DESIGN DESIGNATION

A.A.D.T. - 2024 = 1228A.A.D.T. - 2044 = 1549

> T = 11% V = 55 M.P.H.

FUNCTIONAL CLASSIFICATION- MAJOR COLLECTOR

NO R/W IS REQUIRED FOR THIS PROJECT

CONVENTIONAL SYMBOLS

BUILDINGS AND STRUCTURES ㄷ늘=의 GUARD RAIL 0000 •••• GUARD CARLE 0000 •••• CONCRETE RIGHT-OF-WAY MARKER STEEL RIGHT-OF-WAY MARKER LOCATION SURVEY MARKER \circ UTILITIES FIBER OPTICS – FO – -FO-OVERHEAD CABLE TV -OTV-—UTV —OT —UT— —OE— UNDERGROUND CABLE TV OVERHEAD TELEPHONE -UTV-- OT-- UT -- OE -- UE -- S -UNDERGROUND TELEPHONE OVERHEAD POWER UNDERGROUND POWER —UE— —S— SANITARY SEWER STORM SEWER WATER SAN HYD WV WV MANHOLE FIRE HYDRANT WATER VALVE WATER METER " DROP INLET = DITCH BLOCK GROUND MOUNTED SIGN LIGHT POLE H-FRAME POWER POLE TELEPHONE PEDESTAL Δ FENCE CHAIN LINK WOVEN WIRE GATE POST \boxtimes BENCHMARK

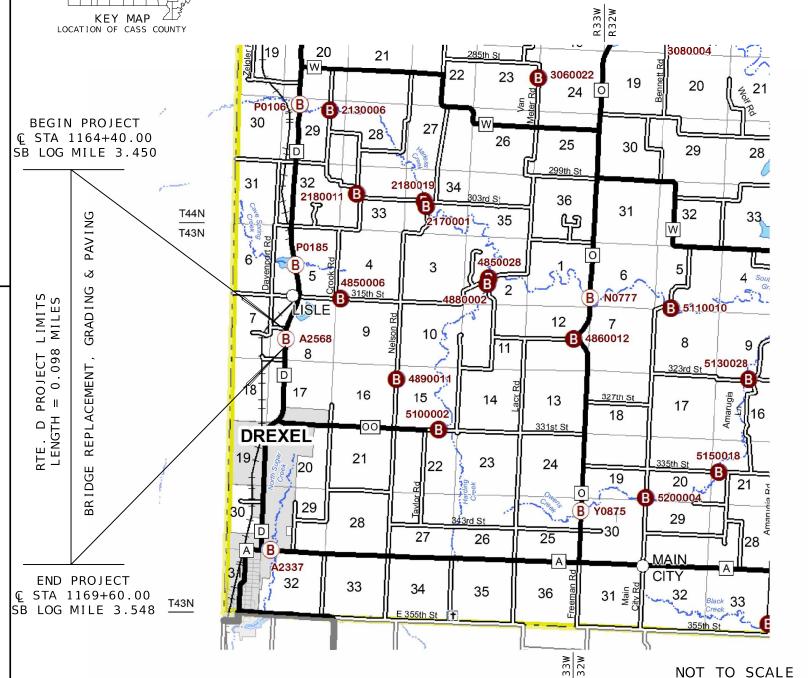
NOTE: DASHED OR OPEN SYMBOLS INDICATE EXISTING FEATURES

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

PLANS FOR PROPOSED

STATE HIGHWAY

CASS COUNTY



THE EXISTENCE AND APPROXIMATE LOCATION OF UTILITY FACILITIES KNOWN TO EXIST, AS SHOWN ON THE PLANS, ARE BASED ON THE BEST INFORMATION AVAILABLE TO THE COMMISSION AT THIS TIME. THIS INFORMATION IS PROVIDED BY THE COMMISSION "AS-IS" AND THE COMMISSION EXPRESSLY DISCLAIMS ANY REPRESENTATION OR WARRANTY AS TO THE COMPLETENESS, ACCURACY, OR SUITABILITY OF THE INFORMATION FOR ANY USE. RELIANCE UPON THIS INFORMATION IS DONE AT THE RISK AND PERIL OF THE USER, AND THE COMMISSION SHALL NOT BE LIABLE FOR ANY DAMAGES THAT MAY ARISE FROM ANY ERROR IN THE INFORMATION. IT IS, THEREFORE, THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY THE EXISTENCE, LOCATION AND STATUS OF ANY FACILITY. SUCH VERIFICATION INCLUDES DIRECT CONTACT WITH THE LISTED UTILITIES.

INDEX OF SHEETS

DESCRIPTION	SHEET NUMBER
TITLE SHEET	1
TYPICAL SECTIONS (TS) (2 SHEETS)	2
QUANTITIES (QU) (2 SHEETS)	3
PLAN-PROFILE (PP)	4
REFERENCE POINTS (RP)	5
COORDINATE POINTS (CP)	6
TRAFFIC CONTROL SHEETS (TC)	7 - 9
EROSION CONTROL SHEETS (EC)	10
BRIDGE DRAWINGS (B)	
A9428-COMPOSITE STEEL ALT	1-21
A9428-P/S CONCRETE ALT	1-21
CROSS SECTIONS (XS)	1 - 4

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		PRO	DJE	СТ	NO.							
BRIDGE NO. A9428												
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LENGTH OF PROJECT

BEGINNING OF PROJECT STA. 1164 + 40.00 END OF PROJECT STA. 1169 + 60.00 520.00 FEET APPARENT LENGTH

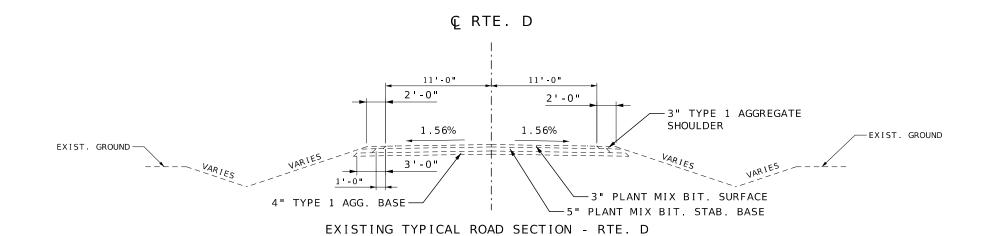
EQUATIONS AND EXCEPTIONS:

NONE

1 ACRES

TOTAL CORRECTIONS 0.00 FEET NET LENGTH OF PROJECT 520.00 FEET STATE LENGTH 0.098 MILES FOR INFORMATION ONLY ESTIMATED DISTURBED ACRES



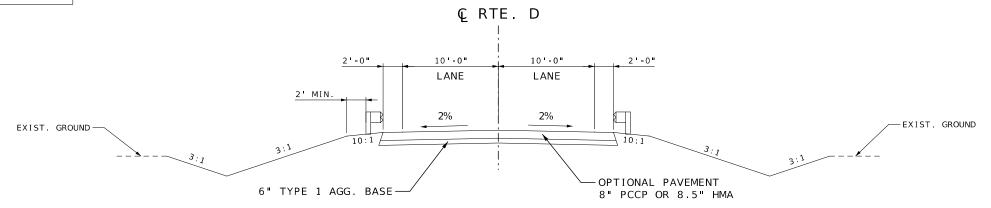


© STA. 1164+42.50 TO 1166+63.15 © STA. 1167+48.22 TO 1169+51.80

ESTIMATE FACTORS
(FOR INFORMATION ONLY)

BP-1 W/PG 64-22 - 1.984 TONS/C.Y. BB W/PG 64-22 - 1.997 TONS/C.Y. TACK COAT - 0.08 GAL/S.Y. OPTIONAL PAVEMENTS

HMA DESIGN
2" BP-1 W/ PG 64-22 OVER
6.5" BB W/ PG 64-22 OR
OR
PCCP DESIGN
8.0" JPCP WIDENED SLAB,
W/15' JOINT SPACING &
1.25" DOWELS



PROPOSED TYPICAL ROAD SECTION - RTE. D

© STA. 1164+42.50 TO 1166+58.18

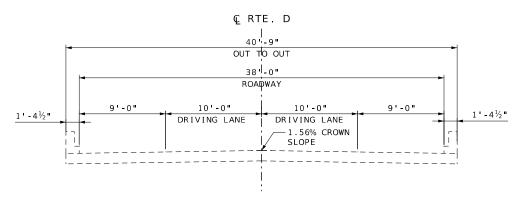
© STA. 1167+51.81 TO 1169+51.80

ROAD TYPICAL SHEET SHEET 1 OF 2

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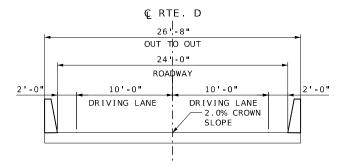
MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION

105 WEST CAPITOL
JEFFERSON CITY, MO 65102



EXISTING TYPICAL BRIDGE SECTION

BRIDGE A2568 OVER COLDWATER CREEK STA. 1166+62.20 TO 1167+47.78



PROPOSED TYPICAL BRIDGE SECTION

BRIDGE A9428 OVER COLDWATER CREEK STA. 1166+58.18 TO 1167+51.81



	REMOVAL OF IMPROVEMENTS									
SHEET	STA	STA	LOCATION	DESCRIPTION	QUANTITY	UNITS	REMARKS			
4	1164+42.50		RTE. D	PAVEMENT REMOVAL SAWCUT	22	LF	N. PAVEMENT SAWCUT			
4	1169+51.80		RTE. D	PAVEMENT REMOVAL SAWCUT	21	LF	S. PAVEMENT SAWCUT			
4	1164+42.50	1166+63.15	RTE. D	CONCRETE PAVEMENT REMOVAL	521	SY	NORTH SIDE PAVEMENT			
4	1166+62.20	1167+47.80	RTE. D	BRIDGE REMOVAL	388	SY	EXIST. BR. A2568			
4	1167+48.22	1169+51.80	RTE. D	CONCRETE PAVEMENT REMOVAL	484	SY	SOUTH SIDE PAVEMENT			
4	1666+42.41	1666+99.29	RTE. D	GROUTED RIP RAP REMOVAL	276	SY	N SIDE BR. SPILL SLOPE			
4	1167+17.85	1167+78.13	RTE. D	GROUTED RIP RAP REMOVAL	267	SY	S SIDE BR. SPILL SLOPE			
4	1164+88.44	1166+61.75	RTE. D	GUARDRAIL REMOVAL	173	LF	NW GUARDRAIL			
4	1165+16.11	1166+51.61	RTE. D	GUARDRAIL REMOVAL	136	LF	NE GUARDRAIL			
4	1167+59.77	1168+95.31	RTE. D	GUARDRAIL REMOVAL	136	LF	SW GUARDRAIL			
4	1167+49.56	1169+22.59	RTE. D	GUARDRAIL REMOVAL	173	LF	SE GUARDRAIL			
			•	1 LUMP SUM	•					

CLEARING & GRUBBING						
		AREA				
STA	STA	ACRE				
1164+42.50	1169+51.80	0.2				
	TOTAL	0.2				
	PAY TOTAL	1				

ЭB	ILIZ.	ATION
1	LUMP	SUM

CONTRACTOR FURNISHED SURVEYING AND STAKING 1 LUMP SUM

	PAVEMENT								
				OPTIONAL	6" TYPE 5				
			LENGTH	PAVEMENT	BASE				
STA.	STA.	LOCATION	FT	SY	SY	REMARKS			
1164+42.50	1166+63.15	RTE. D	220.65	588.40	588.40	N. PAVEMENT REPLACEMENT			
1167+48.22	1169+51.80	RTE. D	203.58	542.88	542.88	S. PAVEMENT REPLACEMENT			
		TOTAL	1131.28	1131.28					
			PAY TOTAL	1131.3	1131				

	EARTHWORK									
		CLASS A	COMPACTING	COMPACTING						
		EXCAVATION	EMBANKMENT	IN CUT						
STA	STA	CY	CY	STA						
1164+42.50	1169+51.80	350.29	32.02	4.24						
	TOTAL	350.3	32.0	4.24						
	PAY TOTAL	350	32	4.2						

	GUARDRA I L										
					MGS GUARDRAIL	TYPE A CRASHWORTHY	MGS BRIDGE APPROACH				
					6 FT. POSTS,	END TERMINAL 50 FT.	TRANSITION				
SHEET	BEGIN	END			6 FT 3 IN. SPACING	(MASH)	(SAFETY BARRIER - BRIDGE)				
NO.	STATION	STATION	ROADWAY	LOCAT I ON	(L.F.)	(EA.)	(EACH)				
4	1165+14.11	1166+51.61	RTE. D	SURVEY CL LT. (NE)	50.0	1	1				
4	1164+61.75	1166+61.75	RTE. D	SURVEY CL RT. (NW)	112.5	1	1				
4	1167+59.77	1168+97.27	RTE. D	SURVEY CL RT. (SW)	50.0	1	1				
4	1167+49.56	1169+49.56	RTE. D	SURVEY CL LT. (SE)	112.5	1	1				
				TOTAL	325.0	4	4				
				PAY TOTAL	325	4	4				

	PAVEMENT MARKING								
				4" WHITE STANDARD WATERBORNE PAVEMENT	4" YELLOW STANDARD WATERBORNE PAVEMENT				
			LENGTH	MARKING PAINT, TYPE P BEADS	MARKING PAINT, TYPE P BEADS				
STA	STA	LOCATION	FT	LF	LF				
1164+42.50	1169+51.80	RTE. D	509	1018.0	802.6				
	1		TOTALS	1018.0	802.6				
	PAY TOTAL			1018	803				

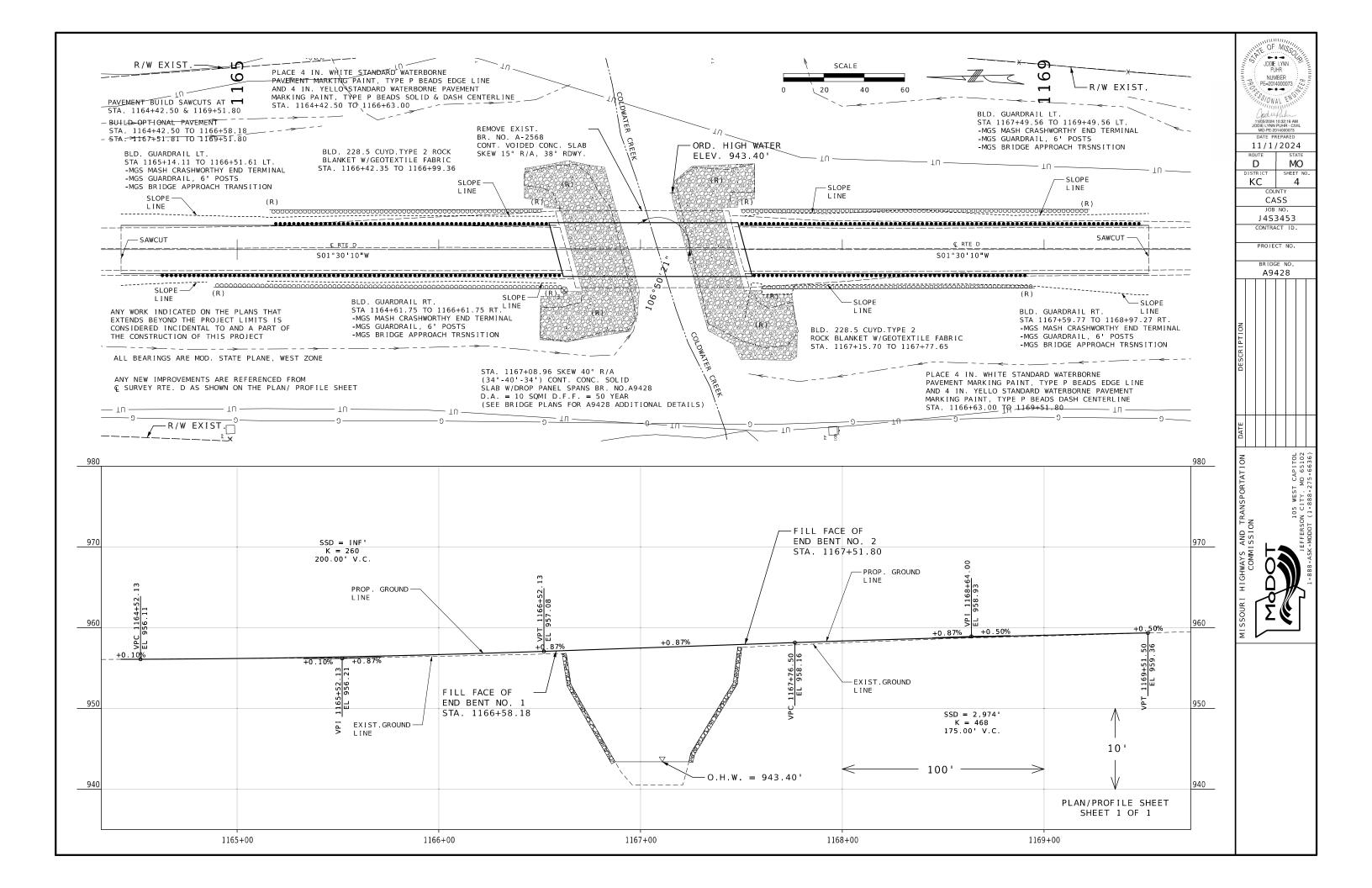
TEMPORARY EROSION CONTROL									
				SILT	SEDIMENT	ALT. DITCH	TEMP. BERM		
				FENCE	REMOVAL	CHECK	TYPE C		
STA	STA	OFFSET	SIDE	LF	CY	LF	LF		
1164+74.44	1167+67.21		LT	211.3	2.1				
1169+85.56	1167+21.64		LT	281.3	2.8				
1166+92.07	1164+78.22		RT	228.7	2.3				
1169+85.43	1167+29.80		RT	281.3	2.8				
1166+66.92		39.6	LT		1.0	15			
1166+91.54			RT		1.0	15			
1167+22.00		38.5	LT.		1.0	15			
1167+30.66		40.9	RT.		1.0	15			
1166+91.69			BOTH		2.0		127.7		
1167+16.76			BOTH		2.0		140.4		
			TOTAL	1002.6	18.0	60	268.1		
			PAY TOTAL	1003	18	60	268		

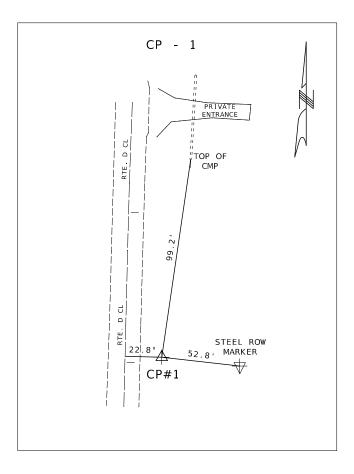
SEEDING									
		AREA COOL SEEDING MIX ADDITIONAL MOBILIZATI							
STA	STA	SF	ACRE	FOR SEEDING					
1164+42.50	1169+51.80	19647.34	0.5	1					
		TOTALS	0.5	1					
		PAY TOTAL	1	1					

	PERMANENT EROSION CONTROL									
STA.	STA. STA. SIDE PLACING TYPE 2 FURNISHING TYPE 2				REMARKS					
			ROCK BLANKET	ROCK BLANKET						
			C.Y.	C.Y.						
1166+42.35	1166+99.36	вотн	228.5	228.5	NORTH SIDE BRIDGE SPILL SLOPE					
1167+15.70	1167+77.65	вотн	228.5	228.5	SOUTH SIDE BRIDGE SPILL SLOPE					
		TOTAL	457.0	457.0						
		DAY TOTAL	157	457						

SUMMARY OF QUANTITIES SHEET 1 OF 2

				TOTAL	OTY TOTAL SIGN	П	1				QTY TOTAL SIGN				EFFECTIVE: 07-01-2024	
	 S I Z	F ARE	$\Delta \mid_{OT}$		RELOCIRELOCI NUM.		SIZE	ARFA	OTY	TOTAL	RELOC RELOC NUM		I ITEM T	OTAL	2112011121 07 02 2021	WE OF MOON
SIGN	IN				EACH SQ.FT. DESCRIPTION	SIGN		1	,	1	EACH SQ FT	DESCRIPTION	1	OTY	DESCRIPTION	IN Off
3101	110	130.1	LAC	_	NING SIGNS	310N	TIV.	30.11.	LACII		DE SIGNS	DESCRITTION	6122008	711	IMPACT ATTENUATOR 40 MPH (SAND BARRELS)	JODIE LYNN FUHR
WO1 - 1L	18X	48 16.0	n l	WAR	TURN (SYMBOL LEFT)	E05-1	36¥48	12.00		G011	DE SIGNS	GORE EXIT	6122009		IMPACT ATTENUATOR 45 MPH (SAND BARRELS)	NUMBER OF SOME
WO1 - 1R		48 16.0			TURN (SYMBOL RIGHT)	E05-2		12.00		+ +		EXIT OPEN	6122010		IMPACT ATTENUATOR 50 MPH (SAND BARRELS)	PE-2014000073
WO1 - 2L		48 16.0	_		CURVE (SYMBOL LEFT)	E05 - 2a		12 00				EXIT CLOSED	6122012		IMPACT ATTENUATOR 55 MPH (SAND BARRELS)	11/18/ONAL ENGLISH
WO1 - 2R		48 16.0			CURVE (SYMBOL RIGHT)	GO20 - 1		10.00				ROAD WORK NEXT XX MILES	6122014		IMPACT ATTENUATOR 60 MPH (SAND BARRELS)	Godufiche
WO1-3L	48X	48 16.0	0		REVERSE TURN (SYMBOL LEFT)	GO20-2	48X24					END ROAD WORK	6122017		IMPACT ATTENUATOR 65 MPH (SAND BARRELS)	09/30/2024 5:46:39 PM JODIE LYNN PUHR - CIVIL
WO1 - 3R	48X	48 16.0	0		REVERSE TURN (SYMBOL RIGHT)	GO20-4	36X18	4.50				PILOT CAR FOLLOW ME	6122019		IMPACT ATTENUATOR 70 MPH (SAND BARRELS)	MO-PE-2014000073 DATE PREPARED
WO1-4L	48X	48 16.0	0		REVERSE CURVE (SYMBOL LEFT)	GO20-4a	42X30	8.75				PILOT CAR IN USE WAIT & FOLLOW	6122020		REPLACEMENT SAND BARREL	9/26/2024
WO1-4R	48X	48 16.0	0		REVERSE CURVE (SYMBOL RIGHT)	GO20-4a	18X12	1.50				PILOT CAR IN USE WAIT & FOLLOW	6122030		IMPACT ATTENUATOR (RELOCATION)	ROUTE STATE
WO1-4bL	48X	48 16.0	0		DOUBLE ARROW REVERSE CURVE (SYMBOL LEFT)	GO20-5aF	36X24	6.00				WORK ZONE (PLAQUE)	6123001		TRUCK MOUNTED ATTENUATOR (TMA)	D MO
WO1-4bR	48X	48 16.0	0		DOUBLE ARROW REVERSE CURVE (SYMBOL RIGHT)	MO4 - 8a	24X18	3.00	2	6.00		END DETOUR	6161008	2	ADVANCED WARNING RAIL SYSTEM	DISTRICT SHEET NO.
WO1-4cL	48X	48 16.0	0		TRIPLE ARROW REVERSE CURVE (SYMBOL LEFT)	MO4 - 9L	48X36	12.00				DETOUR (LEFT)	6161012		BUOYS (BOATS KEEP OUT)	KC 3
WO1-4cR	48X	48 16.0	0		TRIPLE ARROW REVERSE CURVE (SYMBOL RIGHT)	MO4 - 9R	48X36	12.00				DETOUR (RIGHT)	6161013		BUOYS (NO WAKE)	CASS
WO1-6		30 12.5			HORIZONTAL ARROW (SYMBOL)	MO4 - 9P	48X12					STREET NAME (PLAQUE)	6161014		SPECIAL SIGN ASSEMBLY (BOATS KEEP OUT)	JOB NO.
WO1-6a		36 18.0			HORIZ. ARROW (SYMBOL ON PERMANENT BARRICADE)	MO4 - 10L	48X18					DETOUR ARROW (LEFT)	6161025		CHANNELIZER (TRIM LINE)	J4S3453
WO1 - 7		30 12.5			DOUBLE HEAD HORIZONTAL ARROW (SYMBOL)	MO4 - 10R	48X18	6.00		DECLU A		DETOUR ARROW (RIGHT)	6161030	6	TYPE III MOVEABLE BARRICADE	CONTRACT ID.
WO1 - 7 a		36 18.0			DOUBLE HEAD HORIZ. ARROW (SYMBOL ON PERM. BARR.)	 	101/10	12.25		REGULA	TORY SIGNS	STOR	6161033		DIRECTION INDICATOR BARRICADE	
WO1 - 8		24 3.00			CHEVRON (SYMBOL)	R1-1		13.25				STOP	6161040		FLASHING ARROW PANEL	PROJECT NO.
WO1 - 8a	30X				CHEVRON (SYMBOL FOR DIVIDED HIGHWAYS)	R1-2		9.00				YIELD TO ONCOMING TRAFFIC (BLACHE)			TYPE III OBJECT MARKER	BRIDGE NO.
WO3 - 1 WO3 - 2		48 16.0 48 16.0			STOP AHEAD (SYMBOL) YIELD AHEAD (SYMBOL)	R1-2a R1-3P		2.50		+ +		TO ONCOMING TRAFFIC (PLAQUE) ALL WAY (PLAQUE)	6161055		SEQUENTIAL FLASHING WARNING LIGHT TUBULAR MARKER	A9428
WO3 - 2		48 16.0	_		SIGNAL AHEAD (SYMBOL)	R2-1		12.00				SPEED LIMIT XX	6161070		RADAR SPEED ADVISORY SYSTEM	
WO3 - 4		48 16.0			BE PREPARED TO STOP	R3-1		16.00				NO RIGHT TURN (SYMBOL)	5101095		CHANGEABLE MESSAGE SIGN,	
WO3 - 5		48 16.0			SPEED LIMIT AHEAD	R3-2		16.00		+ +		NO LEFT TURN (SYMBOL)	6161096		COMMISSION FURNISHED/RETAINED	
WO4 - 1L		48 16.0			MERGE (SYMBOL FROM LEFT)	R3-3		9.00		+ +		NO TURNS			CHANGEABLE MESSAGE SIGN WITHOUT COMM.	$ \ \ \ \ \ \ $
WO4 - 1R		48 16.0			MERGE (SYMBOL FROM RIGHT)	R3-4		16.00				NO U-TURN (SYMBOL)	6161098A	3	INTERFACE- CONTRACTOR FURNISHED/RETAINED	
WO4 - 1 aL		48 16.0			MERGE (STRIBGE TROPERTIESTE)	R3 - 7L		6.25				LEFT LANE MUST TURN LEFT			CHANGEABLE MESSAGE SIGN WITH COMM.	[2]
WO4 - 1 aR		48 16.0			MERGE (RIGHT)	R3 - 7R		6.25				RIGHT LANE MUST TURN RIGHT	6161099		INTERFACE - CONTRACTOR FURNISHED/RETAINED	[4]
WO5 - 1		48 16.0			ROAD/BRIDGE/RAMP NARROWS	R4-1		12.00				DO NOT PASS	6162000A		WORK ZONE TRAFFIC SIGNAL SYSTEM	SS
WO5 - 3	48X	48 16.0	0		ONE LANE BRIDGE	R4-2	36X48	12.00				PASS WITH CARE	6162002		TEMPORARY LONG-TERM RUMBLE STRIPS	
WO5 - 5	48X	48 16.0	0		NARROW LANES	R4-7a	36X48	12.00				KEEP RIGHT (HORIZONTAL ARROW)			TEMPORARY TRAFFIC BARRIER	$oxed{ }^-oxed{ }$
WO6 - 1	48X	48 16.0	0		DIVIDED HIGHWAY (SYMBOL)	R4-8a	36X48	12.00				KEEP LEFT (HORIZONTAL ARROW)	6173600D		CONTRACTOR FURNISHED/RETAINED	$[\ \ \ \ \ \ \]$
WO6 - 2		48 16.0			DIVIDED HIGHWAY END (SYMBOL)	R5-1	30X30					DO NOT ENTER			TEMPORARY TRAFFIC BARRIER	
WO6 - 3		48 16.0			TWO WAY TRAFFIC (SYMBOL)	R5-1a		6.00				WRONG WAY	6173602B		CONTRACTOR FURNISHED/COMMISSION RETAINED	$ \ \ \ \ \ \ $
WO7 - 3 a		24 5.00			NEXT XX MILES (PLAQUE)	R6-1L		6.75				ONE WAY ARROW (LEFT)	6174000A		TEMP. TRAFFIC BARRIER HEIGHT TRANSITION	
WO8 - 1		48 16.0			BUMP	R6-1R		6.75				ONE WAY ARROW (RIGHT)	6175010A		RELOCATING TEMPORARY TRAFFIC BARRIER	
WO8 - 2		48 16.0			DIP	R6-2L		5.00				ONE WAY (LEFT)			TEMPORARY TRAFFIC BARRIER	
WO8 - 3		48 16.0			PAVEMENT ENDS	R6-2R	24X30			1		ONE WAY (RIGHT)	6176000B		COMMISSION FURNISHED/RETAINED	Z
WO8 - 4		48 16.0			SOFT SHOULDER	R9-9	24X12	2.00				SIDEWALK CLOSED AHEAD	61770000		TEMP. TRAFFIC BARRIER HEIGHT TRANSITION	ION TOL
WO8 - 5 WO8 - 6		48 16.0 48 16.0	_		SLIPPERY WHEN WET (SYMBOL) TRUCK CROSSING	 R9-11L	24X18	3 00				SIDEWALK CLOSED AHEAD, (ARROW LEFT) CROSS HERE	6177000B 6208064A		COMMISSION FURNISHED/RETAINED TEMPORARY RAISED PAVEMENT MARKER	AT AP1 65
WO8 - 6 c		48 16.0 48 16.0	-		TRUCK CROSSING TRUCK ENTRANCE	H_L2-11F	24/18	3.00		+ +		SIDEWALK CLOSED AHEAD,	9029400		TEMPORARY RAISED PAVEMENT MARKER TEMPORARY TRAFFIC SIGNALS	SPORTATI SPORTATI TY, MO 651 888-275-663
WO8 - 7		36 9.00	_		LOOSE GRAVEL	 R9-11R	24X18	3 00				(ARROW RIGHT) CROSS HERE	9029400		TEMPORARY TRAFFIC SIGNALS AND LIGHTING	PO PO Y,
WO8 - 7a		36 9.00			FRESH OIL / LOOSE GRAVEL	R10-6		6.00				STOP HERE ON RED (45^ ARROW)	3023401		SIVILL HALL TO STORALS AND ETGHTING	NSN FILE
WO8 - 7 a		48 16.0			LOW SHOULDER	R11-2		10.00		+ +		ROAD CLOSED				TRANS
WO8 - 11		48 16.0			UNEVEN LANES	11	137.30					ROAD CLOSED XX MILES AHEAD				NO IOS:
WO8 - 12					NO CENTER LINE	R11-3a	60X30	12.50	2	25.00		LOCAL TRAFFIC ONLY				SIC
WO8 - 15					GROOVED PAVEMENT	4		1		25.00		ROAD CLOSED TO THRU TRAFFIC				II S:
WO8 - 15P			_		MOTORCYCLE (PLAQUE)	CONST - 3A						FINE SIGN				S Y S
WO8 - 17L					SHOULDER DROP-OFF (SYMBOL LEFT)	CONST - 3>	56X12	4.67				SPEEDING/PASSING (PLATE)				<u>₹</u> 8 () \
WO8 - 17R	48X	48 16.0	0		SHOULDER DROP-OFF (SYMBOL RIGHT)				M	1I SCELL	ANEOUS SIGNS		'			5 × \
WO8 - 17P	30X	24 5.00)		SHOULDER DROP-OFF (PLAQUE)	CONST - 5	48X36	12.00				POINT OF PRESENCE				
W10-1	42RN	ND. 9.62	2		RAILROAD CROSSING	CONST - 5	96X48	32.00				POINT OF PRESENCE				
WO12-1			_		DOUBLE DOWN ARROW (SYMBOL)	CONST-8						WORK ZONE NO PHONE ZONE				4), 7 g
WO12-2					LOW CLEARANCE (SYMBOL)	SPECIAL	136X60	15.00	24	360.00		DETOUR ROUTE ASSEMBLY (LETTER)				/ / /
W012-2x			_		LOW CLEARANCE (PLAQUE)	 										
WO12-2a			_		OVERHEAD LOW CLEARANCE (FEET AND INCHES)	 										2 V
WO12-4					LOW CLEARANCE XX FT XX IN XX MILES AHEAD	-										
WO12-5					WIDTH RESTRICTION XX FT XX IN XX MILES AHEAD	-										
		30 6.25			ADVISORY SPEED (PLAQUE)	-										
WO16 - 2					XXX FEET (PLAQUE)			-		+ +						
WO16-3				77.00	X MILE (PLAQUE)	-										
		48 16.0 48 16.0	_	32.00 48.00		-										
			_	32.00		616-10	0.5	1		TOTAL						
WO20-3		48 16.0		32.00	ONE LANE ROAD AHEAD	CONSTR		ע כוכמי	5	528						
WO20-4			_		RIGHT/CENTER/LEFT LANE CLOSED AHEAD	616-10		* 210IA	ی	1 320	TOTAL					
WO20-5a					2 RIGHT/CENTER/LEFT LANES CLOSED AHEAD	RELOCA		IGNS			0					
WO20-5a			_		RIGHT/CENTER/LEFT LANE CLOSED	I NELOCA	3	. 0.10								
WO20 - 7a					FLAGGER (SYMBOL)	1										
WO21-2					FRESH OIL	1										
WO21-5					SHOULDER WORK / SHOULDER WORK AHEAD	1										
		48 16.0			BLASTING ZONE AHEAD	1										
WO22-2					TURN OFF 2-WAY RADIO AND PHONE]									SUMMARY OF QUANTITIES SHEET	
WO22-3	42X	36 10.5	0		END BLASTING ZONE]									SHEET 2 OF 2	
GO22-1	21X	15 2.19)		WET PAINT (ARROW PIVOTS)]									•	

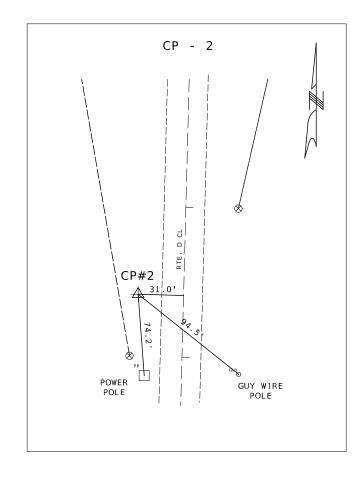




CP # 1 IS A MODNR REFERENCE MONUMENT WITH AN ALUMINUM CAP. THE CODE IS CA-112 AND WAS SET IN 1996. THE MONUMENT IS BURIED 0.25' ± BELOW THE GROUND. THE MONUMENT IS 4.0 MI. NORTH OF DREXEL AND 6.6 MI. SOUTH OF WEST LINE.

MODIFIED STATE PLANE COORDINATES

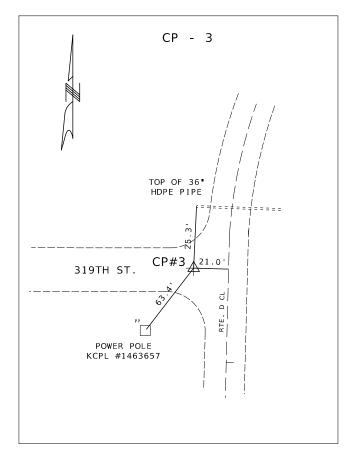
NORTHING: 864462.843 EASTING: 2761437.172 ELEVATION: 954.471'



CP # 2 IS A MODNR REFERENCE MONUMENT WITH AN ALUMINUM CAP. THE CODE IS CA-112A AND WAS SET IN 1995. THE MONUMENT IS SET FLUSH WITH THE GROUND. THE MONUMENT IS 0.3 MI. SOUTH OF CA-112.

MODIFIED STATE PLANE COORDINATES

NORTHING: 863101.646 EASTING: 2761348.133 ELEVATION: 971.195'



CP # 3 IS A 3/8" IRON BAR THAT LIES IN THE CENTERLINE OF 319TH ST. AT THE INTERSECTION OF HWY D AND SERVES AS A SECTION CORNER. THE BAR IS BURIED 0.5' ± BELOW THE SURFACE OF THE ROAD.

MODIFIED STATE PLANE COORDINATES

NORTHING: 865014.940 EASTING: 2761428.024 ELEVATION: 958.611'



ALL COORDINATES ON THIS SHEET ARE MODIFIED STATE PLANE (GROUND) MISSOURI WESTERN ZONE, NAD 1983 PROJECTION FACTOR: 1.00009861 AVERAGE GRID FACTOR: 0.99990139

ALL PROJECT COORDINATES HAVE BEEN PROJECTED FROM THE MISSOURI STATE PLANE COORDINATE (SPC) SYSTEM OF 1983 USING AN AVERAGE PROJECT PROJECTION (GRID TO GROUND) FACTOR. TO GET BACK TO STATE PLANE COORDINATES MULTIPY THE PROJECT COORDINATES BY THE AVERAGE GRID FACTOR AS SHOWN IN THE "REFERENCE CONTROL INFORMATION" PORTION OF THIS TABLE.

PROJECT COORDINATE INFORMATION								
COORDINATE SYSTE	М	MODIFIED S	TATE	PLANE (GRO	OUND)			
HORIZONTAL DATUM	1	NAD83						
VERTICAL DATUM		NAVD88						
GEOID MODEL		GEOID18						
ELEVATIONS		CDC ODCEDV	/A T I ON	1				
DETERMINED BY		GPS OBSERVATION						
PROJECT PROJECTI	ON	1.00009861						
REFERENCE CON	TR	OL INFORM	1AT I C	N				
COORDINATE SYSTE	М	MO COORDIN	IATE S	SYSTEM OF	1983			
CONTROL STATION		MO GEOGRAP	HIC F	REFERENCE	SYSTEM			
DESIGNATION	CA	A-112						
CORS_ID								
PID	37	7112						
LATITUDE 38°32'26.30822"								

EXAMPLE OF PROJECT COORDINATE TO S.P.C.

94°35'46.67762"

263462.8570

841604.7580

PROJECT NORTHING X AVERAGE GRID FACTOR = STATE PLANE NORTHING PROJECT EASTING X AVERAGE GRID FACTOR = STATE PLANE EASTING

WEST PROJECT AVERAGE GRID FACTOR 0.99990140

LONGITUDE

ZONE

NORTHING (M)

EASTING (M)

EXAMPLE: CONTROL POINT #___ N _____ X _.__ = N ____ = E ____ X _.__ = E

LINEAR UNIT CONVERSION 1 METER = 3.280833333 US SURVEY FEET (USFT)

	COORDINATE POINT LISTING								
				MODIFIE	D STATE PLANE (GROUND)			
			OFFSET	NORTHING	EASTING	ELEVATION		GPK	
SHEET NO	STATION	LOCATION	(USFT)	(US SURVEY FT)	(US SURVEY FT)	(US SURVEY FT)	DESCRIPTION	POINT ID	
PROJECT CO	NTROL POINTS	5							
	1162+95.86	LT	23.0163	864,462.84	2,761,437.17	954.47	CA-112 MoDNR Monument Aluminum Cap	6489	
	1176+58.93	RT	30.2941	863,101.65	2,761,348.13	971.20	CA-112A MoDNR Monument Aluminum Cap	6495	
				865,014.94	2,761,428.02	958.61	#4 3/8" Iron Bar (**Point is north of survey cover	ge)	
AL I GNMENTS	5								
	1160+08.00		0	864,751.21	2,761,421.71		Begin project RTEDBLEX alignment	501	
	1180+00.00		0	862,759.89	2,761,369.47		End project RTEDBLEX alignment	502	
	1164+42.50		0			956.10	Start Vertical profile RTEDPRO1		
	1164+52.13		0			956.11	VPC Parabolic Sag Curve		
	1165+52.13		0			956.21	VPI Parabolic Sag Curve		
	1166+52.13		0			957.08	VPT Parabolic Sag Curve		
	1166+58.18		0			957.13	VPI Linear		
	1167+76.50		0			958.16	VPC Parabolic crest curve		
	1168+64.00		0			958.93	VPI Parabollic Crest Curve		
	1169+51.50		0			959.37	VPT Parabolic Crest Curve		
	1169+51.80		0			959.37	End Vertical profile RTEPRO1		

JODIE LYNN PUHR NUMBER PE-2014000073

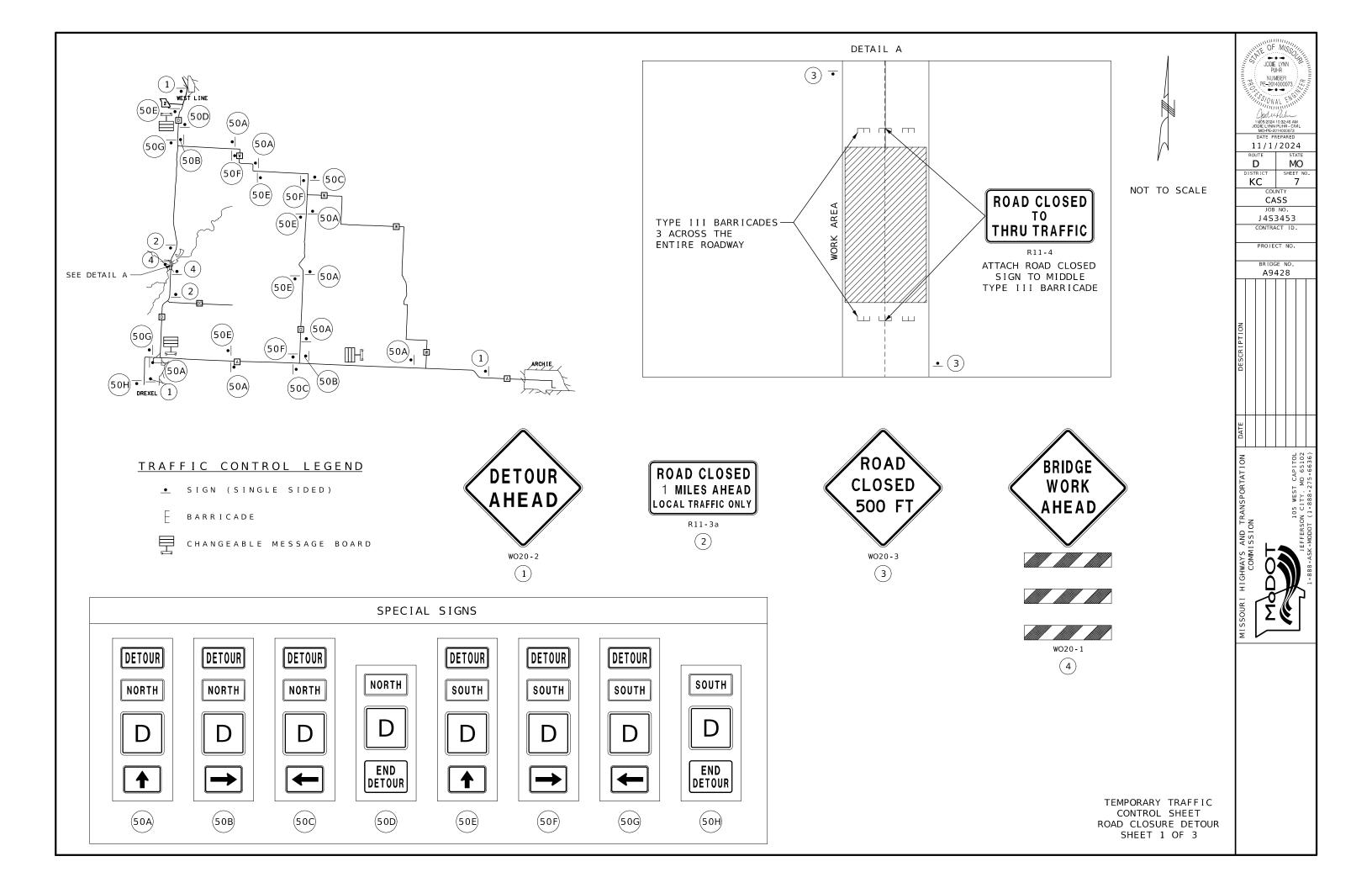
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SHEET NO. KC 6 CASS JOB NO. J4S3453 CONTRACT ID. PROJECT NO. BRIDGE NO. A9428

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COORDINATE POINT SHEET SHEET 1 OF 1



Sign Spacing, Device Spacing and Channelizing Taper Lengths

SIGN SPACING	(S) FOR ADVANCE SIG	GN SERIES (1) (2)		
SPEED MPH	UNDIVIDED HIGHWAYS	DIVIDED HIGHWAYS		
0 - 35	200	200		
40 - 45	350	500		
50-55	500	1000		
60-70	1000	SA - 1000 SB - 1500 SC - 2640		

LONGITUDINA	AL BUFFER SPACE (B)
SPEED MPH	BUFFER SPACE
0-35	250
40-45	360
50-55	495
60-70	730

	TAPER LENGTHS AND SPACING OF CHANNELIZING DEVICES								
SPEED	TAPER	MINIMUM R LENGTH	S (L)	MAXIMUM CHANNELIZER SPACING					
MPH		NE WIDT	, , ,	THROUGH	THROUGH				
	10 FT	11 FT	12 FT	TAPER	WORK AREA				
0-35	205	225	245	35	40				
40-45	450	495	540	40	80				
50-55	550	605	660	50	80				
60-70	700	770	840	60	120				

TAPER LENGTH (L):

L = WS FOR 40 MPH OR MORE

 $L = \frac{WS^2}{60} FOR 35 MPH OR LESS$

FOR SHOULDER TAPER USE L/3

WHERE:

L = TAPER LENGTH IN FEET

W = LATERAL SHIFT IN FEET

S = POSTED SPEED IN MPH

NOTES:

DIMENSIONS IN FEET UNLESS OTHERWISE NOTED.

- (1) SPACING BETWEEN SIGNS AND SPACING BETWEEN LAST SIGN AND FLAGGER, BEGINNING OF TAPER, OR SIGNED CONDITION
- (2) SPACINGS MAY BE ADJUSTED AS NECESSARY TO MEET FIELD CONDITIONS

TRAFFIC CONTROL DEVICES

GENERAL NOTES:

- 1. SEE STANDARD PLAN 616.10 FOR DETAILS
 AND ITEMS NOT SHOWN
- EXISTING SIGNS SHALL BE COVERED DURING WORKING HOURS ONLY IF IN CONFLICT WITH TRAFFIC CONTROL PLANS.
- NO DIRECT PAYMENT WILL BE MADE FOR RELOCATING, COVERING, UNCOVERING OR REMOVING SIGNS.
- 4. CONES ALLOWABLE FOR DAYTIME OPERATIONS ONLY.
- 6. LOCATE FLASHING ARROW PANEL AT BEGINNING OF
 TAPER WHEN FEASIBLE, ARROW PANELS ARE ALWAYS
 LOCATED BEHIND CHANNELIZERS OR CONES.

TRAFFIC CONTROL LEGEND

- SIGN (SINGLE SIDED)
- SIGN (DOUBLE SIDED)

FLAGGER

- ▲ DIRECTIONAL INDICATOR BARRICADE
- CHANNELIZER

E BARRICADE

CHANGEABLE MESSAGE BOARD

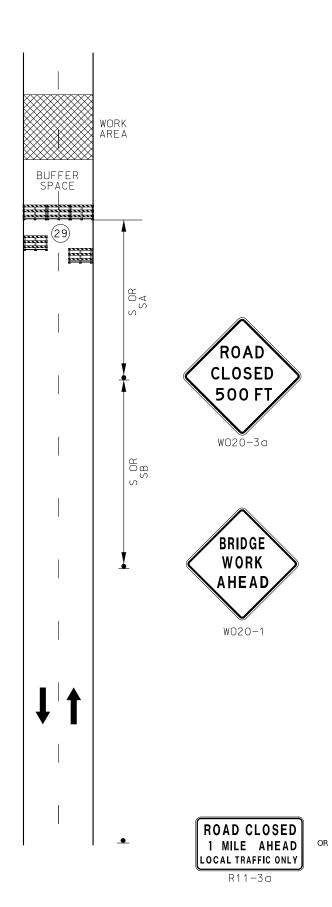
TRUCK MOUNTED ATTENUATOR

W/ FLASHING ARROW

THIIIIII.	PR PR	PE	PL NUN	LYN JHR MBEF 4000		2	THE PERSON		
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NOTES:

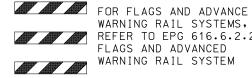
SEE TRAFFIC CONTROL SHEET 1 FOR WORK ZONE SPEED LIMIT GUIDELINES.

SEE TRAFFIC CONTROL SHEET 1 FOR SIGN SPACING, DEVICE SPACING AND CHANNELIZING TAPER LENGTHS.

WHEN A SIDE ROAD INTERSECTS THE HIGHWAY WITHIN THE TTC ZONE ADDITIONAL TTC DEVICES SHALL BE PLACED AS NEEDED.

"ROAD CLOSED" SIGN MAY BE PLACED 7-10 FEET BEHIND THE BARRICADES AND AT A SIGN HEIGHT APPROPRIATE TO THE TYPE OF ROADWAY, ONE BARRICADE SHOULD BE REQUIRED TO COMPLETELY CLOSE EACH 8-FEET OF PAVEMENT. PAVED SHOULDERS SHALL BE INCLUDED IN THE AREA TO BE CLOSED.

IF USED, THE "ROAD CLOSED XX MILES AHEAD LOCAL TRAFFIC ONLY" OR "ROAD CLOSED TO THRU TRAFFIC" SIGN SHOULD BE LOCATED AT THE FIRST STATE ROUTE OR, UPON THE DISCRETION OF THE ENGINEER, ANY OTHER INTERSECTION IN ADVANCE OF THE CLOSURE. ADDITIONAL BARRICADES MAY BE USED AND OFFSETTED TO FACILITATE ACCESS FOR WORK VEHICLES, LOCAL TRAFFIC, ETC.



WARNING RAIL SYSTEMS, REFER TO EPG 616.6.2.2 FLAGS AND ADVANCED WARNING RAIL SYSTEM



ROAD CLOSED

TΟ

THRU TRAFFIC

R11-4

HIGHWAY CLOSURE ON UNDIVIDED HIGHWAY TEMPORARY TRAFFIC CONTROL SHEET 3 OF 3

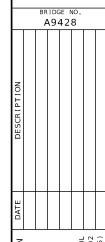
JODIE LYNN PUHR NUMBER PE-2014000073 THE SOLONAL EN Goduflihu

11/05/2024 10:33:13 AM JODIE LYNN PUHR - CIVIL MO-PE-2014000073 DATE PREPARED

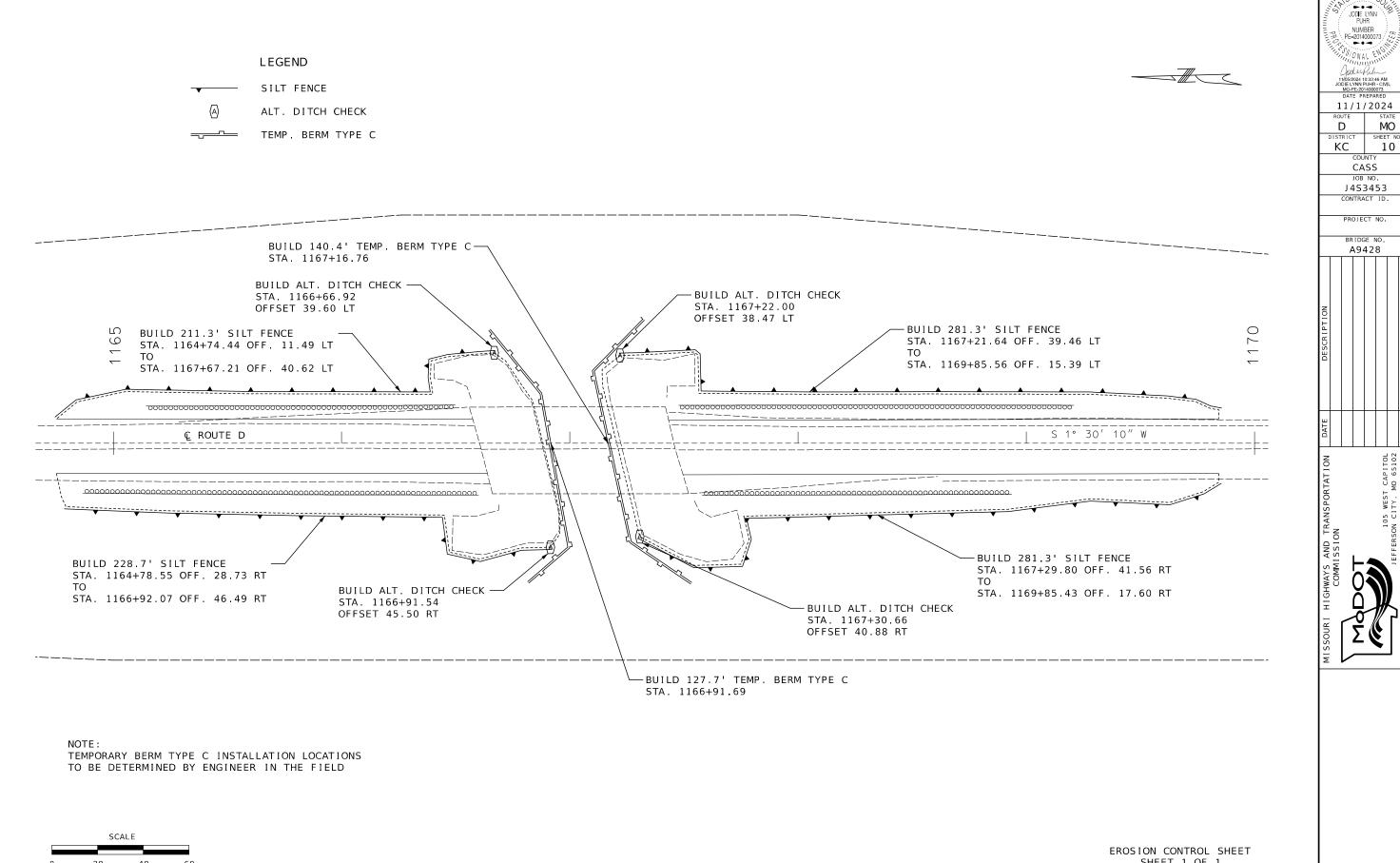
11/1/2024 D MO SHEET NO KC 9

CASS LOB NO J4S3453 CONTRACT ID.

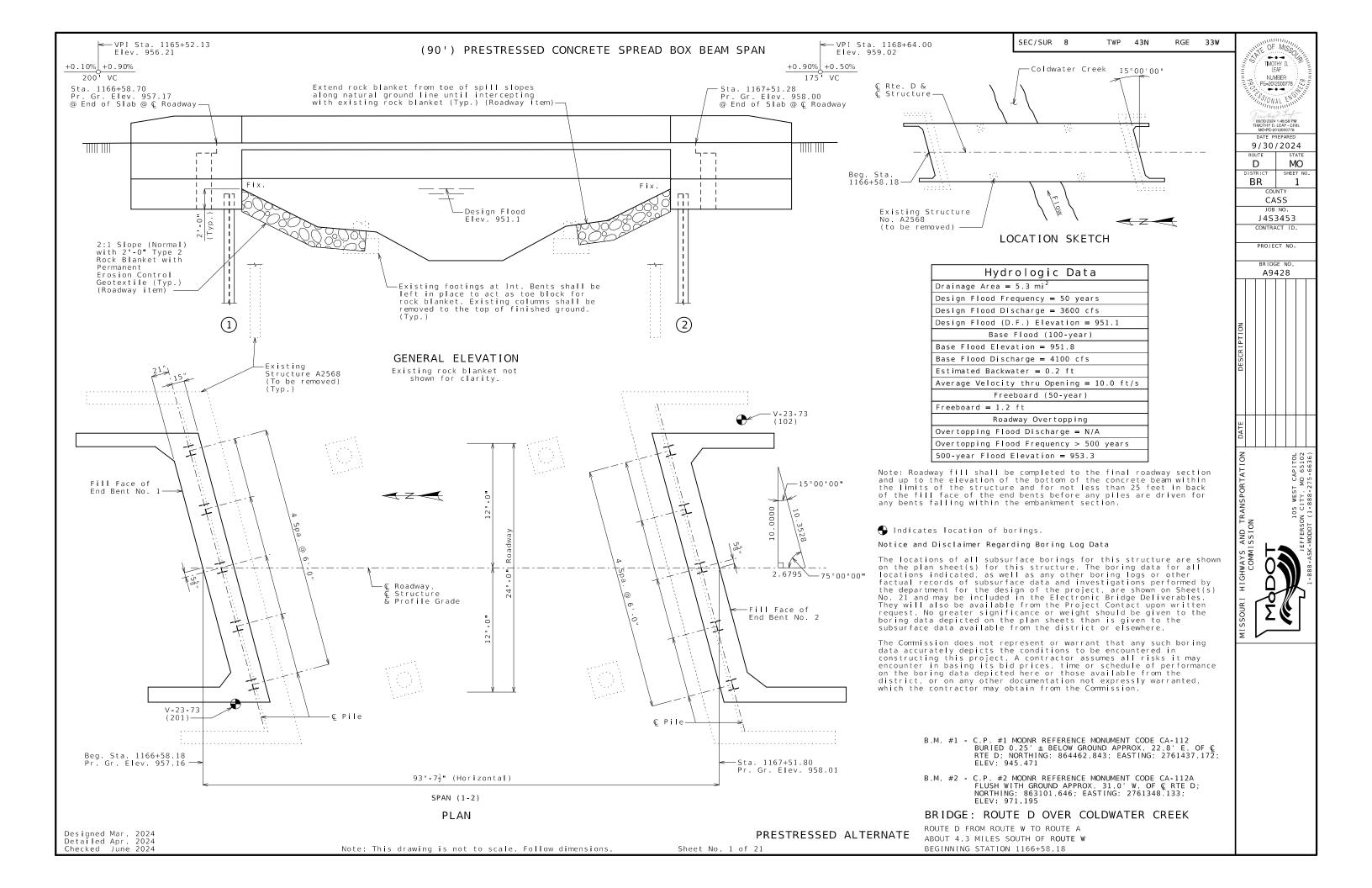
PROJECT NO.







SHEET 1 OF 1



General Notes:

Design Specifications:

2020 AASHTO LRFD Bridge Design Specifications (9th Ed.)

Seismic Design Category = A

Design Loading:

Vehicular = HL-93

Future Wearing Surface = 35 lb/sf
Earth = 120 lb/cf
Equivalent Fluid Pressure = 45 lb/cf (min.)

Superstructure: Non-Composite for dead load. Composite for live load.

Design Unit Stresses:

Class B Concrete (Substructure) f'c = 3,000 psi

Class B-2 Concrete (Superstructure, except Prestressed Beams and Barrier) f'c = 4.000 psif'c = 4.000 psiClass B-1 Concrete (Barrier)

Reinforcing Steel (ASTM A615 Grade 60) fy = 60,000 psi

Structural Steel HP Pile (ASTM A709 Grade 50) fy = 50,000 psi

For prestressed box beam stresses, see Sheet No. 10.

Neoprene Pads:

Neoprene bearing pads shall be 60 durometer and shall be in accordance with Sec 716.

Joint Filler:

All joint filler shall be in accordance with Sec 1057 for preformed sponge rubber expansion and partition joint filler, except as noted.

Reinforcing Steel:

Minimum clearance to reinforcing steel shall be 1 1/2", unless

Traffic Handling:

Structure to be closed during construction. Traffic to be maintained on other routes. See roadway plans for traffic control.

Outline of existing work is indicated by light dashed lines. Heavy lines indicate new work.

Estimated	Quantitie	e s		
I t em		Substr.	Superstr.	Total
Removal of Miscellaneous ACM (Non-Friable)	sq. foot			8
Class 1 Excavation	cu. yard	70		70
Removal of Bridges (A2568)	lump sum			1
Bridge Approach Slab (Minor)	sq. yard		109	109
Galvanized Structural Steel Piles (12 in.)	linear foot	170		170
Pre-Bore for Piling	linear foot	150		150
Pile Point Reinforcement	each	10		10
Class B Concrete (Substructure)	cu. yard	24.8		24.8
Type D Barrier	linear foot		223	223
Slab on Concrete Beam	sq. yard		274	274
39 in., Prestressed Concrete Spread Box Beam	linear foot		272	272
Slab Drain	each		12	12
Vertical Drain at End Bents	each			2
Plain Neoprene Bearing Pad	each		6	6

Foundation Data							
		Bent Number					
Type	Design Data		1	2			
	Pile Type and Size		HP12x53	HP12x53			
	Numb e r	ea	5	5			
	Approximate Length Per Each	ft	17	17			
	Pile Point Reinforcement	ea	ALL	ALL			
Bearing Pile	Min. Galvanized Penetration (Elev.)	ft	Full length	Full length			
1110	Pile Driving Verification Method		DF	DF			
	Resistance Factor		0.4	0.4			
	Minimum Nominal Axial Compressive Resistance	kip	506	506			

DF = FHWA-modified Gates Dynamic Pile Formula

Minimum Nominal Axial Compressive Resistance = Maximum Factored Loads

Prebore for piles at Bents No. 1 and 2 to elevations 934.6 and 935.4, respectively.

All piles shall be galvanized down to the minimum galvanized penetration Pile point reinforcement need not be galvanized. Shop drawings will not be

required for pile point reinforcement.

The contractor shall make every effort to achieve the minimum galvanized penetration (elevation) shown on the plans for all piles. Deviations in penetration less than 5 feet of the minimum will be considered acceptable provided the contractor makes the necessary corrections to ensure the minimum penetration is achieved on subsequent piles.

HP piles are anticipated to be driven to refusal on rock. Review all borings for depth of rock and restrict driving as appropriate to comply with hard rock driving criteria in accordance with Sec 702. When pile refusal on rock occurs, as approved by the engineer, the minimum nominal axial compressive resistance is verified and no additional pile driving verification method is required.

Estimated Quantities for Slab on Concrete Beam						
I t em						
Class B-2 Concrete cu. yard						
Reinforcing Steel (Galvanized) pound	26,530					

The table of Estimated Quantities for Slab on Concrete Beam represents the quantities used by the State in preparing the cost estimate for concrete slabs. The area of the concrete slab will estimate for concrete slabs. The area of the concrete slab will be measured to the nearest square yard longitudinally from end of slab to end of slab and transversely from out to out of bridge slab. Payment for conventional forms and all concrete and galvanized reinforcing steel will be considered completely covered by the contract unit price for the slab. Variations may be encountered in the estimated quantities but the variations cannot be used for an adjustment in the contract unit price.

Method of forming the slab shall be as shown on the plans and in accordance with Sec 703. All hardware for forming the slab to be left in place as a permanent part of the structure shall be coated in accordance with ASTM A123 or ASTM B633 with a thickness class SC 4 and a finish type I. II or III.

Slab shall be cast-in-place with conventional forms or stay-in-place corrugated steel forms. Precast prestressed panels will not be

All concrete above the construction joint in the end bents is included in the Estimated Quantities for Slab on Concrete Beam.

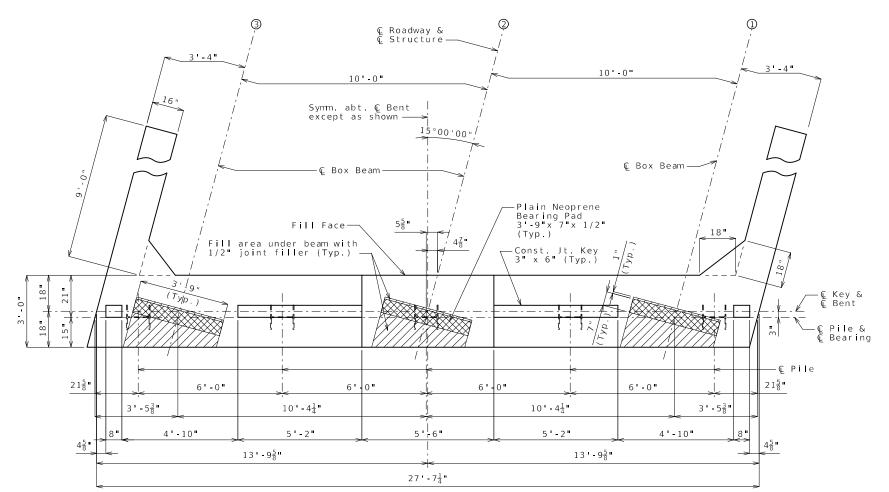
All reinforcement in the end bents is included in the Estimated Quantities for Slab on Concrete Beam.

Specification, materials, zinc coating process and construction practice shall be in accordance with ASTM A1094/A1094M-18.

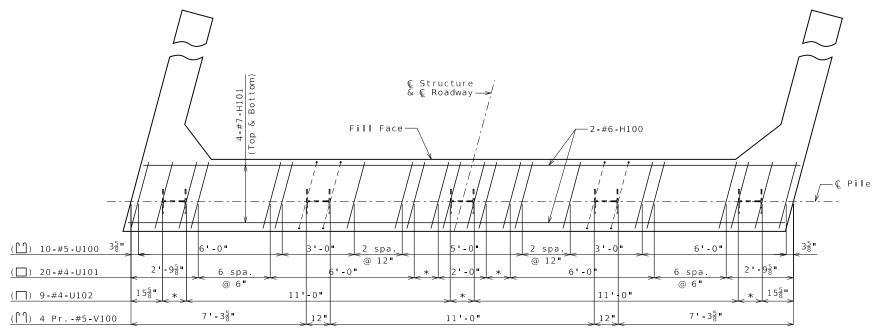
Galvanized reinforcing steel shall not come in contact with uncoated reinforcing steel or prestressing strands. Nylon, PVC, or Polyethylene spacers shall be used where necessary. Nylon cable ties shall be used to bind the spacers to the reinforcement.

TIMOTHY D. LEAF NUMBER PE-2012000778 SONAL 9/30/2024 D MO SHEET NO BR 2 CASS J4S3453 CONTRACT ID. PROJECT NO.

> BRIDGE NO A9428







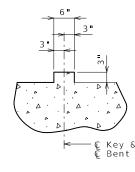
PLAN OF BEAM SHOWING REINFORCEMENT (Keys and beam steps not shown for clarity.)

* 2 spa. @ 6"

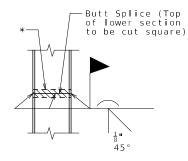
Detailed Apr. 2024 Checked June 2024

END BENT NO. 1

Note: This drawing is not to scale. Follow dimensions. Sheet No. 3 of 21



SECTION THRU KEY



STEEL PILE SPLICE

(If required)

* Galvanizing material shall be omitted or removed one inch clear of weld locations in accordance with Sec 702.

Notes:

Work this sheet with Sheets No. 4 & 5.

Reinforcing steel shall be shifted to clear piles. U bars shall clear piles by at least 1 1/2".

The U bars and pairs of V bars shall be placed parallel to centerline of Roadway.

Substructure Quantity Table	for Bent N	lo. 1
I t em		Quantity
Class 1 Excavation	cu. yard	35
Galvanized Structural Steel Piles (12 in.)	linear foot	85
Pre-Bore for Piling	linear foot	75
Pile Point Reinforcement	each	5
Class B Concrete (Substructure)	cu. yard	12.4

These quantities are included in the Estimated Quantities table on

NUMBER PE-2012000778

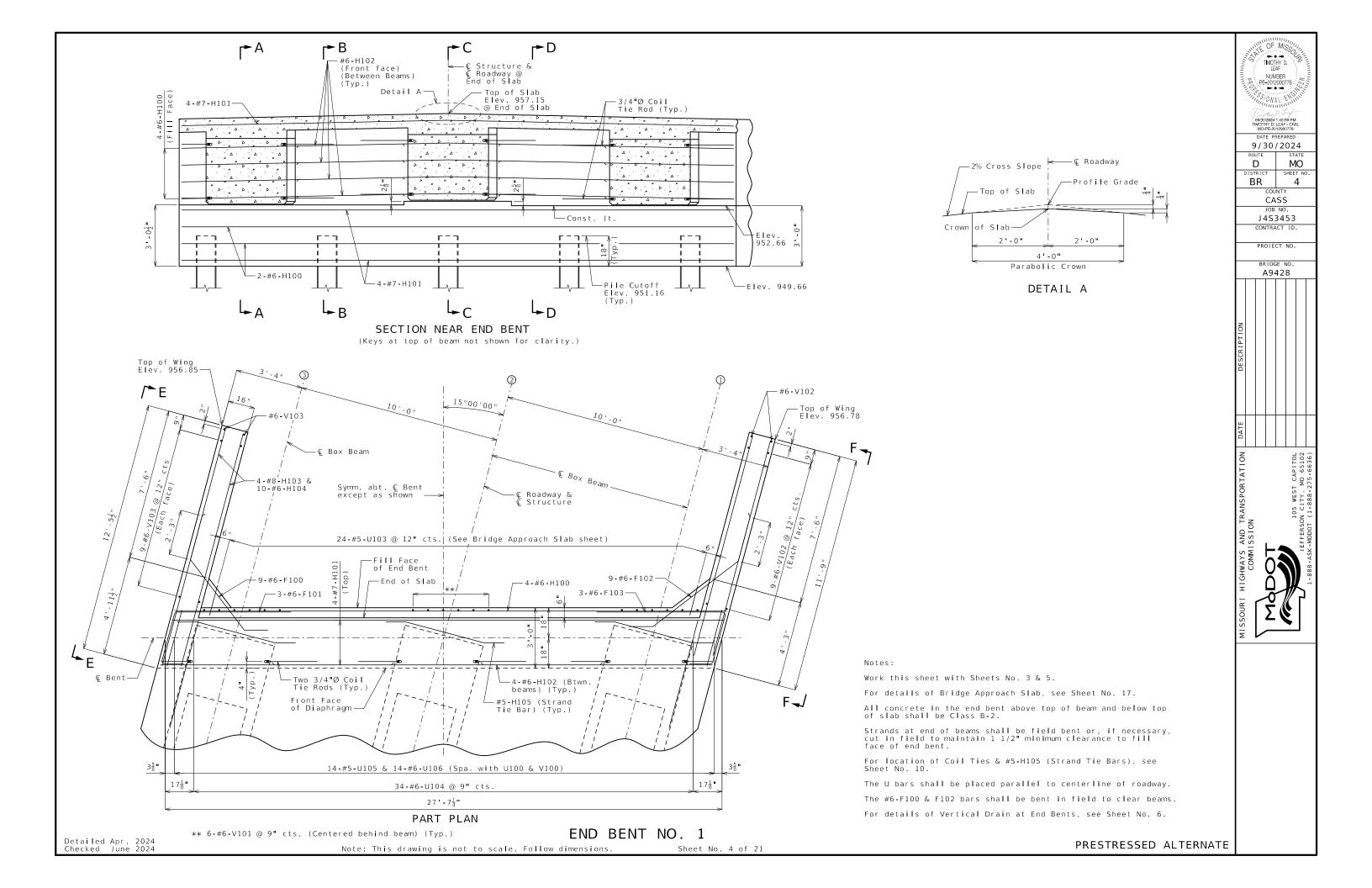
9/30/2024

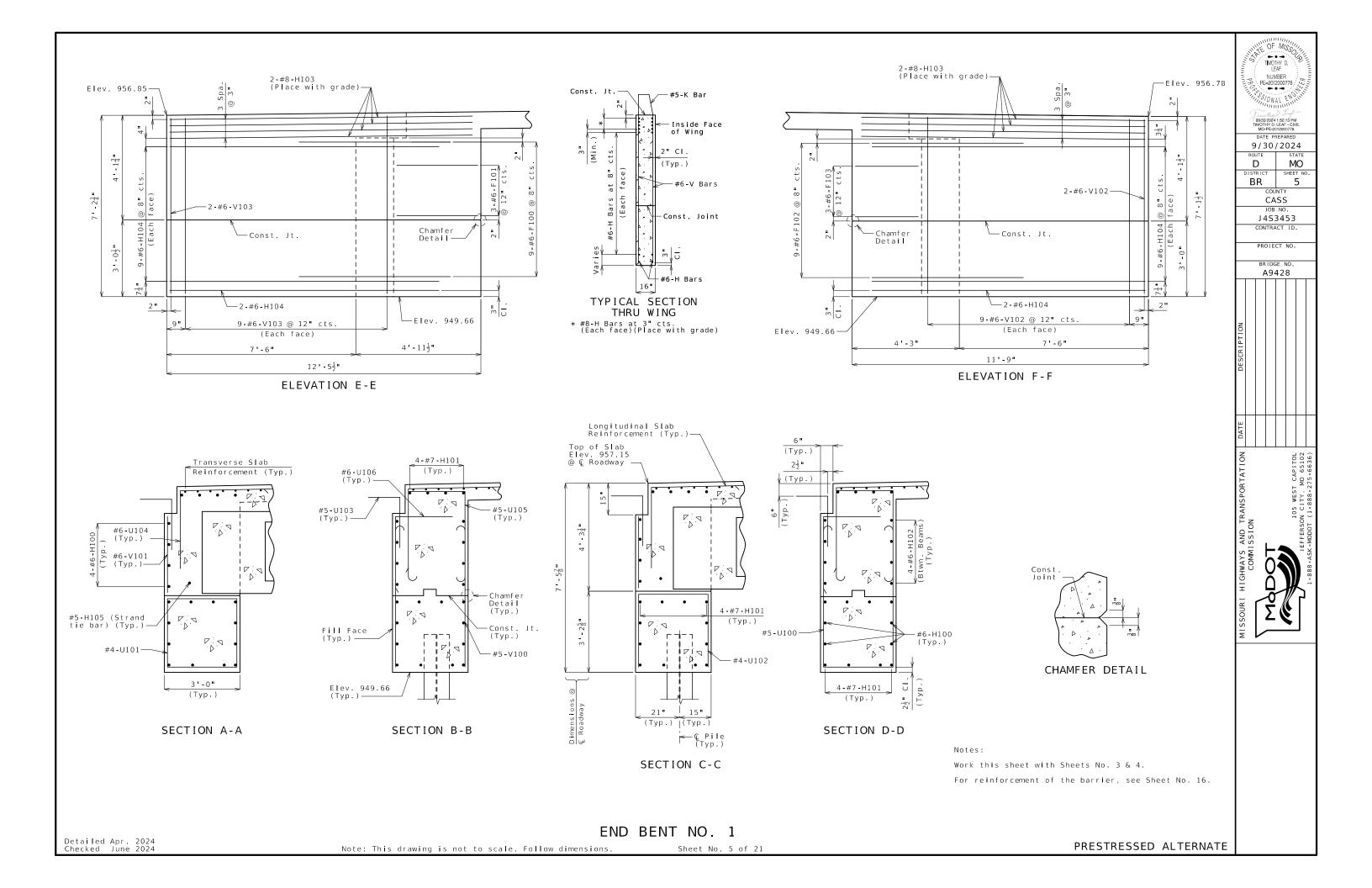
D MO SHEET NO BR 3

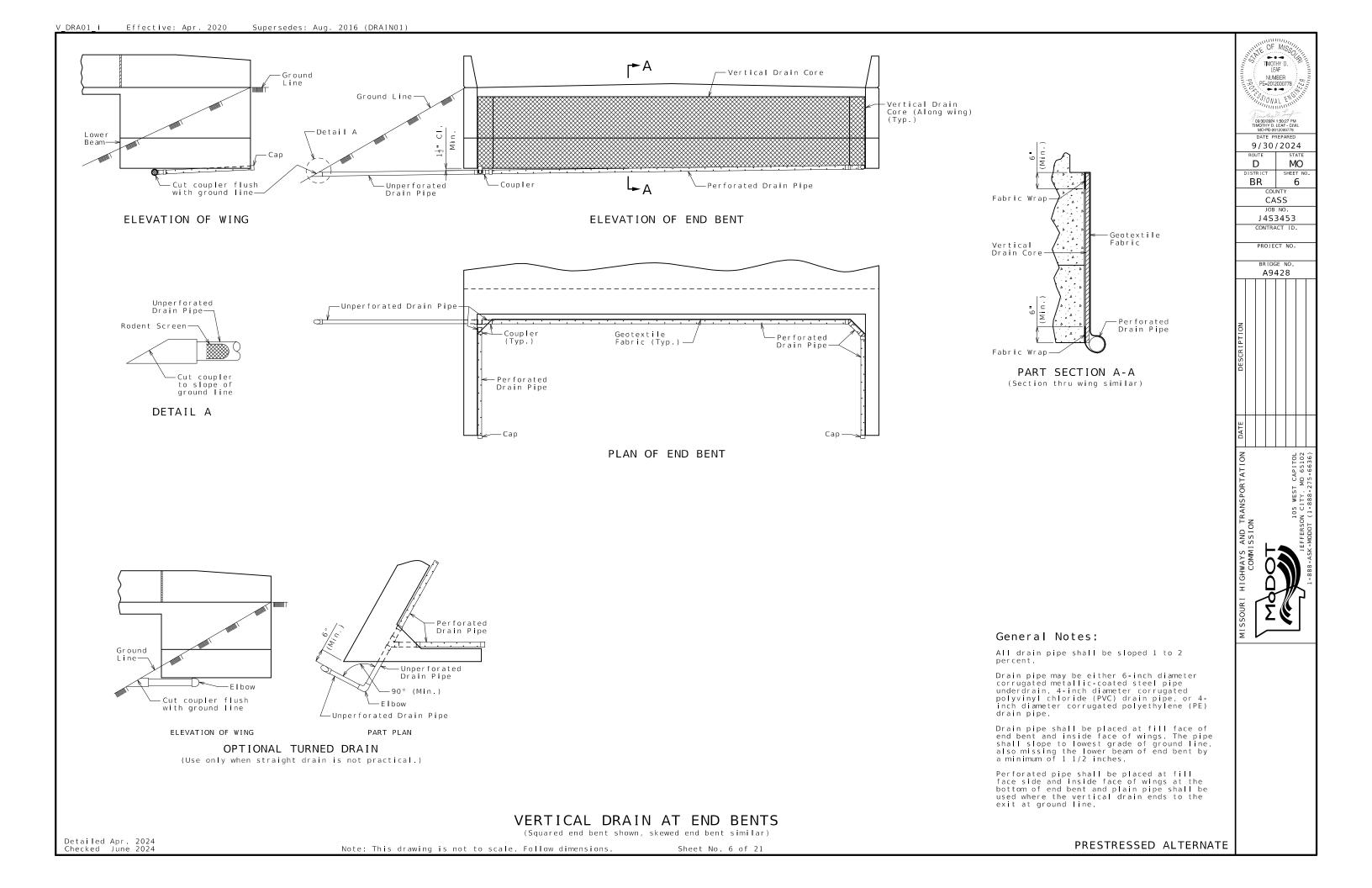
CASS IOB NO J4S3453 CONTRACT ID.

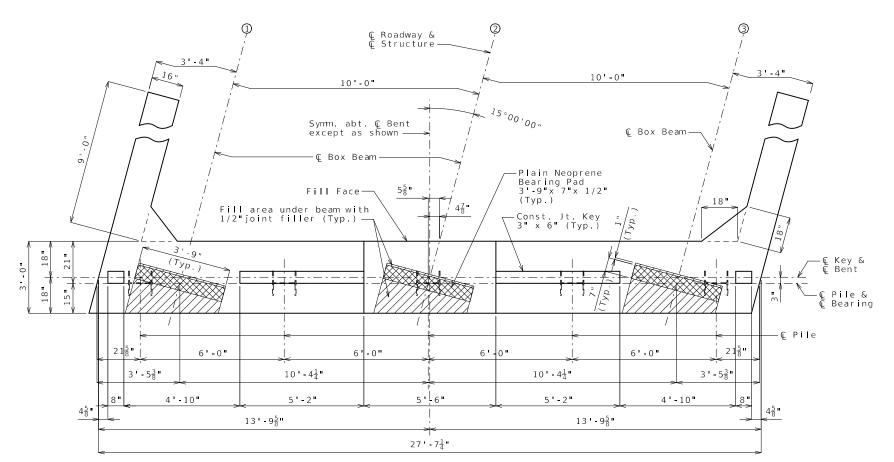
PROJECT NO.



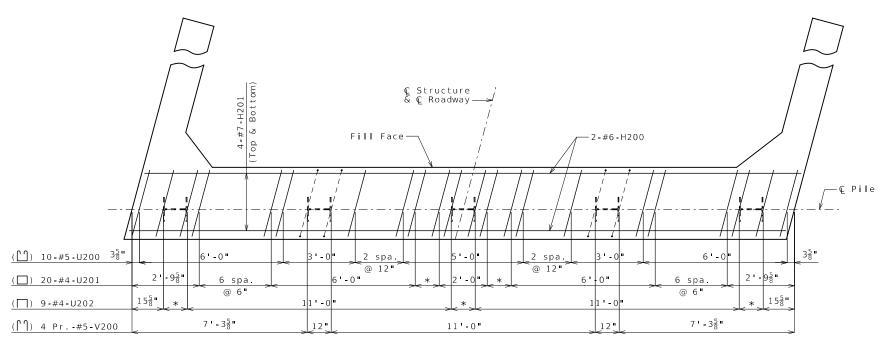








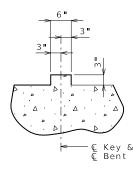




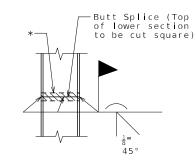
PLAN OF BEAM SHOWING REINFORCEMENT (Keys and beam steps not shown for clarity.)

* 2 spa. @ 6"

END BENT NO. 2



SECTION THRU KEY



STEEL PILE SPLICE

(If required)

* Galvanizing material shall be omitted or removed one inch clear of weld locations in accordance with Sec 702

Notes:

Work this sheet with Sheets No. 8 & 9.

Reinforcing steel shall be shifted to clear piles. U bars shall clear piles by at least 1 1/2".

The U bars and pairs of V bars shall be placed parallel to centerline of Roadway.

Substructure Quantity Table	for Bent N	o . 2
I t em		Quantity
Class 1 Excavation	cu. yard	35
Galvanized Structural Steel Piles (12 in.)	linear foot	85
Pre-Bore for Piling	linear foot	75
Pile Point Reinforcement	each	5
Class B Concrete (Substructure)	cu. yard	12.4

These quantities are included in the Estimated Quantities table on

TIMOTHY D NUMBER PE-2012000778

9/30/2024

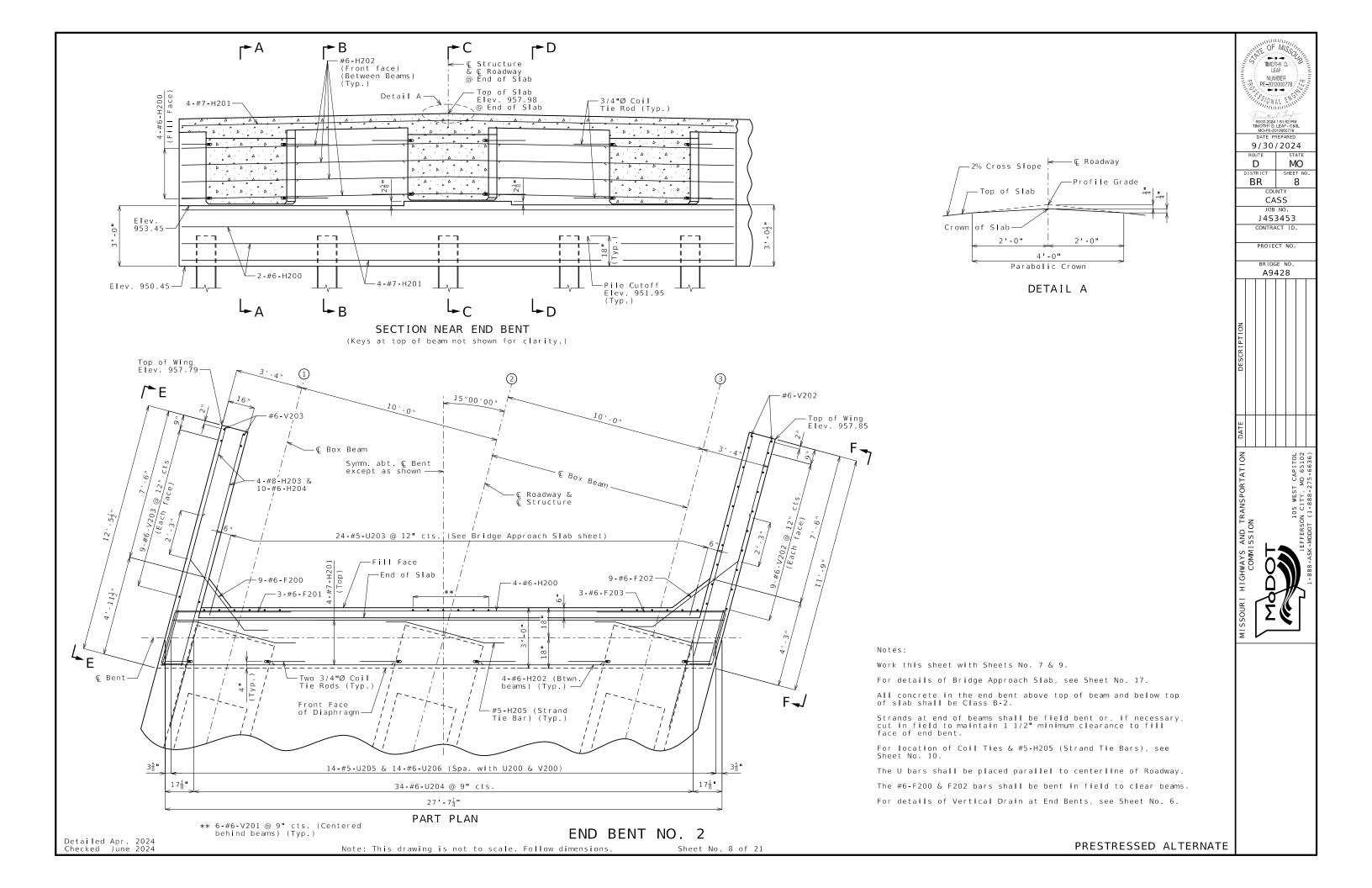
D MO SHEET NO BR 7

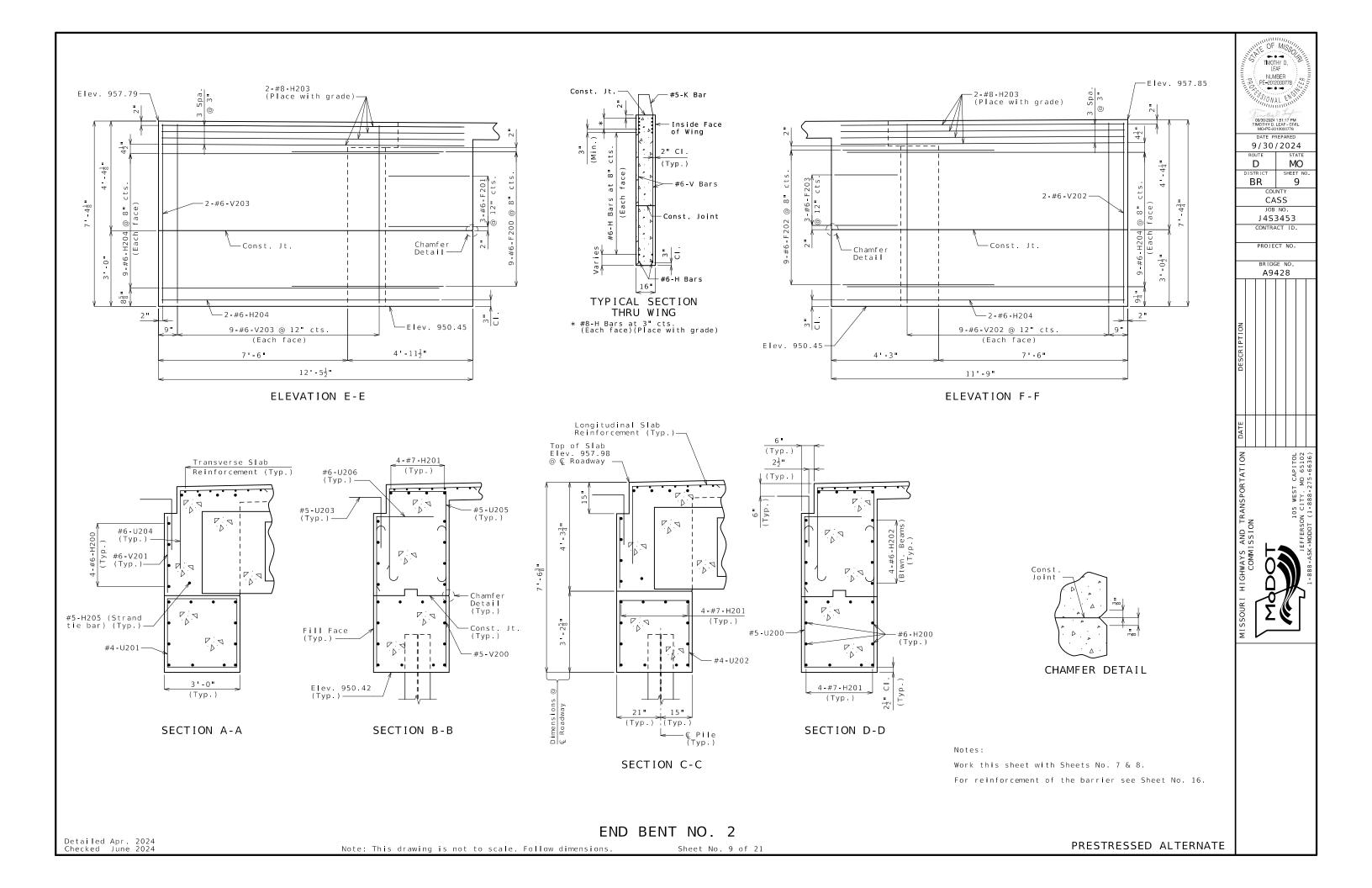
CASS IOB NO. J4S3453 CONTRACT ID.

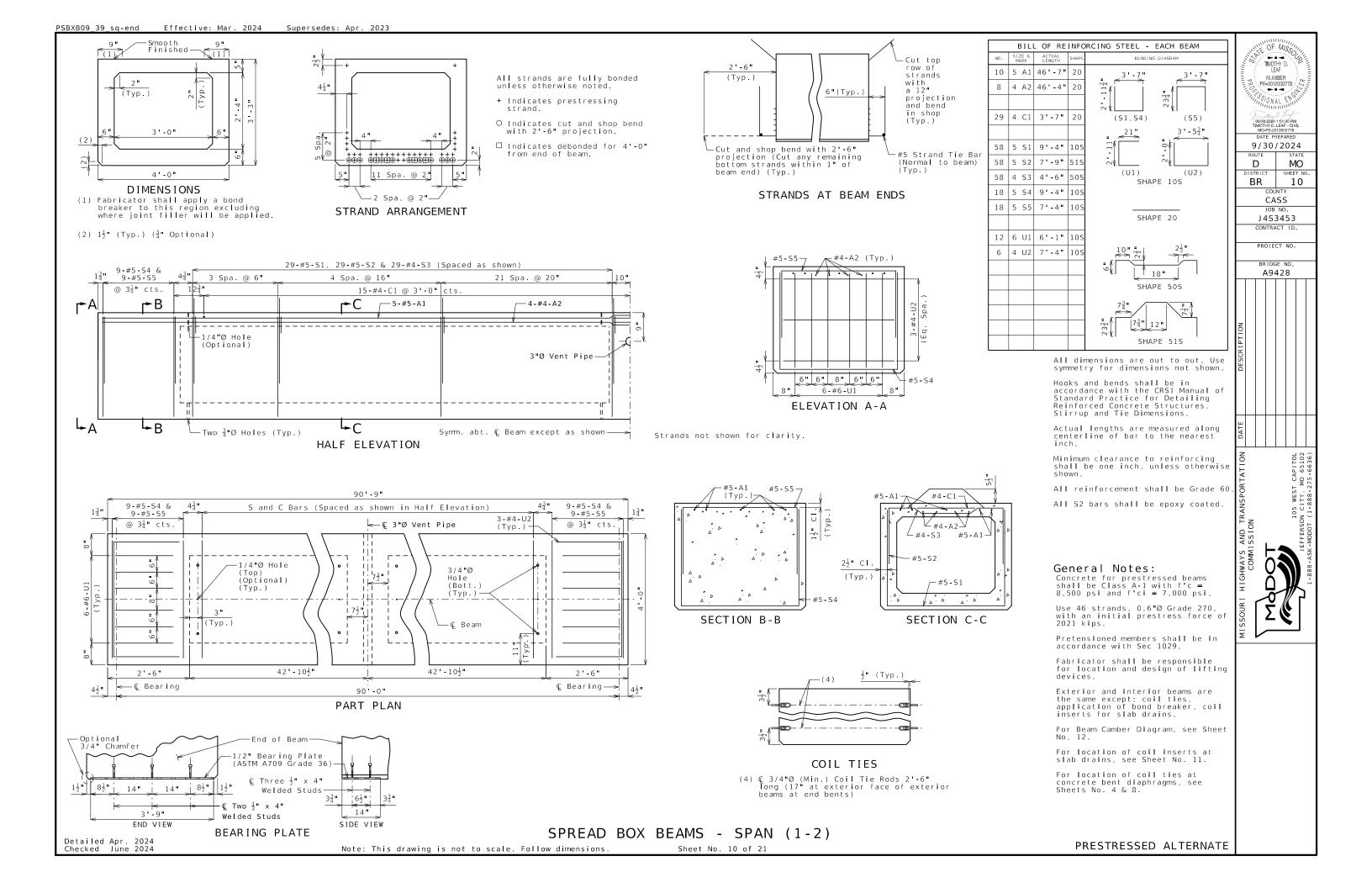
PROJECT NO.

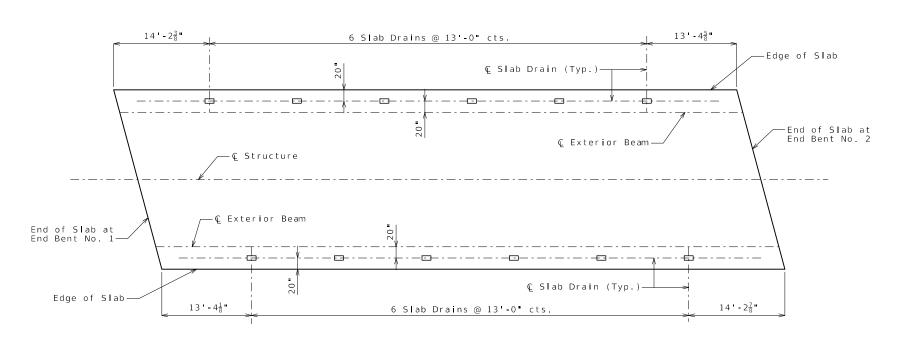
BRIDGE NO A9428

Detailed Apr. 2024 Checked June 2024

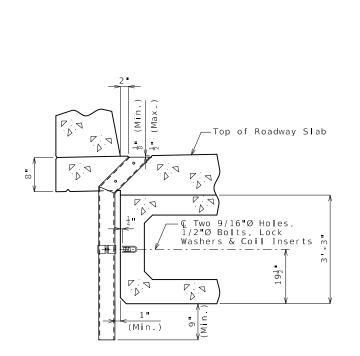








PLAN OF SLAB SHOWING SLAB DRAIN LOCATIONS

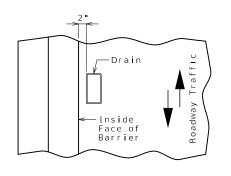


PART SECTION NEAR DRAIN

Detailed Apr. 2024

© 5/8"Ø Steel Spacer to maintain 1"(min.) clearance Ç 9/16"Ø Holes for 1/2"Ø bolt with lock to box beam (Typ.) washer and nut (Typ.) Prestressed Box Beam Coil Insert & 9/16"Ø Hole for 1/2"Ø bolt with lock washer (4) Bent Strip $(Min.) \times 2$

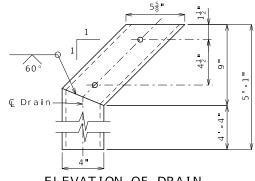
PART SECTION SHOWING BRACKET ASSEMBLY



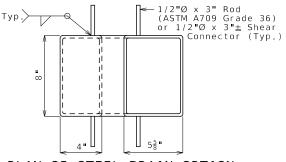
PART PLAN OF SLAB AT DRAIN

SLAB DRAINS

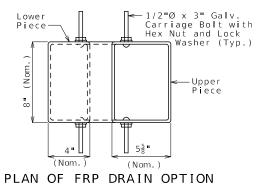




ELEVATION OF DRAIN



PLAN OF STEEL DRAIN OPTION



General Notes:

Contractor shall have the option to construct either steel or FRP slab drains. All drains shall be of same type.

Slab drain bracket assembly shall be ASTM A709 Grade 36 steel

Locate drains in slab by dimensions shown in Part Section Near Drain.

Reinforcing steel shall be shifted to

The coil inserts and bracket assembly shall be galvanized in accordance with ASTM A123.

All bolts, hardened washers, lock washers and nuts shall be galvanized in accordance with AASHTO M 232 (ASTM A153),

All 1/2 o bolts shall be ASTM A307.

Shop drawings will not be required for the slab drains and the bracket assembly.

The coil inserts required for the bracket assembly attachment shall be located on the prestressed beam shop drawings.

Coil inserts shall have a concrete pullout strength (ultimate load) of at least 2,500 pounds in 5,000 psi concrete.

The bolts required to attach the slab drain bracket assembly to the prestressed beam shall be supplied by the prestressed beam fabricator.

Notes for Steel Drain:

Slab drains may be fabricated of either 1/4' welded sheets of ASTM A709 Grade 36 steel or from 1/4' structural steel tubing ASTM A500 or A501.

Outside dimensions of drains are 8" x 4".

The drains shall be galvanized in accordance with ASTM A123.

Notes for FRP Drain:

Drains shall be machine filament-wound thermosetting resin tubing meeting the requirements of ASTM D2996 with the following exceptions:

Shape of drains shall be rectangular with outside nominal dimensions of $8\ x\ 4\ x$.

Minimum reinforced wall thickness shall be 1/4 inch.

The resin used shall be ultraviolet (UV) resistant and/or have UV inhibitors mixed throughout. Drains may have an exterior coating for additional UV resistance.

The color of the slab drain shall be gray (Federal Standard 26373). The color shall be uniform throughout the resin and any coating used.

The combination of materials used in the manufacture of the drains shall be tested for UV resistance in accordance with ASTM D4329 Cycle A. The representative material shall withstand at least 500 hours of testing with only minor discoloration and without attention of the strong testing with only minor discoloration. without any physical deterioration. The contractor shall furnish the results of the required ultraviolet testing prior to acceptance of the slab drains.

At the contractor's option, drains may be field cut. The method of cutting FRP slab drain shall be as recommended by the manufacturer to ensure a smooth, chip free

Both upper and lower drain pieces shall be rigidly connected to each other. Drain flow shall not be obstructed. Approval of the engineer is required.

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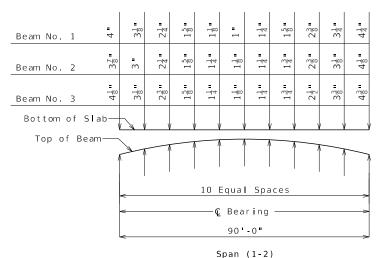
BR 11 CASS LOB NO J4S3453

PROJECT NO.

CONTRACT ID.

BRIDGE NO A9428

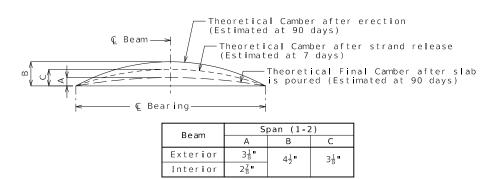
PRESTRESSED ALTERNATE



THEORETICAL SLAB HAUNCHING DIAGRAM (ESTIMATED AT 90 DAYS)

If beam camber is different from that shown in the camber diagram, in order to maintain minimum slab thickness, an adjustment of the slab haunches, an increase in slab thickness or a raise in grade uniformly throughout the structure shall be necessary. No payment will be made for additional labor or materials required for variation in haunching, slab thickness or grade adjustment.

Concrete in the slab haunches is included in the Estimated Quantities for Slab on Concrete Beam.

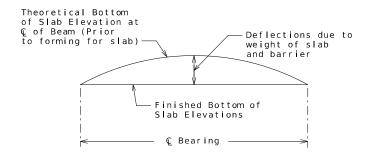


BEAM CAMBER DIAGRAM

Conversion Factor for Beam Camber (Estimated at 90 days):

```
0.1 pt. = 0.314 x 0.5 pt.
0.2 pt. = 0.593 x 0.5 pt.
0.3 pt. = 0.813 x 0.5 pt.
0.4 pt. = 0.952 x 0.5 pt.
```


Elevations are based on a constant slab thickness of 8" and include allowance for theoretical dead load deflections due to weight of slab and barrier.



TYPICAL SLAB ELEVATIONS DIAGRAM



DATE PREPARED

9/30/2024

ROUTE STATE

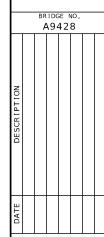
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DISTRICT SHEET NO

BR 12

COUNTY
CASS
JOB NO.
J4S3453
CONTRACT ID.

PROJECT NO.

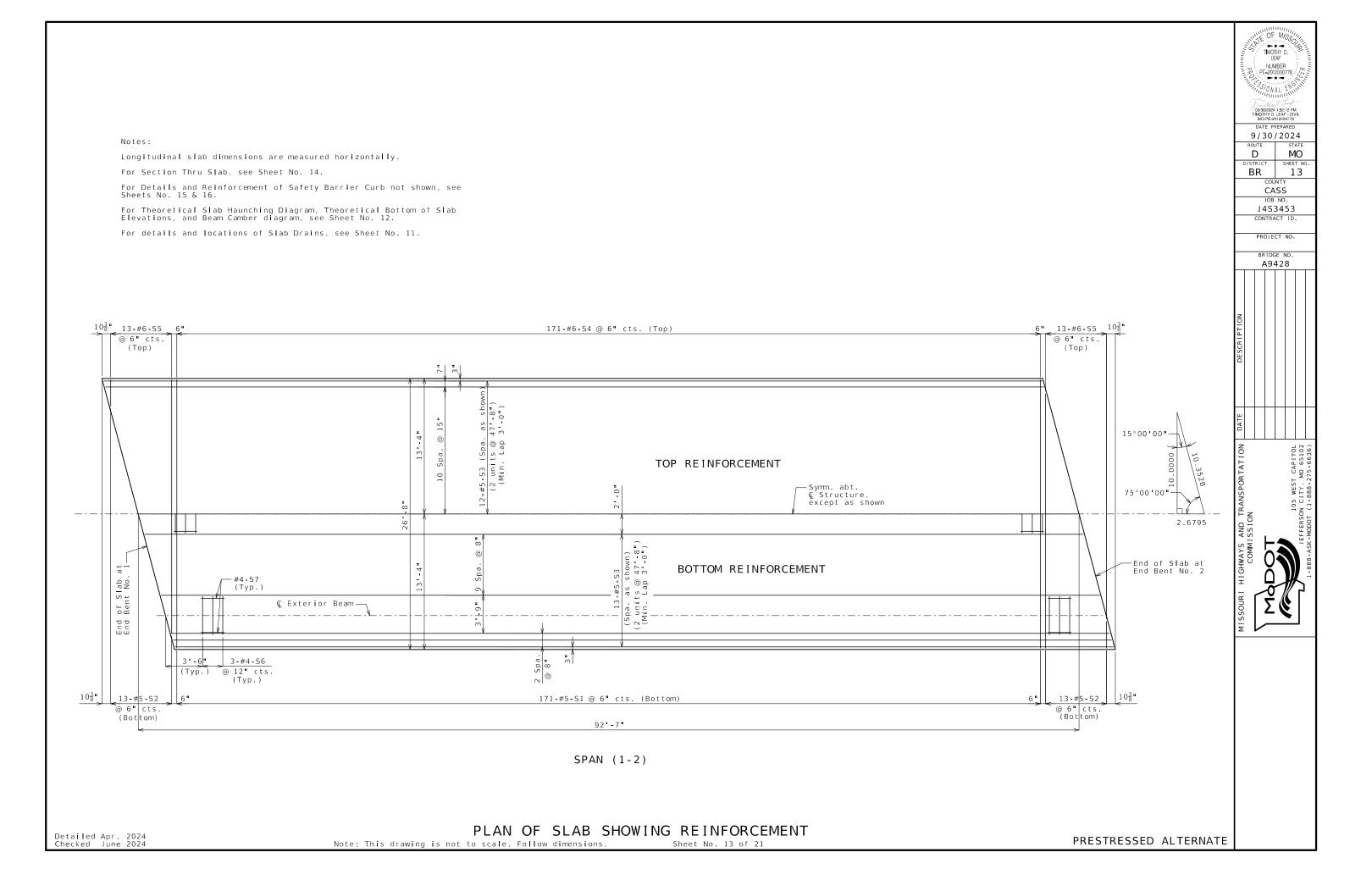


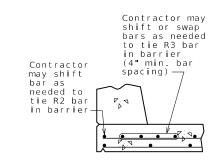
SSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION

MODOT

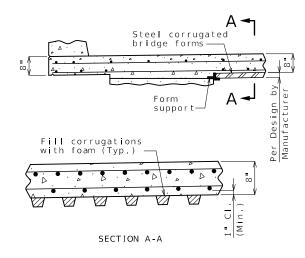
105 WEST CAPITOL
LIFFERSON CITY, MO 65102

PRESTRESSED ALTERNATE





OPTIONAL SHIFTING
TOP BARS AT BARRIER



OPTIONAL STAY-IN-PLACE FORM DETAILS

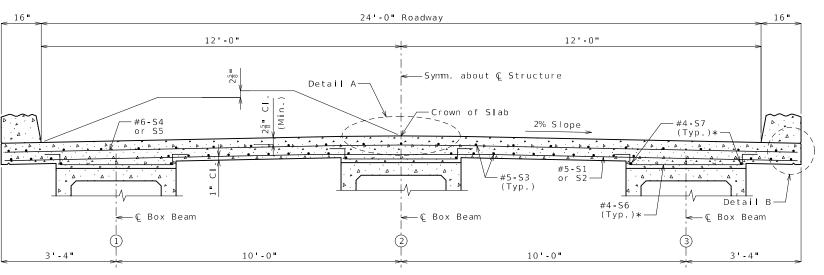
Stay-In-Place Forms:

Corrugated steel forms, supports, closure elements and accessories shall be in accordance with grade requirement and coating designation G165 of ASTM A653. Complete shop drawings of the permanent steel deck forms shall be required in accordance with Sec 1080.

Corrugations of stay-in-place forms shall be filled with an expanded polystyrene material. The polystyrene material shall be placed in the forms with an adhesive in accordance with the manufacturer's recommendations.

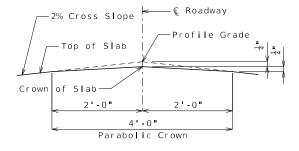
Form sheets shall not rest directly on the top of beam. Sheets shall be securely fastened to form supports with a minimum bearing length of one inch on each end. Form supports shall be placed in direct contact with the top of beam. Drilling holes in the beam will not be permitted. All steel fabrication and construction shall be in accordance with Sec 1080 and 712. Certified field welders will not be required for welding of the form supports.

The design of stay-in-place corrugated steel forms is per manufacturer which shall be in accordance with Sec 703 for false work and forms. Maximum actual weight of corrugated steel forms allowed shall be 4 psf assumed for beam loading.

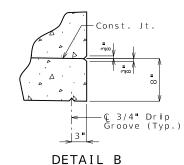


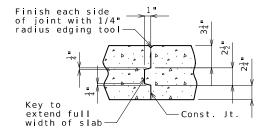
SECTION THRU SLAB

* Near end bents, where haunch is 4 inches or greater



DETAIL A





SLAB CONSTRUCTION JOINT

Notes:

For reinforcement of barrier not shown, see Sheets No. 15 & 16.

The contractor shall furnish an approved retarder to retard the set of the concrete to 2.5 hours and shall pour and satisfactorily finish the slab pours at not less than 25 cubic yards per hour.

The concrete diaphragm at the integral end bents shall be poured a minimum of 30 minutes and a maximum of 2 hours before the slab is poured.

For Plan of Slab Showing Reinforcement, see Sheet No. 13.

SLAB DETAILS

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J4S3453

CONTRACT ID.

PROJECT NO.

BRIDGE NO

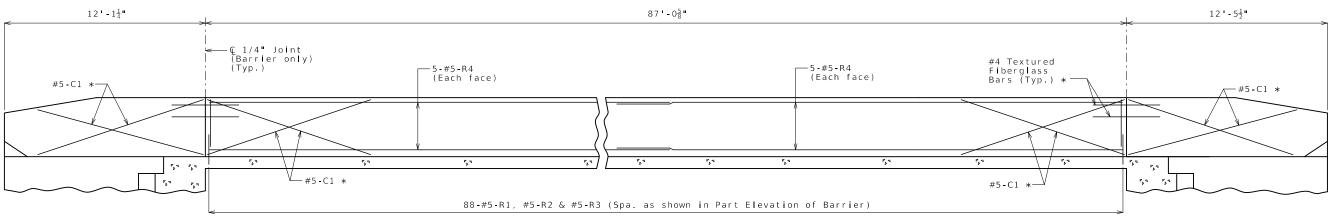
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SHEET NO

14

D

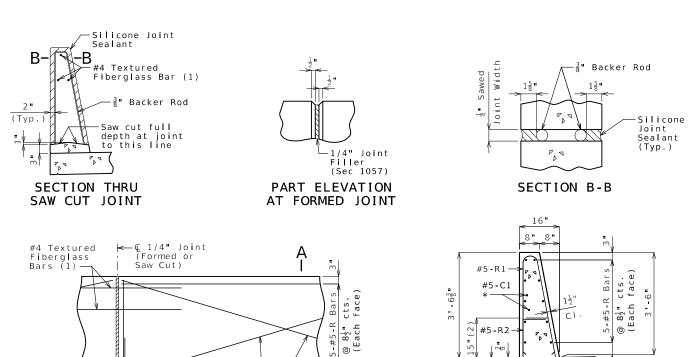
BR



(Span 1-2)

ELEVATION OF BARRIER

(Left barrier shown, right barrier similar) Longitudinal dimensions are horizontal.



PART ELEVATION OF BARRIER

P 4

DV

#5-C1 *-

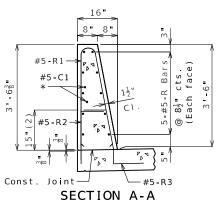
#5-R1, R2 and R3 @ abt. 12" cts.

D . A

(1) Four feet long, centered on joint, slip-formed option only

Detailed Apr. 2024

Checked June 2024



Use a minimum lap of 3 -1" for #5 horizontal barrier bars.

The cross-sectional area above the slab is 3.52 square feet.

(2) To top of bar

Bar (3) - #5 - R Bar (3) Joint #5-R2

R-BAR PERMISSIBLE ALTERNATE SHAPE

(3) The R1 bar may be separated into two bars as shown, at the contractor's option, only when slip forming is not used. (All dimensions are out to out.)

General Notes:

* Slip-formed option only.

Conventional forming or slip forming may be used. Saw cut joints may be used with conventional forming.

Top of barrier shall be built parallel to grade.

All exposed edges of barrier shall have either a 1/2-inch radius or a 3/8-inch bevel, unless otherwise noted.

Payment for all concrete and reinforcement, complete in place, will be considered completely covered by the contract unit price for Type D Barrier per linear foot.

Concrete in barrier shall be Class B-1.

Measurement of barrier is to the nearest linear foot for each structure, measured along the outside top of slab from end of

Concrete traffic barrier delineators shall be placed on top of the barrier as shown on Missouri Standard Plan 617.10 and in accordance with Sec 617.
Delineators on bridges with two-lane, twoway traffic shall have retroreflective sheeting on both sides. Concrete traffic barrier delineators will be considered completely covered by the contract unit price for Type D Barrier.

Joint sealant and backer rods shall be in accordance with Sec 717 for silicone joint sealant for saw cut and formed

For slip-formed option, both sides of barrier shall have a vertically broomed finish and the top shall have a transversely broomed finish.

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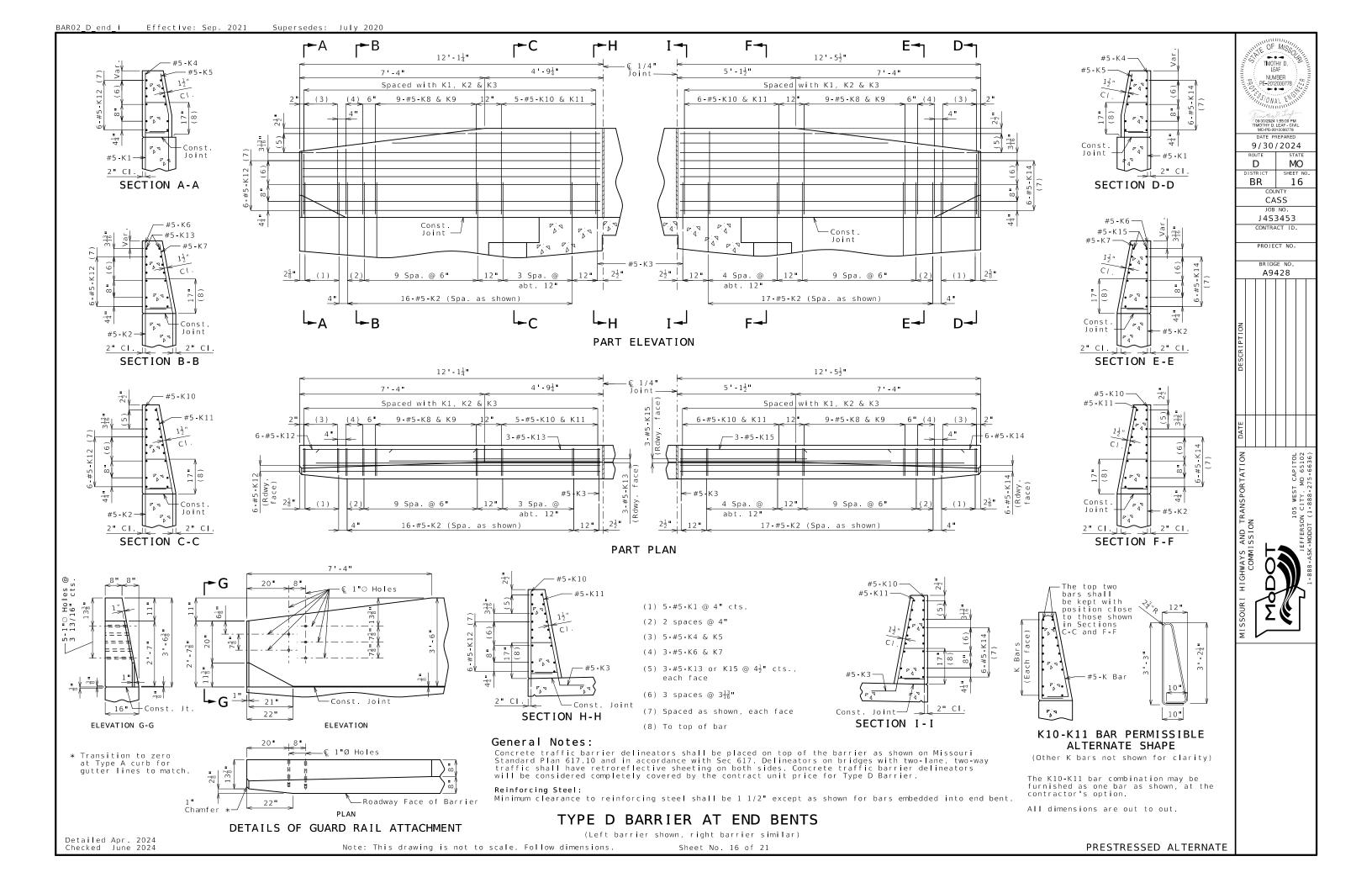
9/30/2024 D MO SHEET NO BR 15

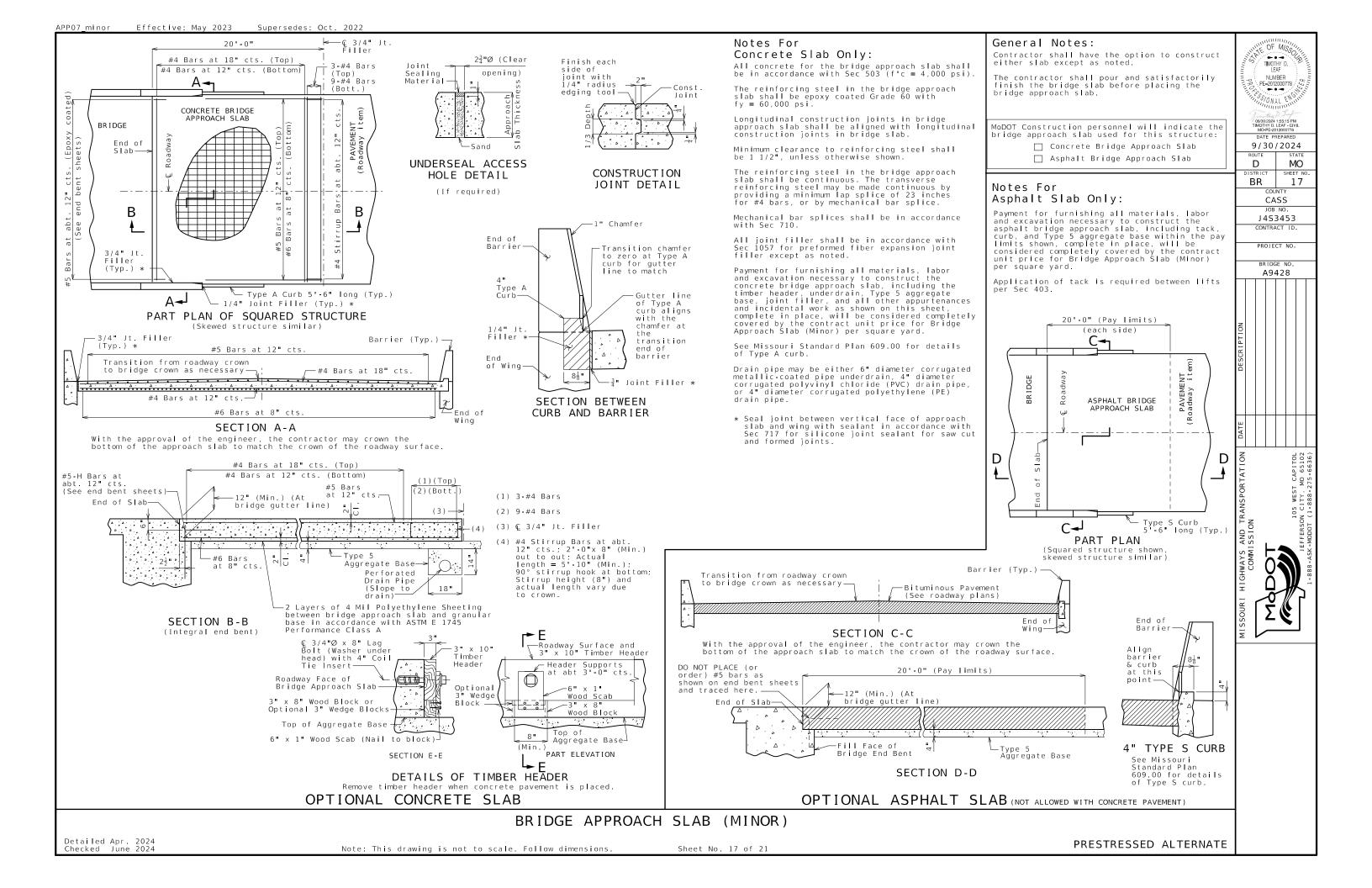
CASS LOB NO J4S3453 CONTRACT ID.

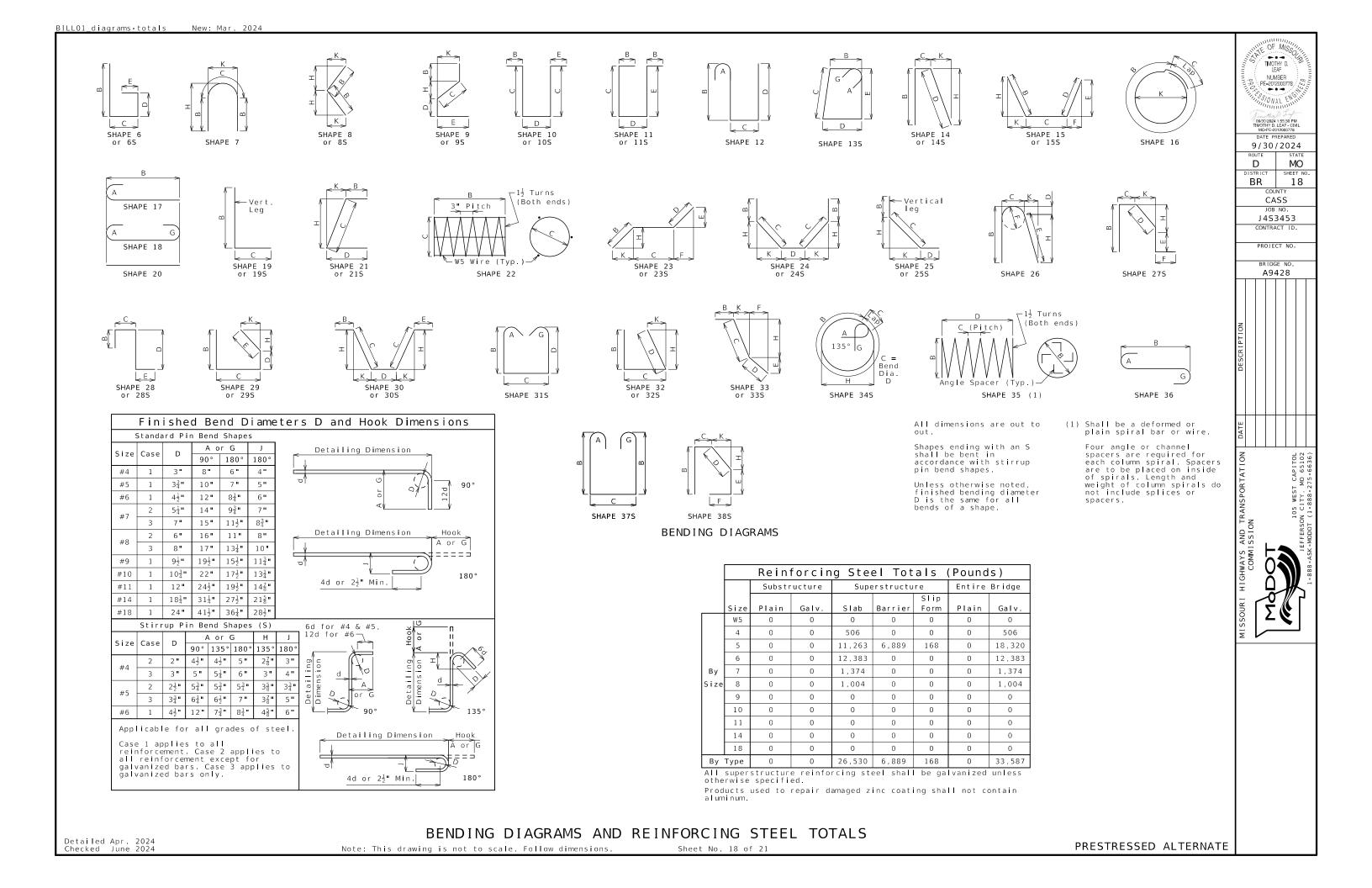
PROJECT NO.

BRIDGE NO A9428

TYPE D BARRIER Sheet No. 15 of 21







BILLO2 data New: Mar. 2024

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Nominal lengths are based on out to out dimensions shown in bending diagrams and are listed to the nearest inch for fabricator's use. Actual lengths are measured along centerline bar to the nearest inch. Weights are based on actual lengths.

All bars shall be Grade 60.

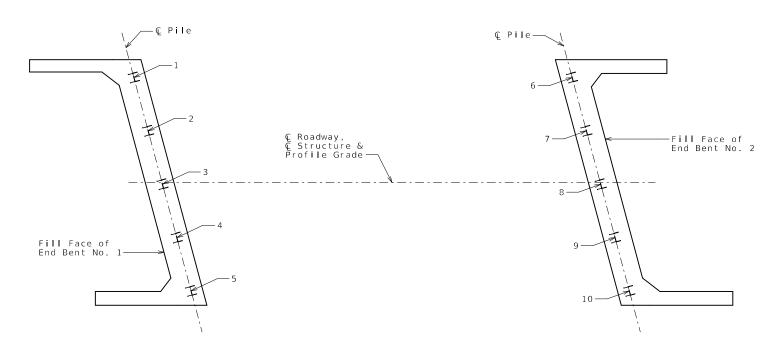
Codes: C = Required coatings, where E = Epoxy Coated and <math>G = Galvanized.

SH = Required shape, see bending diagrams.

V = Sets of varied bars and number of bars of each length. Bar dimensions vary in equal increments between dimensions shown on this line and the following line and the actual length dimension shown on this line and the following line vary by the specified increment.

PRESTRESSED
ALTERNATE

For	bending	diagrams	and	steel	reinforcing	totals,	see	Sheet	No.	18.	



PART PLAN SHOWING PILE NUMBERING FOR RECORDING AS-BUILT PILE DATA

			As-Built Pile Data
Pile No.	Length in Place (ft)	Compressive	Remarks
			End Bent No. 1
1			
2			
3			
4			
5			
			End Bent No. 2
6			
7			
8			
9			
10			

Note: Indicate in remarks column: A. Pile type and grade B. Batter C. Driven to practical refusal

This sheet to be completed by MoDOT construction personnel.

TIMOTHY D. LEAF NUMBER PE-2012000778 9/30/2024 ROUTE D MO SHEET NO. BR CASS JOB NO.
J4S3453
CONTRACT ID. PROJECT NO. BRIDGE NO A9428

AS-BUILT PILE DATA

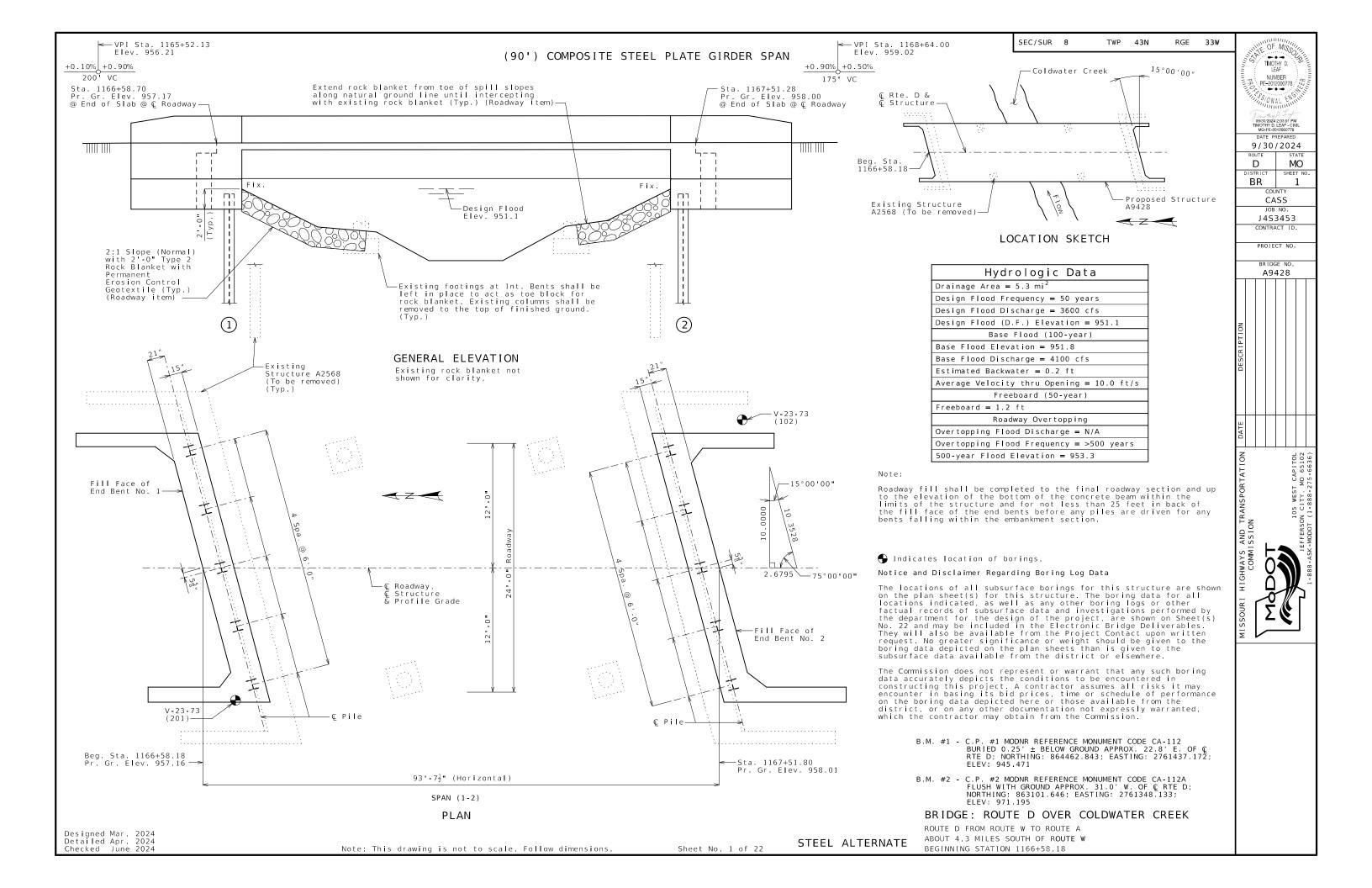
Detailed Apr. 2024 Checked June 2024

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 20 of 21

PRESTRESSED ALTERNATE

BOR01 BoringLogTemplate Effective: Apr. 2021 Supersedes: Aug. 2018 TIMOTHY D. LEAF NUMBER PE-2012000778 SUBSURFACE DIAGRAM | USCS Poorly-graded Gravel USCS Low Plasticity Gravelly Clay USCS Low Plasticity Clay Missouri Dept of Transportation Highly Weathered Limestone Limestone PROJECT NAME Bridge Replacement Shale PROJECT LOCATION 1.0 Mile South of Lisleca CLIENT PROJECT NUMBER J4S3453 9/30/2024 D MO SHEET NO BR 21 V-23-73 (102) 1167+55.45 15.2 L ELEV.957.50 V-23-73 (201) 1166+61.6 12.8 R ELEV.956.70 CASS IOB NO. J4S3453 PP N_m or (tsf) [RQD] CONTRACT ID. PROJECT NO. 955 955 BRIDGE NO A9428 1.75 14 1.0 6 950 950 1.0 9 2.0 16 Elevation (ft) 945 945 1.75 12 [72] [56] 940 940 922 892 [80] 935 935 2510 152 [66] [82] 2490 930 930 36.6 [92] [90] 925 925 ☐ ☐ ☐ 1140 ☐ 1430 Distance Along Baseline (ft) BORING DATA Note: For locations of borings, see Sheet No. 1. Detailed Apr. 2024 Checked June 2024 PRESTRESSED ALTERNATE Note: This drawing is not to scale. Follow dimensions. Sheet No. 21 of 21



General Notes:

Design Specifications:

2020 AASHTO LRFD Bridge Design Specifications (9th Ed.) Seismic Design Category = A

Design Loading:

Vehicular = HL-93 Future Wearing Surface = 35 lb/sf Earth = 120 lb/cf Equivalent Fluid Pressure = 45 lb/cf (min.)

Design Unit Stresses:

Class B Concrete (Substructure) f'c = 3,000 psiClass B-2 Concrete (Superstructure, f'c = 4,000 psiClass B-1 Concrete (Barrier) f'c = 4,000 psiReinforcing Steel (ASTM A615 Grade 60) fy = 60,000 psiStructural Steel (ASTM A709 Grade 50) fy = 50,000 psiStructural Steel HP Pile (ASTM A709 Grade 50) fv = 50.000 psi

Neoprene Pads:

Neoprene bearing pads shall be 60 durometer and shall be in accordance

Fabricated Steel Connections:

Field connections shall be made with 3/4-inch diameter ASTM F3125 Grade A325 Type 1 bolts and 13/16-inch diameter holes, except as noted.

All joint filler shall be in accordance with Sec 1057 for preformed sponge rubber expansion and partition joint filler, except as noted.

Reinforcing Steel:

Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise shown.

Structure to be closed during construction. Traffic to be maintained on other routes. See roadway plans for traffic control.

Outline of existing work is indicated by light dashed lines. Heavy lines indicate new work.

Estimated Quantiti	es			
I t em		Substr.	Superstr.	Total
Removal of Miscellaneous ACM (Non-Friable)	sq. foot			8
Class 1 Excavation	cu. yard	70		70
Removal of Bridges (A2568)	lump sum			1
Bridge Approach Slab (Minor)	sq. yard		109	109
Galvanized Structural Steel Piles (12 in.)	linear foot	170		170
Pre-Bore for Piling	linear foot	150		150
Pile Point Reinforcement	each	10		10
Class B Concrete (Substructure)	cu. yard	24.6		24.6
Slab on Steel	sq. yard		274	274
Type D Barrier	linear foot		223	223
Fabricated Structural Low Alloy Steel (Plate Girder) A709, Grade 50	pound		49,150	49,150
Slab Drain	each		8	8
Galvanizing Structural Steel	lump sum			1
Vertical Drain at End Bents	each			2
Laminated Neoprene Bearing Pad	each		6	6

	Foundation Da	ta			
			Bent 1	Number	
Type	Design Data		1	2	
	Pile Type and Size		HP12x53	HP12x53	
	Numb e r	еa	5	5	
	Approximate Length Per Each	ft	17		
Load	Pile Point Reinforcement	ea	a AII AII		
Bearing Pile	Min. Galvanized Penetration (Elev.)	ft	Full length	Full length	
1110	Pile Driving Verification Method		DF	DF	
	Resistance Factor		0.4	0.4	
	Minimum Nominal Axial Compressive Resistance	kip	506	506	

DF = FHWA-modified Gates Dynamic Pile Formula

Minimum Nominal Axial Compressive Resistance = Maximum Factored Loads
Resistance Factor

Prebore for piles at Bents No. 1 and 2 to elevations 934.6 and 935.4, respectively

All piles shall be galvanized down to the minimum galvanized penetration elevation.

Pile point reinforcement need not be galvanized. Shop drawings will not be required for pile point reinforcement.

The contractor shall make every effort to achieve the minimum galvanized penetration (elevation) shown on the plans for all piles. Deviations in penetration less than 5 feet of the minimum will be considered acceptable provided the contractor makes the necessary corrections to ensure the minimum penetration is achieved on subsequent piles.

HP piles are anticipated to be driven to refusal on rock. Review all borings for depth of rock and restrict driving as appropriate to comply with hard rock driving criteria in accordance with Sec 702. When pile refusal on rock occurs, as approved by the engineer, the minimum nominal axial compressive resistance is verified and no additional pile driving verification method is required.

Estimated Quantities for Slab on Steel	
I t em	Total
Class B-2 Concrete cu. yard	93
Reinforcing Steel (Galvanized) pound	28,060

The table of Estimated Quantities for Slab on Steel represents The table of Estimated Quantities for Slab on Steel represents the quantities used by the State in preparing the cost estimate for concrete slabs. The area of the concrete slab will be measured to the nearest square yard longitudinally from end of slab to end of slab and transversely from out to out of bridge slab. Payment for conventional forms and all concrete and galvanized reinforcing steel will be considered completely covered by the contract unit price for the slab. Variations may be encountered in the estimated quantities but the variations cannot be used for an adjustment in the contract unit price.

Method of forming the slab shall be as shown on the plans and in accordance with Sec 703. All hardware for forming the slab to be left in place as a permanent part of the structure shall be coated in accordance with ASTM A123 or ASTM B633 with a thickness class SC 4 and a finish type I, II or III.

Slab shall be cast-in-place with conventional forms or stay-in-place corrugated steel forms. Precast prestressed panels will not be

All concrete above the construction joint in the end bents is included in the Estimated Quantities for Slab on Steel.

All reinforcement in the end bents is included in the Estimated Quantities for Slab on Steel.

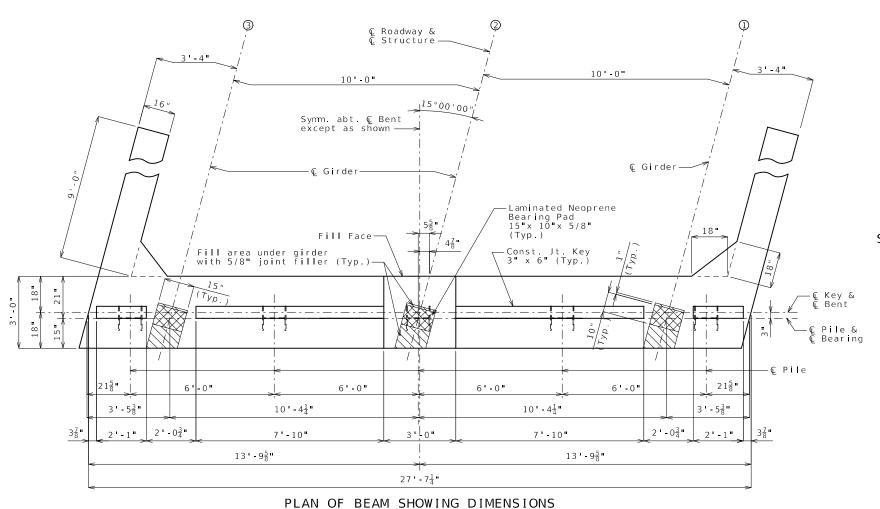
Specification, materials, zinc coating process and construction practice shall be in accordance with ASTM Al094/Al094M-18.

Galvanized reinforcing steel shall not come in contact with uncoated reinforcing steel or prestressing strands. Nylon, PVC, or Polyethylene spacers shall be used where necessary. Nylon cable ties shall be used to bind the spacers to the reinforcement

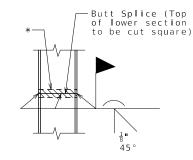
TIMOTHY D NUMBER PE-20120007 SSIONAL " 9/30/2024 D MO SHEET NO BR 2 CASS J4S3453 CONTRACT ID PROJECT NO

		€ 1 42		
DESCRIPTION				
DATE				



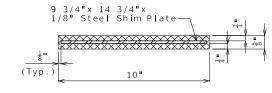


SECTION THRU KEY



STEEL PILE SPLICE (If required)

 Galvanizing material shall be omitted or removed one inch clear of weld locations in accordance with Sec 702.



TYPICAL SECTION THRU LAMINATED NEOPRENE BEARING PAD

2 spa. @ 12"

PLAN OF BEAM SHOWING REINFORCEMENT (Keys and beam steps not shown for clarity.)

11'-0"

3' 0" 3 spa. @ 12"

* 2 spa. @ 6"

END BENT NO. 1

5'-0"

— 2 - #6 **-** H101

13 -35

Note: This drawing is not to scale. Follow dimensions.

© Structure & © Roadway

Fill Face-

_2 spa

@ 12"

5'-0"

13'-35"

2 ' - 95"

158" 12"

© Pile & © Bearing−

(<u></u> 16-#5-U100

(□) 3-#4-U102

Detailed Apr. 2024 Checked June 2024

Sheet No. 3 of 22

12 | 15 | 15 |



Work this sheet with Sheets No. 4 & 5.

Reinforcing steel shall be shifted to clear piles. U bars shall clear piles by at least 1 1/2 inches.

The U bars and pairs of V bars shall be placed parallel to centerline of roadway.

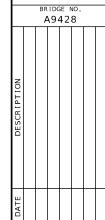
Substructure Quantity Table	for Bent N	lo. 1
I t em		Quantity
Class 1 Excavation	cu. yard	35
Galvanized Structural Steel Piles (12 in.)	linear foot	85
Pre-Bore for Piling	linear foot	75
Pile Point Reinforcement	each	5
Class B Concrete (Substructure)	cu. yard	12.3

These quantities are included in the Estimated Quantities table on Sheet No. 2.

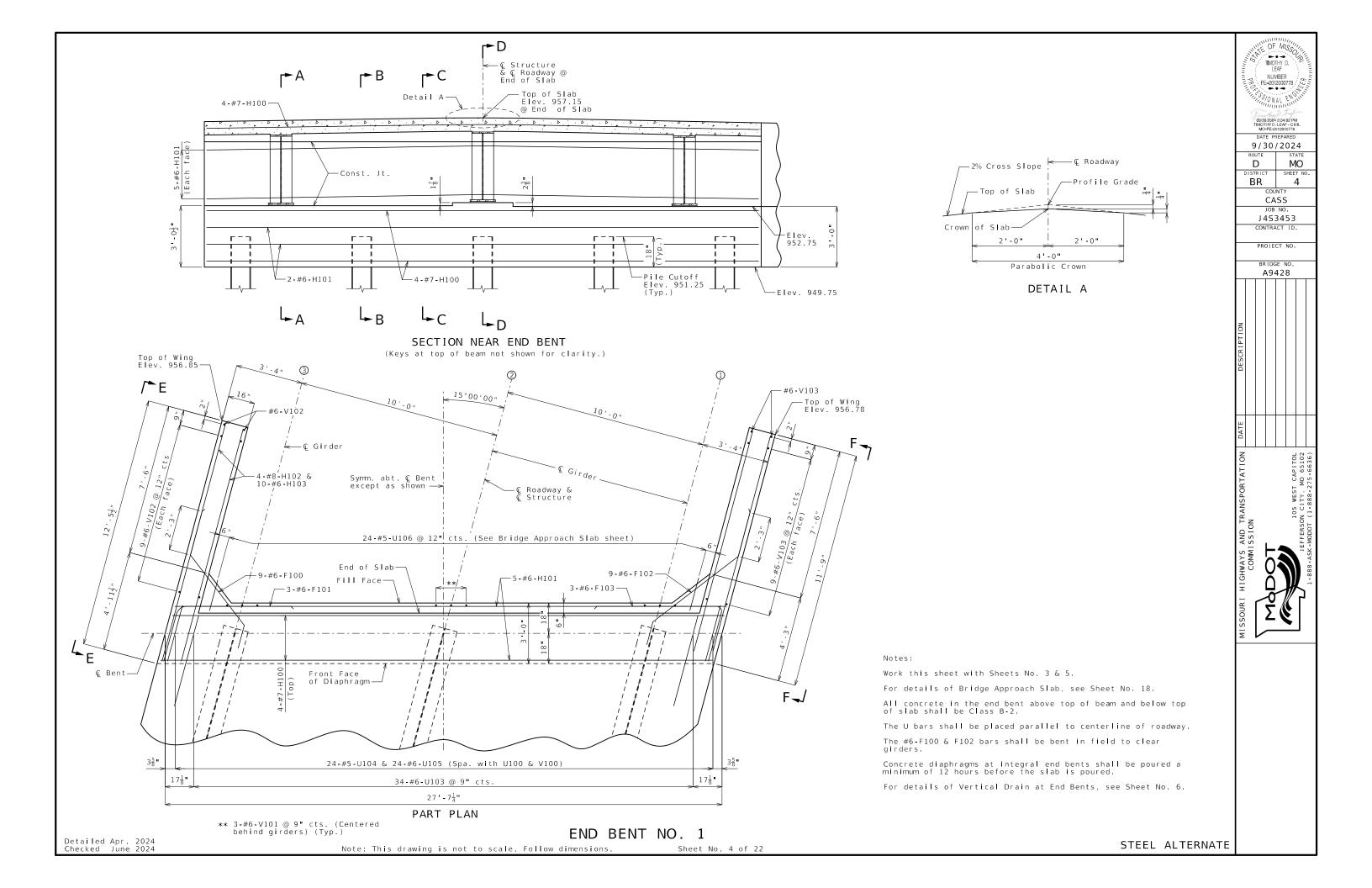
9/30/2024 D MO SHEET NO BR 3

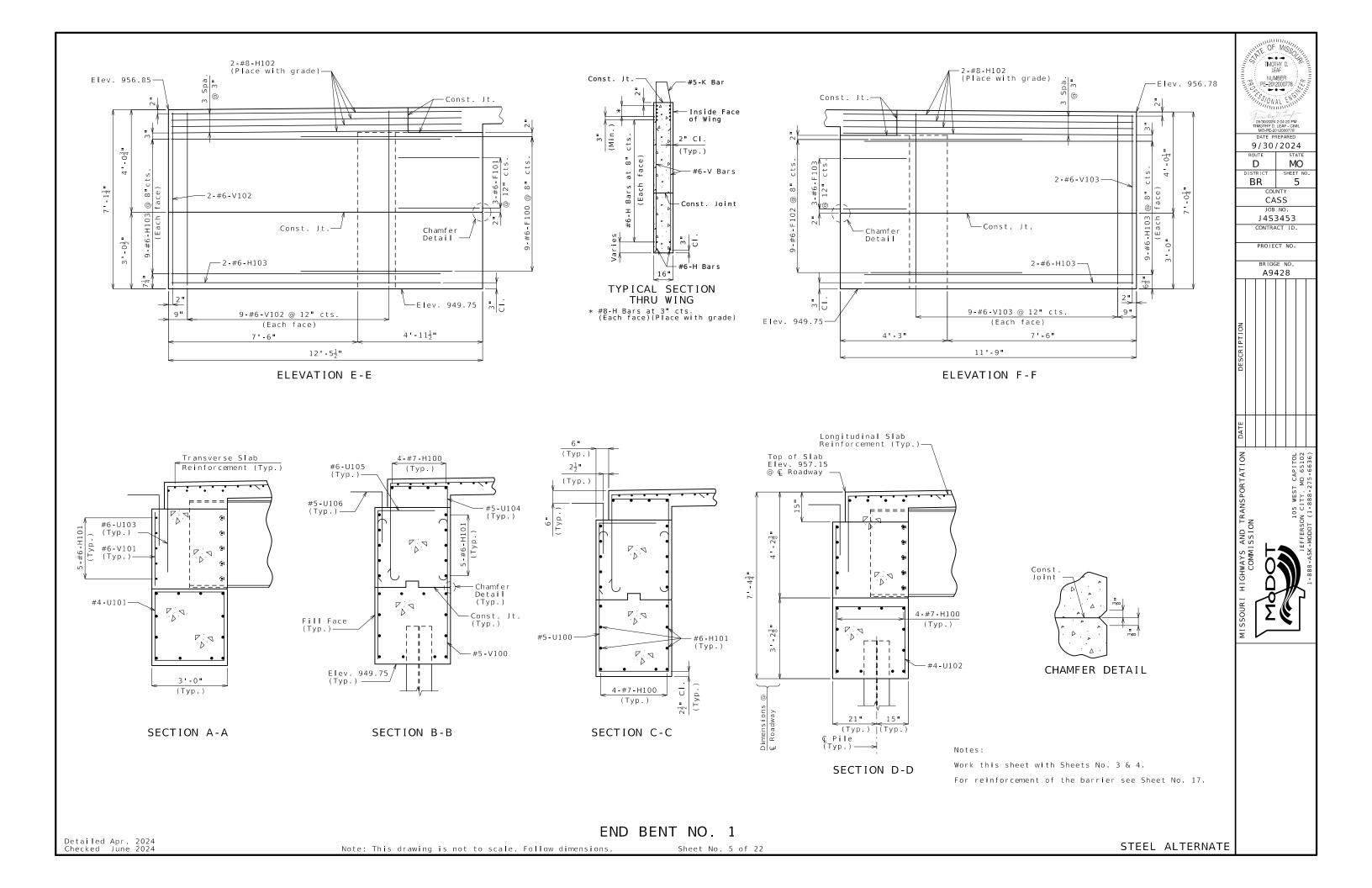
CASS JOB NO J4S3453 CONTRACT ID.

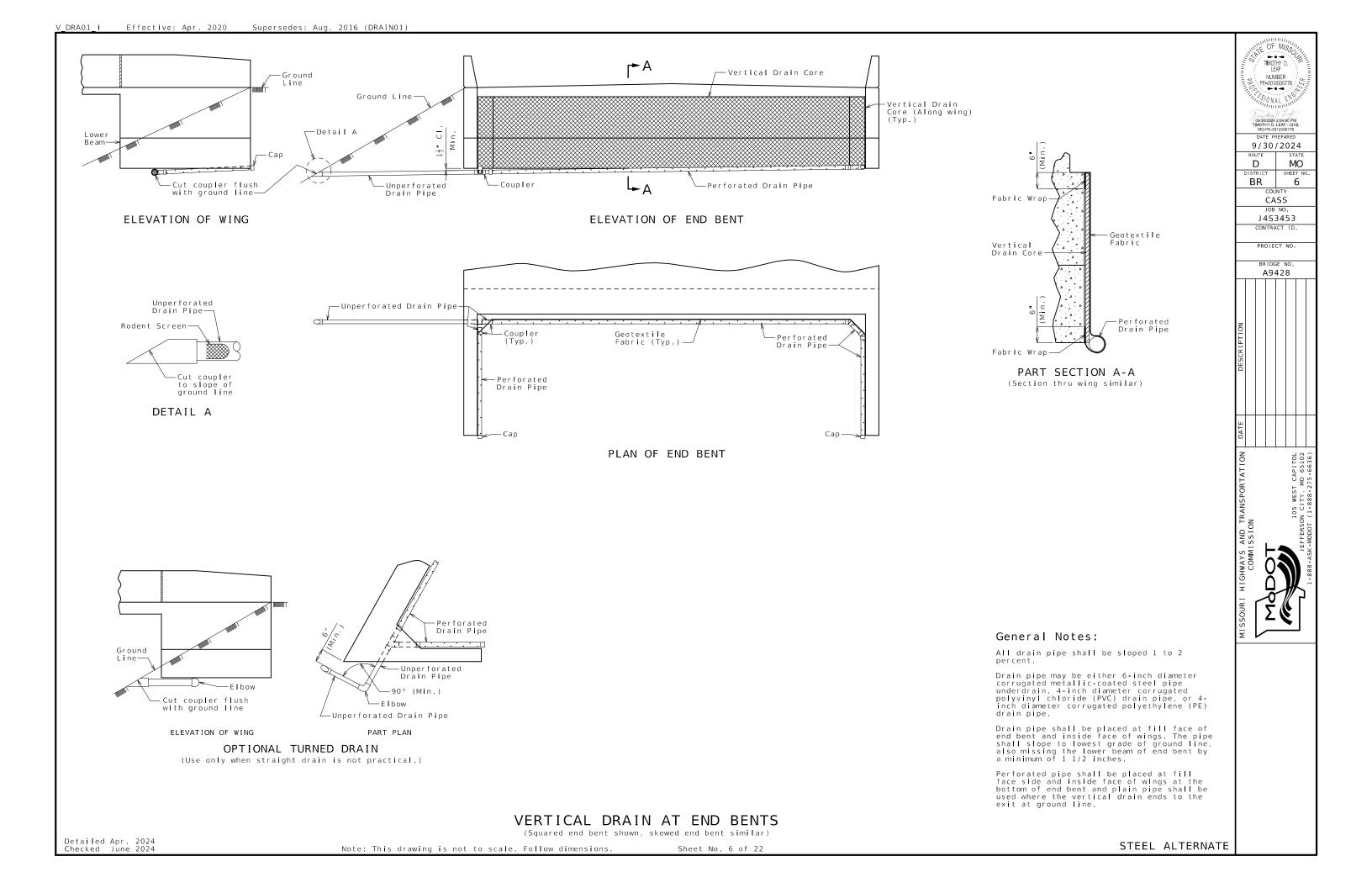
PROJECT NO.

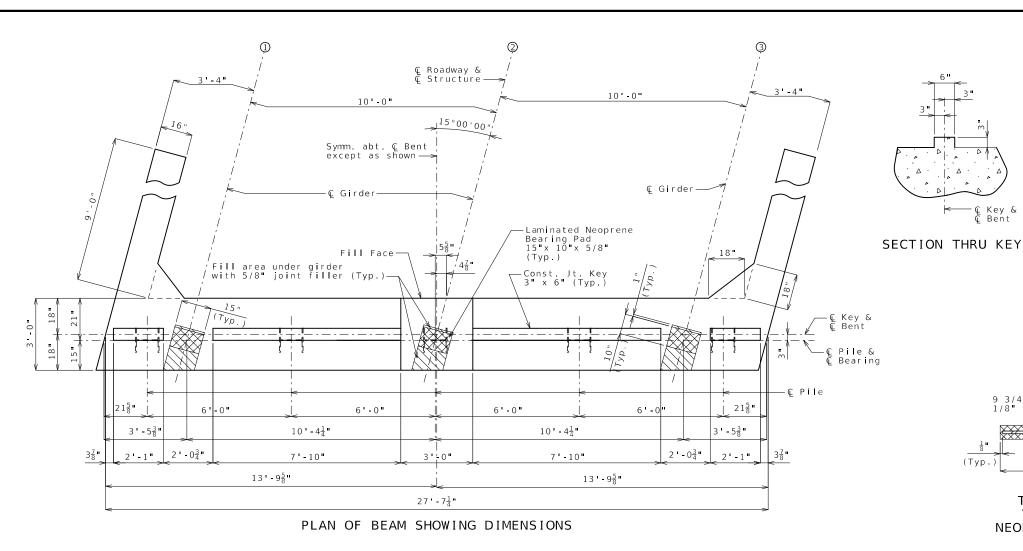


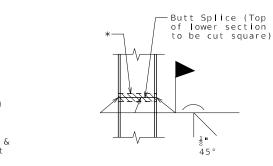
STEEL ALTERNATE







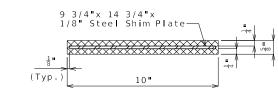




STEEL PILE SPLICE

(If required)

* Galvanizing material shall be omitted or removed one inch clear of weld locations in accordance with Sec 702.



TYPICAL SECTION THRU LAMINATED NEOPRENE BEARING PAD

Notes:

Work this sheet with Sheets No. 8 & 9.

Reinforcing steel shall be shifted to clear piles. U bars shall clear piles by at least 1 1/2 inches.

The U bars and pairs of V bars shall be placed parallel to centerline of roadway.

Substructure Quantity Table	for Bent N	lo. 2
I t em		Quantity
Class 1 Excavation	cu. yard	35
Galvanized Structural Steel Piles (12 in.)	linear foot	85
Pre-Bore for Piling	linear foot	75
Pile Point Reinforcement	each	5
Class B Concrete (Substructure)	cu. yard	12.3

These quantities are included in the Estimated Quantities table on Sheet No. 2.

€ Structure & € ₽^~ € Roadway - 2 **-** #6 **-** H201 Fill Face-Ç Pile & € Bearing- $(\underline{\square})$ 16-#5-U200 $3\frac{5}{8}$ " 3 spa. @ 12 _2 spa _2 spa. @ 12" @ 12" 2 ' - 95" 13'-35" 13 - 35 () 3-#4-U202 158" 12" 12" 158" 5'-0" 11'-0" 5'-0"

PLAN OF BEAM SHOWING REINFORCEMENT (Keys and beam steps not shown for clarity.)

* 2 spa. @ 6"

END BENT NO. 2

Detailed Apr. 2024
Checked June 2024
Note: This drawing is not to scale. Follow dimensions.

Sheet No. 7 of 22



INOITH D. LEAF NUMBER PE-2012000778

DATE PREPARED 9/30/2024

ROUTE STATE D MO
DISTRICT SHEET NO. BR 7

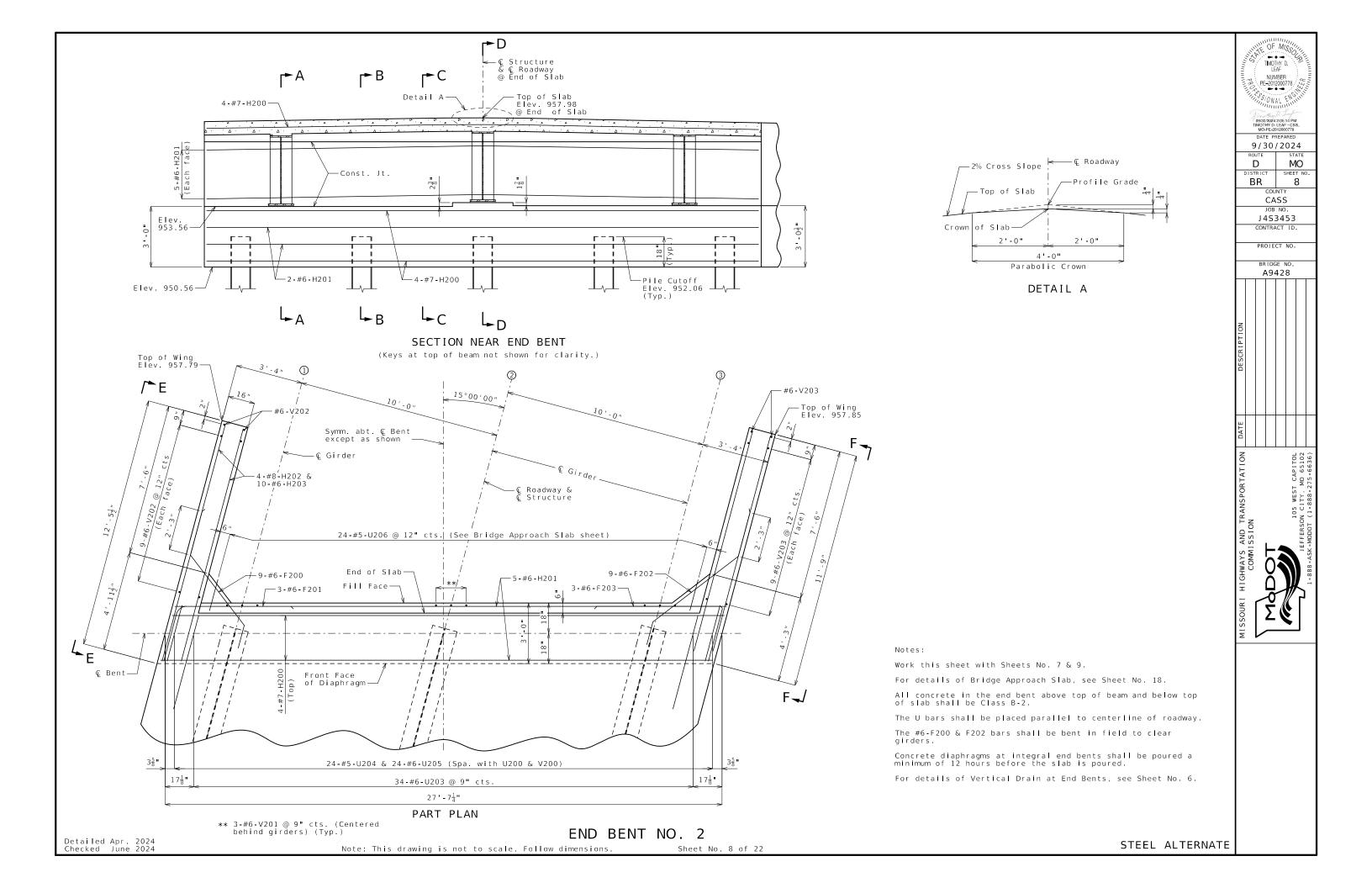
COUNTY CASS
JOB NO. J 453453

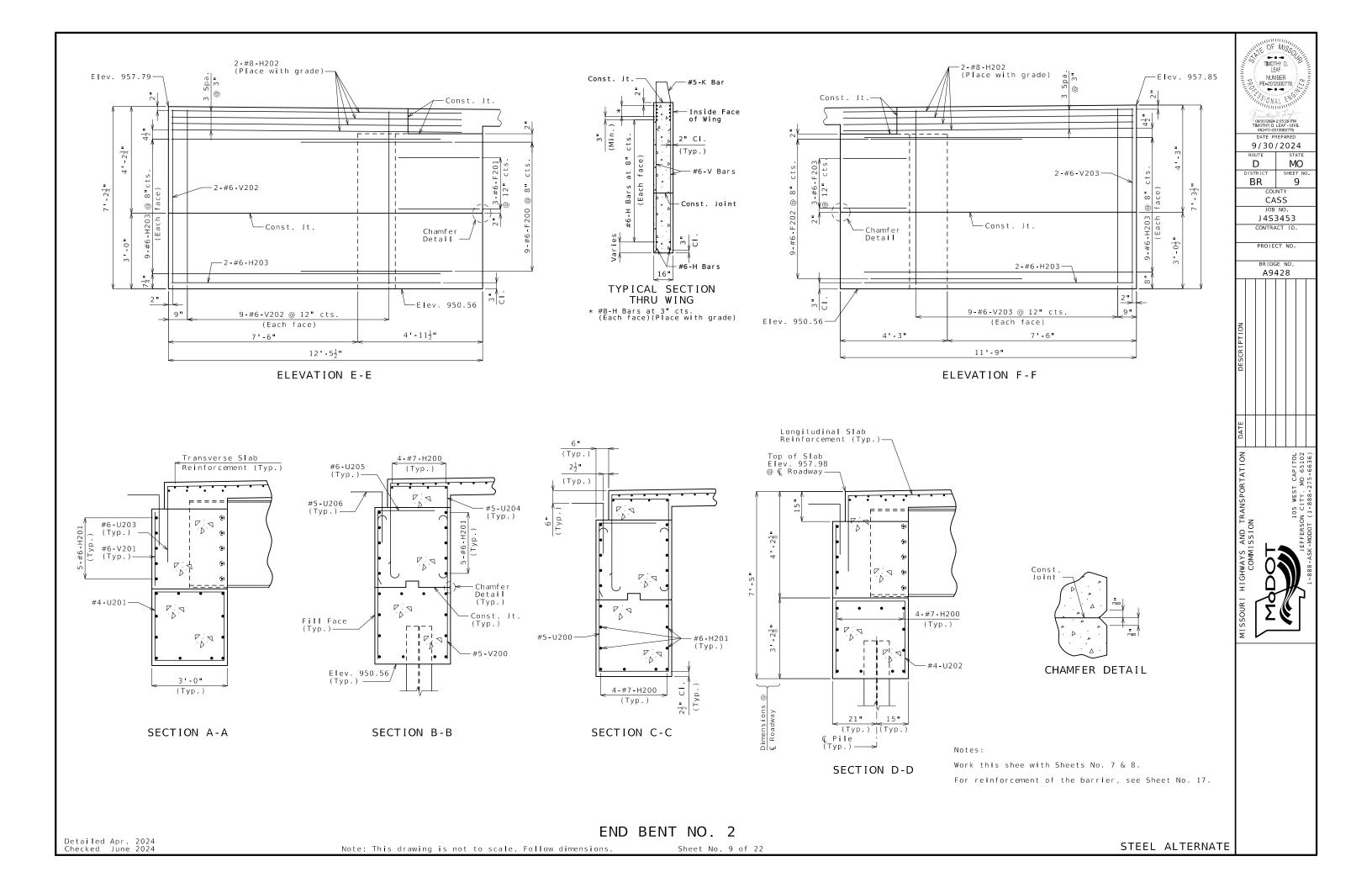
CONTRACT ID.

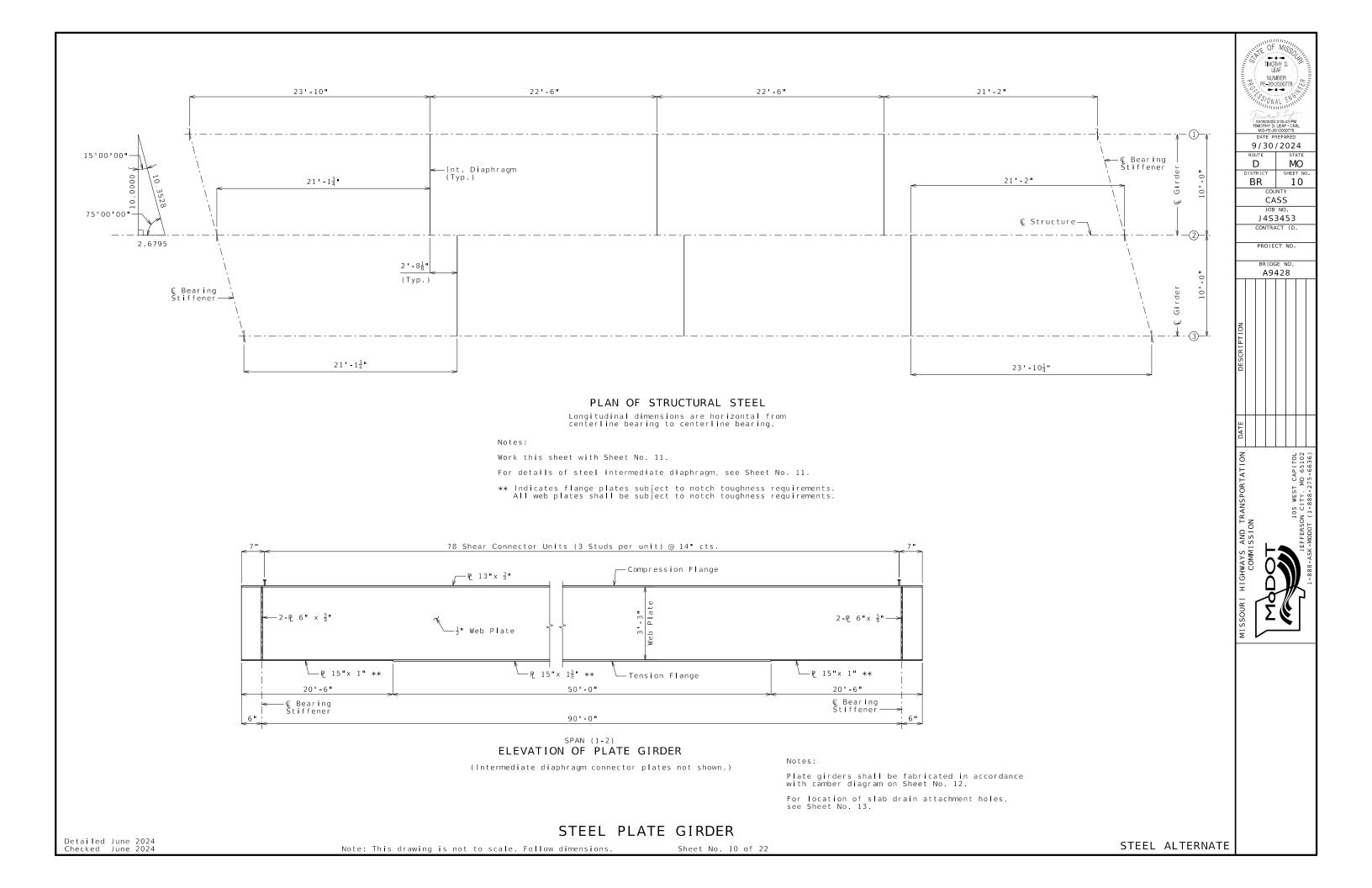
PROJECT NO.

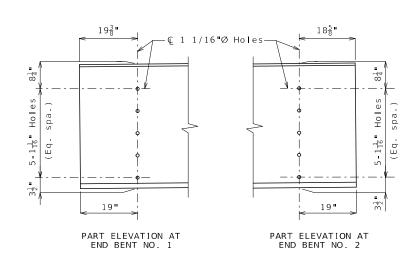
BRIDGE NO.
A9428



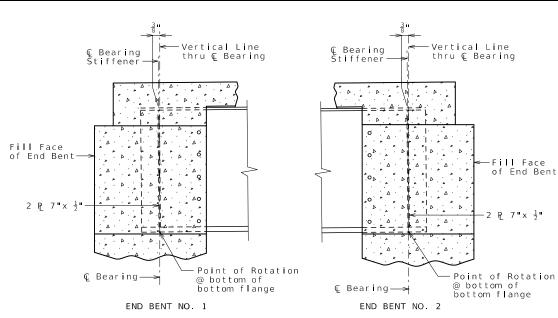




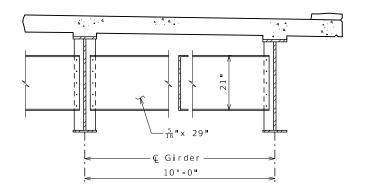




DETAILS OF WEB HOLES AT END BENTS

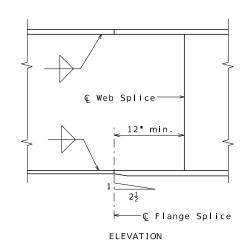


PART LONGITUDINAL SECTION



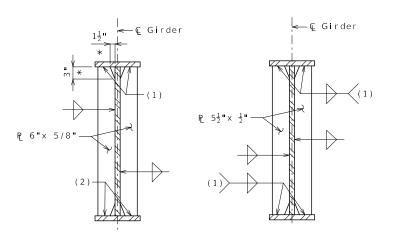
TYPICAL PART SECTION SHOWING INTERMEDIATE DIAPHRAGMS

For location of intermediate diaphragms, see Sheet No. 10.



WELDED SHOP WEB AND FLANGE SPLICE

Welded shop web and flange splices may be permitted when detailed on the shop drawings and approved by the engineer. No additional payment will be made for optional welded shop web and flange splices.



BEARING STIFFENER (NO ATTACHMENTS)

INTERMEDIATE DIAPHRAGM CONNECTION PLATE

WELDING DETAILS

- (1) Tight fit
- (2) Grind or mill to bear.
- * Typical for all intermediate web stiffeners, intermediate diaphragm connection plates and bearing stiffeners.

Notes:

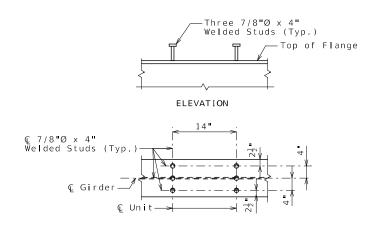
Work this sheet with Sheet No. 10.

Weight of 569 pounds of shear connectors is included in the weight of Fabricated Structural Low Alloy Steel (Plate Girder) A709, Grade 50.

Shear connectors shall be in accordance with Sec 712, 1037, & 1080.

Fabricated structural steel shall be ASTM A709, Grade 50, except as noted.

At the contractor's option, holes in the diaphragm plate of non slab bearing diaphragms may be made 3/16" larger than the nominal diameter of the bolt. A hardened washer shall be used under the bolt head and nut when this option is used. Holes in the girder diaphragm connection plate or transverse web stiffener shall be standard size.



PLAN OF SHEAR CONN. (3 PER UNIT)

DETAILS OF SHEAR CONNECTORS

STEEL DETAILS

Detailed Apr. 2024 Checked June 2024

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 11 of 22

STEEL ALTERNATE

TIMOTHY D LEAF NUMBER PE-201200077/

9/30/2024

D MO BR 11

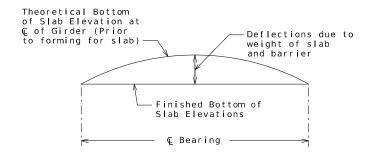
CASS LOB NO J4S3453 CONTRACT ID.

PROJECT NO.

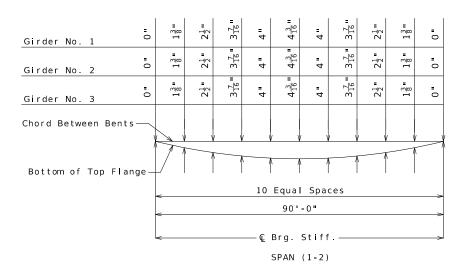
BRIDGE NO A9428

The	Theoretical Bottom of Slab Elevations at Centerline of Girder (Prior to forming for slab)													
Girder	Span (1-2) (90'-0" @ Brg @ Brg.)													
Number	© Brg.	. 10	. 20	.30	.40	.50	.60	.70	. 80	. 90	€ Brg.			
1	956.29	956.47	956.63	956.78	956.90	956.99	957.06	957.10	957.12	957.11	957.10			
2	956.49	956.67	956.84	956.98	957.10	957.20	957.26	957.30	957.32	957.32	957.30			
3	956.34	956.51	956.68	956.82	956.94	957.04	957.11	957.15	957.17	957.16	957.15			

Elevations are based on a constant slab thickness of 8" and include allowance for theoretical dead load deflections due to weight of slab (including precast panel) and barrier.



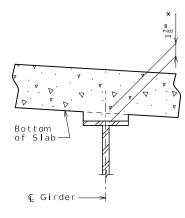
TYPICAL SLAB ELEVATIONS DIAGRAM



DEAD LOAD DEFLECTION

15% of dead load deflection is due to the weight of structural steel.

Dead load deflection includes weight of structural steel, concrete slab, and barrier.



THEORETICAL SLAB HAUNCH

* Dimension (bottom of slab to top of web) may vary if girder camber after erection differs from plan camber by more than the % of Dead Load Deflection due to weight of structural steel. No payment will be made for any adjustment in forming or additional concrete required for variation in haunching.

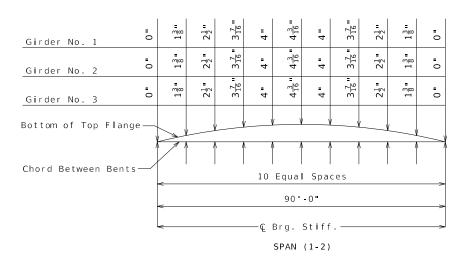


PLATE GIRDER CAMBER DIAGRAM

Camber includes allowance for vertical curve, and dead load deflection due to concrete slab, barrier, and structural steel.

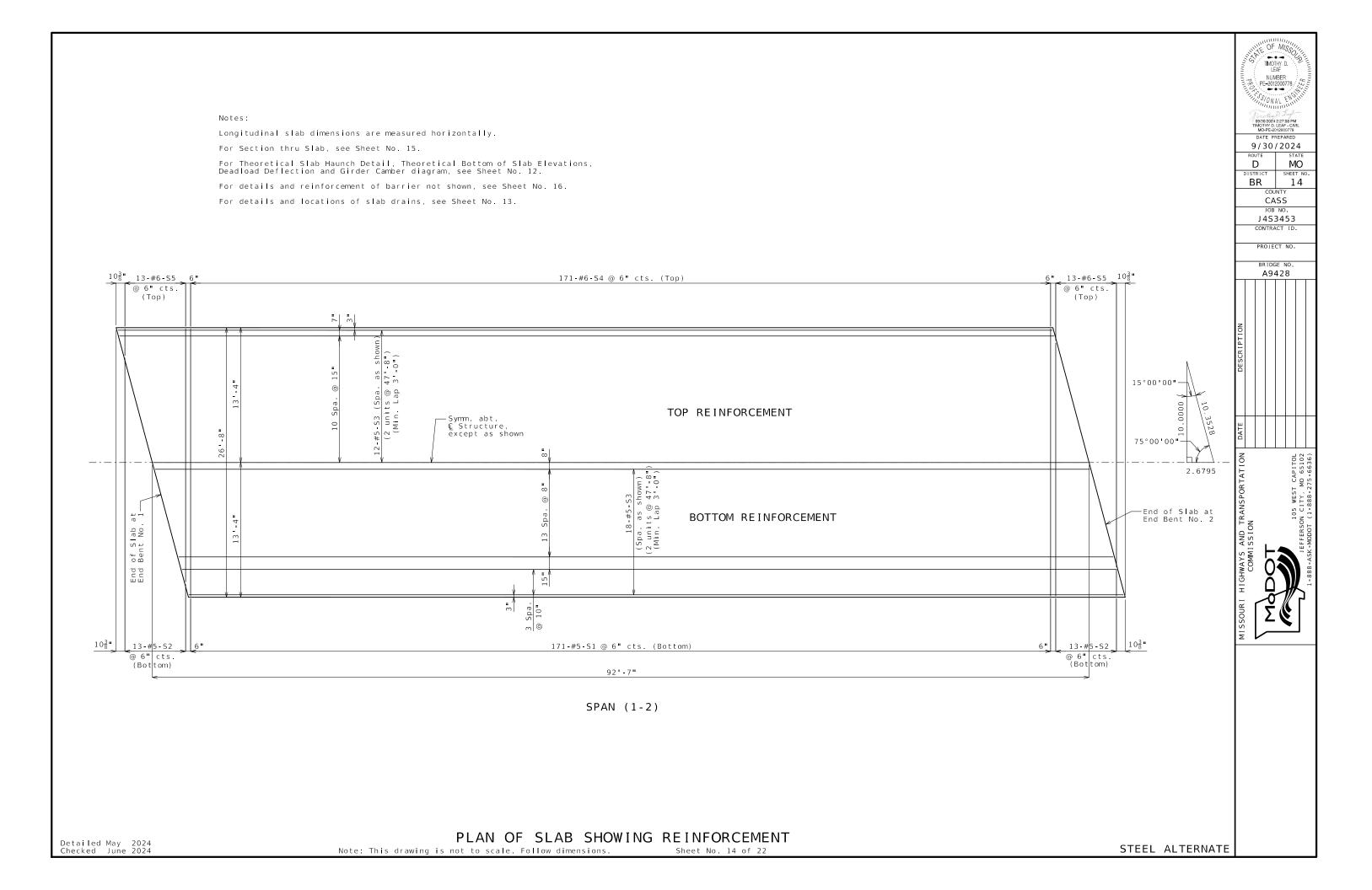
TMOTHY D. LEAF COVIDENT OF MISSON OF

MACOURI HIGHWAYS AND TRANSPORTATION
COMMISSION

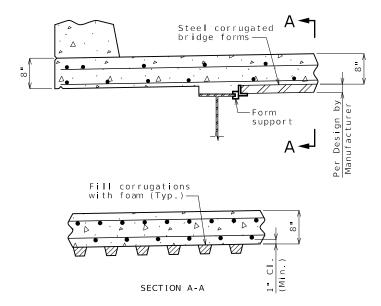
MACOOT

105 WEST CAPITOL

JEFFERSON CITY, MO 65102



OPTIONAL SHIFTING TOP BARS AT BARRIER



OPTIONAL STAY-IN-PLACE FORM DETAILS

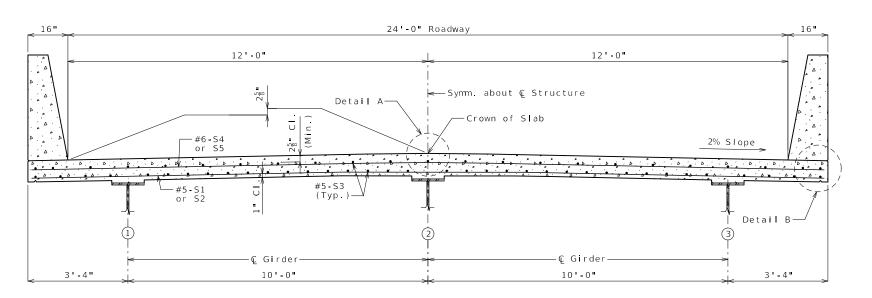
Stay-In-Place Forms:

Corrugated steel forms, supports, closure elements and accessories shall be in accordance with grade requirement and coating designation G165 of ASTM A653. Complete shop drawings of the permanent steel deck forms shall be required in accordance with Sec 1080.

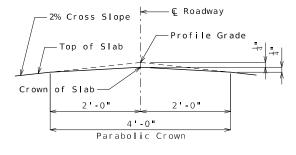
Corrugations of stay-in-place forms shall be filled with an expanded polystyrene material. The polystyrene material shall be placed in the forms with an adhesive in accordance with the manufacturer's recommendations.

Form sheets shall not rest directly on the top of girder flanges. Sheets shall be securely fastened to form supports with a minimum bearing length of one inch on each end. Form supports shall be placed in direct contact with the flange. Drilling holes in the girder flanges will not be permitted. All steel fabrication and construction shall be in accordance with Sec 1080 and 712. Certified field welders will not be required for welding of the form supports.

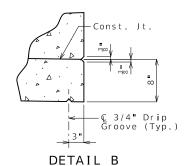
The design of stay-in-place corrugated steel forms is per manufacturer which shall be in accordance with Sec 703 for false work and forms. Maximum actual weight of corrugated steel forms allowed shall be 4 psf assumed for girder loading.



SECTION THRU SLAB



DETAIL A



SLAB CONSTRUCTION JOINT

Finish each side of joint with 1/4" radius edging tool

Key to extend full width of slab-

Notes:

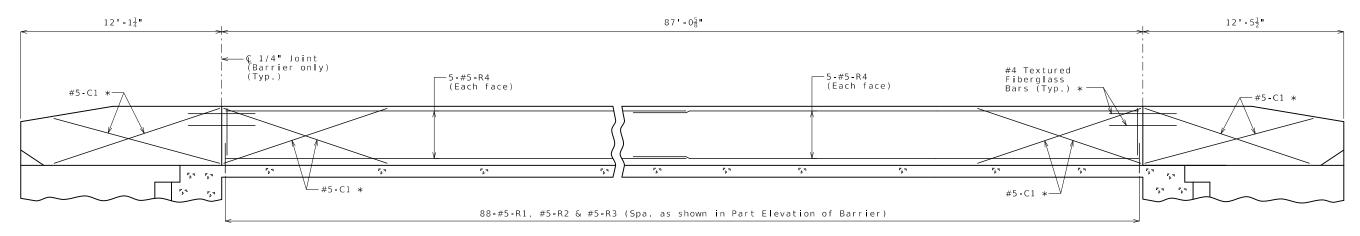
For reinforcement of barrier not shown, see Sheet No. 16 & 17. For Plan of Slab Showing Reinforcement, see Sheet No. 14.

The contractor shall pour up grade and satisfactorily finish the roadway slab at a rate of not less than 25 cubic yards per hour.

PE-2012000778 9/30/2024 D MO BR 15 CASS J4S3453 CONTRACT ID. PROJECT NO. BRIDGE NO A9428

TIMOTHY D. LEAF -Silicone Joint

Sealant



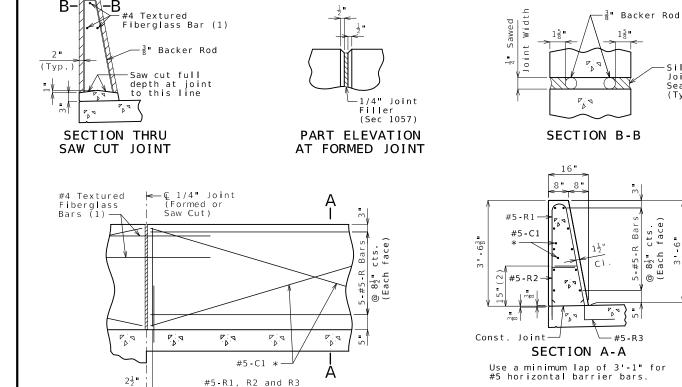
(Span 1-2)

Silicone

Sealant (Typ.)

ELEVATION OF BARRIER

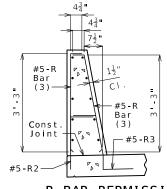
(Left barrier shown, right barrier similar)
Longitudinal dimensions are horizontal.



@ abt. 12" cts.

PART ELEVATION OF BARRIER

(1) Four feet long, centered on joint, slip-formed option only



R-BAR PERMISSIBLE ALTERNATE SHAPE

(3) The R1 bar may be separated into two bars as shown, at the contractor's option, only when slip forming is not used. (All dimensions are out to out.)

General Notes:

* Slip-formed option only.

Conventional forming or slip forming may be used. Saw cut joints may be used with conventional forming.

Top of barrier shall be built parallel to grade.

All exposed edges of barrier shall have either a 1/2-inch radius or a 3/8-inch bevel, unless otherwise noted.

Payment for all concrete and reinforcement, complete in place, will be considered completely covered by the contract unit price for Type D Barrier per linear foot.

Concrete in barrier shall be Class B-1.

Measurement of barrier is to the nearest linear foot for each structure, measured along the outside top of slab from end of wing to end of wing.

Concrete traffic barrier delineators shall be placed on top of the barrier as shown on Missouri Standard Plan 617.10 and in accordance with Sec 617.
Delineators on bridges with two-lane, two-way traffic shall have retroreflective sheeting on both sides. Concrete traffic barrier delineators will be considered completely covered by the contract unit price for Type D Barrier.

Joint sealant and backer rods shall be in accordance with Sec 717 for silicone joint sealant for saw cut and formed joints.

For slip-formed option, both sides of barrier shall have a vertically broomed finish and the top shall have a transversely broomed finish.

TMOTHY D.
LEAF
NUMBER
PE-2012000778

09/30/2024 2:28:33 PM TIMOTHY D. LEAF - CIVIL MO-PE-2012000778 DATE PREPARED

9/30/2024

ROUTE STATE
D MO

DISTRICT SHEET NO
BR 16

COUNTY
CASS

J4S3453
CONTRACT ID.

PROJECT NO.

LOB NO

BRIDGE NO. A9428

N DATE DESCRIPTION

COMMISSION

COMMISSION

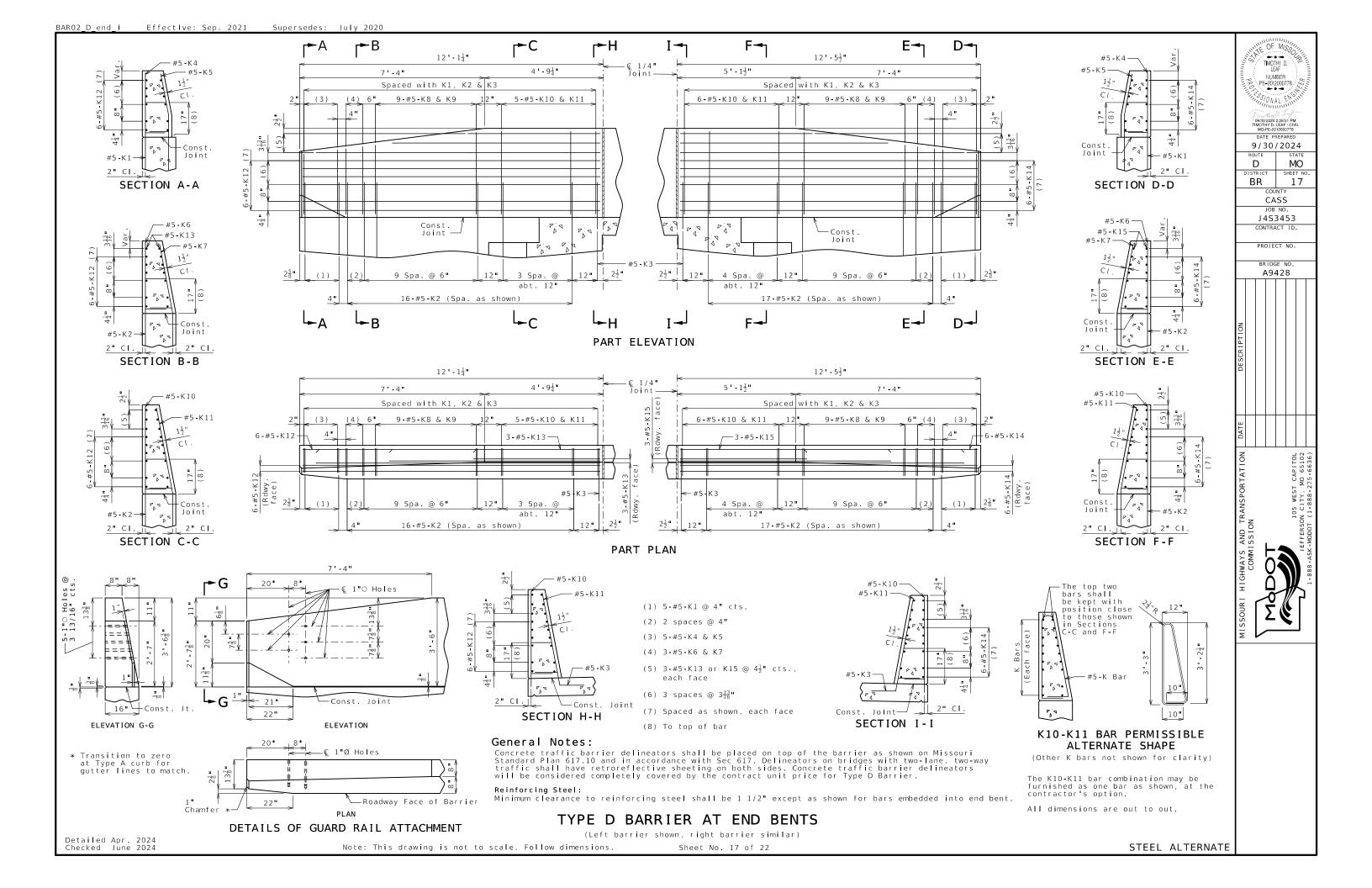
1.588-54K-MONDT (1.888-245-6834)

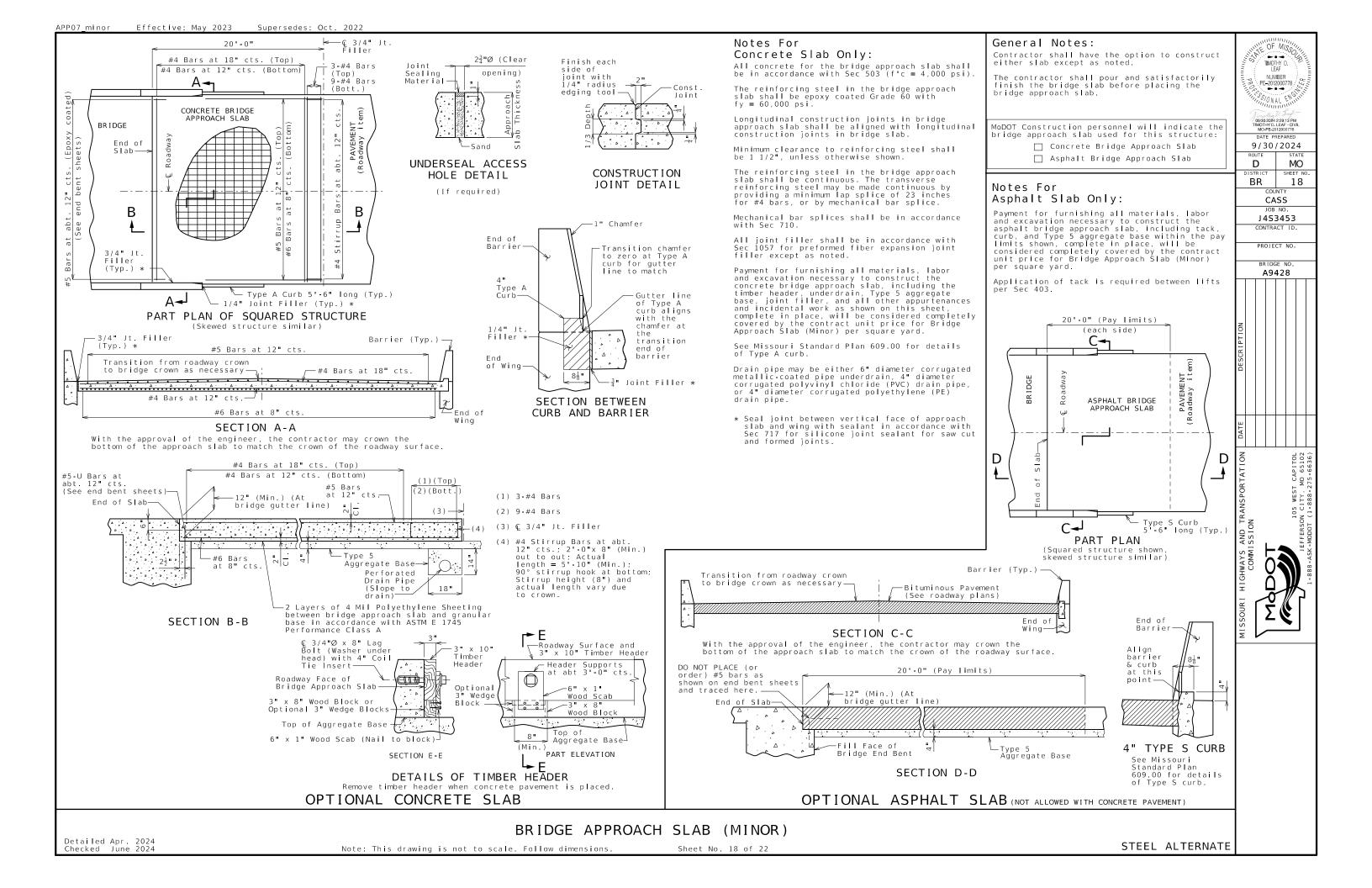
TYPE D BARRIER

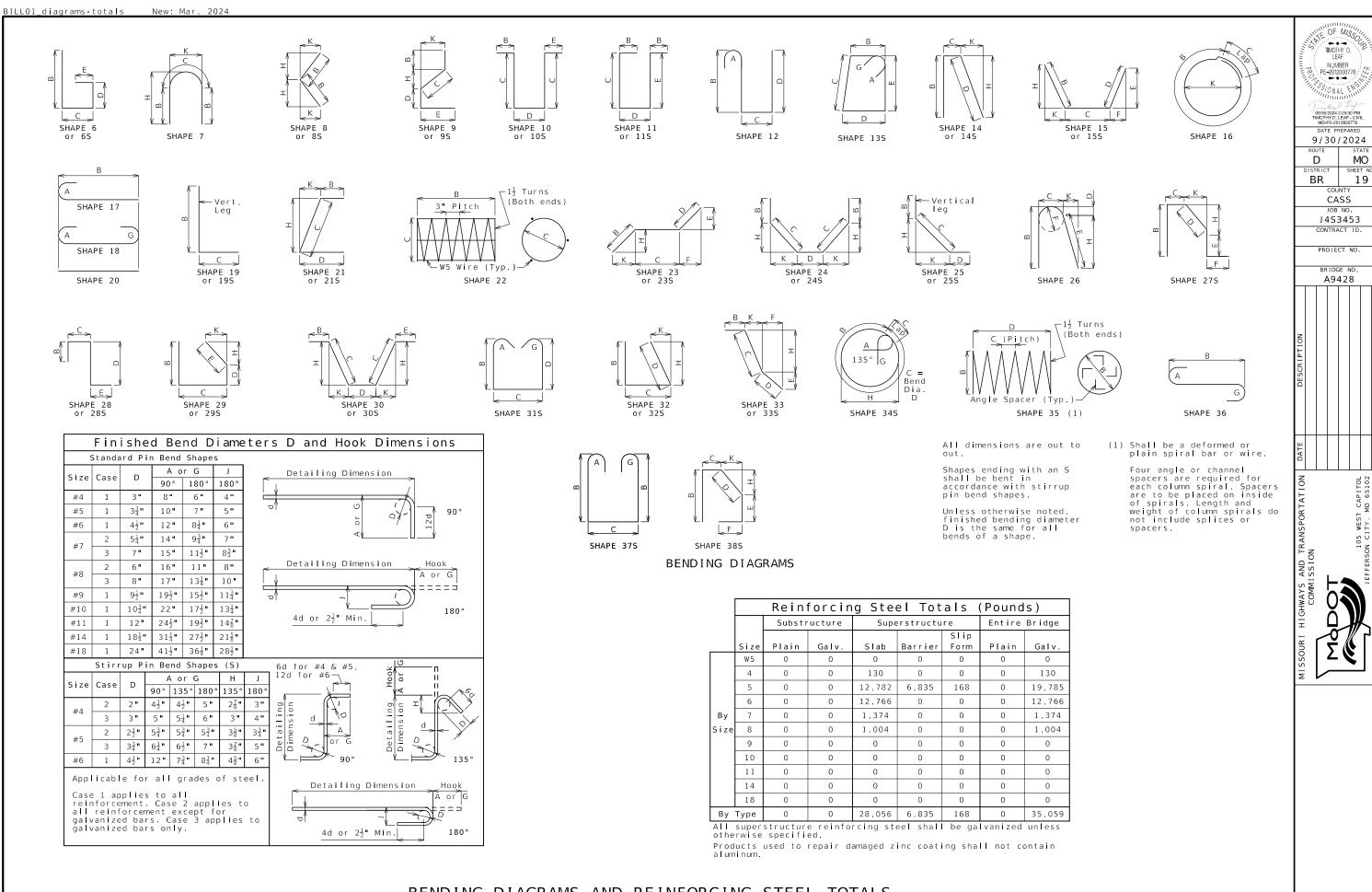
Sheet No. 16 of 22 STEEL ALTERNATE

(2) To top of bar

The cross-sectional area above the slab is 3.52 square feet.







BILLO2 data New: Mar. 2024

BILL02	_uata	New: Mar. 2024	4																						
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				_	RIII O	f Reinforc	ing St	eeı				Bill of Reinforcing Steel						ITTE OF MISS	6/1/2						
						Dimensio	ns		Nom	. Actua	l I						Dir	nension	S		Nom.	Actual	1 1	TIMOTHY D.	9=
No.	Size/		Codes	В	С	D E	F	Н	K Leng	th Length	Weight	No. Size		Codes	В	С	D	E	F H	K	Length	Length	Weight	_ EAF	1
Req.	Mark	Location	C SH \	/ft in.	ft in.	ft in. ft in.	ft in.	ft in.	ft in. ft	in. ft in	ı. Ib	Req. Mark	Location	C SH \	/ft in.	ft in. f	t in. f	in.	ft in. ft in.	ft in.	ft in.	ft in.	Ιb	NUMBER 2012000779	, ja==
																								PE-2012000778	100
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												36 5 K8	BARRIER	G 195 4	4 2 8.50	10.00				1	3 7	3 5	133	William .	
		End Bent 1											Incr. = 0.750"		3 2.50	10.00				1	4 1	3 11	138	1 in thy 2 2 29:48 F	DM
9	6 F100	WING BRACE	G 23S	2 3.00	4 5.00	14.00 11.0	0 8.50	21.50	16.50 7	10 7 9	108									1				TIMOTHY D. LEAF - C MO-PE-201200077	,IVIL
3	6 F101	DIAPHRAGM	G 15S	_	4 7.00			2 8.00	8.50 7	4 7 1	33	36 5 K9	BARRIER	G 21S 4	4	2 8.50	10.00		2 7.75	6.75	3 7	3 5		DATE PREPARE	
9	6 F102	WING BRACE	G 23S	_	5 8.00	14.00 11.0	0 8.50	21.50	16.50 9	1 9	125		Incr. = 0.750"			3 2.50	10.00		3 1.75	7.75	4 1	3 11	142	9/30/20	24
3	6 F103	DIAPHRAGM	G 21S		2 9.00			2 8.00	8.50 7	11 7 10										1					STATE
												22 5 K10	BARRIER	G 19S	3 3.00	10.00				1	4 1	3 11	93		MO
12	7 H100	BM & DIAPH	G 20	27 4.00					27	4 27 4	687	22 5 K1	BARRIER	G 21S	1	3 3.00	10.00		3 2.25	7.75	4 1	4	95		EET NO.
	6 H101	BM & DIAPH		27 4 00					27			24 5 K12		G 20	11 10.00							11 10	306		20
	8 H102	WING		11 6.00					11		502	12 5 K13			4 11 1.00						11 1	11 1		COUNTY	-
40	6 H103	WING		10 8.00					10		_		Incr. = 36.000"		5 1.00					1	5 1	5 1	105	CASS	
															1						+			JOB NO.	
16	5 U100	BEAM	G 37S	5 9.00	2 10.00				15	6 15 3	263	24 5 K14	BARRIER	G 20	12 2.00						12 2	12 2	315	J4S345	3
	4 U101	BEAM	G 135	_		2 10.00 2 8.00)		11			12 5 K15			4 11 5 00						11 5	11 5		CONTRACT I	D.
	4 U102	BEAM	G 105			2 10.00				2 7 1		1	Incr. = 36.000"		5 5.00						5 5		109		
	6 U103	DIAPHRAGM	G 195	3 3 00	4 8 00				7											·				PROJECT NO	٠.
	5 U104	DIAPHRAGM	G 37S		2 4 00				10		274	176 5 R1	BARRIER	G 26	3 3.00	5.50	2.25 3	3.50	5.50 3 3.00	6.75	7 2	6 11	1,314		
	6 U105	DIAPHRAGM	G 195		2 10.00					6 5 4	198	176 5 R2	BARRIER	G 195	20.50						2 6	2 4	443	BRIDGE NO	
	5 U106	DIAPHRAGM	G 195	2 0.00					3		80	176 5 R3	BARRIER	G 275		9.50	15.25	5.00	12.00 15.00	3.00		3 3	617	A9428	
			1000						-			40 5 R4	BARRIER		45 0.00		10.10		12.00		45	45	1,942		
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	6 F201	DIAPHRAGM	G 15S		4 7 00	11100 1110	0.30	2 8 00	8.50 7		33			++							 				
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	6 U205	DIAPHRAGM	G 195		2 10.00				5												1	†		P.	105 WEST CAPITOL RSON CITY, MO 65102 OT (1-888-275-6636)
	5 U206	DIAPHRAGM	G 195	_					3		80										 			S	¥ 7 7 388
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 Nomin 	al Lan	oths are based or	a out to	out dim	oncione c	cnown in honding	. diagram	and are					Codoc. C	- Pogu	irod coat	tinac whoi	- A E - En	2 V V C 2	ated and $G = Galv$	120 1 70d					

Nominal lengths are based on out to out dimensions shown in bending diagrams and are listed to the nearest inch for fabricator's use. Actual lengths are measured along centerline bar to the nearest inch. Weights are based on actual lengths.

All bars shall be Grade 60.

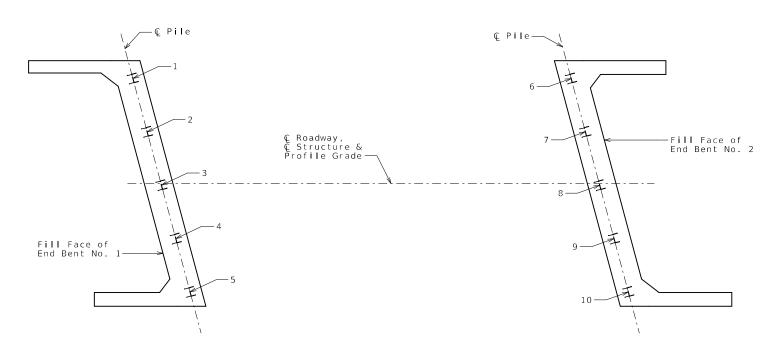
Codes: C = Required coatings, where E = Epoxy Coated and <math>G = Galvanized.

SH = Required shape, see bending diagrams.

V = Sets of varied bars and number of bars of each length. Bar dimensions vary in equal increments between dimensions shown on this line and the following line and the actual length dimension shown on this line and the following line vary by the specified increment.

STEEL ALTERNATE

For	bendi	ng	diagrams	and	steel	reinforcing	totals,	see	Sheet	No.	19.
			e 2024 e 2024						Note:	This	dr



PART PLAN SHOWING PILE NUMBERING FOR RECORDING AS-BUILT PILE DATA

	As-Built Pile Data									
Pile No.	Length in Place (ft)	Compressive	Remarks							
			End Bent No. 1							
1										
2										
3										
4										
5										
			End Bent No. 2							
6										
7										
8										
9										
10										

Note: Indicate in remarks column: A. Pile type and grade B. Batter C. Driven to practical refusal

This sheet to be completed by MoDOT construction personnel.

TIMOTHY D. LEAF NUMBER PE-2012000778 ILESSIONAL ENG 9/30/2024 ROUTE D MO SHEET NO. BR CASS JOB NO.
J4S3453
CONTRACT ID. PROJECT NO. BRIDGE NO A9428

BOR01 BoringLogTemplate Effective: Apr. 2021 Supersedes: Aug. 2018 TIMOTHY D. LEAF NUMBER PE-2012000778 SUBSURFACE DIAGRAM | USCS Poorly-graded Gravel USCS Low Plasticity Gravelly Clay USCS Low Plasticity Clay Missouri Dept of Transportation Highly Weathered Limestone Limestone PROJECT NAME Bridge Replacement Shale PROJECT LOCATION 1.0 Mile South of Lisleca CLIENT PROJECT NUMBER J4S3453 9/30/2024 D MO SHEET NO BR 22 V-23-73 (102) 1167+55.45 15.2 L ELEV.957.50 V-23-73 (201) 1166+61.6 12.8 R ELEV.956.70 CASS IOB NO. PP N_{an} or (tsf) [RQD] J4S3453 PP N_m or (tsf) [RQD] CONTRACT ID. PROJECT NO. 955 955 BRIDGE NO A9428 1.75 14 1.0 6 950 950 1.0 9 2.0 16 Elevation (ft) 945 945 1.75 12 [72] [56] 940 940 922 892 [80] 935 935 2510 152 [66] [82] 2490 930 930 36.6 [92] [90] 925 925 1430 Distance Along Baseline (ft) BORING DATA Note: For locations of borings, see Sheet No. 1. Detailed May 2024 Checked June 2024 STEEL ALTERNATE Note: This drawing is not to scale. Follow dimensions. Sheet No. 22 of 22