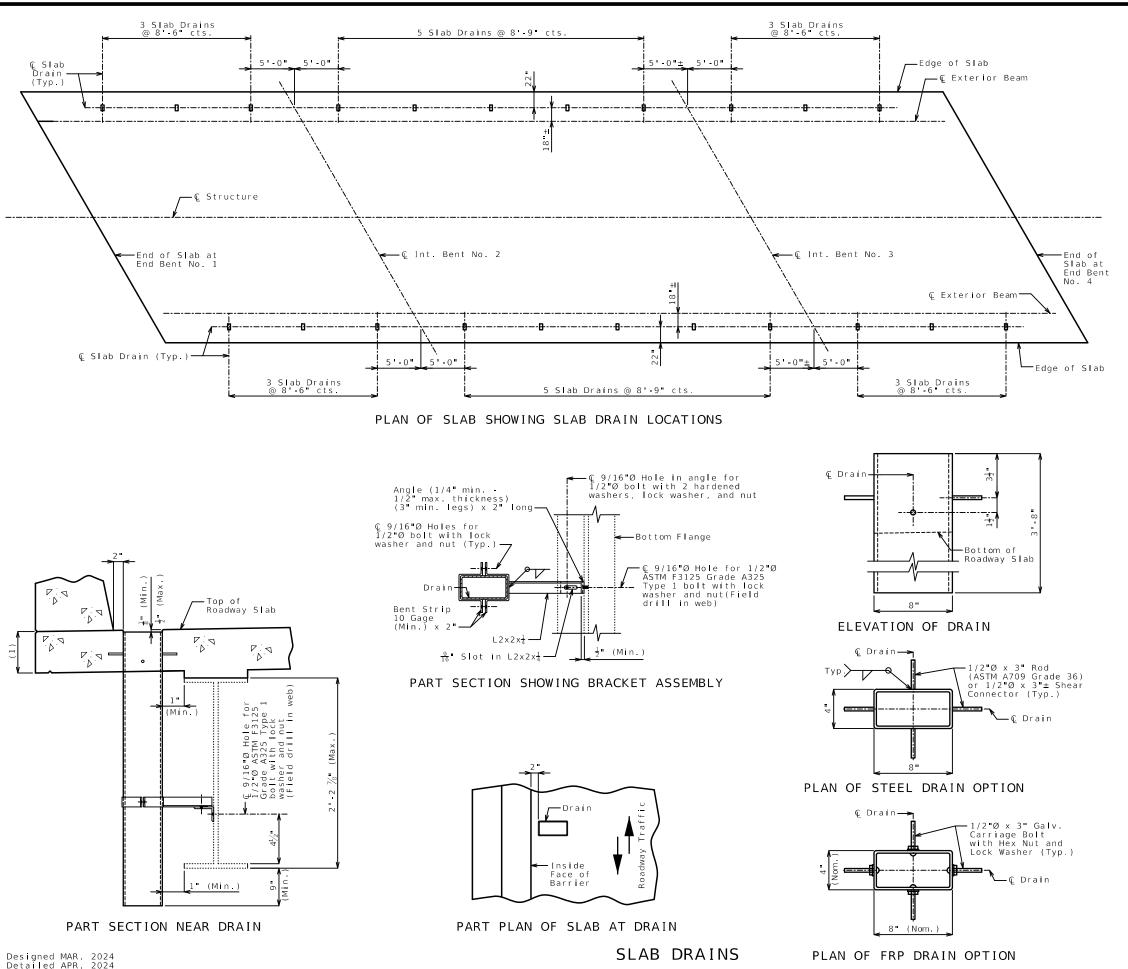
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DETAILS OF SHEAR CONNECTORS



Note: This drawing is not to scale. Follow dimensions.

Checked JUL

General Notes:

Contractor shall have the option to construct either steel or FRP slab drains. All drains shall be of same type.

Slab drain bracket assembly shall be ASTM A709 Grade 36 steel.

Locate drains in slab by dimensions shown in Part Section Near Drain.

Reinforcing steel shall be shifted to clear drains.

The bracket assembly shall be galvanized in accordance with ASTM A123.

All bolts, hardened washers, lock washers and nuts shall be galvanized in accordance with AASHTO M 232 (ASTM A153), Class C.

All 1/2"Ø bolts shall be ASTM A307, except as shown.

Shop drawings will not be required for the slab drains and the bracket assembly.

The bolt hole for the bracket assembly attachment shall be shifted to the minimum extent necessary to field drill in the existing web.

(1) See front sheet for slab thickness.

Notes for Steel Drain:

Slab drains may be fabricated of either 1/4" welded sheets of ASTM A709 Grade 36 steel or from 1/4" structural steel tubing ASTM A500 or A501.

Outside dimensions of drains are 8" x 4".

The drains shall be galvanized in accordance with ASTM A123.

Notes for FRP Drain:

Drains shall be machine filament-wound thermosetting resin tubing meeting the requirements of ASTM D2996 with the following exceptions:

Shape of drains shall be rectangular with outside nominal dimensions of $8" \times 4"$.

Minimum reinforced wall thickness shall be

The resin used shall be ultraviolet (UV) resistant and/or have UV inhibitors mixed throughout. Drains may have an exterior coating for additional UV resistance.

The color of the slab drain shall be gray (Federal Standard 26373). The color shall be uniform throughout the resin and any coating used.

The combination of materials used in the manufacture of the drains shall be tested for UV resistance in accordance with ASTM D4329 Cycle A. The representative material shall withstand at least 500 hours of testing with only minor discoloration and without any physical deterioration. The contractor shall furnish the results of the required ultraviolet testing prior to acceptance of the slab drains.

At the contractor's option, drains may be field cut. The method of cutting FRP slab drain shall be recommended by the manufacturer to ensure a smooth, chip free



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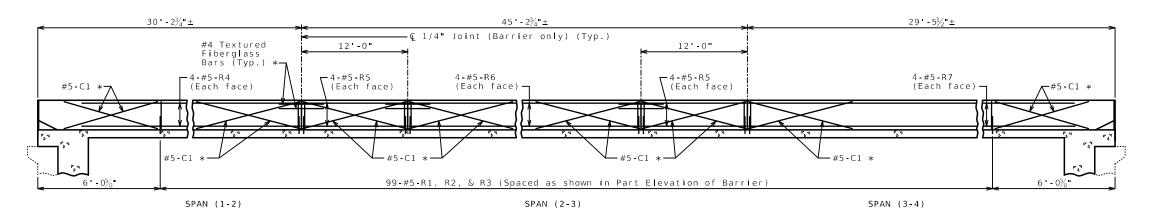
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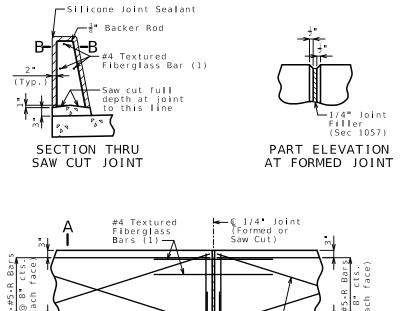
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Sheet No. 8 of 12



ELEVATION OF BARRIER

(Left barrier shown, right barrier similar) Longitudinal dimensions are horizontal.



PART ELEVATION OF BARRIER

0 0

21 "

-#5-C1 (Typ.) *

#5-R1 R2 & R3

P 0

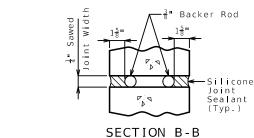
#5-R1. R2. & R3

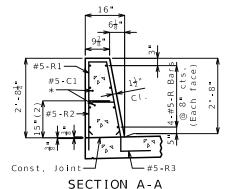
@ abt. 12" cts.

Designed MAR. 2024 Detailed APR. 2024

Checked JUL

(1) Four feet long, centered on joint, slip-formed option only



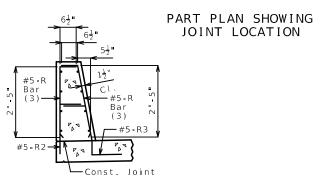


Use a minimum lap of 3'-1" for #5 horizontal barrier bars.

The cross-sectional area above the slab is 2.89 square feet.

(2) To top of bar

© 1/4" Joint (Barrier only) (Typ.) Roadway Face of Barrier



R-BAR PERMISSIBLE ALTERNATE SHAPE

(3) The R1 bar may be separated into two bars as shown, at the contractor's option, only when slip forming is not used. (All dimensions are out to out.)

General Notes:

* Slip-formed option only.

Conventional forming or slip forming may be used. Saw cut joints may be used with conventional forming.

Top of barrier shall be built parallel to grade and barrier joints normal to grade.

All exposed edges of barrier shall have either a 1/2-inch radius or a 3/8-inch bevel, unless otherwise noted.

Payment for all concrete and reinforcement, complete in place, will be considered completely covered by the contract unit price for Type H Barrier per linear foot.

Concrete in barrier shall be Class B-1.

Measurement of barrier is to the nearest linear foot for each structure, measured along the outside top of slab from end of slab to end of slab.

Concrete traffic barrier delineators shall be placed on top of the barrier as shown on Missouri Standard Plan 617.10 and in accordance with Sec 617. Delineators on bridges with two-lane, two-way traffic shall have retroreflective sheeting on both sides. Concrete traffic barrier delineators will be considered completely covered by the contract unit price for Type H Barrier.

Joint sealant and backer rods shall be in accordance with Sec 717 for silicone joint sealant for saw cut and formed joints.

For slip-formed option, both sides of barrier shall have a vertically broomed finish and the top shall have a transversely broomed finish.



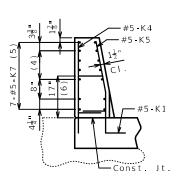
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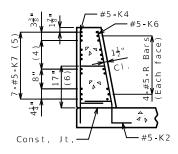
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benesch 115 main Plaza, 4435 Main St., Sulte 115 mass City, MO 64111

TYPE H BARRIER



ELEVATION A-A

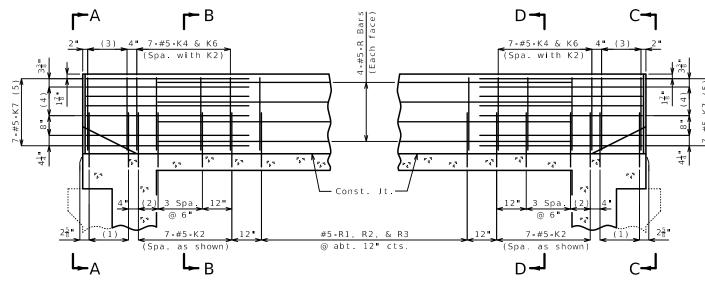


SECTION B-B

r►E

1" Chamfer *—

DETAILS OF GUARD RAIL ATTACHMENT



PART ELEVATION

(1) 5-#5-K1 @ 4 cts

(2) 2 Spaces @ 4"

Ç 1"Ø Holes

└─Const. Joint

1"Ø Holes

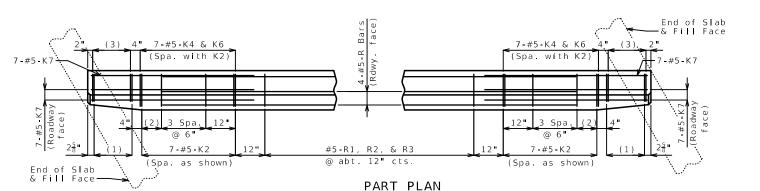
— Roadway Face ⊣®

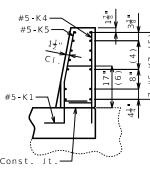
of Barrier

ELEVATION

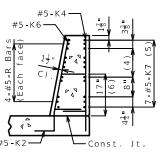
22"

- (3) 5-#5-K4 and 5-#5-K5, spaced with K1
- (4) 3 Spaces @ 3¹³"
- (5) Spaced as shown, each face
- (6) To top of bar

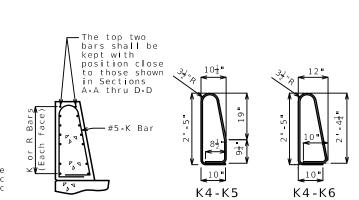




ELEVATION C-C



SECTION D-D



PERMISSIBLE ALTERNATE SHAPES

(Other K bars not shown for clarity)

The K4-K5 and K4-K6 bar combination may be furnished as one bar as shown, at the contractor's option.

All dimensions are out to out.

General Notes:

Concrete traffic barrier delineators shall be placed on top of the with Sec 617. Delineators on bridges with two-lane, two-way traffic shall have retroreflective sheeting on both sides. Concrete traffic barrier delineators will be considered completely covered by the contract unit price for Type H Barrier.

Minimum clearance to reinforcing steel shall be 1 1/2".

Use a minimum Tap of 3'-1" between K7 bars and R bars.

TYPE H BARRIER AT END BENTS

(Left barrier shown, right barrier similar)

Designed MAR. 2024 Detailed APR. 2024 Checked JUL. 2024

ELEVATION E-E

* Transition to zero at Type A curb for gutter lines to match.

5-1"Ø Holes @ 3 13/16" cts.

Note: This drawing is not to scale. Follow dimensions.

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