DESIGN DESIGNATION

RTE N

A.A.D.T. 2022 = 923 A.A.D.T. - 2042 = 1,200

D.H.V. = 10% T = 5%

V = 35 M.P.H.

D = 50%/50% FUNCTIONAL CLASSIFICATION- MAJOR COLLECTOR

MO - 6

A.A.D.T. - 2022 = 765 A.A.D.T. - 2042 = 1,033

V = 60 M.P.H.

D = 50%/50%

FUNCTIONAL CLASSIFICATION- MINOR ARTERIAL

NO RIGHT OF WAY ACQUISITION

CONVENTIONAL SYMBOLS (USED IN PLANS)

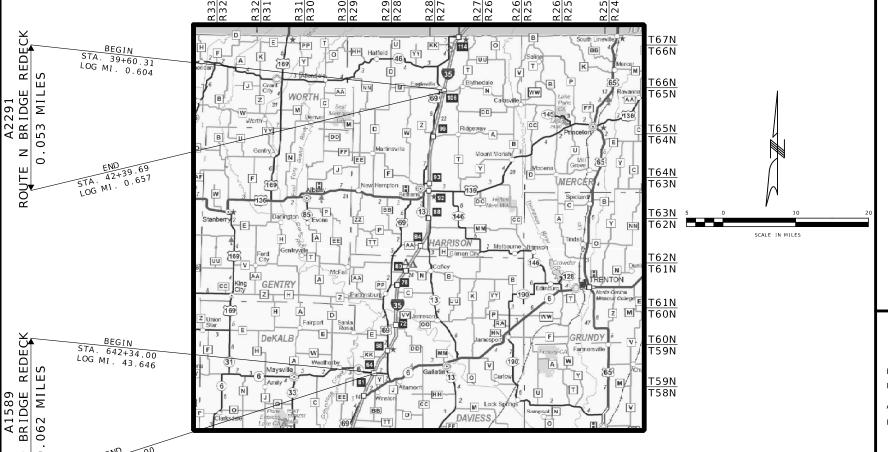
(OSED IN LEANS	,,	
	EXISTING	NEW
BUILDINGS AND STRUCTURES GUARD RAIL GUARD CABLE CONCRETE RIGHT-OF-WAY MARKER STEEL RIGHT-OF-WAY MARKER LOCATION SURVEY MARKER UTILITIES	0000	0
FIBER OPTICS OVERHEAD CABLE TV UNDERGROUND CABLE TV OVERHEAD TELEPHONE UNDERGROUND TELEPHONE OVERHEAD POWER UNDERGROUND POWER SANITARY SEWER STORM SEWER GAS WATER	- FOOTVUTV OT UT OE UE SS G W	—FO—OTV—UTV—OT—UT—OE——VE——S——S——G——W
MANHOLE	SAN E)
FIRE HYDRANT		Ì
WATER VALVE	w _v G)
WATER METER	ww.)
DROP INLET	"[
DITCH BLOCK	=	⊨
GROUND MOUNTED SIGN	SIGN	-
LIGHT POLE		
H-FRAME POWER POLE		
TELEPHONE PEDESTAL FENCE CHAIN LINK WOVEN WIRE GATE POST	PED V	·
BENCHMARK	· · · ·)

NOTE: DASHED OR OPEN SYMBOLS INDICATE EXISTING FEATURES

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

PLANS FOR PROPOSED STATE HIGHWAY

DAVIESS & HARRISON COUNTY





THE EXISTENCE AND APPROXIMATE LOCATION OF UTILITY FACILITIES KNOWN TO EXIST, AS SHOWN ON THE PLANS, ARE BASED ON THE BEST INFORMATION AVAILABLE TO THE COMMISSION AT THIS TIME. THIS INFORMATION IS PROVIDED BY THE COMMISSION "AS-IS" AND THE COMMISSION EXPRESSLY DISCLAIMS ANY REPRESENTATION OR WARRANTY AS TO THE COMPLETENESS, ACCURACY, OR SUITABILITY OF THE INFORMATION FOR ANY USE. RELIANCE UPON THIS INFORMATION IS DONE AT THE RISK AND PERIL OF THE USER, AND THE COMMISSION SHALL NOT BE LIABLE FOR ANY DAMAGES THAT MAY ARISE FROM ANY ERROR IN THE INFORMATION. IT IS, THEREFORE, THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY THE EXISTENCE, LOCATION AND STATUS OF ANY FACILITY. SUCH VERIFICATION INCLUDES DIRECT CONTACT WITH THE LISTED UTILITIES.

INDEX OF SHEETS

DESCRIPTION	SHEET NUMBER
TITLE SHEET	1
TYPICAL SECTIONS (TS) (1 SHEET)	2
QUANTITIES (QU) (2 SHEETS)	3
RTE 6	
PLAN-PROFILE (PP)	4
TRAFFIC CONTROL SHEETS (TC)	5 - 10
EROSION CONTROL SHEETS (EC)	11
PAVEMENT MARKING (PM)	12
CROSS SECTIONS (XS)	1 - 5
RTE N	
PLAN-PROFILE (PP)	13
TRAFFIC CONTROL SHEETS (TC)	14-19
EROSION CONTROL SHEETS (EC)	20
PAVEMENT MARKING (PM)	21
CROSS SECTIONS (XS)	1 - 4
BRIDGE DRAWINGS (B)	
A1589	1 - 9
A2291	1-15

10 (11)(A)	PROT THE SEALE		O N		ENG SIGNE	D. CALLY.	W 1841/1/15	ICALLY SEALED
	6 ,	TE N RIC	20	/ 2	HEE	4 ATE 10	Э.	SEEN ELECTRON
DA	· V]	J N CON	JOB	& NO 1 СТ	1 7 7 1 D NO	2	R.	ON THIS SHEET IT HAS
DESCRIPTION								IF A SEAL IS PRESENT (
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LENGTH OF PROJECT

DTE N

RIE	N		
BEGINNING	STA.	39+60.31	
END	STA.	42+39.69	
APPARENT LENGTH		279.38	FEET
EQUATIONS AND EXCEPTIONS:			
NONE			
TOTAL CORRECTIONS		0.0	FEET
MO - 6			
BEGINNING	STA.	642+34.00	
END	STA.	645+63.00	
APPARENT LENGTH		329.00	FEET
EQUATIONS AND EXCEPTIONS:			
NONE			
TOTAL CORRECTIONS		0.0	FEET

NET LENGTH OF PROJECT

FOR INFORMATION ONLY ESTIMATED DISTURBED ACRES

STATE LENGTH



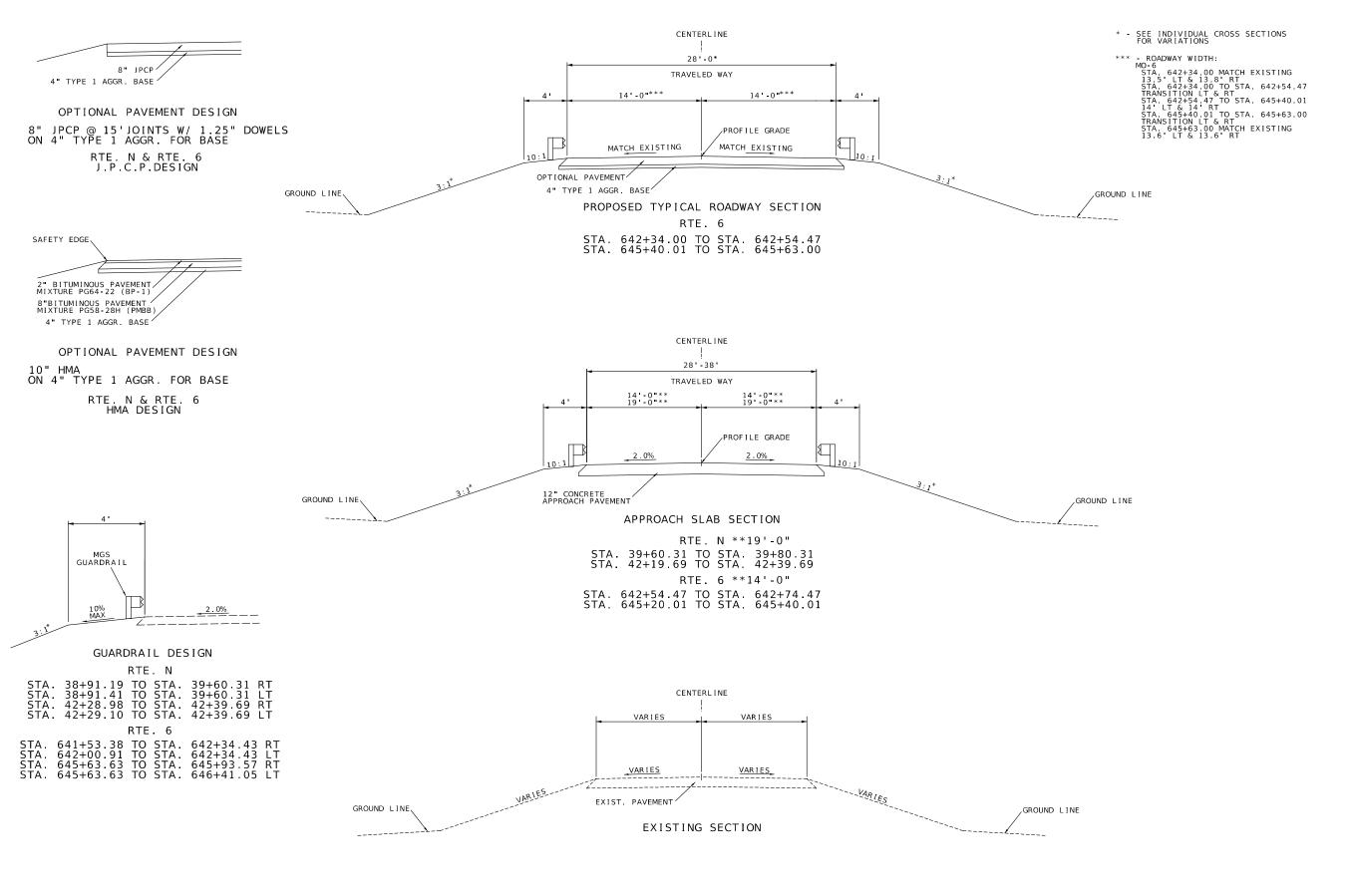
C S 0 C Ф 2

608.38 FEET

0.115 MILES

0.29 ACRES

MICHELE R



TYPICAL SECTIONS SHEET 1 OF 1

DATE PREPARED

8/7/2024

ROUTE
6/N MO
DISTRICT SHEET NO.

JNW0112

CONTRACT ID.

PROJECT NO.

BRIDGE NO.

HIGHWAYS AND TRANSPORTATION DATE
COMMISSION

105 WEST CAPITOL
HEFERSON CITY, MO 65102

benesch MASS MAIN STRET, SUITE 1150 ANSAG CITY, MO 6411-1468

	REMO	OVAL OF	IMPROVEMENT:	S	
STATION	STATION	SIDE	DESCRIPTION	QUANTITY	UNITS
RTE 6					
641+54.49	642+63.71	RT	GUARDRAIL	109.2	LF
642+04.08	642+73.89	LT	GUARDRAIL	69.8	LF
642+34.00	642+74.47	CL	PAVEMENT	121.0	SY
642+42.15		LT	SIGN	1	EA
642+49.77		RT	SIGN	1	EA
645+20.01	645+63.00	CL	PAVEMENT	129.3	SY
645+21.27	645+90.77	RT	GUARDRAIL	69.5	LF
645+30.81	646+38.04	LT	GUARDRAIL	107.2	LF
645+46.00		RT	SIGN	1	EA
			SUBTOTAL	1	LS
RTE N					
38+99.14	39+68.93	RT	GUARDRAIL	69.8	LF
38+99.61	39+68.88	LT	GUARDRAIL	69.3	LF
39+60.31	39+80.31	CL	PAVEMENT	83.7	SY
42+19.69	42+39.69	CL	PAVEMENT	83.7	SY
42+30.85	43+13.50	LT	GUARDRAIL	82.6	LF
42+30.96	43+00.46	RT	GUARDRAIL	69.5	LF
			SUBTOTAL	1	LS
			PAY TOTAL	1	LS

OPTIONAL PAVEMENT							
			TYPE 1				
BEGIN	END	OPTIONAL	AGGREGATE				
STATION	STATION	PAVEMENT	BASE (4")				
		(SY)	(SY)				
RTE 6			_				
642+34.00	642+54.47	62.8	62.8				
645+40.01	645+63.00	70.5	70.5				
	SUBTOTALS	133.3	133.3				
	PAY TOTALS	133	133				

	GUARDRAIL						
				TYPE A	MGS		
				CRASHWORTHY	BRIDGE		
BEGIN	END		MGS	END TERMINAL	APPROACH		
STATION	STATION	SIDE	GUARDRAIL	(FLEAT)	TRANSITION		
			(LF)	(EA)	(EA)		
RTE 6							
641+53.38	642+69.43	RT	37.5	1	1		
642+00.91	642+79.51	LT		1	1		
645+14.97	645+93.57	RT		1	1		
645+25.06	646+41.05	LT	37.5	1	1		
	SUI	BTOTALS	75.0	4	4		
RTE N							
38+91.19	39+70.98	RT		1	1		
38+91.41	39+71.20	LT		1	1		
42+28.98	43+08.77	RT		1	1		
42+29.10	43+08.89	LT		1	1		
	SUI	BTOTALS		4	4		
	PAY	TOTALS	75	8	8		

CLEARING AND GRUBBING					
BEGIN	END				
STATION	STATION	(AC)			
RTE 6					
641+39.65	646+52.25	0.1			
	SUBTOTAL	0.1			
RTE N					
38+87.70	43+12.90	0.1			
	SUBTOTAL	0.1			
	PAY TOTAL	1.0			

CLEARING AND GRUBBING								
BEGIN END								
STATION	STATION	(AC)						
TE 6								
641+39.65	646+52.25	0.1						
	SUBTOTAL	0.1						
TE N								
38+87.70	38+87.70 43+12.90 0.1							
	SUBTOTAL	0.1						

	E	ARTHWORK		
BEGIN	END	CLASS A	COMPACTING	EMBANKMENT
STATION	STATION	EXCAVATION	EMBANKMENT	IN PLACE
		(CY)	(CY)	(CY)
ROUTE 6				
641+57.07	646+34.38	38	16	0
	TOTAL	38	16	0
ROUTE N				
38+80.39	43+20.63	0	0	18
	TOTAL	0	0	18
	PROJECT PAY TOTAL	38	16	18

MOBILIZATION 1 LUMP SUM

CONTRACTOR FURNISHED SURVEYING & STAKING 1 LUMP SUM

		PERM	IANENT PAVEM	ENT MARKING		
			WATERBORNE PAVEMENT MARKING PAINT TYPE P BEADS			
BEGIN	END		4"	4"		
STATION	STATION	SIDE	WHITE	YELLOW	REMARKS	
			(LF)	(LF)		
RTE 6						
642+34.00	645+63.00	LT	329.0		EDGE LINE	
642+34.00	645+63.00	RT	329.0		EDGE LINE	
642+34.00	645+63.00	CL		329.0	SOLID CENTERLINE	
642+34.00	645+63.00	CL		82.3	INTERMITTENT CENTERLINE	
	SL	IBTOTALS	658	411		
RTE N						
39+60.31	42+39.69	LT	279.4		EDGE LINE	
39+60.31	42+39.69	RT	279.4		EDGE LINE	
39+60.31	42+39.69	CL		69.8	INTERMITTENT CENTERLINE	
SUBTOTALS			559	70		
	PA	Y TOTALS	1,217	481		

	TEMPORARY	EROSION CON	TROL	
BEGIN STATION	END STATION	SIDE	SILT FENCE (LF)	SEDIMENT REMOVAL (CY)
RTE 6				
641+44.78	642+60.38	RT	115.6	1
641+90.85	642+71.60	LT	80.75	1
645+24.63	646+04.64	RT	80.01	1
645+34.97	646+52.84	LT	117.87	1
		SUBTOTALS	394.2	4
RTE N				
38+88.55	39+70,31	LT	81.76	1
38+88.92	39+70.42	RT	81.5	1
42+29.57	43+12.53	LT	82.96	1
42+28.97	43+10.92	RT	81.95	1
		SUBTOTALS	328.2	4
		PAY TOTALS	722	8

	SEEDING AN	D MULCHING	
BEGIN		COOL SEASON	
STATION	END STATION	MIXTURES	MULCHING
		(AC)	(AC)
RTE 6			
641+39.65	646+52.25	0.1	0.1
	SUBTOTALS	0.1	0.1
RTE N			
38+87.70	43+12.90	0.1	0.1
	SUBTOTALS	0.1	0.1
	PAY TOTALS	0.2	0.2
		•	

MICHELE R.
KEAL
NUMBER
PE-2005000711
FS/ONAL END 8/20/2024 ROUTE STATE MO DISTRICT SHEET NO COUNTY
DAVIESS & HARR JOB NO.
JNW0112
CONTRACT ID. PROJECT NO.



SUMMARY OF QUANTITIES SHEET 1 OF 2

												EFFECTIVE: 04-01-2023
	TOTAL QTY TOTAL SIGN						QTY	TOTAL SIGN	N I			ETTECTIVE: 04 01 2025
SIZE AREA QTY	AREA RELOC RELOC NUM.			SIZE	AREA QT	Y TOTAL	RELOC	RELOC NUM				
SIGN IN SQ FT EACH	SQ FT EACH SQ FT		SIGN	IN.	SQ FT EAG	CH SQ.FT.	EACH	SQ.FT.		ITEM	TOTAL	
WARN	IING SIGNS	DESCRIPTION		•	GU	IDE SIG	NS		DESCRIPTION	NUMBER	QTY	DESCRIPTION
WO1-1L 48X48 16.00		TURN (SYMBOL LEFT ARROW)	E05-1		12.00				GORE EXIT	6122008		IMPACT ATTENUATOR 40 MPH (SAND BARRELS)
WO1-1R 48X48 16.00		TURN (SYMBOL RIGHT ARROW)	E05-2		12.00				EXIT OPEN	6122009		IMPACT ATTENUATOR 45 MPH (SAND BARRELS)
WO1-2L 48X48 16.00 WO1-2R 48X48 16.00		CURVE (SYMBOL LEFT ARROW) CURVE (SYMBOL RIGHT ARROW)			10.00				ROAD WORK NEXT XX MILES	6122010		IMPACT ATTENUATOR 50 MPH (SAND BARRELS) IMPACT ATTENUATOR 55 MPH (SAND BARRELS)
WO1-3L 48X48 16.00		REVERSE TURN (SYMBOL LEFT ARROW)				64.00			END ROAD WORK	6122014		IMPACT ATTENUATOR 60 MPH (SAND BARRELS)
WO1-3R 48X48 16.00		REVERSE TURN (SYMBOL RIGHT ARROW)	GO20-4		4.50				PILOT CAR FOLLOW ME	6122017		IMPACT ATTENUATOR 65 MPH (SAND BARRELS)
WO1-4L 48X48 16.00		REVERSE CURVE (SYMBOL LEFT ARROW)	GO20-4a						PILOT CAR IN USE WAIT & FOLLOW	6122019		IMPACT ATTENUATOR 70 MPH (SAND BARRELS)
WO1 - 4R 48X48 16.00		REVERSE CURVE (SYMBOL RIGHT ARROW)	GO20 - 4a			24.00			PILOT CAR IN USE WAIT & FOLLOW	6122020		REPLACEMENT SAND BARREL
WO1-4bL 48X48 16.00 WO1-4bR 48X48 16.00		DOUBLE ARROW REVERSE CURVE (SYMBOL LT ARROWS) DOUBLE ARROW REVERSE CURVE (SYMBOL RT ARROWS)	GO20-5aP MO4-8a		6 00 4 3 3 00 4				WORK ZONE (PLAQUE) END DETOUR	6122030 6123000A		IMPACT ATTENUATOR (RELOCATION) TRUCK OR TRAILER MOUNTED ATTENUATOR (TMA)
WO1-4cL 48X48 16.00		TRIPLE ARROW REVERSE CURVE (SYMBOL LT ARROWS)	MO4-9L		5 12 00	0.00			DETOUR (LEFT ARROW)	6161008	4	ADVANCED WARNING RAIL SYSTEM
WO1-4cR 48X48 16.00		TRIPLE ARROW REVERSE CURVE (SYMBOL RT ARROWS)	MO4 - 9R	48X36	12.00				DETOUR (RIGHT ARROW)	6161012		BUOYS (BOATS KEEP OUT)
WO1-6 60X30 12.50		HORIZONTAL ARROW (SYMBOL)			4.00				STREET NAME (PLAQUE)	6161013		BUOYS (NO WAKE)
WO1-6a 72X36 18.00		HORIZ. ARROW (SYMBOL ON PERMANENT BARRICADE)	MO4 - 10L						DETOUR (ARROW LEFT)	6161014	242	SPECIAL SIGN ASSEMBLY (BOATS KEEP OUT)
WO1-7 60X30 12.50 WO1-7a 72X36 18.00		DOUBLE HEAD HORIZONTAL ARROW (SYMBOL) DOUBLE HEAD HORIZ. ARROW (SYMBOL ON PERM. BARR.)	MO4 - 10R	48X18		 GULATOF	Y SIG	NS	DETOUR (ARROW RIGHT)	6161025 6161030	212	CHANNELIZER (TRIM LINE) TYPE III MOVEABLE BARRICADE
WO1-8 18X24 3.00		CHEVRON (SYMBOL)	R1 - 1	48X48	3 13 . 25	J	1 310		STOP	6161033	27	DIRECTION INDICATOR BARRICADE
WO1-8a 30X36 7.50		CHEVRON (SYMBOL FOR DIVIDED HIGHWAYS)	R1-2		6.93				YIELD	6161040	4	FLASHING ARROW PANEL
WO3-1 48X48 16.00		STOP AHEAD (SYMBOL)	R1-2a		9.00				TO ONCOMING TRAFFIC (PLAQUE)	6161047		TYPE III OBJECT MARKER
WO3-2 48X48 16.00		YIELD AHEAD (SYMBOL)	R1-3P		2.50	0000			ALL WAY (PLAQUE)	6161055		SEQUENTIAL FLASHING WARNING LIGHT
WO3-3 48X48 16.00 WO3-4 48X48 16.00		SIGNAL AHEAD (SYMBOL) BE PREPARED TO STOP	R2 - 1 R3 - 1		3 12.00 8 3 16.00	96.00			SPEED LIMIT XX NO RIGHT TURN (SYMBOL)	6161070 6161095		TUBULAR MARKER RADAR SPEED ADVISORY SYSTEM
	64.00	SPEED LIMIT AHEAD	R3-1		16.00				NO LEFT TURN (SYMBOL)	0101032		CHANGEABLE MESSAGE SIGN,
WO4-1L 48X48 16.00		MERGE (SYMBOL FROM LEFT)	R3-3		9.00				NO TURNS	6161096		COMMISSION FURNISHED/RETAINED
WO4-1R 48X48 16.00		MERGE (SYMBOL FROM RIGHT)	R3-4	48X48	16.00				NO U-TURN (SYMBOL)			CHANGEABLE MESSAGE SIGN W/O COMM.
WO4-1aL 48X48 16.00		MERGE (ARROW SYMBOL)	R3-7L		6.25				LEFT LANE MUST TURN LEFT	6161098A	8**	INTERFACE - CONTRACTOR FURNISHED/RETAINED
	64.00	MERGE (ARROW SYMBOL)	R3 - 7R		6.25				RIGHT LANE MUST TURN RIGHT	6161000		CHANGEABLE MESSAGE SIGN WITH COMM.
WO5-1 48X48 16.00 WO5-3 48X48 16.00		ROAD/BRIDGE/RAMP NARROWS ONE LANE BRIDGE	R4 - 1 R4 - 2		3 12.00 3 12.00				DO NOT PASS PASS WITH CARE	6161099 6162000A		INTERFACE - CONTRACTOR FURNISHED/RETAINED WORK ZONE TRAFFIC SIGNAL SYSTEM
WO5-5 48X48 16.00		NARROW LANES	R4-8a		3 12.00				KEEP LEFT (HORIZONTAL ARROW)	6162002		TEMPORARY LONG-TERM RUMBLE STRIPS
WO6-1 48X48 16.00		DIVIDED HIGHWAY (SYMBOL)	R4 - 7a		12.00				KEEP RIGHT (HORIZONTAL ARROW)			TEMPORARY TRAFFIC BARRIER
WO6-2 48X48 16.00		DIVIDED HIGHWAY END (SYMBOL)	R5-1		6.25				DO NOT ENTER	6173600D	4	CONTRACTOR FURNISHED/RETAINED
WO6-3 48X48 16.00		TWO WAY TRAFFIC (SYMBOL)	R5-1a		6.00				WRONG WAY			TEMPORARY TRAFFIC BARRIER
WO7-3a 30X24 5.00 WO8-1 48X48 16.00		NEXT XX MILES (PLAQUE) BUMP	R6-1L R6-1R		6.75				ONE WAY ARROW (LEFT) ONE WAY ARROW (RIGHT)	6173602B 6174000A		CONTRACTOR FURNISHED/COMMISSION RETAINED TEMP. TRAFFIC BARRIER HEIGHT TRANSITION
WO8-2 48X48 16.00		DIP	R6-2L		5.00				ONE WAY (LEFT)	6175010A		RELOCATING TEMPORARY TRAFFIC BARRIER
WO8-3 48X48 16.00		PAVEMENT ENDS	R6-2R		5.00				ONE WAY (RIGHT)	01700107		TEMPORARY TRAFFIC BARRIER
WO8-4 48X48 16.00		SOFT SHOULDER	R9-9	24X12	2.00				SIDEWALK CLOSED	6176000B		COMMISSION FURNISHED/RETAINED
WO8-5 48X48 16.00		SLIPPERY WHEN WET (SYMBOL)							SIDEWALK CLOSED AHEAD,			TEMP. TRAFFIC BARRIER HEIGHT TRANSITION
WO8-6 48X48 16.00 WO8-6c 48X48 16.00		TRUCK CROSSING (WITH FLAGS) TRUCK ENTRANCE	R9-11L	24X18	3.00				(ARROW LEFT) CROSS HERE SIDEWALK CLOSED AHEAD,	6177000B 6208064A		COMMISSION FURNISHED/RETAINED TEMPORARY RAISED PAVEMENT MARKER
WO8-7 36X36 9.00		LOOSE GRAVEL	R9-11R	24X18	3.00				(ARROW RIGHT) CROSS HERE	9029400		TEMPORARY TRAFFIC SIGNALS
WO8-7a 36X36 9.00		FRESH OIL/LOOSE GRAVEL			6.00				STOP HERE ON RED (45^ ARROW)	9029401		TEMPORARY TRAFFIC SIGNALS AND LIGHTING
WO8-9 48X48 16.00		LOW SHOULDER	R11-2	48X30	10.00 10	100.00			ROAD CLOSED			
WO8-11 48X48 16.00		UNEVEN LANES							ROAD CLOSED XX MILES AHEAD			
WO8-12 48X48 16.00		NO CENTER LINE	R11-3a R11-4						LOCAL TRAFFIC ONLY			
WO8-15 48X48 16.00 WO8-15P 30X24 5.00		GROOVED PAVEMENT MOTORCYCLE (PLAQUE)	CONST - 3A						ROAD CLOSED TO THRU TRAFFIC FINE SIGN			
WO8-17 48X48 16.00		SHOULDER DROP-OFF (SYMBOL)	CONST - 3X						SPEEDING/PASSING (PLATE)			
WO8-17P 30X24 5.00		SHOULDER DROP-OFF (PLAQUE)			MI	SCELLA	IEOUS	SIGNS				
W10-1 42RND. 9.62		RAILROAD CROSSING	CONST-5						POINT OF PRESENCE	_		
WO12-1 24X24 4.00		DOUBLE DOWN ARROW (SYMBOL)	CONST - 5						POINT OF PRESENCE			9.01 - MISC. LUMP SUM
WO12-2 48X48 16.00 W012-2X 24X18 3.00		LOW CLEARANCE (SYMBOL) LOW CLEARANCE (PLAQUE)	CONST - 7						RATE OUR WORK ZONE RATE OUR WORK ZONE	-	I EMP	ORARY TRAFFIC CONTROL LUMP SUM = 1
WO12-2a 84X24 14.00		OVERHEAD LOW CLEARANCE (FEET AND INCHES)	CONST-8						WORK ZONE NO PHONE ZONE	L	*NOT I	
WO12-4 120X60 50.00		LOW CLEARANCE XX FT XX IN XX MILES AHEAD			19.50 2			+	DETOUR ASSEMBLY WEST/LEFT			NCLUDED IN TEMPORARY TRAFFIC - 1 LUMP SUM QUANTITY
WO12-5 120X60 50.00		WIDTH RESTRICTION XX FT XX IN XX MILES AHEAD			19.50 2			+	DETOUR ASSEMBLY EAST/LEFT		IOTE - A	LL TEMPORARY TRAFFIC CONTROL ITEMS
WO13-1 30X30 6.25		ADVISORY SPEED (PLAQUE)			19.50 2				DETOUR ASSEMBLY WEST/RT DIAG.	A	RE INC	LUDED IN PAY ITEM 616-99.01, MISC.
WO16-2 30X24 5.00 WO16-3 30X24 5.00		XXX FEET (PLAQUE) X MILE (PLAQUE)			3 19.50 2 3 19.50 1				DETOUR ASSEMBLY EAST/RT DIAG. DETOUR ASSEMBLY WEST/RIGHT			M TEMPORARY TRAFFIC CONTROL, UNLESS SE NOTED. QUANTITIES SHOWN HAVE BEEN
WO20-1 48X48 16.00 12		ROAD/BRIDGE/RAMP WORK AHEAD			19 50 1			50F		E	STIMAT	E FOR BOTH BRIDGE CLOSURES AND ARE
WO20-2 48X48 16.00 4	64.00	DETOUR AHEAD	SPECIAL	36X78	19.50 1	19.50		50G	DETOUR ASSEMBLY WEST/STRAIGHT	9	UBJECT	TO CHANGE BASED ON FIELD CONDITIONS.
WO20-3 48X48 16.00 12		ROAD CLOSED AHEAD			19.50 1			+	DETOUR ASSEMBLY EAST/RIGHT			
WO20-4 48X48 16.00		ONE LANE ROAD AHEAD			19.50 2			+	DETOUR ASSEMBLY WEST/LEFT			
WO20-5 48X48 16.00 8 WO20-5a 48X48 16.00		RIGHT/CENTER/LEFT LANE CLOSED AHEAD 2 RIGHT/CENTER/LEFT LANES CLOSED AHEAD			3 19.50 2 3 19.50 2				DETOUR ASSEMBLY EAST/LEFT DETOUR ASSEMBLY WEST/RT DIAG.			
WO20-6a 48X48 16.00 4		RIGHT/CENTER/LEFT LANE CLOSED AREAD			19 50 2			50L				
WO20-7a 48X48 16.00		FLAGGER (SYMBOL, WITH FLAGS)			19 50 1				DETOUR ASSEMBLY WEST/RIGHT			
WO21-2 36X36 9.00		FRESH OIL			19.50 1				DETOUR ASSEMBLY EAST/RIGHT			
WO21-5 48X48 16.00 8		SHOULDER WORK AHEAD			19.50 1				DETOUR ASSEMBLY WEST/STRAIGHT			
WO22-1 48X48 16.00 WO22-2 42X36 10.50		BLASTING ZONE AHEAD TURN OFF 2-WAY RADIO AND PHONE	SPECIAL 616-10		3 19.50 1	19.50 TOTAL		50Q	DETOUR ASSEMBLY EAST/STRAIGHT			
WO22-2 42X36 10.50 WO22-3 42X36 10.50		END BLASTING ZONE			ON SIGNS		4					SUMMARY OF QUANTITIES
WO24-1 48X48 16.00		DOUBLE REVERSE CURVE	616-10		J.1 J I J I J I J	F 5 5 5 . 0 1	1	TOTAL				SHEET 2 OF 2
			RELOCA		S I GNS							

STATE OF MISSOL MICHELE R. KEAL NUMBER PE-2005000711 PE-2005000711 25

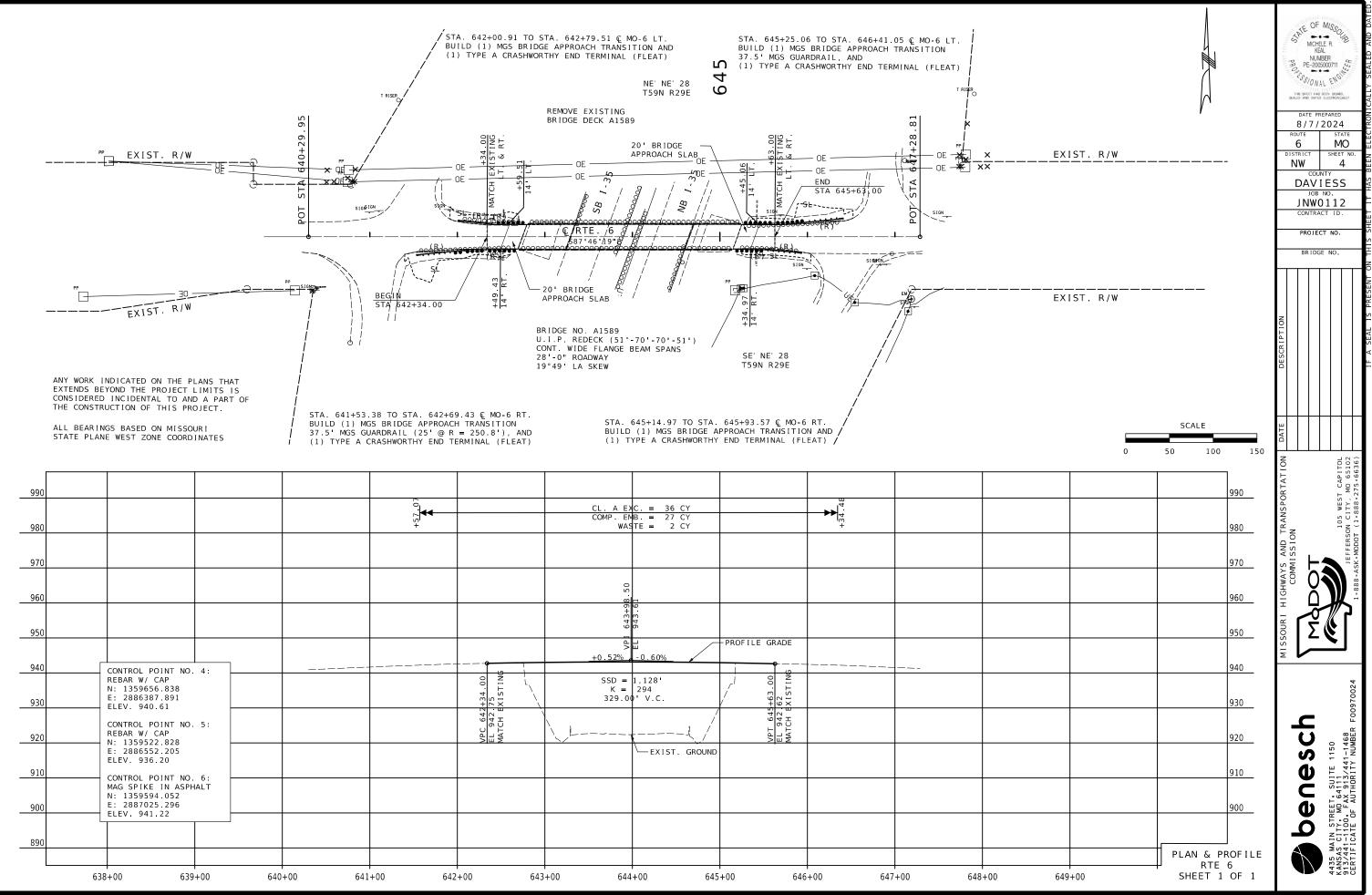
8/21/2024 6/N MO SHEET NO DISTRICT NW 3

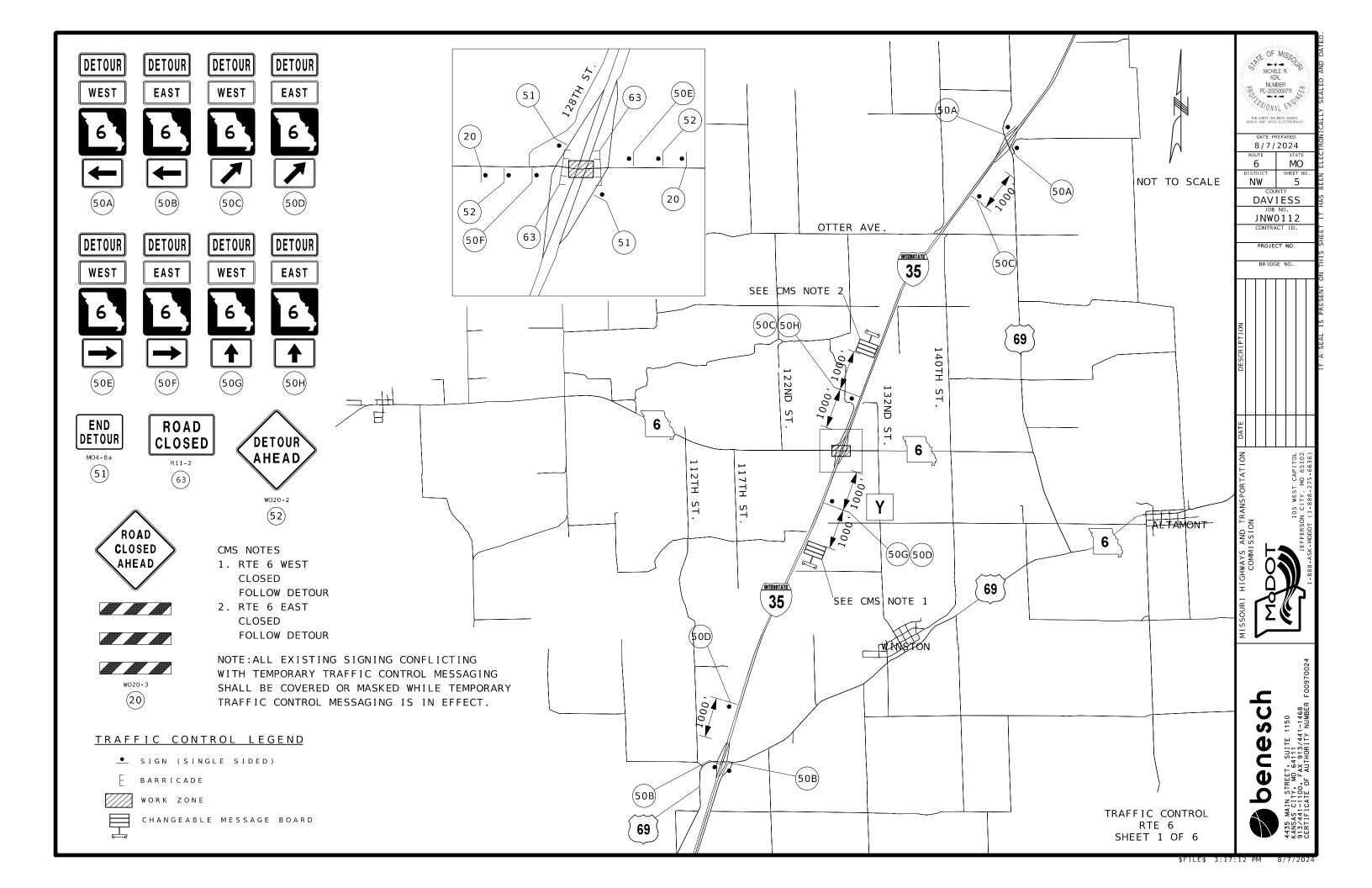
DAVIESS & HARR JNW0112

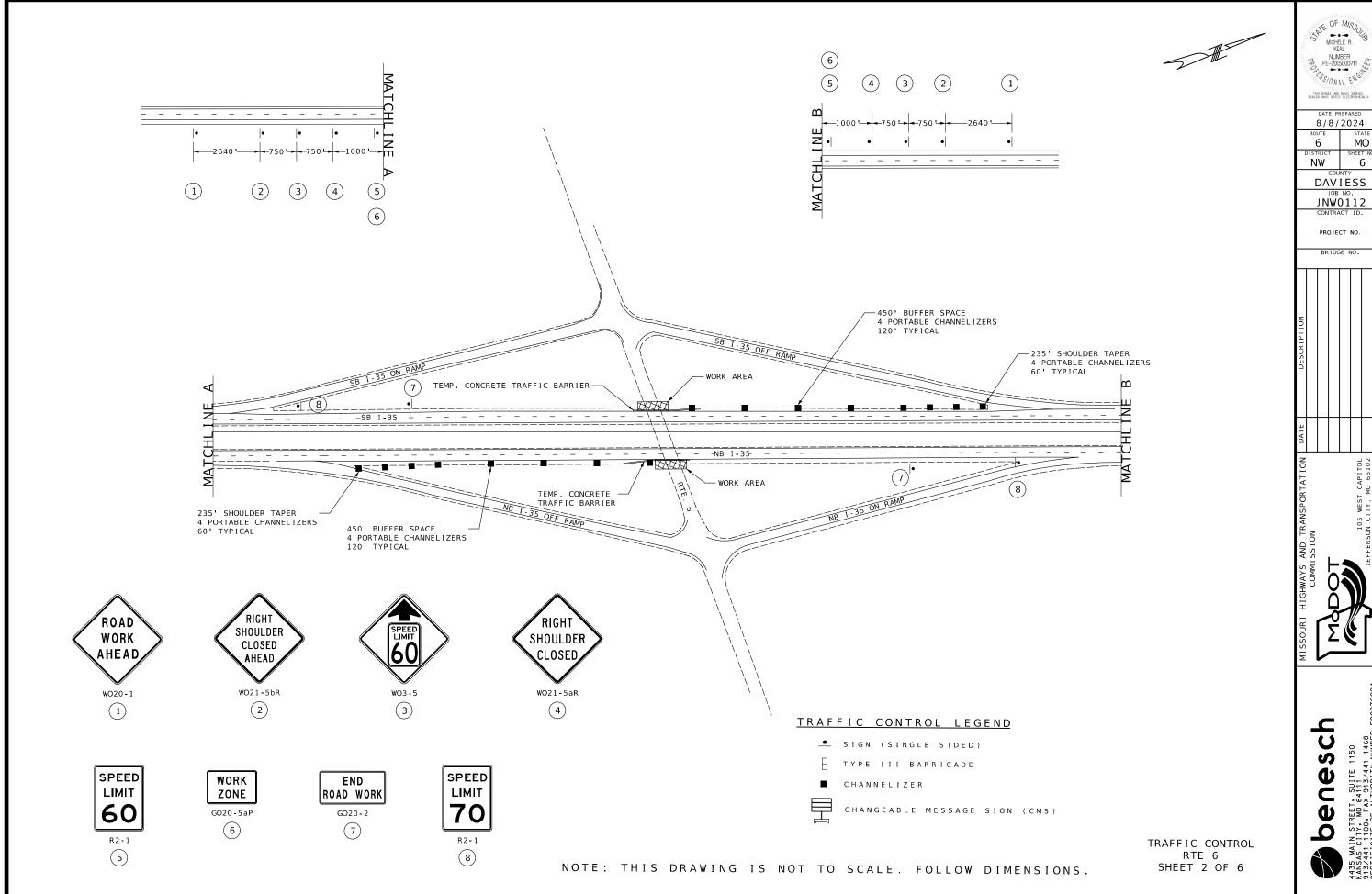
PROJECT NO.

BRIDGE NO.

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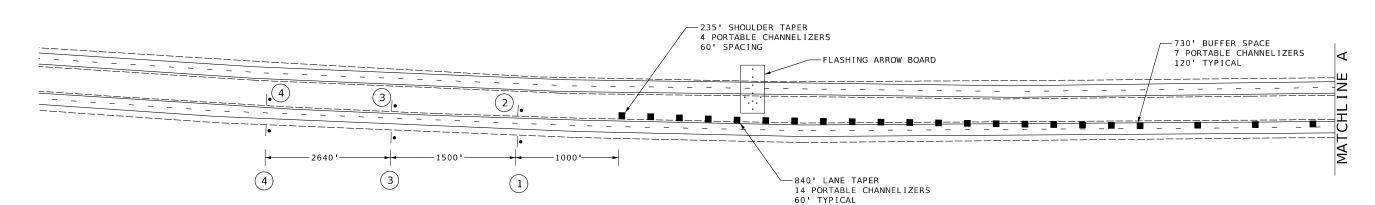












TRAFFIC CONTROL LEGEND

SIGN (SINGLE SIDED)

TYPE III BARRICADE

CHANNELIZER

CHANGEABLE MESSAGE SIGN (CMS)

TRAFFIC CONTROL RTE 6 SHEET 3 OF 6 MISSOURI HIGHWAYS AND TRANSPORTATION DATE

COMMISSION

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MICHELE R. KEAL NUMBER PE-2005000711

8/8/2024

JOB NO.
JNW0112

PROJECT NO.

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SHEET NO

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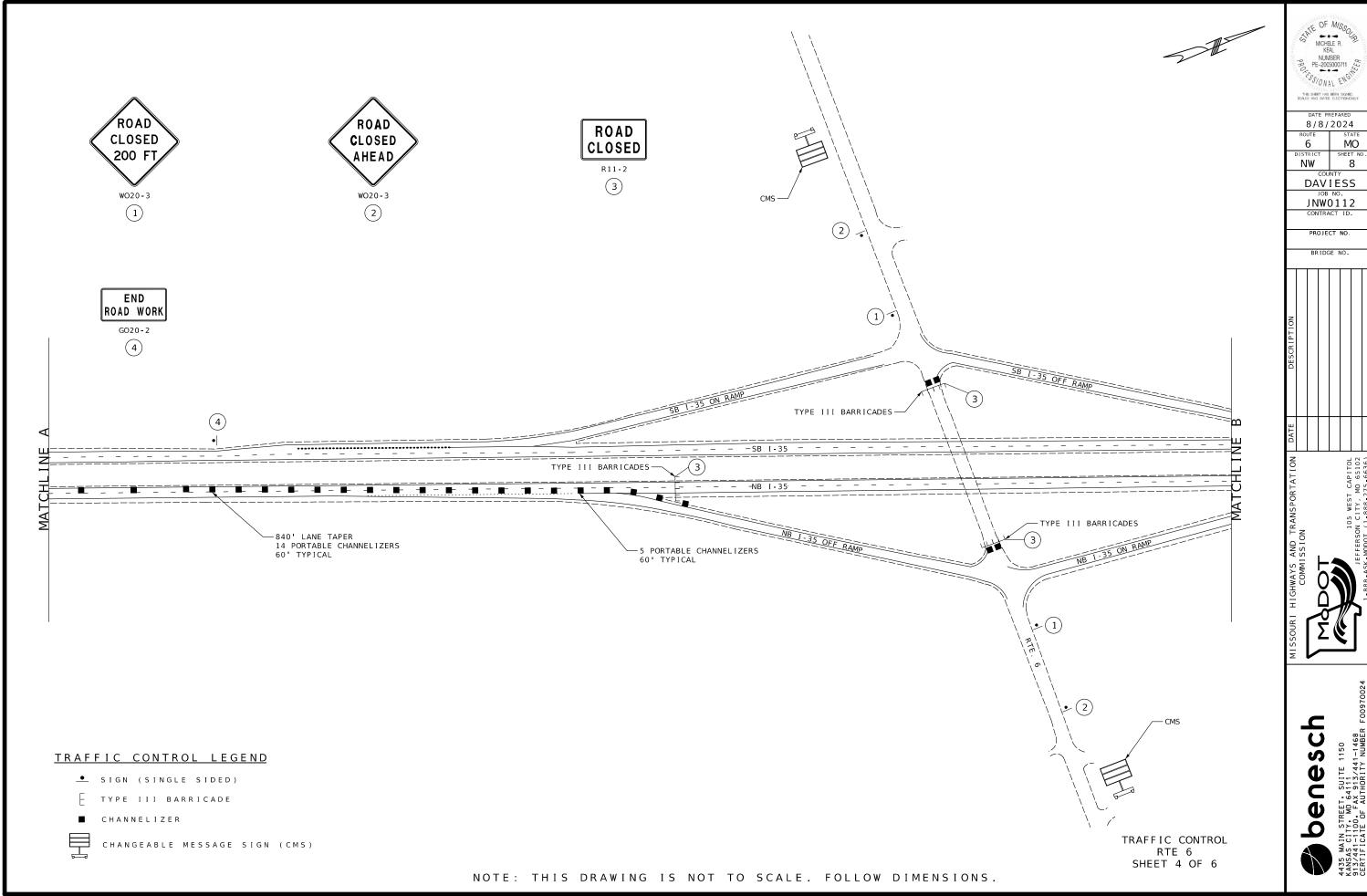
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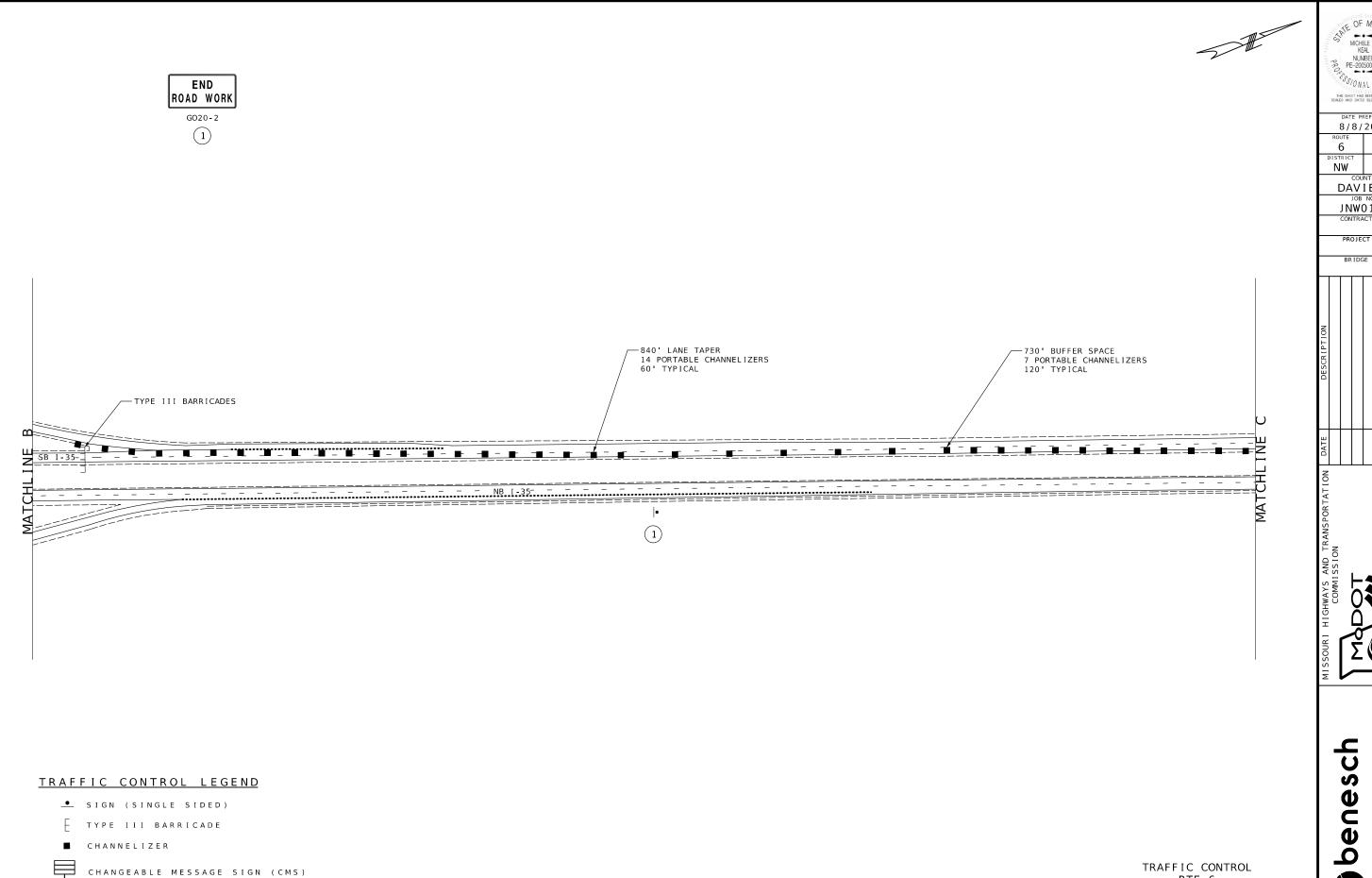
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NOTE: THIS DRAWING IS NOT TO SCALE. FOLLOW DIMENSIONS.

MICHELE R. KEAL NUMBER PE-2005000711 8/8/2024 MO SHEET NO 9 DAVIESS JNW0112 PROJECT NO.

TRAFFIC CONTROL RTE 6 SHEET 5 OF 6





MICHELE R. KEAL NUMBER PE-2005000711 NUMBEH PE-2005000711 ET

8/8/2024

DAVIESS JNW0112

PROJECT NO.

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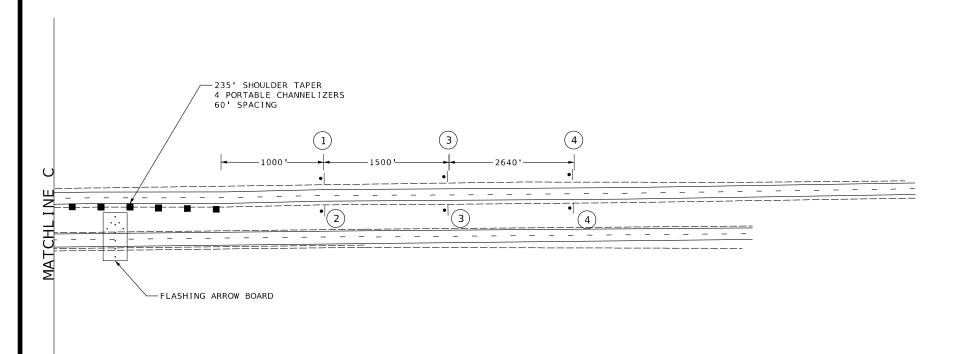
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TRAFFIC CONTROL LEGEND

SIGN (SINGLE SIDED)

TYPE III BARRICADE

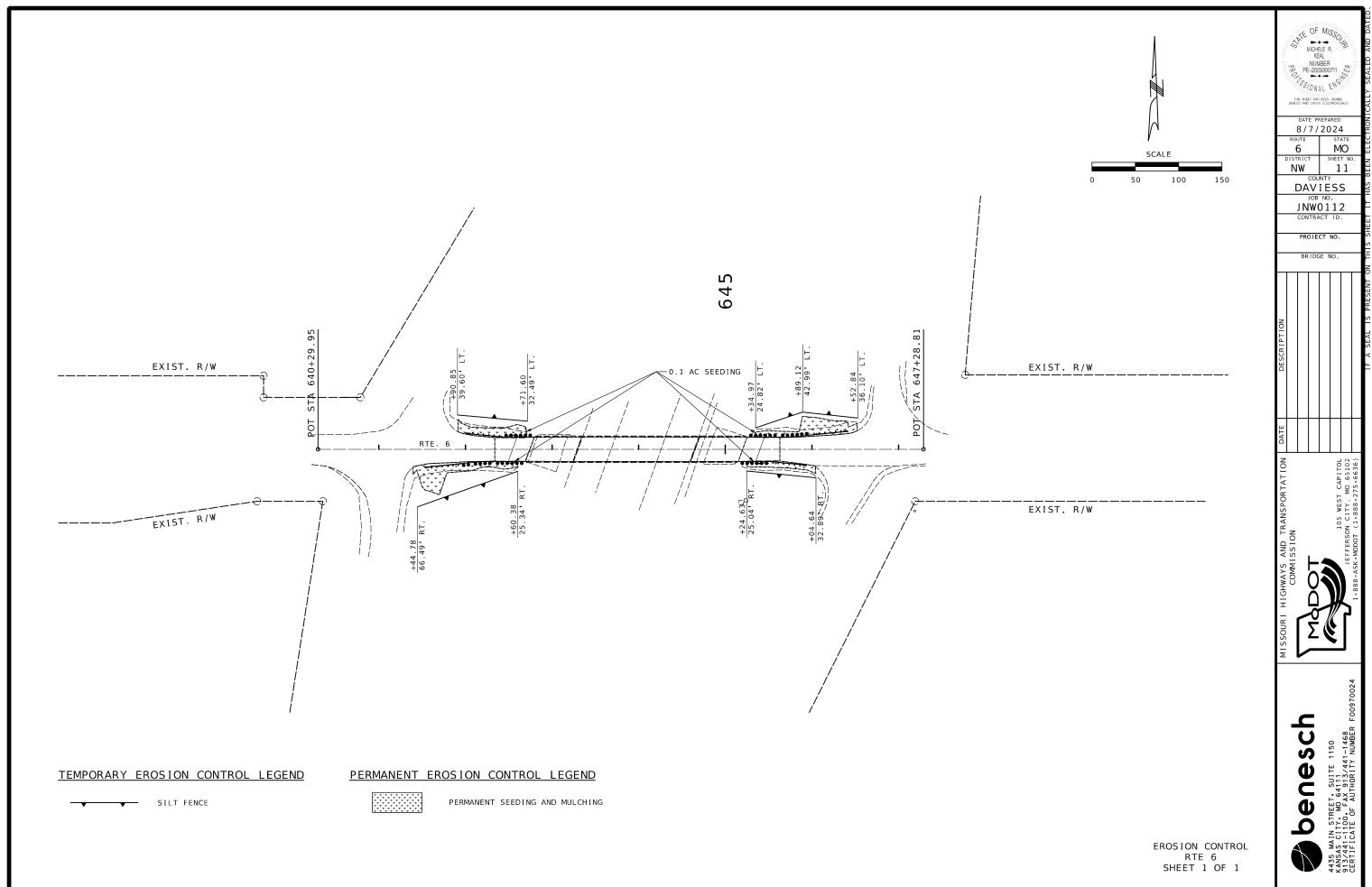
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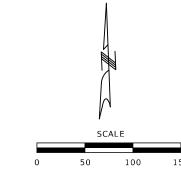
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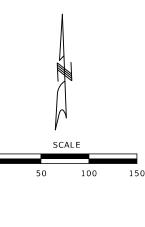
TRAFFIC CONTROL RTE 6 SHEET 6 OF 6

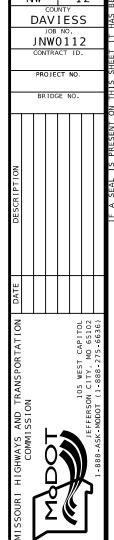
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MOHELE R.
KEAL
NUMBER
PE-2005000711

DATE PREPARED

8 / 7 / 2024

ROUTE

6 MO

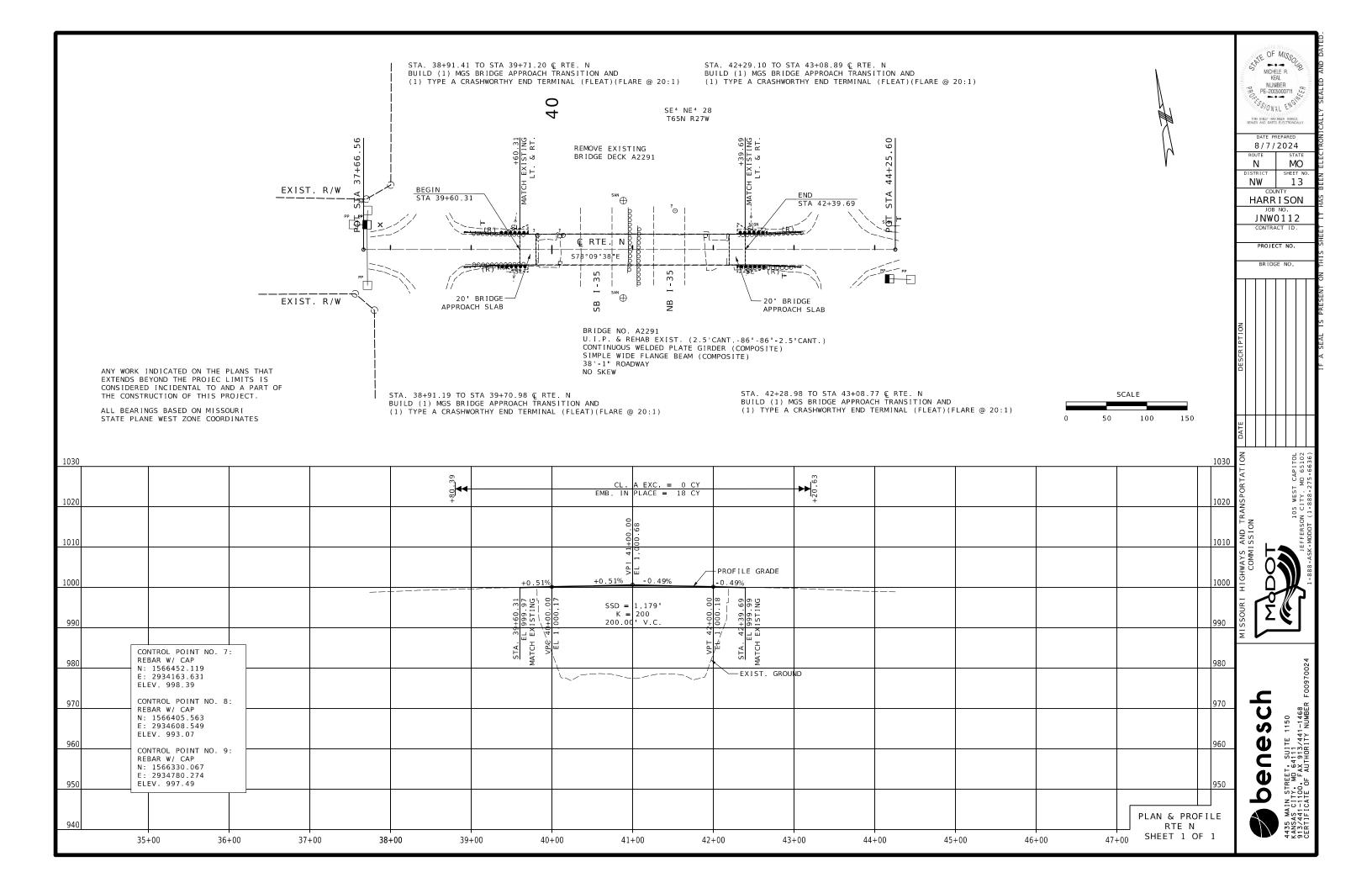
DISTRICT SHEET NO.

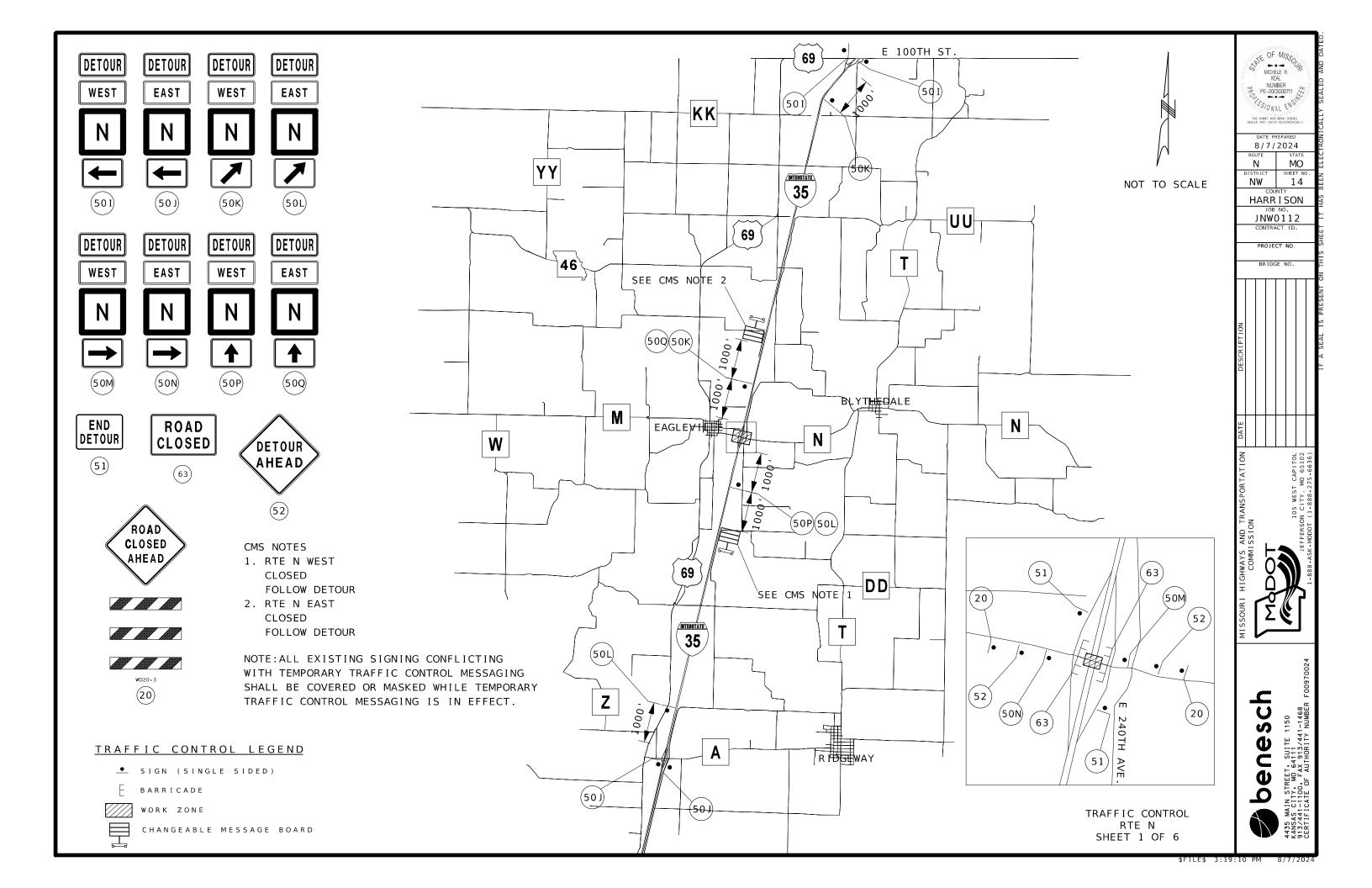
NW 12

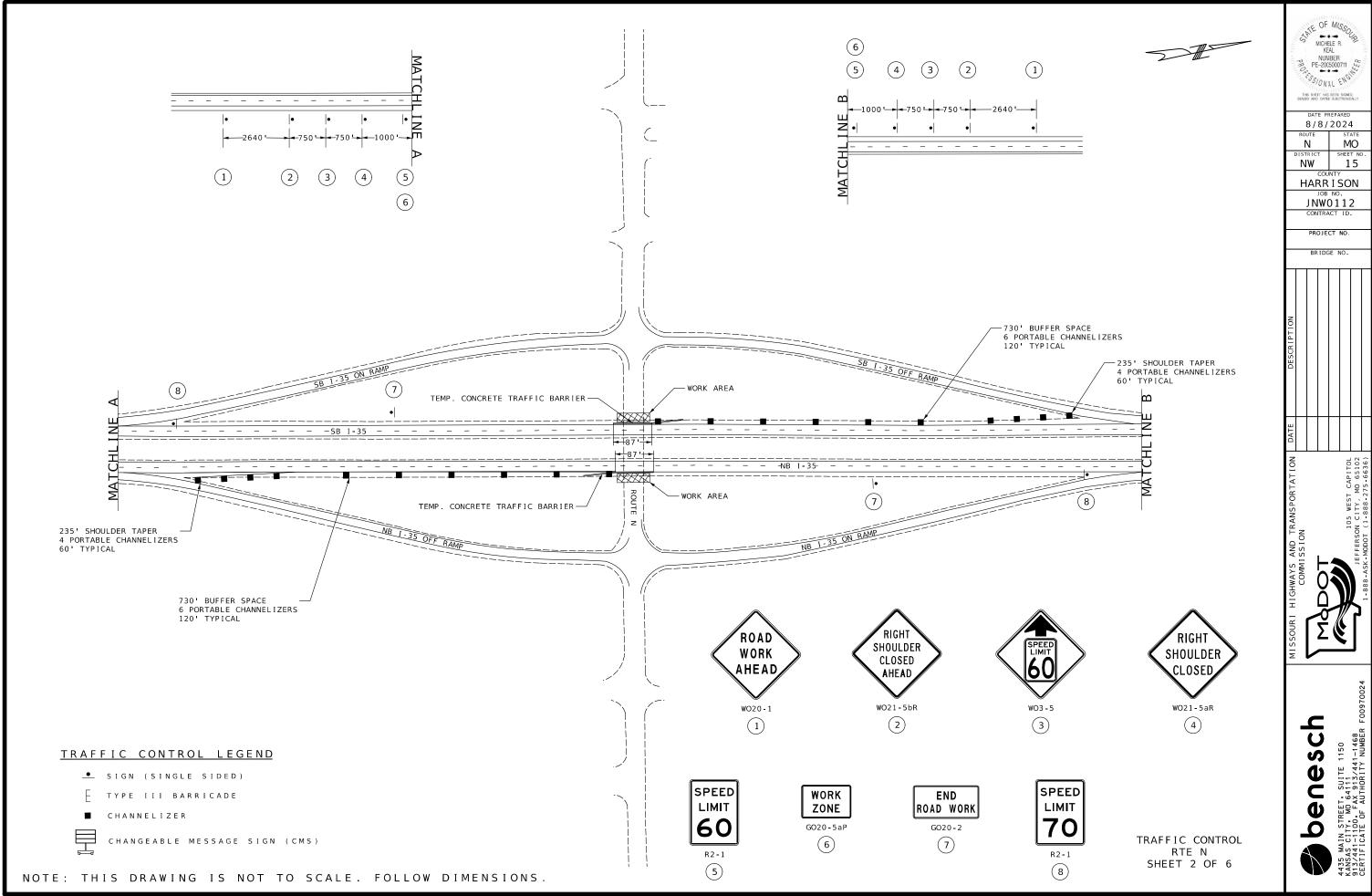
POT STA 640+29.95	\$TA. 642+34.00 WATCH EXISTING PAVEMENT MARKING	645 645 5TA. 645+63.00 MATCH EXISTING PAVEMENT MARKING	POT STA 647+28.81
4	RTE. 6		
	4" S.W. EDGE LINE— 4" S.Y. CENTERLINE, LT. &— 4" I.Y. CENTERLINE, RT.		

PAVEMENT MARKING RTE 6 SHEET 1 OF 1

benesch







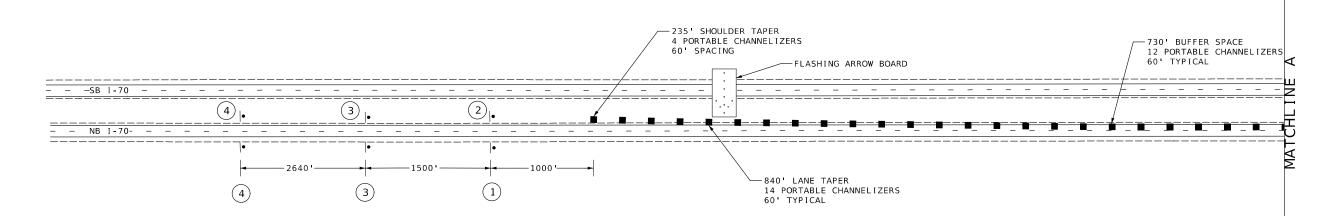












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TRAFFIC CONTROL LEGEND

SIGN (SINGLE SIDED)

TYPE III BARRICADE

CHANNELIZER

CHANGEABLE MESSAGE SIGN (CMS)

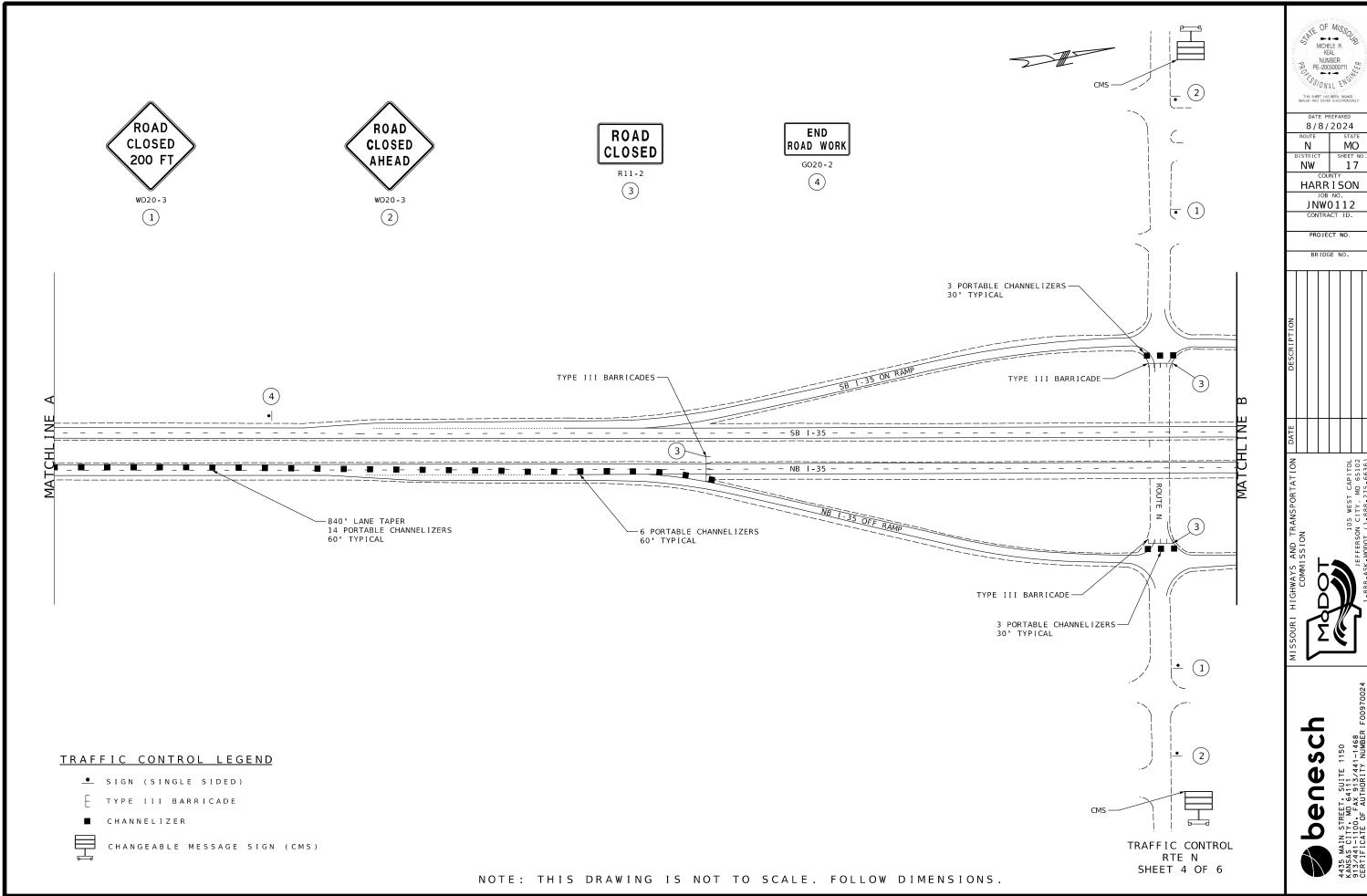
TRAFFIC CONTROL RTE N SHEET 3 OF 6 COMMISSION
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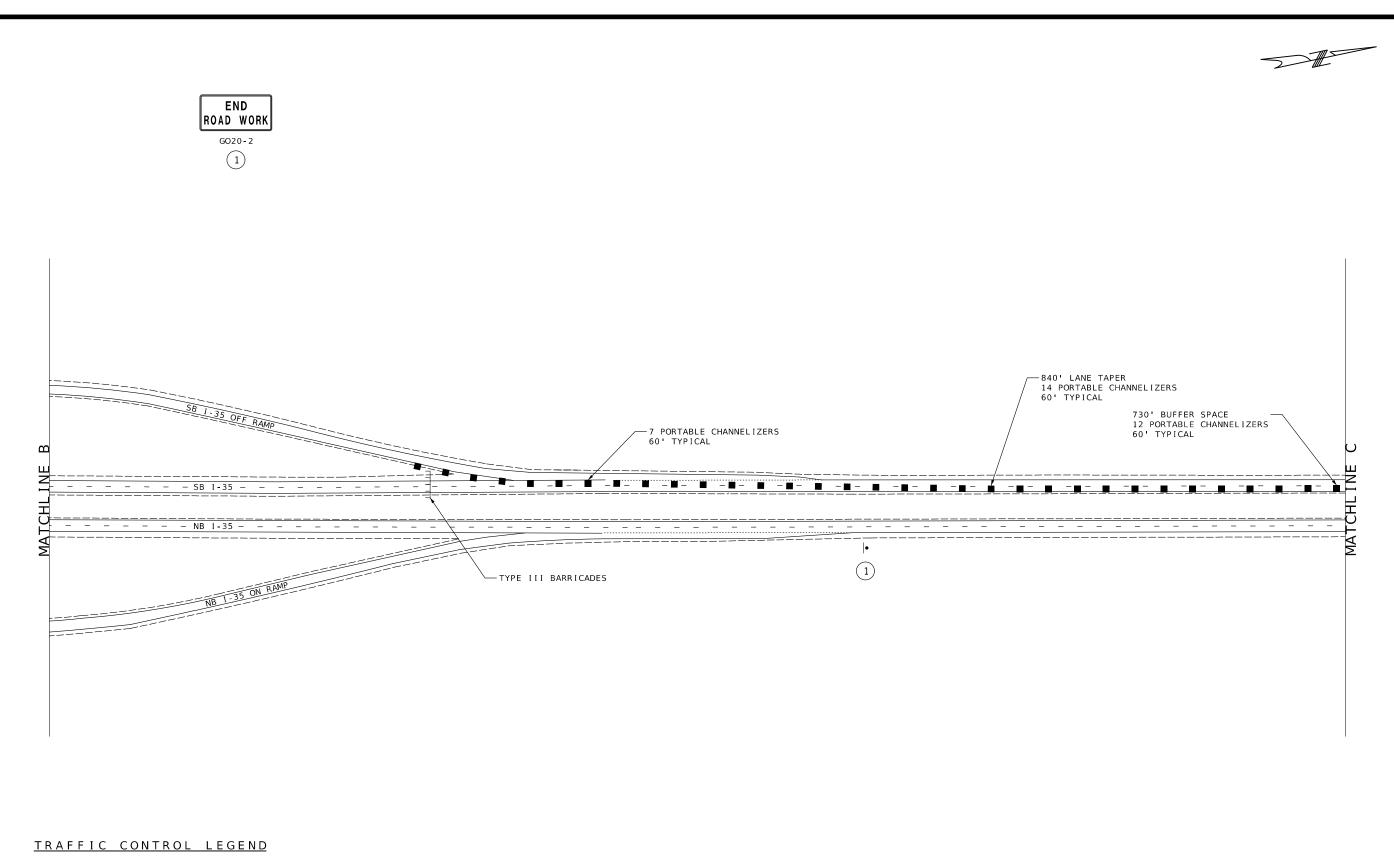
MICHELE R. KEAL NUMBER PE-2005000711

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8/8/2024

SEMAIN STREET. SUITE 1150INSARAT-1100. FAX 913/41-1468





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TYPE III BARRICADE

■ CHANNELIZER

CHANGEABLE MESSAGE SIGN (CMS)

TRAFFIC CONTROL RTE N SHEET 5 OF 6

8/8/2024 N MO SHEET NO NW 18 **HARRISON** JNW0112 PROJECT NO.

FE-2005000711

benesch 4435 MAIN STRET, SUITE 1150 KANSAS CITY, WG 64111 913.441-1100, FAX 913.441-1468 CERTIFICATE OF ANTHORITY NUMBER F009700













235' SHOULDER TAPER 4 PORTABLE CHANNELIZERS 60' SPACING (3) 840' LANE TAPER 14 PORTABLE CHANNELIZERS 60' SPACING 3° (4)°

-FLASHING ARROW BOARD

TRAFFIC CONTROL LEGEND

SIGN (SINGLE SIDED)

TYPE III BARRICADE

CHANNELIZER

CHANGEABLE MESSAGE SIGN (CMS)

TRAFFIC CONTROL RTE N SHEET 6 OF 6

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MICHELE R. KEAL.
NUMBER
PE-2005000711

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8/8/2024

HARRISON

JNW0112

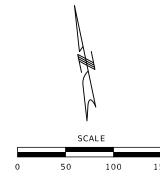
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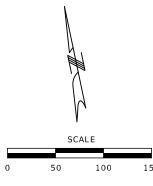
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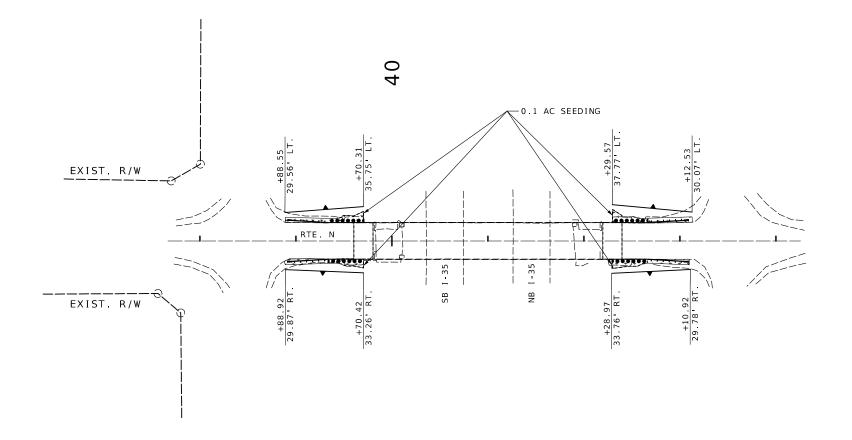
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TEMPORARY EROSION CONTROL LEGEND

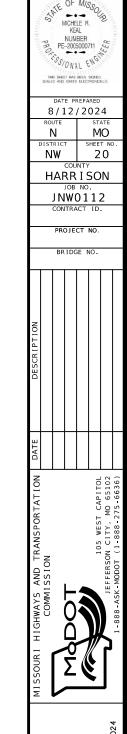
SILT FENCE

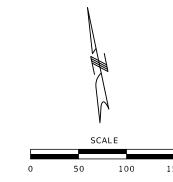
PERMANENT EROSION CONTROL LEGEND

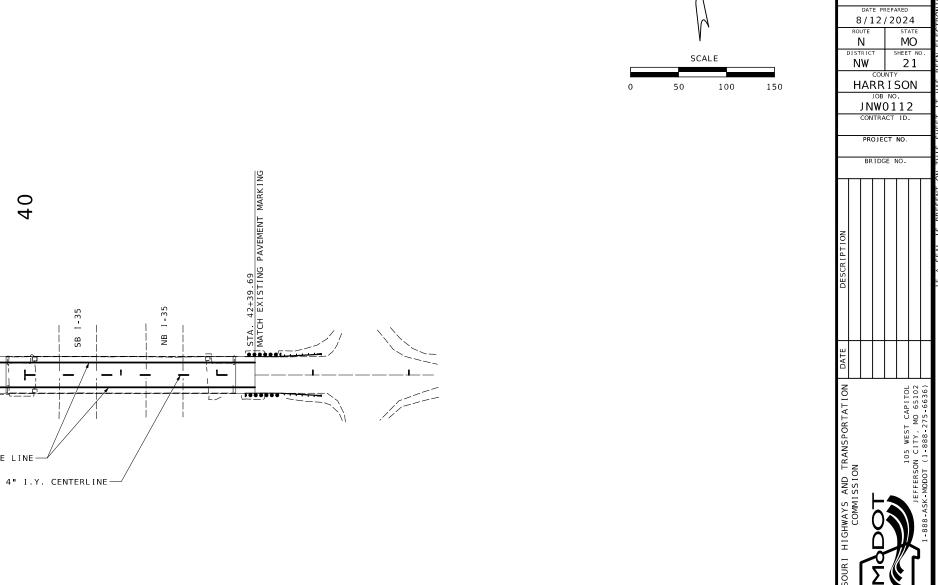


PERMANENT SEEDING AND MULCHING

EROSION CONTROL RTE N SHEET 1 OF 1







) STA. 39+60.31 MATCH EXISTING PAVEMENT MARKING

4" S.W. EDGE LINE

40

SB 1-35

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MICHELE R.
KEAL
NUMBER
PE-2005000711

PAVEMENT MARKING RTE N SHEET 1 OF 1

	Table Showing S2 and S3 Bar Lengths												
Int. Bei	nt No. 2	Int. Bei	nt No. 3	Int. Bent No. 4									
Span 1	Span 2	Span 2	Span 3	Span 3	Span 4								
16'-9"	19'-6"	19'-6"	19'-6"	19'-6"	16'-9"								

	Lap Length Splices **
Bar Size	Splice Length
4	2'-7"
5	3'-3"
6	3'-10"
7	4'-11"

** Unless otherwise shown.

General Notes:

Design Specifications: 2002 AASHTO LFD (17th Ed.) Standard Specifications Seismic Performance Category A

Design Loading:

H15-44 (1961) (Existing) HS20-44 (New Construction) 35 lb/sf Future Wearing Surface Earth - 120 lb/cf, Equivalent Fluid Pressure 45 lb/cf Fatigue Stress - Case III

Design Unit Stresses:

Class B-1 Concrete (Barrier) f'c = 4,000 psiClass B-2 Concrete (End Bents & Superstructure, except Barrier) f'c = 4,000 psiReinforcing Steel (Grade 60) fy = 60,000 psi

Joint Filler:

All joint filler shall be in accordance with Sec 1057 for preformed sponge rubber expansion and partition joint filler, except as noted.

Reinforcing Steel:

Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise shown. #4-56 to be placed at 12" cts. where haunch exceeds 2 inches in shear stud reigions.

Bars bonded in existing concrete not removed shall be cleanly stripped and embedded into new concrete where possible. If length is available, existing bars shall extend into new concrete at least 40 diameters for plain bars and 30 diameters for deformed bars, unless otherwise noted.

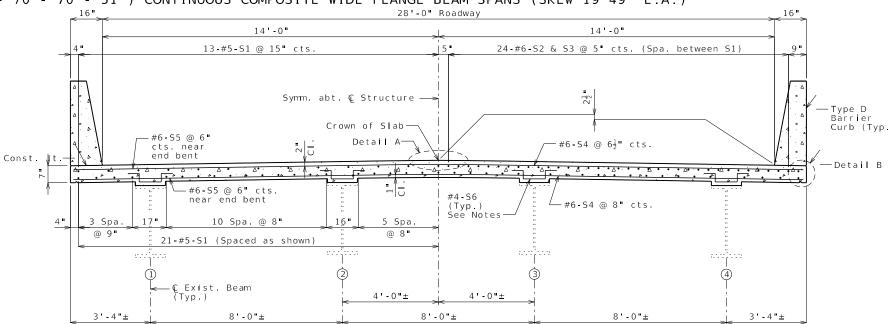
Roadway surfacing adjacent to bridge ends shall match new bridge slab surface. (Roadway item)

Outline of existing work is indicated by light dashed lines. Heavy

Contractor shall verify all dimensions in field before ordering new material.

The area exposed by the removal of concrete and not covered with new concrete shall be coated with an approved qualified special mortar in accordance with Sec 704.

Structure to be closed during construction. Traffic to be maintained on other routes during construction. See roadway plans for traffic control.



HALF SECTION NEAR MIDSPAN AND END BENT

HALF SECTION NEAR INT. BENT

TYPICAL SECTION THRU SLAB

Estimated Quantiti	es	
I t em		Total
Removal of Miscellaneous ACM (Non-Friable)	sq. foot	23
Removal of Existing Bridge Deck	sq. foot	7,551
Partial Removal of Substructure Concrete	lump sum	1
Bridge Approach Slab (Minor)	sq. yard	127
Slab on Steel	sq. yard	837
Type D Barrier	linear foot	527
Substructure Repair (Unformed)	sq. foot	15
Slab Drain	each	20
Non-Destructive Testing	linear foot	74
Vertical Drain at End Bents	each	2
Open Cell Foam Joint Seal	linear foot	57

Cost of any required excavation for bridge will be considered completely covered by the contract unit price for other items

Estimated Quantities fo	or Slab on St	ee I										
I t em		Total										
Class B-2 Concrete	cu. yard	190										
Reinforcing Steel (Epoxy Coated)												

The table of Estimated Quantities for Slab on Steel represents the quantities used by the State in preparing the cost estimate for concrete slabs. The area of the concrete slab will be measured to the nearest square yard longitudinally from end of slab to end of slab and transversely from out to out of bridge slab (or with the horizontal dimensions as shown on the plan of slab). Payment for stay-in-place corrugated steel forms, conventional forms, all concrete and epoxy coated reinforcing steel will be considered completely covered by the contract unit price for the slab. Variations may be encountered in the estimated quantities but the variations cannot be used for an adjustment in the contract unit price.

Method of forming the slab shall be in accordance with Sec 703. All hardware for forming the slab to be left in place as a permanent part of the structure shall be coated in accordance with ASTM A123 or ASTM B633 with a thickness Class SC 4 and a finish Type I, Il or III.

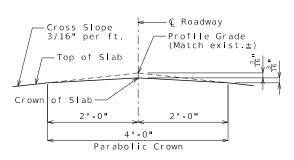
Slab shall be cast-in-place with conventional forming or stay-in-place corrugated steel forms. Precast prestressed panels will not be permitted.

Bridge deck suface may be finished with a vibratory screed. For Optional Stay-In-Place Form Details, see Sheet No. 2.

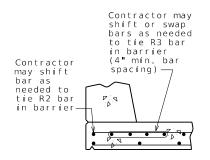
REPAIRS TO BRIDGE: ROUTE 6 OVER ROUTE I-35

ROUTE 6 FROM FROM ROUTE EE TO ROUTE Y ABOUT 3.0 MILES EAST OF ROUTE EE BEGINNING STATION 642+74.50± (MATCH EXISTING)

-Const. Jt. = C 3/4" Drip Groove (Typ) DETAIL B



DETAIL A



OPTIONAL SHIFTING TOP BARS AT BARRIER

DESIGNED BY: KLW MAR 2024 DETAILED BY: JTC MAR 2024 CHECKED BY: DMA APR 2024

NUMBER

E-25019

9/27/2024

DAVIESS

JNW0112

PROJECT NO

A15892

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SHEET NO

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Stav-In-Place Forms:

Corrugated steel forms, supports, closure elements and accessories shall be in accordance with grade requirement and coating designation G165 of ASTM A653. Complete shop drawings of the permanent steel deck forms shall be required in accordance with Sec 1080.

Corrugations of stay-in-place forms shall be filled with an expanded polystyrene material. The polystyrene material shall be placed in the forms with an adhesive in accordance with the manufacturer's recommendations.

Form sheets shall not rest directly on the top of beam flanges. Sheets shall be securely fastened to form supports with a minimum bearing length of one inch on each end. Form supports shall be placed in direct contact with the flange. Welding on or drilling holes in the beam flanges will not be permitted. All steel fabrication and construction shall be in accordance with Sec 1080 and 712. Certified field welders will not be required for welding of the form supports.

The design of stay-in-place corrugated steel forms is per manufacturer which shall be in accordance with Sec 703 for false work and forms. Maximum actual weight of corrugated steel forms allowed shall be 4 psf assumed for beam loading.

The contractor shall provide a method of preventing the direct contact of the stay-in-place forms and connection components with uncoated weathering steel members that is approved by the engineer.

Pouring and Finishing Slab:

The contractor shall provide bracing necessary for lateral and torsional stability of the beams during construction of the concrete slab and remove the bracing after the slab has attained 75% design strength. Contractor shall not weld on or drill holes in the beams. The cost for furnishing, installing, and removing bracing will be considered completely covered by the contract unit price for Slab on

Slab shall be poured upgrade from end to end at a minimum rate of 25 cubic yards per hour.

Alternate pour sequences may be submitted to the engineer for approval. Keyed construction joints shall be provided

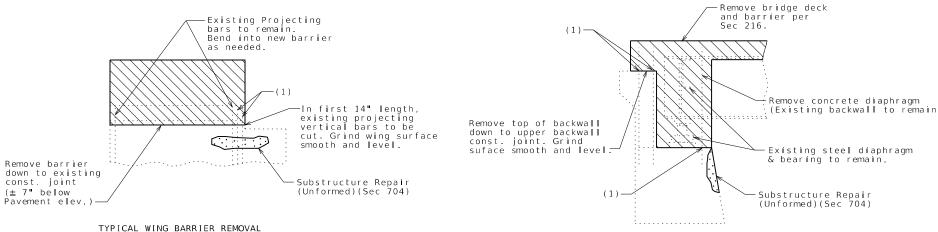
Haunching:

(2) Slab is to be considered a uniform thickness as shown on the plans. Haunching will vary. See front sheet for slab thickness. For adjusted beam deflection due to weight of new deck and barriers, see Bridge Electronic Deliverables.

The contractor shall use one of the qualified resin anchor systems in accordance with Sec 1039.

Cost of furnishing and installing the resin anchor systems, complete in place, will be considered completely covered by the contract unit price for Slab on Steel.

The minimum embedment depth in concrete with f'c = 4,000 psi for the resin anchor systems shall be that required to meet the minimum ultimate pullout strength in accordance with Sec 1039 but shall not be less than 5 inches, unless noted



END BENTS NO. 1 & 5 SECTION

DETAILS OF CONCRETE REMOVAL

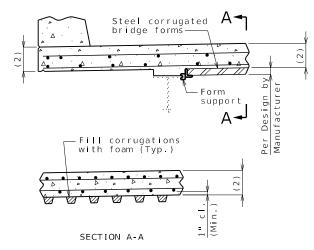
Prior to deck removal, profile grade along bridge shall be recorded at tenth points of each span

The cost of removal of concrete deck and barriers as shown will be considered completely covered by the contract unit price for removal of Existing Bridge

The cost of removal of end bent diaphragms as shown will be considered completely covered by the contract lump sum price for Partial Removal of Substructure Concreté

(1) Vertical reinforcement to be cut off one inch below concrete removal surface and the resulting holes shall be filled with a qualified special mortar.

A smooth, level surface shall be provided at top of backwall removal lines.



OPTIONAL STAY-IN-PLACE FORM DETAILS

PROJECT NO A15892

NUMBER E-25019

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9/27/2024

DAVIESS

JNW0112 CONTRACT ID.

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SHEET NO

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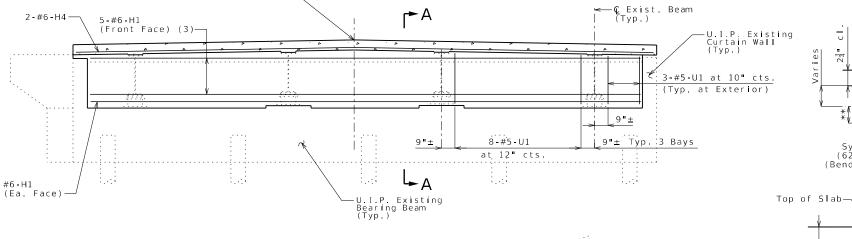
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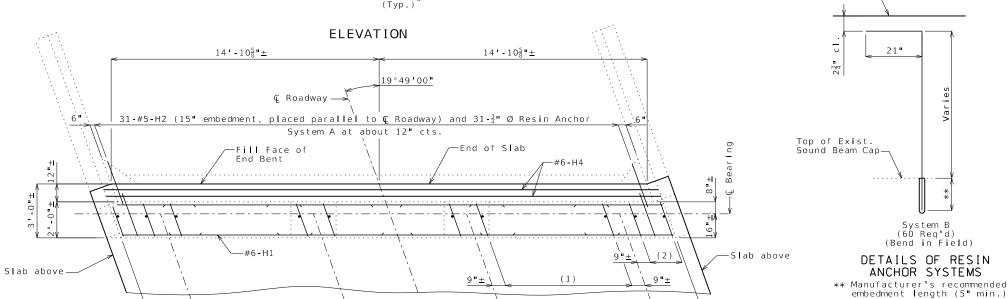
DESIGNED BY: KLW MAR 2024 DETAILED BY: JTC MAR 2024 CHECKED BY: DMA APR 2024

- (1) 8-#5-U1 with 8- $\frac{3}{4}$ " Ø Resin Anchor System B (Typ. 3 bays)
- (2) 3-#5-U1 with 3- $\frac{3}{4}$ Ø Resin Anchor System B (Typ. at Exterior)
- (1yp. at Exterior)

 (3) The H1 bars are segmented for ease of placement through girder web holes. The total bar length for H1 bars shown in Bill of Reinforcing Steel allows for one lap splice with a length of 3'-10". Actual bar segment lengths to be determined by contractor for ease of installing bars. The contractor may use a mechanical bar splice in lieu of a lap splice. When a mechanical bar splice is used, the actual bar segment length will be determined by the contractor to accommodate manufacturer's recommendations for installation and ease of construction. The cost of furnishing and installing the bar splices will be considered completely covered by the contract unit price for Reinforcing Steel. No adjustment of the quantity of reinforcing steel will be allowed for the use of mechanical bar splices.



Profile Grade = C Roadway—



8 6 ±

Approach Slab-

3/4" Ø Resin

Anchor System A

#5-H2-

Backwall

U.I.P. Existing

PLAN
(End Bent No. 1 shown. End Bent No. 5 similar)

8 6 ±

Notes:

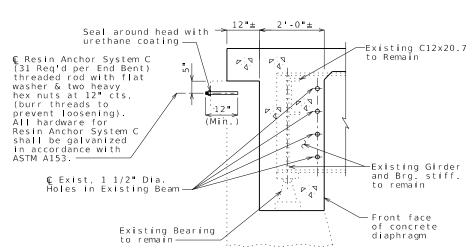
The contractor shall use one of the qualified resin anchor systems in accordance with Sec $1039\,.$

Cost of furnishing and installing the resin anchor systems, complete in place, will be considered completely covered by the contract unit price for Slab on Steel.

The minimum embedment depth in concrete with f'c = 4,000 psi for the resin anchor systems shall be that required to meet the minimum ultimate pullout strength in accordance with Sec 1039 but shall not be less than 5".

An epoxy coated #6 Grade 60 reinforcing bar shall be substituted for the $\frac{3}{4}$ $\!\!\!/\!\!\!/\!\!\!/\!\!\!/$ threaded rod.

The exposed and accessible surfaces of the existing structural steel and bearings that will be encased in concrete shall be cleaned with a minimum of SSPC-SP-3 surface preparation and coated with a minimum of one coat of gray epoxy-mastic primer (non-aluminum) in accordance with Sec 1081 to produce a dry film thickness of not less than 3 mils before concrete is poured. The surface preparation and coating for beams shall extend a minimum of one foot outside the face of the beam encasement. Payment for cleaning and coating steel to be encased in concrete will be considered completely covered by the contract unit price for Slab on Steel.



8 6 ±

BEAM HOLE LOCATION DETAIL (Reinforcing steel not shown for clarity)

END BENTS NO. 1 & 5

#6-H1 (Typ.)

#6-H1 (Typ.)

#6-H1

Chamfer Detail (Typ.)

3/4" Ø Resin Anchor System B

Je Lochner 17 College Boulevard | Lenexa, Kansas 66219

NUMBER E-25019

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DATE PREPARED

10/2/2024

DAVIESS

JNW0112

PROJECT NO

BRIDGE NO.

MO SHEET NO

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—Top of Slab

4 - 3

System A (62 Req'd) (Bend in Field)

—Top of Exist. Backwall

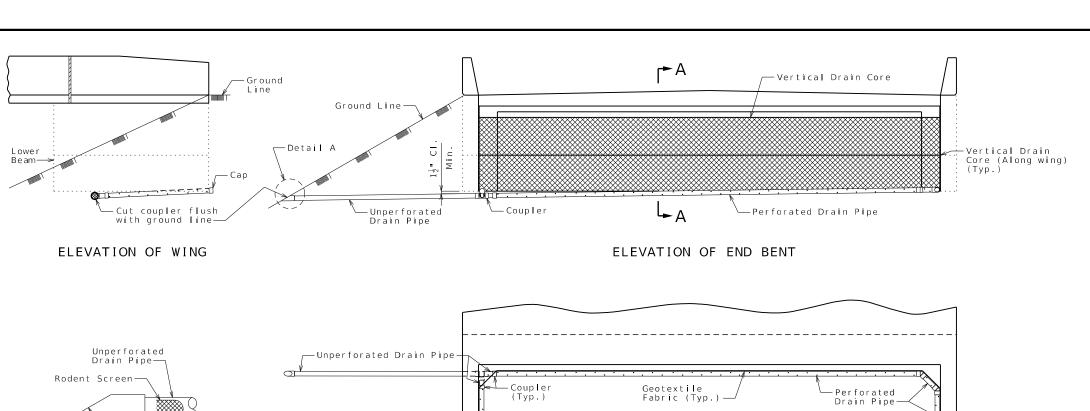
DESIGNED BY: KLW MAR 2024 DETAILED BY: JTC MAR 2024 CHECKED BY: DMA APR 2024 2'-8"±

Existing Beam Spacing

(Match Existing Grade ±) @ End of Slab @ @ Roadway

-#6-H4

SECTION A-A

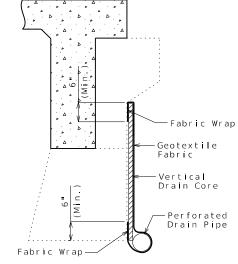


- Perforated

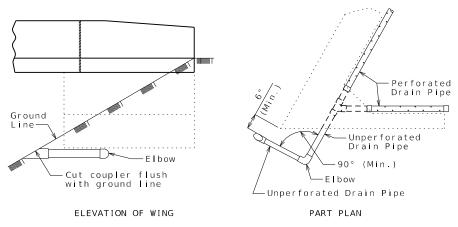
Drain Pipe

PLAN OF END BENT

Сар –



PART SECTION A-A (Section thru wing similar)



Cut coupler

to slope of ground line

DETAIL A

DESIGNED BY: KLW MAR 2024

DETAILED BY: JTC MAR 2024 CHECKED BY: DMA APR 2024

OPTIONAL TURNED DRAIN

(Use only when straight drain is not practical.)

VERTICAL DRAIN AT END BENTS

(Squared end bent shown, skewed end bent similar)

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 4 of 9

General Notes:

All drain pipe shall be sloped 1 to 2 percent.

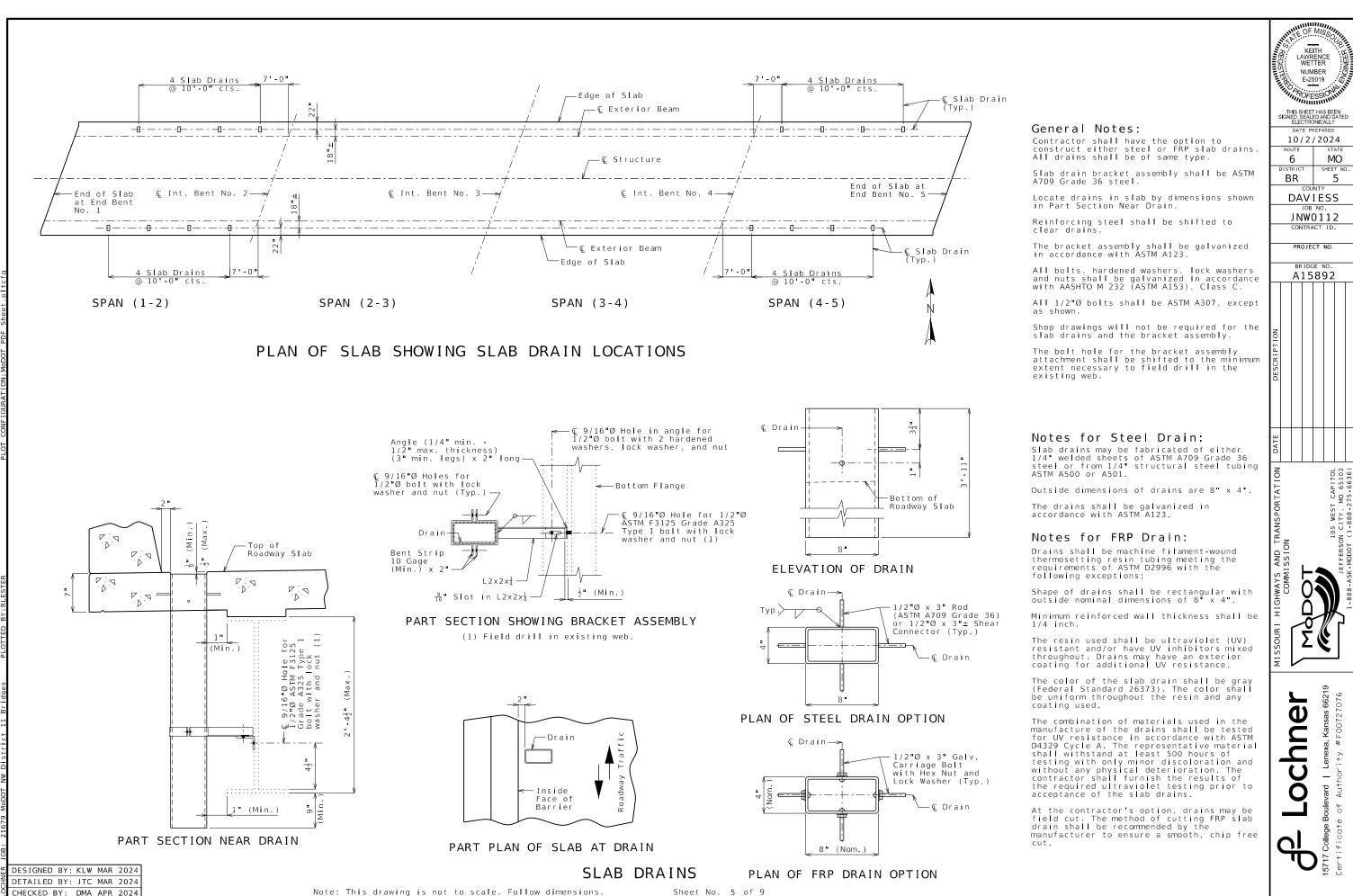
Drain pipe may be either 6-inch diameter corrugated metallic-coated steel pipe underdrain, 4-inch diameter corrugated polyvinyl chloride (PVC) drain pipe, or 4inch diameter corrugated polyethylene (PE) drain pipe.

Drain pipe shall be placed at fill face of end bent and inside face of wings. The pipe shall slope to lowest grade of ground line, also missing the lower beam of end bent by a minimum of 1 1/2 inches.

Perforated pipe shall be placed at fill face side and inside face of wings at the bottom of end bent and plain pipe shall be used where the vertical drain ends to the exit at ground line.

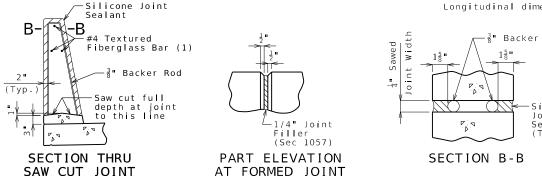


De Lochner 17 college Boulevard | Lenexa, Kansas 66218

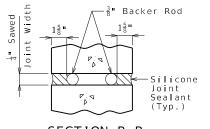


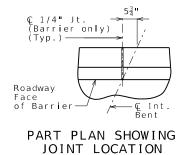
ELEVATION OF BARRIER

(Left barrier shown, right barrier similar) Longitudinal dimensions are taken parallel to grade.



SPAN (3-4)





SPAN (4-5)

(3) #5-R Bar (3) Joint — #5 **-** R3 #5-R2

4" Plastic Waterstop (Centered Joint on joint) Joint DO

R-BAR PERMISSIBLE ALTERNATE SHAPE

(3) The R1 bar may be separated into two bars as shown, at the contractor's option, only when slip forming is not used. (All dimensions are out to

WATERSTOP DETAIL Plastic waterstop shall be

placed in all formed joints, except structures with superelevation, use on lower joints only

Cost of plastic waterstop complete in place, will be considered completely covered by the contract unit price for Type D Barrier.

General Notes:

* Slip-formed option only.

Conventional forming or slip forming may be used. Saw cut joints may be used with conventional forming.

Top of barrier shall be built parallel to grade and barrier joints (except at end bents) normal to grade.

All exposed edges of barrier shall have either a 1/2-inch radius or a 3/8-inch bevel, unless otherwise noted.

Payment for all concrete and reinforcement, complete in place, will be considered completely covered by the contract unit price for Type D Barrier per

Concrete in barrier shall be Class B-1.

Measurement of barrier is to the nearest linear foot for each structure, measured along the outside top of slab from end of wing to end of wing.

Concrete traffic barrier delineators shall be placed on top of the barrier as shown on Missouri Standard Plan 617.10 and in accordance with Sec 617. Delineators on bridges with two-lane, two-way traffic shall have retroreflective sheeting on both sides. Concrete traffic barrier delineators will be considered completely covered by the contract unit price for Type D Barrier

Joint sealant and backer rods shall be in accordance with Sec 717 for silicone joint sealant for saw cut and formed joints.

For slip-formed option, both sides of barrier shall have a vertically broomed finish and the top shall have a transversely broomed finish.

Plastic waterstop shall not be used with saw cut joints.



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NUMBER

E-25019

9/27/2024

DAVIESS

JNW0112

PROJECT NO

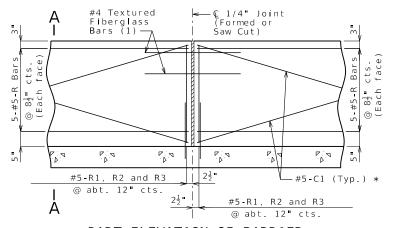
A15892

MO

SHEET NO 6

6

BR



SAW CUT JOINT

DESIGNED BY: KLW MAR 2024

DETAILED BY: JTC MAR 2024 CHECKED BY: DMA APR 2024

PART ELEVATION OF BARRIER

(1) Four feet long, centered on joint, slip-formed option only

Use a minimum lap of 3'-1" for #5 horizontal barrier bars. The cross-sectional area above

SECTION A-A

the slab is 3.52 square feet.

- #5 **-** R3

(2) To top of bar

#5-R1

#5-C1

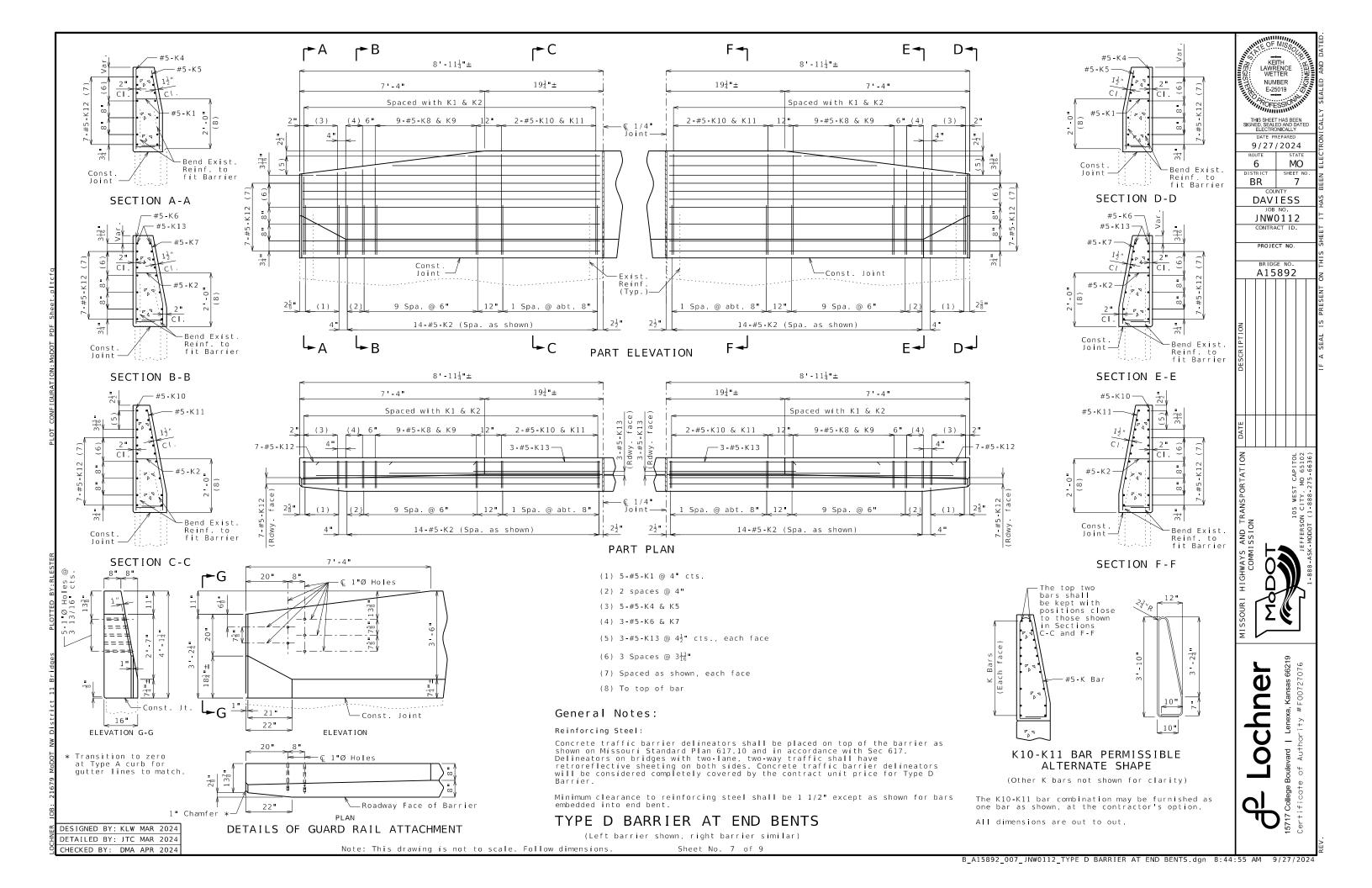
#5-R2

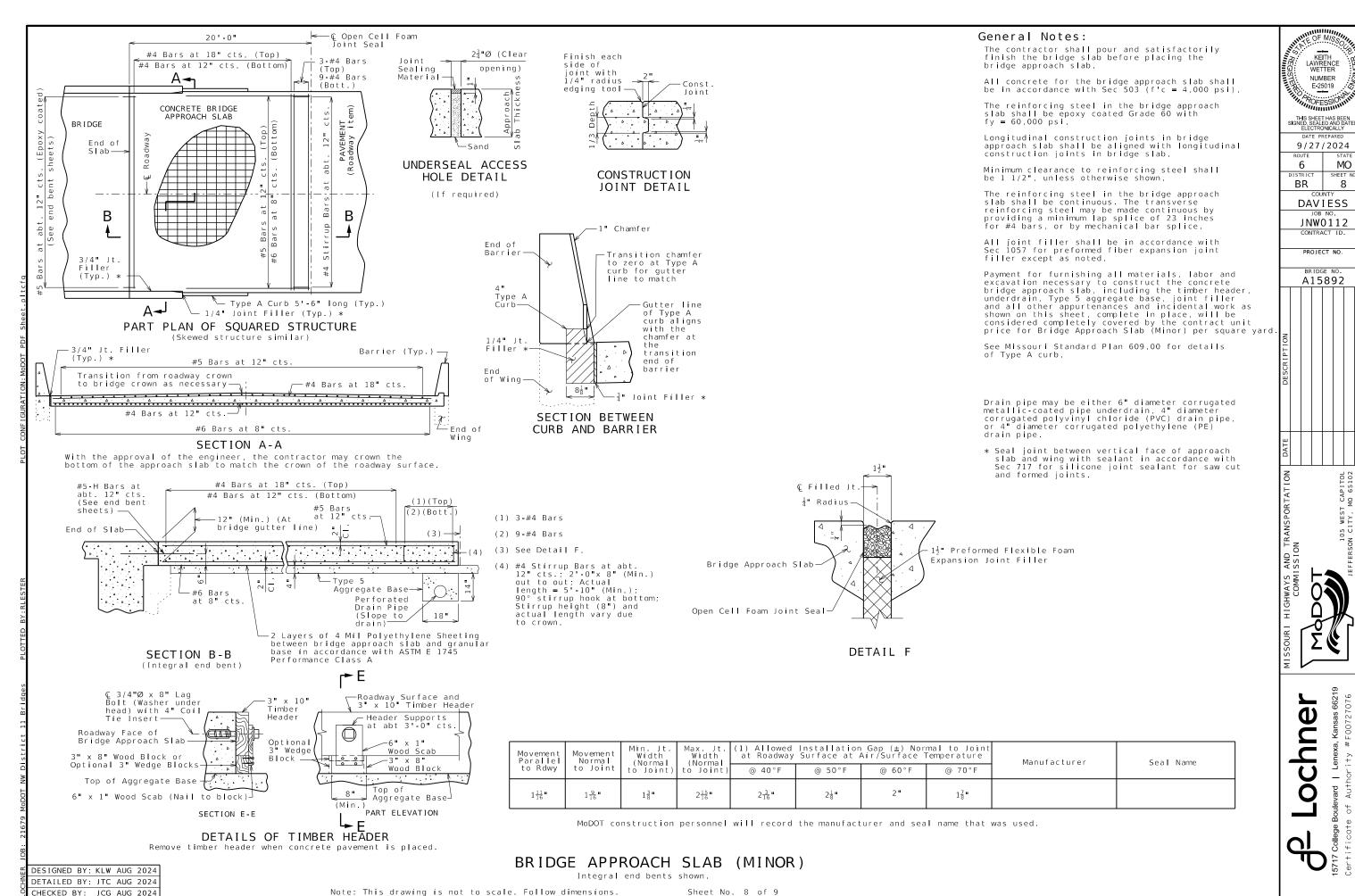
Const. Joint

TYPE D BARRIER Sheet No. 6 of 9

Note: This drawing is not to scale. Follow dimensions.

B_A15892_006_JNW0112_TYPE D BARRIER ELEVATION.dgn 8:44:45 AM 9/27/2024





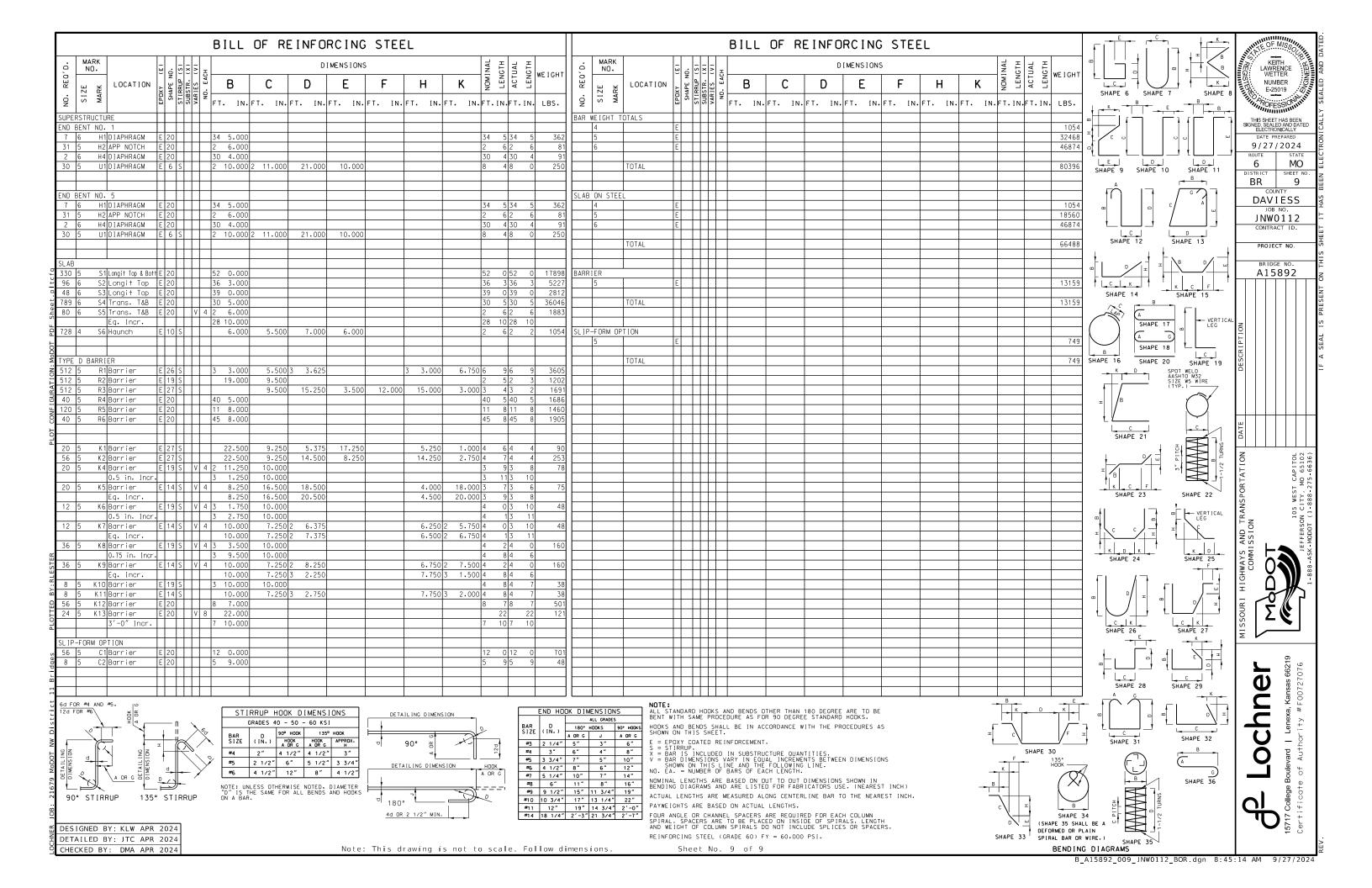
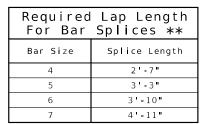
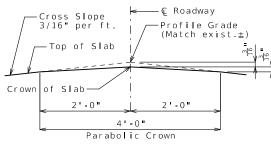


Table Showing Bar Lengths S2 S4 Int. Bent No. 2 & 4 Int. Bent No. 3 Span 1 & 4 Span 2 & 3 Span 2 Span 3 11'-10" 25'-4" 25'-4" 25'-4"

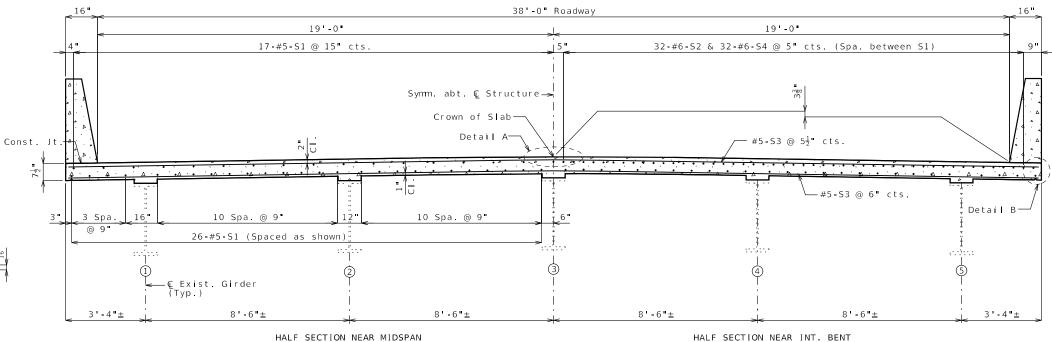


** Unless otherwise shown.



DETAIL A

U.I.P., REDECK AND RECONFIGURE EXISTING (32'-86'-86'-32') CONTINUOUS COMPOSITE PLATE GIRDER SPANS



TYPICAL SECTION THRU SLAB

General Notes:

Design Specifications: 2002 AASHTO LFD (17th Ed.) Standard Specifications Seismic Performance Category A

Design Loading:

H20-44 (1965) (Existing) HS20-44 (New Construction) 135 lb/sf Future Wearing Surface Earth - 120 lb/cf, Equivalent Fluid Pressure 45 lb/cf Fatigue Stress - Case III

Design Unit Stresses:

Class B-1 Concrete (Barrier) f'c = 4,000 psiClass B-2 Concrete (End Bents & Superstructure, f'c = 4,000 psiexcept Barrier)
Reinforcing Steel (Grade 60) fy = 60,000 psiStructural Carbon Steel (ASTM A709 Grade 36) fy = 36,000 psi

All joint filler shall be in accordance with Sec 1057 fo preformed sponge rubber expansion and partition joint filler, except as noted

Reinforcing Steel:

Minimum clearance to reinforcing steel shall be 1 1/2", unless

Bars bonded in existing concrete not removed shall be cleanly stripped and embedded into new concrete where possible. If length is available, existing bars shall extend into new concrete

Teast 40 diameters for plain bars and 30 diameters for deformed bars, unless otherwise noted.

Roadway surfacing adjacent to bridge ends shall match new bridge slab surface (Roadway item) Outline of existing work is indicated by light dashed lines. Heavy

lines indicate new work. Contractor shall verify all dimensions in field before ordering

The area exposed by the removal of concrete and not covered with new concrete shall be coated with an approved qualified special mortar in accordance with Sec 704.

For adjusted girder deflection due to weight of new deck and barriers, see Bridge Electronic Deliverables.

Rubblized concrete from existing bridge deck that qualifies as clean fill may be placed on spill slopes at end bents above ordinary high water line (Roadway Item).

Designed By: CEA 04/24 Detailed By: CEA 04/24 Checked By: CWT 05/24



DETAIL B

OPTIONAL SHIFTING TOP BARS AT BARRIER

Sheet No. 1 of 15

Contractor may

Traffic Handling:

Note: This drawing is not to scale. Follow dimensions.

Structure to be closed during construction. Traffic to be maintained on other routes during construction. See roadway plans for traffic control.

Slab:

The table of Estimated Quantities for Slab on Steel represents the quantities used by the State in preparing the cost estimate for concrete slabs. The area of the concrete slab will be measured to the nearest square yard longitudinally from end of slab to end of slab and transversely from out to out of bridge slab (or with the horizontal dimensions as shown on the plan of slab). Payment for stay-in-place corrugated steel forms, conventional forms, all concrete and epoxy coated reinforcing steel will be considered completely covered by the contract unit price for the slab. Variations may be encountered in the estimated quantities but the variations cannot be used for an adjustment in the contract unit price.

Method of forming the slab shall be in accordance with Sec 703. All hardware for forming the slab to be left in place as a permanent part of the structure shall be coated in accordance with ASTM AI23 or ASTM B633 with a thickness Class SC 4 and a finish Type I, II or III.

Slab shall be cast-in-place with conventional forming or stay-in-place corrugated steel forms. Precast prestressed panels will not be permitted.

For Optional Stay-In-Place Form Details, see Sheet No. 2.

5	
	Total
sq. foot	26
lump sum	1
sq. foot	9,658
lump sum	1
sq. yard	181
sq. yard	1,082
linear foot	519
sq. foot	256
pound	97,620
each	12
sq. foot	6,000
sq. foot	1,300
lump sum	1
each	2
each	10
each	10
linear foot	81
	sq. foot lump sum sq. foot lump sum sq. yard sq. yard linear foot sq. foot pound each sq. foot lump sum each sq. foot

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HARRISON

JNW0112 CONTRACT ID

PROJECT NO.

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Cost of any required excavation for bridge will be considered completely covered by the contract unit price for other items

Estimated Quantities for Slab on Stee										
I t em		Total								
Class B-2 Concrete	cu. yard	257								
Reinforcing Steel (Epoxy Coated)	pound	80,830								

REPAIRS TO BRIDGE: ROUTE N OVER ROUTE I-35

ABOUT 0.6 MILE EAST OF ROUTE 69

ROUTE N FROM ROUTE 69 TO ROUTE T BEGINNING STATION 439+80.31± (MATCH EXISTING)

General Notes Cont'd:

Stay-In-Place Forms:

Corrugated steel forms, supports, closure elements and accessories shall be in accordance with grade requirement and coating designation G165 of ASTM A653. Complete shop drawings of the permanent steel deck forms shall be required in accordance with Sec 1080.

Corrugations of stay-in-place forms shall be filled with an expanded polystyrene material. The polystyrene material shall be placed in the forms with an adhesive in accordance with the manufacturer's recommendations.

Form sheets shall not rest directly on the top of girder flanges. Sheets shall be securely fastened to form supports with a minimum bearing length of one inch on each end. Form supports shall be placed in direct contact with the flange. Drilling holes in the girder flanges will not be permitted. All steel fabrication and construction shall be in accordance with Sec 1080 and 712. Certified field welders will not be required for welding of the form supports

The design of stay in place corrugated steel forms is per manufacturer which shall be in accordance with Sec 703 for false work and forms. Maximum actual weight of corrugated steel forms allowed shall be 4 psf assumed for girder loading.

The contractor shall provide a method of preventing the direct contact of the stay-in-place forms and connection components with uncoated weathering steel members that is approved by the engineer.

Pouring and Finishing Slab:

The contractor shall provide bracing necessary for lateral and torsional stability of the girders during construction of the concrete slab and remove the bracing after the slab has attained 75% design strength. Contractor shall not weld on or drill holes in the girders. The cost for furnishing, installing, and removing bracing will be considered completely covered by the contract unit price for Slab on Steel.

Alternate pour sequences may be submitted to the engineer for approval. Keyed construction joints shall be provided between pours.

- (1) Slab is to be considered a uniform thickness as shown on the plans. See front sheet for slab thickness. Haunching will vary. For adjusted girder deflection due to weight of proposed deck and barriers, see Sheet No. 3 and Bridge Electronic Deliverables.
- Structural Steel Protective Coatings (New Steel):

Protective Coating: System G in accordance with Sec 1081.

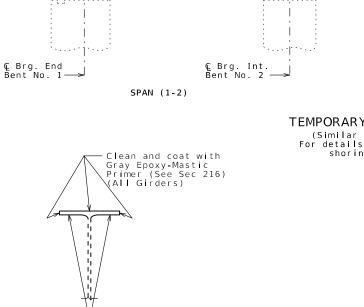
Prime Coat: The cost of the inorganic zinc prime coat will be considered completely covered by the contract unit price for the fabricated structural steel.

Field Coat(s): The color of the field coat(s) shall be Gray (Federal Standard #26373). The cost of the intermediate field coat will be considered completely covered by the contract price per sq. foot for Intermediate Field Coat (System G). The cost of the finish field coat will be considered completely covered by the contract price per sq. foot for Finish Field

At the option of the contractor, the intermediate and finish field coats may be applied in the shop. The contractor shall exercise extreme care during all phases of loading, hauling, handling, erection and pouring of the slab to minimize damage and shall be fully responsible for all repairs and cleaning of the coating systems as required by the engineer.

Structural Steel Protective Coating (Top Flange):

In accordance with Sec. 216.50 and 1081, the top and additionally the sides and bottom of the top flange shall be coated with not less than 3.0 mils of Gray Epoxy Mastic-Primer (non-aluminum) applied over an SSPC-SP3 surface preparation. Payment for coating steel will be considered completely covered by the contract sq. foot price for Removal of Existing Bridge

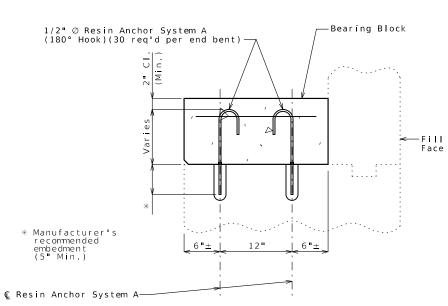


TYPICAL SECTION THRU GIRDER SHOWING PROTECTIVE COATING

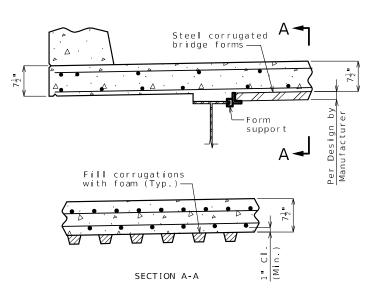
(Not applicable to sections of new steel girders)

·Clean and coat with Gray Epoxy-Mastic

Primer (See Sec 1081) (All Girders)

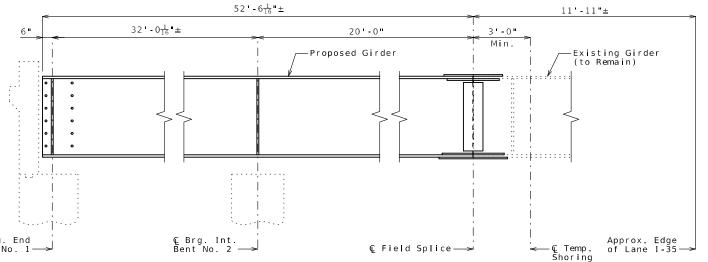


DETAILS OF RESIN ANCHOR SYSTEM A @ END BENTS NO. 1 & 5



OPTIONAL STAY-IN-PLACE

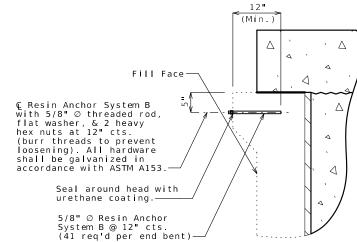
FORM DETAILS



TEMPORARY SHORING DIAGRAM

PART SPAN (2-3)

(Similar at Bents No. 4 and 5) For details of removal of existing shoring see Sheet No. 4.



DETAILS OF RESIN ANCHOR SYSTEM B @ END BENTS NO. 1 & 5

Resin Anchor System Notes:

The Contractor shall use one of the qualified resin anchor systems in accordance with Sec 1039.

Cost of furnishing and installing the resin anchor systems, complete in place, will be considered completely covered by the contract unit price for Slab on Steel.

The minimum embedment depth in concrete with f'c = 4.000psi for the resin anchor systems shall be that required to meet the minimum ultimate pullout strength in accordance with Sec 1039 but shall not be less than 5" for System A and as shown for System B

NUMBER

10/01/2024

9/30/2024 MO N DISTRICT SHEET NO BR 2

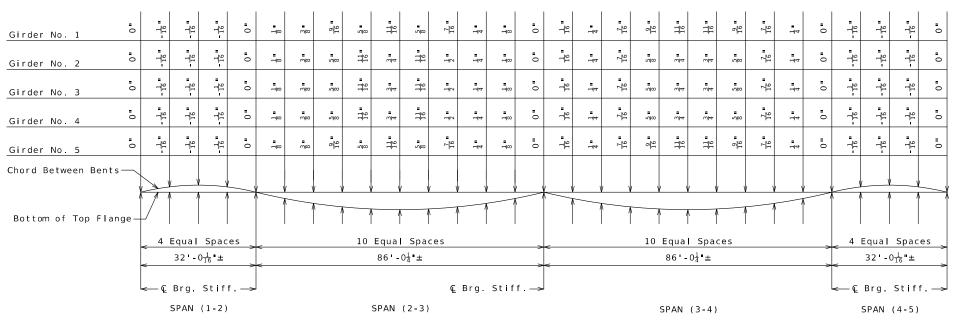
HARRISON JNW0112 CONTRACT ID

PROJECT NO

A22912

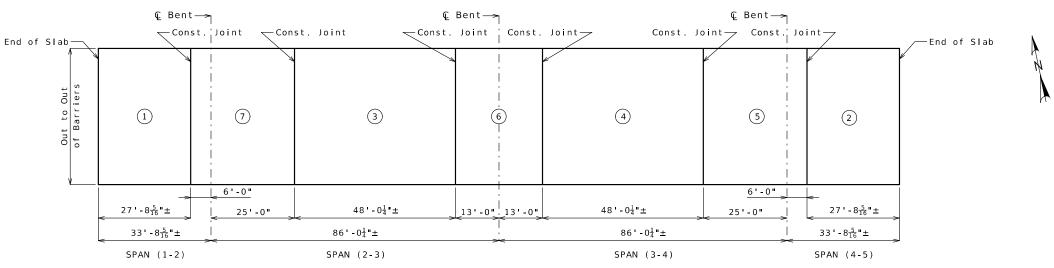
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Designed By: CEA 04/24 Detailed By: CEA 04/24 Checked By: CWT 05/24



DEAD LOAD DEFLECTION

13% of dead load deflection is due to the weight of structural steel. Dead load deflection includes weight of structural steel, concrete slab, and barrier.



The contractor shall provide bracing necessary for lateral and torsional stability of the beams during construction of the concrete slab and remove the bracing after the slab has attained 75% design strength. Contractor shall not weld on or drill holes in the beams. The cost for furnishing, installing, and removing bracing will be considered completely covered by the contract unit price for Slab on Steel.

Slab shall be poured upgrade from end to end at a minimum rate of 25 cubic yards per hour.

Alternate pour sequences may be submitted to the engineer for approval. Keyed construction joints shall be provided between pours.

		:	Min. Rate of Pou Cu. Yds./Hr.										
Direction With Retarder R													
Basic Sequence	1	2	25	25									
Alternate pours to the basic skip sequence are subject to the approval of the engineer in accordance with Sec 703.													
Alternate A Pours	1 End t	& :o 7	2 End to		+ 3 + 6	+ 4 + to 2	- 25	41					

The contractor shall pour and satisfactorily finish the slab pours at the rate given. Retarder, if used, shall be an approved type and retard the set of concrete to 2.5 hours.

SLAB POURING SEQUENCE

MISCELLANEOUS DETAILS

Designed By: CEA 04/24 Detailed By: CEA 04/24 Checked By: CWT 05/24

Note: This drawing is not to scale. Follow dimensions.

10/01/2024 9/30/2024 N MO DISTRICT SHEET NO 3 BR **HARRISON** JNW0112 PROJECT NO. A22912

> ch S O C O 2

Remove bridge — Cut off vertical deck and barrier per Sec 216 (4) reinforcement (2) -Remove top of backwall down to approach slab seat (3)(5) Substructure Repair (1) (Typ.)

TYPICAL SECTION OF REMOVAL AT END BENT

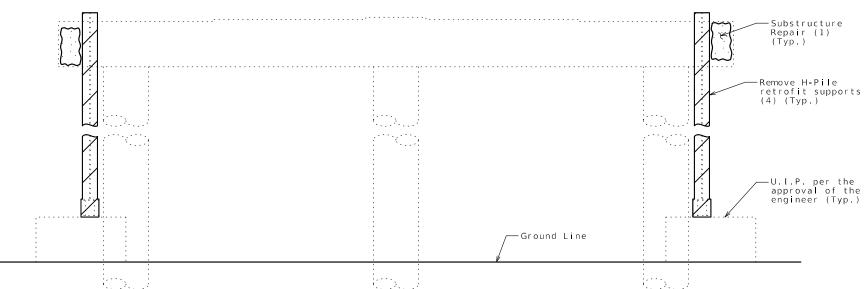
-Cut off vertical reinforcement (2) (Typ.)

TYPICAL WING BARRIER REMOVAL

Remove barrier and curb to 2" below approach slab seat (3)(5)

ELEVATION OF END BENT

(End Bent No. 1 shown, End Bent No. 5 similar)



ELEVATION OF INTERMEDIATE BENT

(Int. Bent No. 4 shown, Int. Bent No. 2 similar.)

Notes:

- (1) See Sec. 704 for Substructure Repair (Unformed). (3) A smooth, level surface shall be provided at removal lines.

 End Bent No. 1 140 sq. foot
 Int. Bent No. 4 6 sq. foot
 End Bent No. 5 110 sq. foot

 (4) The cost of conrete and H-Pile retrofit supported by the contract unit release for Departs levels and the contract unit relea
- (2) Vertical reinforcement to be cut off one inch below concrete removal surface. The resulting holes shall be filled with a qualified special
- (4) The cost of conrete and H-Pile retrofit support removal shall be considered completely covered by the contract unit price for Removal of Existing Bridge Deck.
- (5) The cost of conrete removal shall be considered completely covered by the contract unit price for Partial Removal of Substructure Concrete.

CONCRETE REMOVAL AND SUBSTRUCTURE REPAIR

Designed By: CEA 04/24 Detailed By: CEA 04/24 Checked By: CWT 05/24



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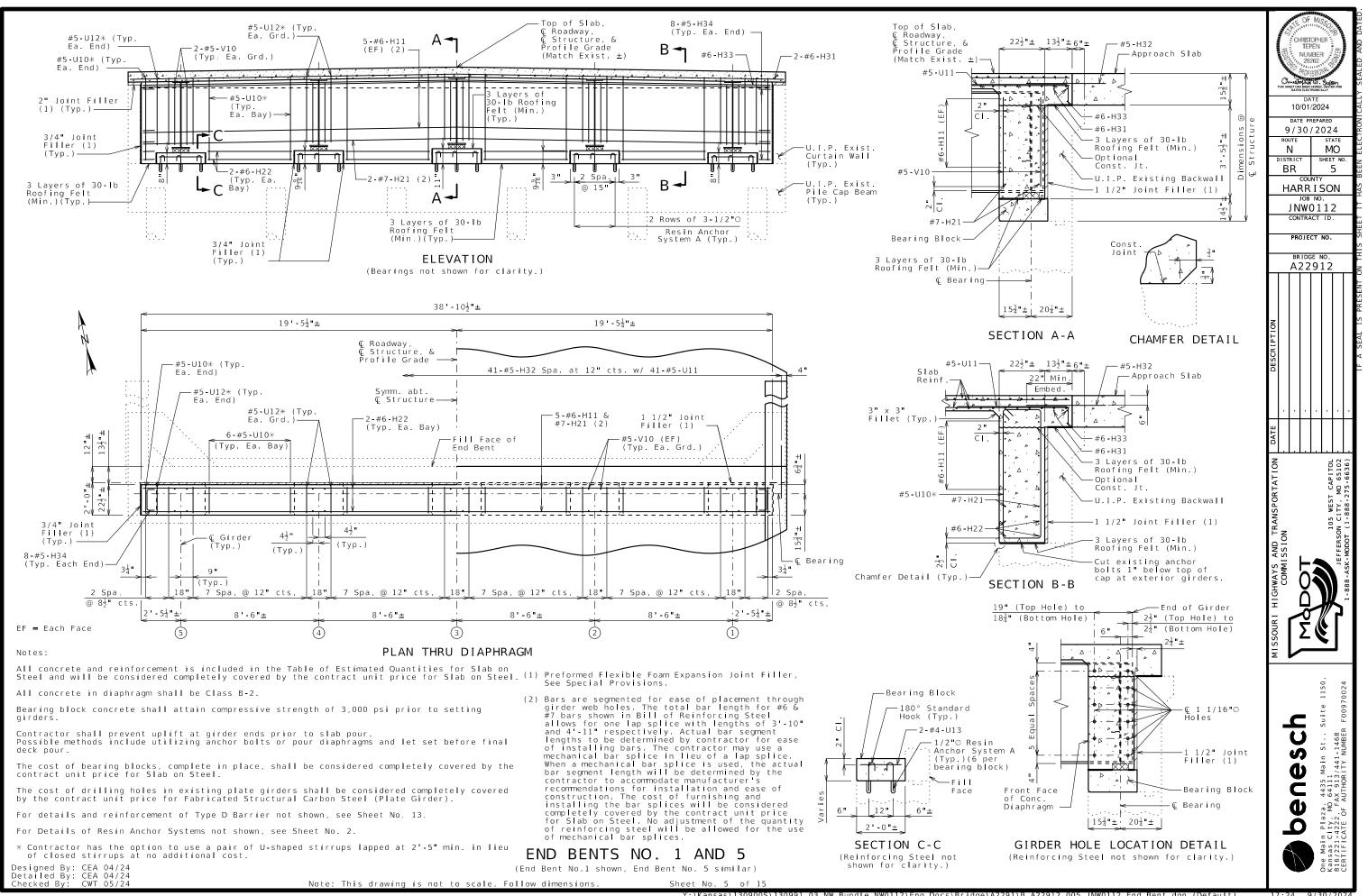
N MO DISTRICT SHEET NO BR 4

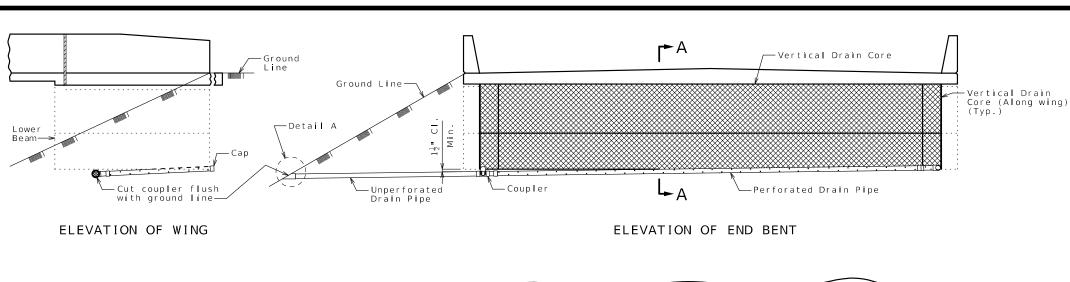
HARRISON JNW0112 CONTRACT ID.

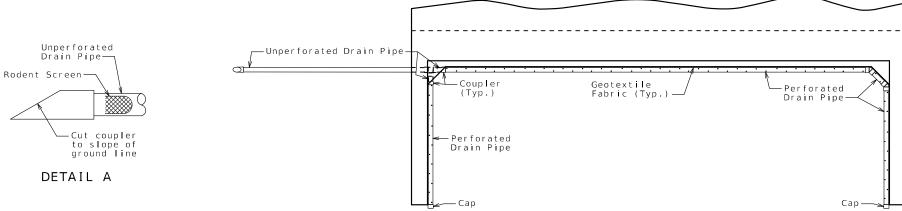
PROJECT NO.

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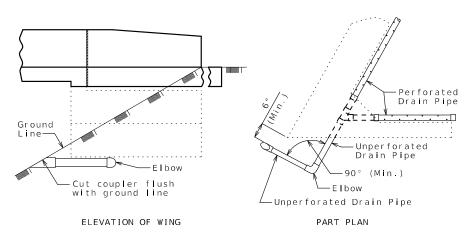
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PLAN OF END BENT

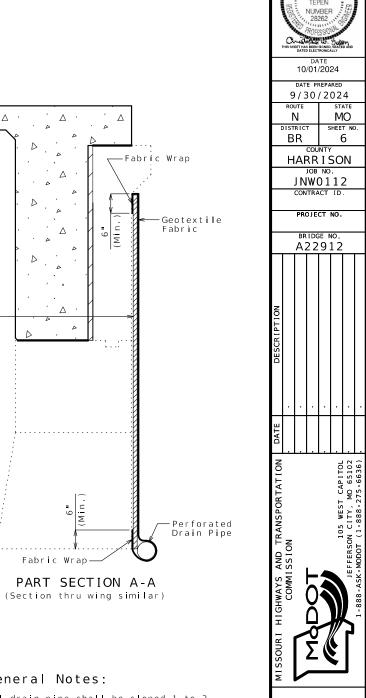


OPTIONAL TURNED DRAIN

(Use only when straight drain is not practical.)

VERTICAL DRAIN AT END BENTS

Designed By: CEA 04/24 Detailed By: CEA 04/24 Checked By: CWT 05/24



General Notes:

Fabric Wrap-

Vertical Drain Core

All drain pipe shall be sloped 1 to 2 percent.

Drain pipe may be either 6-inch diameter corrugated metallic-coated steel pipe underdrain, 4-inch diameter corrugated polyvinyl chloride (PVC) drain pipe, or 4-inch diameter corrugated polyethylene (PE)

Drain pipe shall be placed at fill face of end bent and inside face of wings. The pipe shall slope to lowest grade of ground line, also missing the lower beam of end bent by a minimum of 1 1/2 inches.

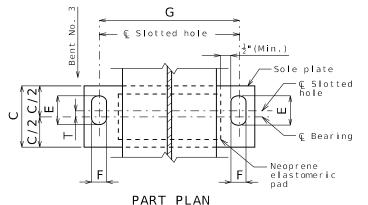
Perforated pipe shall be placed at fill face side and inside face of wings at the bottom of end bent and plain pipe shall be used where the vertical drain ends to the exit at ground line.

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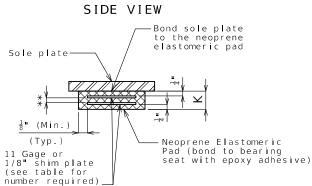
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(Brg. Plate & Brg. Extension not shown for clarity.)

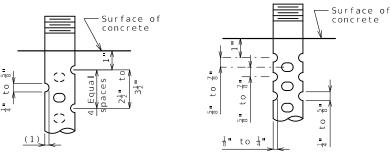


** Layers of 1/2" elastomer alternating with 11 gage or 1/8" shim plate

NEOPRENE ELASTOMERIC PAD

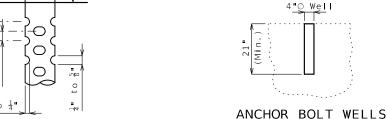
	EXPANSION BEARINGS																							
BENT NO.	Α	В	С	D	Е	F	G	J	K	Г	М	N	Р	Q	R	Т	С	٧	W	Χ	Υ	Z	NUMBER OF SHIM PLATES *	NUMBER REQUIRED
2	20"	10"	13"	24"	5 1 "	2 1 "	18"	$4\frac{1}{4}$ "	2½"	13"	1½"	2½"	0"	3"	1 m	0"	1 ¹ / ₂ "	14"	13"	12"	$11\frac{3}{4}$ "	5 5 "	4	5
4	20"	10"	13"	24"	5 ¹ / ₄ "	2 1 "	18"	$4\frac{1}{4}$ "	2½ "	13"	1½"	2 1 "	0"	3"	1 m	0"	1 1 "	14"	13"	12"	$11\frac{3}{4}$ "	5 § "	4	5

* The required shim plate shall be placed between layers of elastomer and molded together to form an integral unit.



DETAIL OF 3/4"Ø THRU 2 1/2"Ø ANCHOR BOLTS

Designed By: CEA 04/24 Detailed By: CEA 04/24 Checked By: CWT 05/24



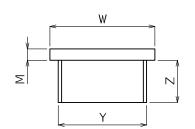
OPTIONAL DETAIL OF 1 3/8"Ø THRU 2 1/2"Ø ANCHOR BOLTS

SWEDGE ANCHOR BOLT DETAILS

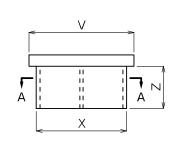
(1) $\frac{1}{8}$ for $\frac{3}{4}$ Ø thru $1\frac{1}{4}$ Ø anchor bolts

 $\frac{1}{8}$ to $\frac{1}{4}$ for $1\frac{3}{8}$ Ø thru $2\frac{1}{2}$ Ø anchor bolts

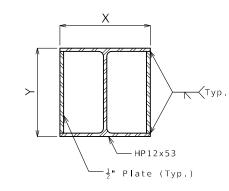
LAMINATED NEOPRENE BEARING PAD ASSEMBLY



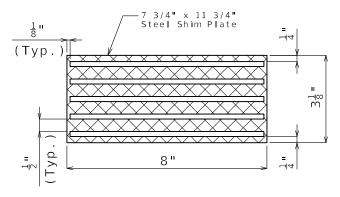
SIDE ELEVATION OF BEARING EXTENSION AND BEARING PLATE



END ELEVATION OF BEARING EXTENSION AND BEARING PLATE



SECTION A-A



TYPICAL SECTION THRU 8" x 12" x 3 1/8"
LAMINATED NEOPRENE BEARING PAD

DETAIL OF BEARINGS FOR END BENTS NO. 1 & 5

GENERAL NOTES:

Anchor bolts shall be 2"Ø ASTM F1554 Grade 55 swedged bolts and shall extend 18 into the concrete with ASTM A563 Grade A Heavy Hex nuts. Actual manufacturer's certified mill test reports(chemical and mechanical) shall be provided. Swedging shall be 1 less than extension into the concrete.

Anchor bolt shall be at the centerline of slotted hole at 60°F. Bearing position shall be adjusted ${\pmb R}$ for each 10° fall or rise in temperature at installation.

Anchor bolts and heavy hex nuts shall be coated with a minimum of two coats of inorganic zinc primer to provide a total dry film thickness of 4 mils minimum, 6 mil maximum, or galvanized in accordance with Sec 1081.

Neoprene Elastomeric Pads shall be 60 Durometer.

Structural steel for sole plate shall be ASTM A709 Grade 50 and shall be coated with a minimum of two coats of inorganic zinc primer to provide a total dry film thicknes of 4 mils minimum, 6 mils maximum.

Laminated Neoprene Bearing Pad Assembly shall be in accordance with Sec 716. ISSOURI HIGHWAYS AND TRANSPORTATION

COMMISSION

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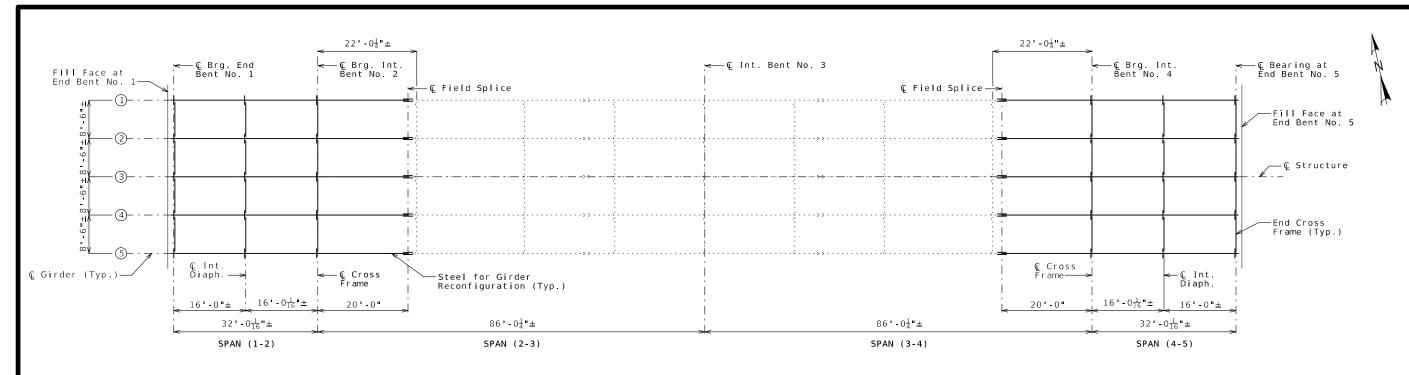
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benessary, 4435 Main St., Suite 36,2221-4222, FAX 913/441-1468

Note: This drawing is not to scale. Follow dimensions.



PLAN OF STRUCTURAL STEEL

Notes

Longitudinal dimensions are horizontal.

Fabricated structural steel shall be ASTM A709, Grade 36.

At the Contractor's option, holes in the diaphragm plate of non slab bearing diaphragms may be made 3/16" larger than the nominal diameter of the bolt. A hardened washer shall be used under the bolt head and nut when this option is used. Holes in the girder diaphragm connection plate or transverse web stiffener shall be standard size.

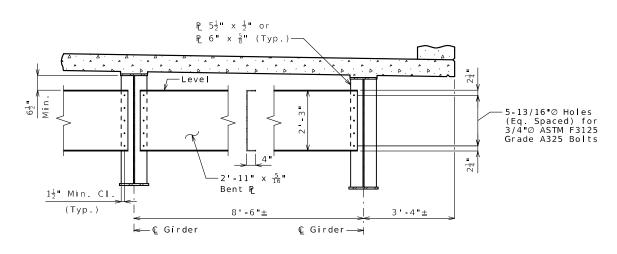
For details of Girders, see Sheet No. 9.

For details of Stiffeners, see Sheet No. 9.

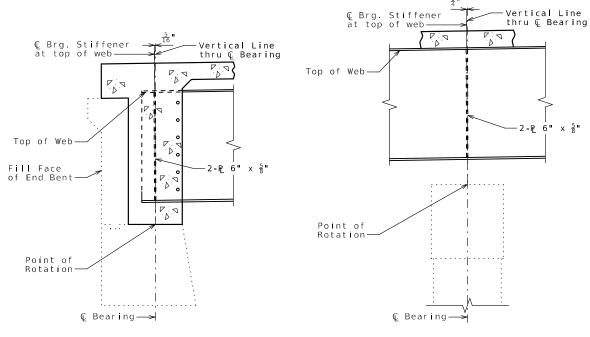
For details and spacing of Shear Connectors, see Sheet No. 9.

For details of Bolted Field Splices, see Sheet No. 10.

For location of Slab Drain attachment holes, see Sheet No. 11.



TYPICAL PART SECTION SHOWING CROSS FRAMES AND INTERMEDIATE DIAPHRAGMS



Exp.

END BENTS

(End Bent No. 1 shown, End Bent No. 5 similar)

INTERMEDIATE BENTS
(Int. Bent No. 2 shown, Int. Bent No. 4 similar)

PART LONGITUDINAL SECTIONS

RECONFIGURATION OF EXISTING GIRDERS

Designed By: CEA 04/24 Detailed By: CEA 04/24 Checked By: CWT 05/24



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PART ELEVATION OF GIRDER NEAR BENTS NO. 1 AND 2 SHOWING STRUCTURAL STEEL REMOVAL

(Similar at Bents No. 4 and 5)

Notes:

Designed By: CEA 04/24 Detailed By: CEA 04/24 Checked By: CWT 05/24

Payment for removal of pins, pin plates, intermediate diaphragms, end diaphragms, bearings, shear connectors, and any other incidental material included in the girder section being removed, and any coating repair will be considered completely covered by the contract lump sum price for Reconfigure Existing Structural Steel

Any weld material remaining after removal shall be ground flush.

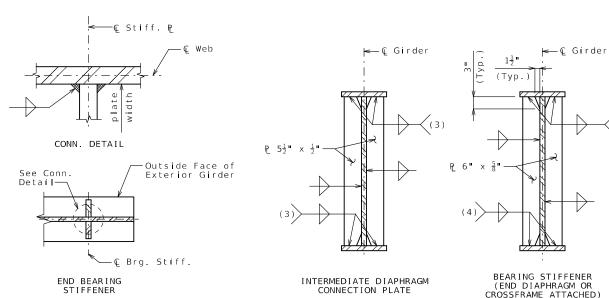
The cost of supplying and installing shear connectors will be considered completely covered by the contract unit price for Fabricated Structural Carbon Steel (Plate Girder).

Shear Connectors shall be in accordance with Sec 712, 1037, & 1080

Required temporary support load of 17 kips per girder right of Int. Bent No. 2 (Span 2-3) and left of Int. Bent No. 4 (Span 3-4) is a service dead load without a factor of safety. It includes existing girder weight (existing or proposed deck weight not included) and a construction load of 50 psf. See Special

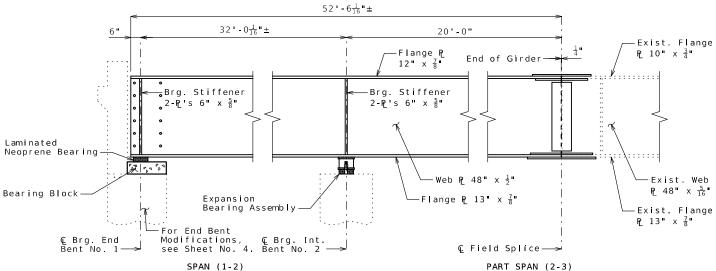
For location of Temporary Shoring, see Sheet No. 2.

- (1) The cut end of existing girders and any damaged areas of existing paint shall be coated with Gray
- (2) Any Shear Connectors interfering with splice plate installation shall be removed and flange surface ground smooth.



(3) Tight fit

(4) Grind or mill to bear



PART ELEVATION OF GIRDER NEAR BENTS NO. 1 AND 2 SHOWING STRUCTURAL STEEL RECONFIGURATION (Similar at Bents No. 4 and 5)

Notes:

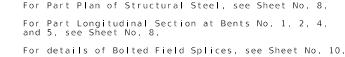
Longitudinal dimensions are horizontal.

All Flange and Web Plates shall be subject to notch toughness requirements.

All Fabricated Structural Steel, shall be ASTM A709, Grade 36

For details of Laminated Neoprene Bearing Pad Assembly, see Sheet No. 7.

For details of Cross Frames, see Sheet No.



For Slab Drain attachment hole locations, see Sheet

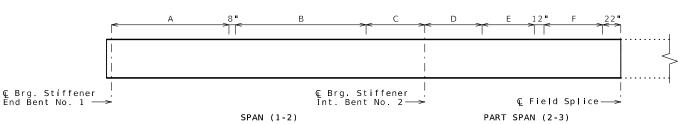
TABLE SHOWING SHEAR CONNECTOR UNIT SPACING

12 Spa. @ 12" 20 Spa. @ 8" $6' - 0\frac{1}{16}" \pm$

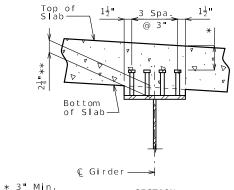
5 -10"

8 Spa. @ 8"

6 Spa. @ 12"

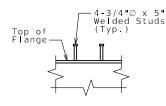


ELEVATION SHOWING SHEAR CONNECTOR SPACING FOR GIRDER (4 Shear Connectors per Unit) (Similar at Part Span (3-4) and Span (4-5))

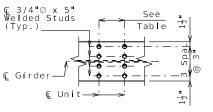


SECTION

** Dimension (bottom of slab to top of web) may vary if girder camber after erection differs from plan camber by more than the % of Dead Load Deflection due to weight of structural steel. payment will be made for any adjustment in forming or additional concrete required for variation in haunching



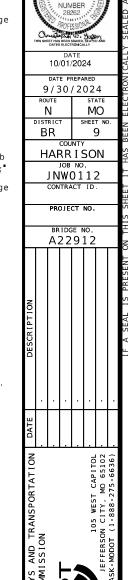
TOP OF FLANGE ELEVATION



PLAN OF SHEAR CONNECTOR (4 PER UNIT)

GIRDERS

SHEAR CONNECTOR DETAILS

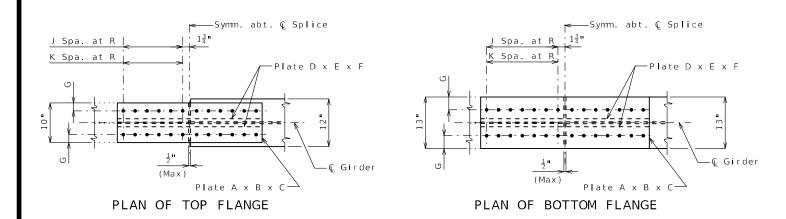


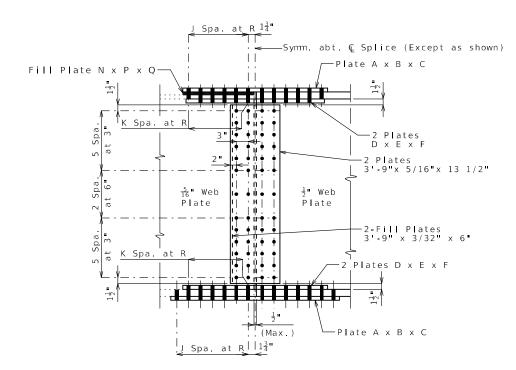
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WELDING DETAILS

Note: This drawing is not to scale. Follow dimensions

Sheet No. 9 of 15





DETAIL OF BOLTED FIELD SPLICE

Bolts shall be 7/8-inch diameter ASTM F3125 Grade A325 Type 1 in 15/16-inch diameter holes.

Contact surfaces shall be in accordance with Sec 1081 for surface preparation.

	TABLE OF DIMENSIONS - FIELD SPLICE														
LOCATION				D E F			G				N	Р	Q	R	
Top Flange	10"	1/2 m	3'-0 ¹ / ₂ "	4"	<u>5</u> 	2'-6½"	2 1 "	5	5	-	10"	1 "	15"	3 "	
Bottom Flange	13"	<u>5</u> 	3'-6½"	5 1 "	<u>3</u> 	3'-0 1 "	3 1 "	6	6	-	-	-	-	3 "	

BOLTED FIELD SPLICE	B	Οl	_T	ΕI	D	F	I	Ε	L	D	S	Ρ	L	I	C	Ε	
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Designed By: CEA 04/24 Detailed By: CEA 04/24 Checked By: CWT 05/24

TEPEN NUMBER 28262

10/01/2024

9/30/2024

HARRISON

JNW0112

PROJECT NO.

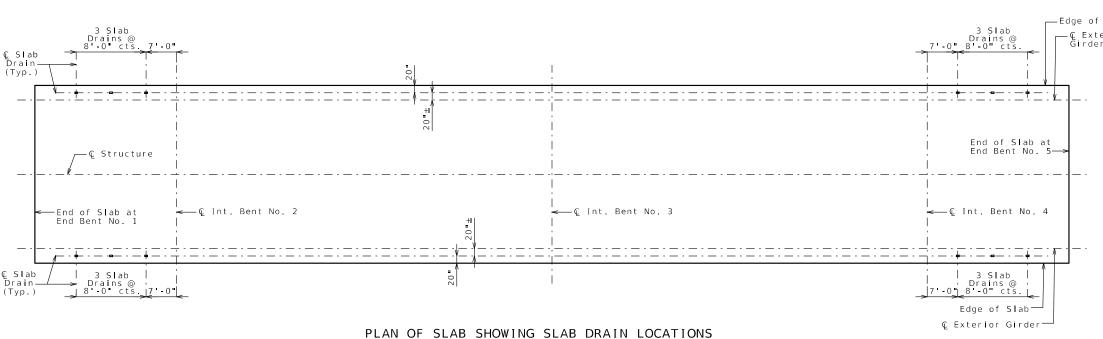
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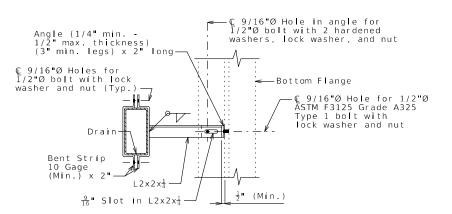
MO SHEET NO

10

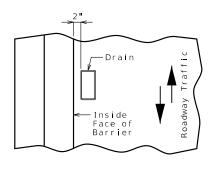
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PART SECTION SHOWING BRACKET ASSEMBLY



PART PLAN OF SLAB AT DRAIN

SLAB DRAINS

Note: This drawing is not to scale. Follow dimensions.

·Top of Roadway Slab

(Min.)

1" (Min.)

PART SECTION NEAR DRAIN

Designed By: CEA 04/24 Detailed By: CEA 04/24 Checked By: CWT 05/24



C Drain

General Notes:

Slab drain bracket assembly shall be ASTM A709 Grade 36 steel.

Locate drains in slab by dimensions shown in Part Section Near Drain.

Reinforcing steel shall be shifted to clear drains.

The bracket assembly shall be galvanized in accordance with ASTM A123.

All bolts, hardened washers, lock washers and nuts shall be galvanized in accordance with AASHTO M 232 (ASTM A153), Class C.

All 1/2"Ø bolts shall be ASTM A307, except as shown.

Shop drawings will not be required for the slab drains and the bracket assembly.

The bolt hole for the bracket assembly attachment shall be located on the plate girder shop drawings.

Notes for Steel Drain:

Slab drains may be fabricated of either 1/4" welded sheets of ASTM A709 Grade 36 steel or from 1/4" structural steel tubing ASTM A500 or A501.

Outside dimensions of drains are $8" \times 4"$.

The drains shall be galvanized in accordance with ASTM A123.

Notes for FRP Drain:

Drains shall be machine filament-wound thermosetting resin tubing meeting the requirements of ASTM D2996 with the following exceptions:

Shape of drains shall be rectangular with outside nominal dimensions of 8 " \times 4".

Minimum reinforced wall thickness shall be

The resin used shall be ultraviolet (UV) resistant and/or have UV inhibitors mixed throughout. Drains may have an exterior coating for additional UV resistance.

The color of the slab drain shall be gray (Federal Standard 26373). The color shall be uniform throughout the resin and any coating used.

The combination of materials used in the manufacture of the drains shall be tested for UV resistance in accordance with ASTM D4329 Cycle A. The representative material shall withstand at least 500 hours of testing with only minor discoloration and without any physical deterioration. The contractor shall furnish the results of the required ultraviolet testing prior to acceptance of the slab drains.

At the contractor's option, drains may be field cut. The method of cutting FRP slab drain shall be recommended by the manufacturer to ensure a smooth, chip free



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-Bottom of

-1/2"Ø x 3" Rod (ASTM A709 Grade 36) or 1/2"Ø x 3"± Shear Connector (Typ.)

–Ç Drain

1/2"Ø x 3" Galv. Carriage Bolt with Hex Nut and

Lock Washer (Typ.)

−¢ Drain

Roadway

Slab

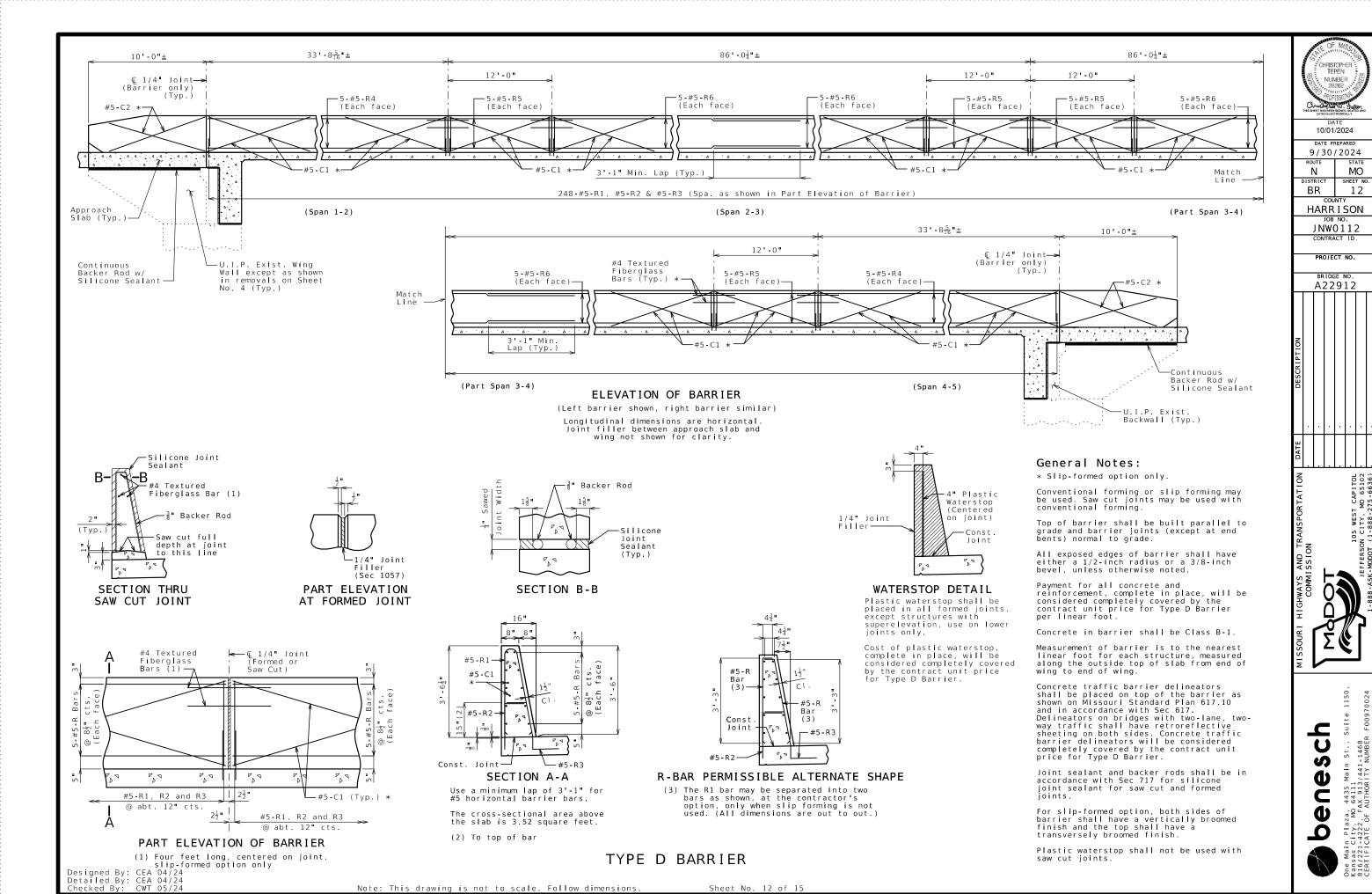
ELEVATION OF DRAIN

PLAN OF STEEL DRAIN OPTION

PLAN OF FRP DRAIN OPTION

– € Drain

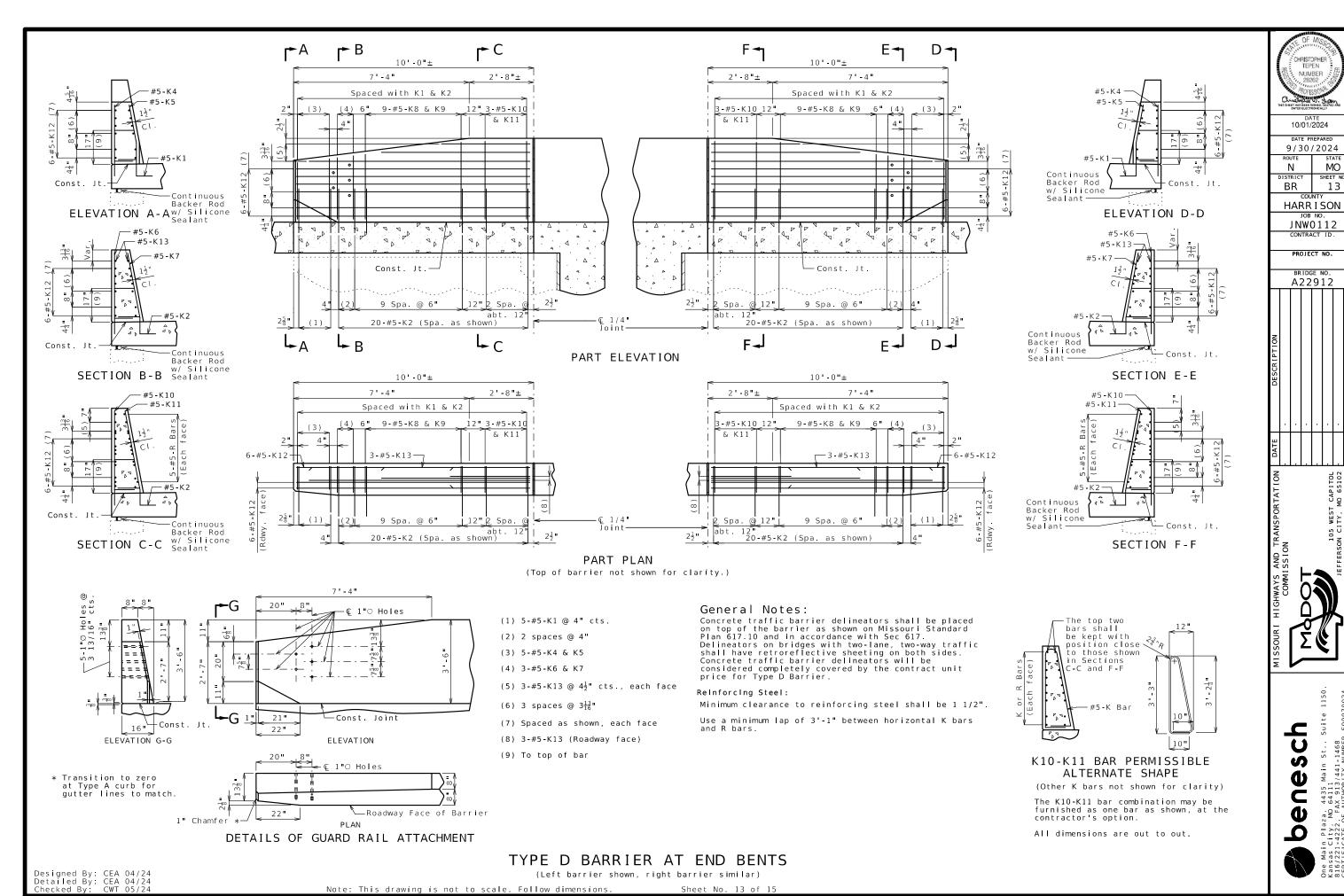
4" (Nom.)

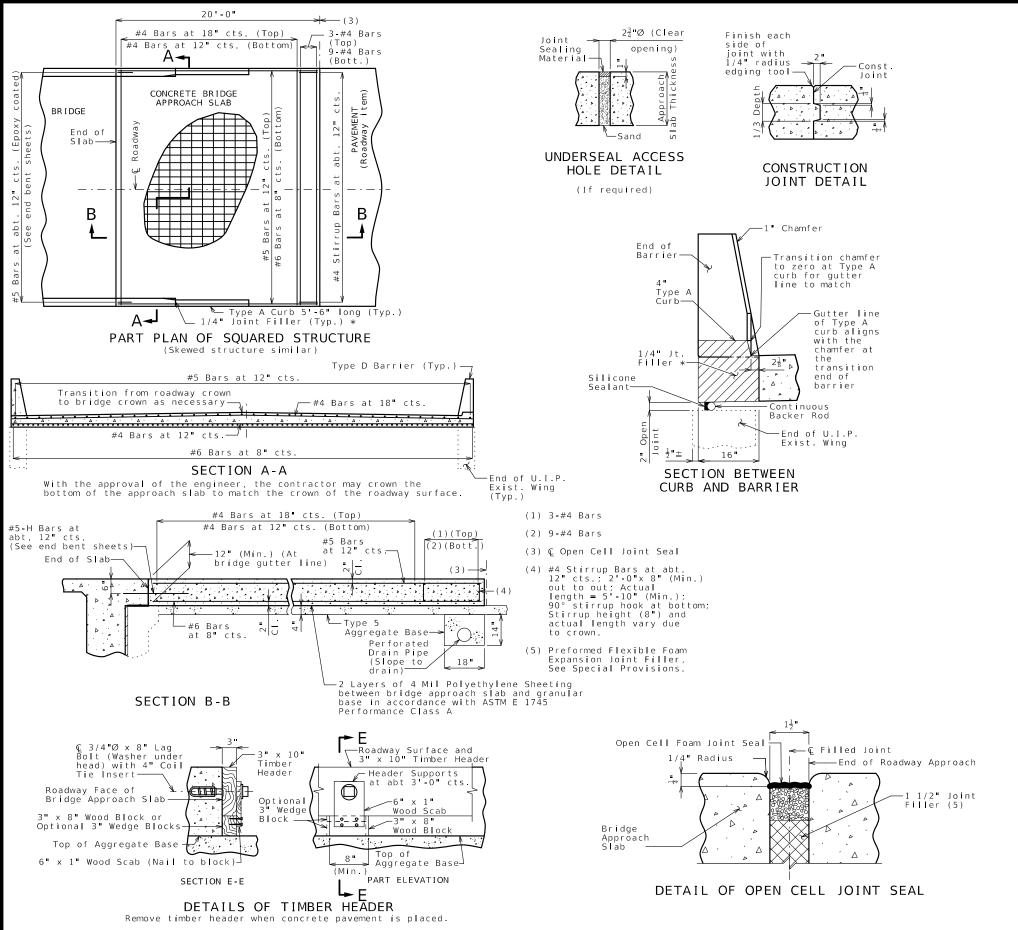


Sheet No. 12 of 15

TYPE D BARRIER

Note: This drawing is not to scale. Follow dimensions





General Notes:

The contractor shall pour and satisfactorily finish the bridge slab before placing the bridge approach slab.

All concrete for the bridge approach slab shall be in accordance with Sec 503 (f'c = 4,000 psi)

The reinforcing steel in the bridge approach slab shall be epoxy coated Grade 60 with fy = 60,000 psi

Longitudinal construction joints in bridge approach slab shall be aligned with longitudinal construction joints in bridge slab.

Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise shown.

The reinforcing steel in the bridge approach slab shall be continuous. The transverse reinforcing steel may be made continuous by providing a minimum lap splice of 23 inches for #4 bars, or by mechanical bar splice.

All joint filler shall be in accordance with Sec 1057 for preformed fiber expansion joint filler except as noted.

Payment for furnishing all materials, labor and excavation necessary to construct the concrete bridge approach slab, including the timber header, underdrain, Type 5 aggregate base, joint filler, and all other appurtenances and incidental work as shown on this sheet, complete in place, will be considered completely covered by the contract unit price for Bridge Approach Slab (Minor) per square yard.

See Missouri Standard Plan 609.00 for details of Type A curb.

Drain pipe may be either 6" diameter corrugated metallic-coated pipe underdrain, 4" diameter corrugated polyvinyl chloride (PVC) drain pipe, or 4" diameter corrugated polyethylene (PE) drain pipe.

* Seal joint between vertical face of approach slab and wing with sealant in accordance with Sec 717 for silicone joint sealant for saw cut and formed joints.



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HARRISON

JNW0112 CONTRACT ID.

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BRIDGE APPROACH SLAB (MINOR)

Sheet No. 14 of 15

Designed By: CEA 04/24 Detailed By: CEA 04/24 Checked By: CWT 05/24

