DESIGN DESIGNATION

ROUTE 46 BRIDGE

A.A.D.T. - 2025 = 1102 A.A.D.T. - 2045 = 1248 D.H.V. = 9.93% T = 17.55% V = 55 M.P.H.

D = 49.5%/50.5%

D = 47.3%/52.7%

FUNCTIONAL CLASSIFICATION-MAJOR COLLECTOR

DESIGN DESIGNATION

ROUTE W BRIDGE

A.A.D.T. - 2025 = 249 A.A.D.T. - 2045 = 283 D.H.V. = 15.64% T = 11.52% V = 55 M.P.H.

FUNCTIONAL CLASSIFICATION-MAJOR COLLECTOR

NO RIGHT OF WAY ACQUISITION

CONVENTIONAL SYMBOLS

	EXISTING	NEW
BUILDINGS AND STRUCTURES GUARD RAIL GUARD CABLE CONCRETE RIGHT-OF-WAY MARKER STEEL RIGHT-OF-WAY MARKER LOCATION SURVEY MARKER UTILITIES	0000	••••
FIBER OPTICS OVERHEAD CABLE TV UNDERGROUND CABLE TV OVERHEAD TELEPHONE UNDERGROUND TELEPHONE OVERHEAD POWER UNDERGROUND POWER SANITARY SEWER STORM SEWER GAS WATER	- FOOTVUTV OT UT OE UE S S S G W SAN	UT
MANHOLE	HYD.	\ni
FIRE HYDRANT	w _v C	3
WATER VALVE	,, MM)
WATER METER	₽‴)
DROP INLET	<u>"</u>	
DITCH BLOCK	=	⊨
GROUND MOUNTED SIGN	SIGN	-
LIGHT POLE]
H-FRAME POWER POLE		
TELEPHONE PEDESTAL FENCE CHAIN LINK WOVEN WIRE	PED \ \ \	·—
GATE POST	BM C	7
BENCHMARK	⊗	9

NOTE: DASHED OR OPEN SYMBOLS INDICATE EXISTING FEATURES

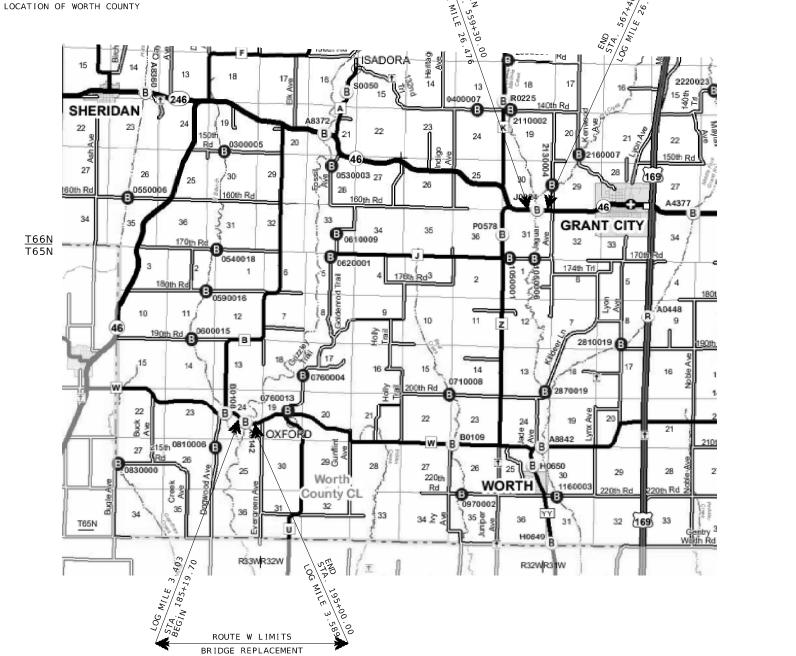
MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

PLANS FOR PROPOSED

STATE HIGHWAY 46/STATE HIGHWAY W

WORTH COUNTY





THE EXISTENCE AND APPROXIMATE LOCATION OF UTILITY FACILITIES KNOWN TO EXIST, AS SHOWN ON THE PLANS, ARE BASED ON THE BEST INFORMATION AVAILABLE TO THE COMMISSION AT THIS TIME. THIS INFORMATION IS PROVIDED BY THE COMMISSION "AS-IS" AND THE COMMISSION EXPRESSLY DISCLAIMS ANY REPRESENTATION OR WARRANTY AS TO THE COMPLETENESS, ACCURACY, OR SUITABILITY OF THE INFORMATION FOR ANY USE. RELIANCE UPON THIS INFORMATION IS DONE AT THE RISK AND PERIL OF THE USER, AND THE COMMISSION SHALL NOT BE LIABLE FOR ANY DAMAGES THAT MAY ARISE FROM ANY ERROR IN THE INFORMATION. IT IS, THEREFORE, THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY THE EXISTENCE, LOCATION AND STATUS OF ANY FACILITY. SUCH VERIFICATION INCLUDES DIRECT CONTACT WITH THE LISTED UTILITIES.

INDEX OF SHEETS

DESCRIPTION	SHEET NUMBER
TITLE SHEET	1
TYPICAL SECTIONS (TS) (2 SHEETS)	2
QUANTITIES (QU) (3 SHEETS)	3
PLAN-PROFILE (PP)	4 - 6
REFERENCE POINTS (RP)	7 - 8
COORDINATE POINTS (CP)	9-10
SPECIAL SHEETS (SS)	11-13
TRAFFIC CONTROL (TC)	14-16
EROSION CONTROL (EC)	17 - 18
SIGNING & STRIPING (SN & PM)	19-22
BRIDGE DRAWINGS (B)	
BRIDGE A9467 ROUTE 46	1-30
BRIDGE A9468 ROUTE W	1-36
CROSS SECTIONS ROUTE 46 (XS)	1 - 6
CROSS SECTIONS ROUTE W (XS)	1 - 8

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DESCRIPTION								SEAL
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LENGTH OF PROJECT

BEGINNING OF ROUTE 46	STA. 559 + 30.00
END OF ROUTE 46	STA. 567 + 40.00
APPARENT LENGTH	810.00 FEET
EQUATIONS AND EXCEPTIONS:	
TOTAL CORRECTIONS	.00 FEET
TOTAL CONNECTIONS	
NET LENGTH	810.00 FEET
FOR INFORMATION ONLY ESTIMATED DISTURBED ACRES	0.76 ACRES
BEGINNING OF ROUTE W	STA. 185 + 19.70
END OF ROUTE W	STA. 195 + 00.00
APPARENT LENGTH	980.30 FEET
APPARENT LENGTH	900.30 FEET
EQUATIONS AND EXCEPTIONS:	
TOTAL CORRECTIONS	.00 FEET
NET LENGTH	980.30 FEET

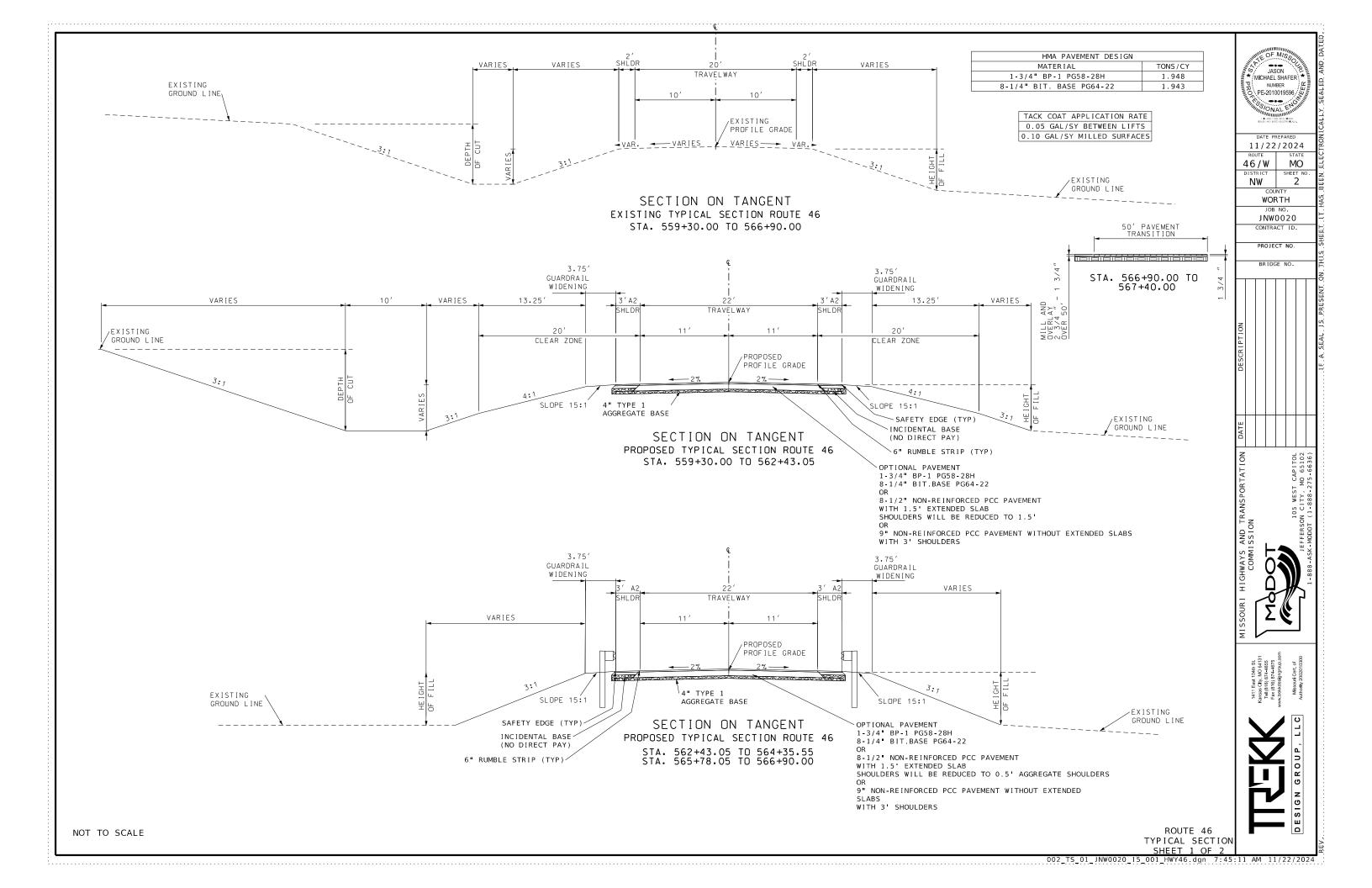
NET LENGTH OF PROJECT	1,790.30	FEET
STATE LENGTH	0.339	MILES
FOR INFORMATION ONLY ESTIMATED DISTURBED ACRES	0.731	ACRES

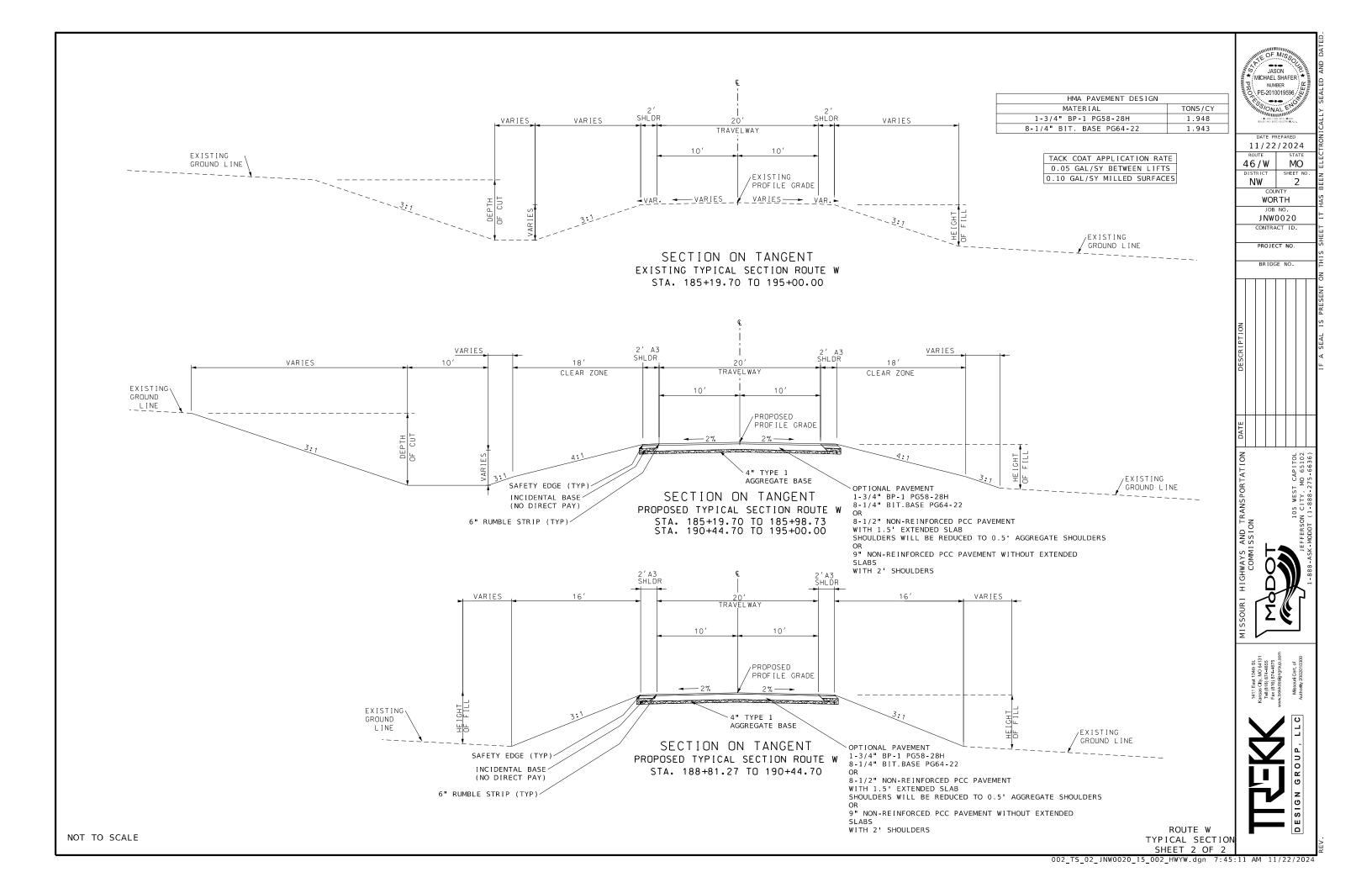


1411 East 104th St.
Kansas Cly, MO 64131
Tel (816) 874-4675
Fax (816) 874-4675
www.trekkdesigngroup.com
Missouri Cert. of



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MOBILIZATION					
PROJECT TOTAL	1 LUMP SUM				

ADDITIONAL MOBILIZ	ZATION FOR SEEDING
PROJECT TOTAL	3 EA

CONTRACTOR	FURNISHED	SURVEYING	& STAKING
PROJECT	TOTAL	1 LUM	P SUM

ENTRANCES								
			GRAVEL A	15"	15"			
			CRUSHED STONE B	GROUP C	GROUP C			
			4 "	PIPE	FES			
LOCATION	STATION	SIDE	(SY)	(LF)	(EA)			
ROUTE 46	561+22.21	LT	82.6	24	2			
ROUTE 46	561+23.60	RT	44.8	32	2			
		TOTALS	127	56	4			

SEED AND MULCH							
SEEDING - COOL							
			SEASON GRASSES	MULCHING			
LOCATION	STATION	STATION	(ACRE)	(ACRE)	REMARKS		
ROUTE 46	559+30.00	564+15.55	0.5	0.5	WEST OF BRIDGE		
ROUTE 46	565+98.05	567+73.80	0.2	0.2	EAST OF BRIDGE		
		TOTALS	0.7	0.7			

	MILL AND OVERLAY							
			1.75"	TACK	MODIFIED			
			BP-1	COAT	COLDMILLING			
			PG 58-28H	0.10 GAL/SY	(DEPTH TRANSITIONS)			
LOCATION	STATION	STATION	(TON)	(GAL)	(SY)	REMARKS		
ROUTE 46	566+90.00	567+40.00	14.4	15.2	152	DEPTH TRANSITION		
		TOTALS	14.4	15.2	152			

PERMANENT EROSION CONTROL								
			FURNISHING	PLACING	PERMANENT			
			TYPE 2	TYPE 2	EROSION CONTROL			
			ROCK BLANKET	ROCK BLANKET	GEOTEXTILE			
LOCATION	STATION	SIDE	(CY)	(CY)	(SY)			
ROUTE 46	564+38.02	LT/RT	426.7	426.7	640			
ROUTE 46	565+75.55	LT/RT	148.7	148.7	223			
		TOTALS	575	575	863			

	BASE AND PAVEMENT											
TYPE 1 AGG.												
				OPTIONAL	TYPE A2	FOR BASE						
				PAVEMENT	SHOULDER	(4 IN. THICK)						
LOCATION	STATION	STATION	SIDE	(SY)	(SY)	(SY)	REMARKS					
ROUTE 46	559+30.00	564+15.55	LT/RT	1186.9	321.0	1508	WEST OF BRIDGE					
ROUTE 46	565+98.05	566+90.00	LT/RT	224.8	58.9	284	EAST OF BRIDGE					
			TOTALS	1411.7	379.9	1792						

		TEMPOR	ARY EF	ROSION	CONTR	.OL	
					ROCK	TYPE C	
				SILT	DITCH	TEMPORARY	SEDIMENT
				FENCE	CHECK	BERM	REMOVAL
LOCATION	STATION	STATION	SIDE	(LF)	(LF)	(LF)	(CY)
ROUTE 46	559+44	561+77	LT		150		10
ROUTE 46	559+44	562+00	RT		165		11
ROUTE 46	561+80	564+60	LT	281			2.8
ROUTE 46	562+04	564+57	RT	254			2.5
ROUTE 46	564+82		LT/RT			150	1.5
ROUTE 46	565+54		LT/RT			126	1.3
ROUTE 46	565+60	567+21	RT	180			1.8
ROUTE 46	565+66	568+05	LT	244			2.4
			TOTALS	959	315	276	33

		EA	RTHWORK							
			UNCLASSIFIED	COMPACTING	EMBANKMENT					
			EXCAVATION	EMBANKMENT	IN PLACE					
LOCATION	STATION	STATION	(CY)	(CY)	(CY)					
ROUTE 46	559+30.00	564+38.02	114	91	2700					
ROUTE 46	565+75.55	566+90.00	108	86	434					
	TOTALS 222 177 3134									

			PAVE	MENT MARKING		
				4" WHITE STANDARD	4" YELLOW STANDARD	
				WATERBORNE PVMT	WATERBORNE PVMT	
				MARKING PAINT	RUMBLE	
				WITH TYPE P BEADS	WITH TYPE P BEADS	STRIP
LOCATION	STATION	STATION	SIDE	(LF)	(LF)	(STA)
ROUTE 46	559+30	567+40		1620	558	
ROUTE 46	559+30	564+35.55	LT			5
ROUTE 46	559+30	564+35.55	RT			5
ROUTE 46	565+78.05	567+40	LT			1.6
ROUTE 46	565+78.05	567+40	RT			1.6
		TOTALS		1620	558	13.2

		REMO	VAL OF II	MPROVE	MENTS		
SHEET	LOCATION	STATION	STATION	SIDE	DESCRIPTION	QUANT I TY	UNITS
4	ROUTE 46	561+11.71	561+35.76	RT	15" CMP	24	LF
4	ROUTE 46	561+11.88	561+35.55	LT	15" RCP	24	LF
4	ROUTE 46	563+99.94		LT	OM-3	1	EA
4	ROUTE 46	564+01.72		RT	OM-3	1	EA
4	ROUTE 46	564+15.55		RT	OM-3	1	EA
4	ROUTE 46	564+20.48		LT	OM-3	1	EA
4	ROUTE 46	564+21.30		RT	OM-3	1	EA
4	ROUTE 46	564+28.05	564+41.00	RT	GUARD RAIL	13	LF
4	ROUTE 46	564+40.11		LT	OM-3	1	EA
4	ROUTE 46	565+71.72	565+86.75	LT	GUARD RAIL	15	LF
4	ROUTE 46	565+72.19	565+86.76	RT	GUARD RAIL	15	LF
4	ROUTE 46	565+73.87		RT	OM-3	1	EA
4	ROUTE 46	565+73.88		LT	OM-3	1	EA
4	ROUTE 46	565+94.26		LT	OM-3	1	EA
4	ROUTE 46	565+95.37		RT	OM-3	1	EA
4	ROUTE 46	566+14.08		LT	OM-3	1	EA
4	ROUTE 46	566+14.63		RT	OM-3	1	EA
	INCLUDE ROUT	E W REMOVALS	(SEE NEXT SH	IEET)	TOTAL	1	LS

				GU.	ARDRA I L			
						"TYPE A" MASH	SHAPING	
				BRIDGE		CRASHWORTHY	SLOPES	
				ANCHOR	MGS	END	CLASS III	REMARKS
				SECTION	GUARDRAIL	TERMINAL		
LOCATION	STATION	STATION	SIDE	(EA)	(LF)	(EA)	(100F)	
ROUTE 46	562+39.80	564+28.05	RT	1	100	1	0.75	ADJACENT LANE
ROUTE 46	563+02.30	564+28.05	LT	1	37.5	1	0.75	OPPOSING LANE
ROUTE 46	565+85.55	567+11.30	RT	1	37.5	1	0.75	OPPOSING LANE
ROUTE 46	565+85.55	567+73.80	LT	1	100	1	0.75	ADJACENT LANE
			TOTALS	4	275	4	3	

CLEARING AND GRUBBING											
		CLEARING AND									
		GRUBB I NG									
LOCATION STATION	ION STATION STATION SID		ACRE	REMARKS							
ROUTE 46 559+30.00	564+15.55	LT/RT	0.5	WEST OF BRIDGE							
ROUTE 46 565+98.05	567+73.80	LT/RT	0.2	EAST OF BRIDGE							
		TOTAL	1								

ROUTE 46 SUMMARY OF QUANTITIES SHEET 1 OF 3

	1 16 N	DATE // PRICE W JI CON	COULTRA	NTY RTI NO	ST. M SHEE	24 IO T NO	D.
DATE DESCRIPTION							
MISSOURI HIGHWAYS AND TRANSPORTATION DA	COMMISSION		Modol		105 WEST CAPITOL	JEFFERSON CITY, MO 65102	1-888-ASK-MODOT (1-888-275-6636)

	ENT	RANCE:	S						
			GRAVEL A						
			CRUSHED STONE B						
			4 "						
LOCATION	STATION	SIDE	(SY)						
ROUTE W	190+94.15	LT	52.1						
		TOTALS	52						

	BASE AND PAVEMENT											
						TYPE 1 AGG.						
				OPTIONAL	TYPE A3	FOR BASE						
				PAVEMENT	SHOULDER	(4 IN. THICK)						
LOCATION	STATION	STATION	SIDE	(SY)	(SY)	(SY)	REMARKS					
ROUTE W	185+19.70	185+78.73	LT/RT	132.4	12.9	145	WEST OF BRIDGE					
ROUTE W	189+01.27	195+00.00	LT/RT	1329.5	255.0	1585	EAST OF BRIDGE					
			TOTALS	1461.9	267.9	1730						

	EARTHWORK										
			UNCLASSIFIED	COMPACTING	EMBANKMENT	COMPACTING					
			EXCAVATION	EMBANKMENT	IN PLACE	IN CUT	REMARKS				
LOCATION	STATION	STATION	(CY)	(CY)	(CY)	(STA)					
ROUTE W	185+19.70	185+98.73	140	2		1	WEST OF BRIDGE				
ROUTE W	193+95.00	195+00.00	263	210	681	1.1	EAST OF BRIDGE				
·	TOTALS 403 212 681 2.1										

		REM	OVAL OF IM	IPROVE	MENTS		
SHEET	LOCATION	STATION	STATION	SIDE	DESCRIPTION	QUANTITY	UNITS
5	ROUTE W	185+67.91		LT	OM-3	1	EA
5	ROUTE W	185+68.32		RT	OM-3	1	EA
5	ROUTE W	185+88.05		LT	OM-3	1	EA
5	ROUTE W	185+88.28		RT	OM- 3	1	EA
5	ROUTE W	186+07.60		RT	OM- 3	1	EA
5	ROUTE W	186+07.68		LT	OM- 3	1	EA
5	ROUTE W	186+08.15	188+70.94	RT	GUARDRA I L	263	LF
5	ROUTE W	186+08.51	188+70.50	LT	GUARDRA I L	262	LF
5	ROUTE W	188+70.55		RT	OM- 3	1	EA
5	ROUTE W	188+70.96		LT	OM- 3	1	EA
5	ROUTE W	188+90.62		LT	OM-3	1	EA
5	ROUTE W	189+11.29		LT	OM-3	1	EA
5	ROUTE W	189+11.34		RT	OM-3	1	EA

		CLEARING	AND (GRUBB I NG	
				CLEARING AND	
				GRUBB I NG	
LOCATION	STATION	STATION	SIDE	ACRE	REMARKS
ROUTE W	185+19.70	185+80.49	LT/RT	0.1	WEST OF BRIDGE
ROUTE W	188+99.15	195+00.00	LT/RT	0.4	EAST OF BRIDGE
			TOTAL	1	

		SE	ED AND MULCH					
	SEEDING - COOL							
			SEASON GRASSES	MULCHING				
LOCATION	STATION	STATION	(ACRE)	(ACRE)	REMARKS			
ROUTE W	185+19.70	185+80.49	0.1	0.1	WEST OF BRIDGE			
ROUTE W	188+99.15	195+00.00	0.4	0.4	EAST OF BRIDGE			
		TOTALS	0.5	0.5				

_						
			PERMAN	ENT EROSION	CONTROL	
ſ				FURNISHING	PLACING	PERMANENT
-				TYPE 2	TYPE 2	EROSION CONTROL
-				ROCK BLANKET	ROCK BLANKET	GEOTEXTILE
1	LOCATION	STATION	SIDE	(CY)	(CY)	(SY)
	ROUTE W	186+01.27	LT/RT	176	176	264
	ROUTE W	188+78.73	LT/RT	172	172	257
	ROUTE W	196+40.11	RT	22	22	33
ſ			TOTALS	370	370	554

	TEN	/IPORARY	EROS	ION CO	NTROL	
					TYPE C	
				SILT	TEMPORARY	SEDIMENT
				FENCE	BERM	REMOVAL
LOCATION	STATION	STATION	SIDE	(LF)	(LF)	(CY)
ROUTE W	185+19	186+00	LT	87		0.9
ROUTE W	185+19	186+04	RT	88		0.9
ROUTE W	186+36		LT/RT		103	1.0
ROUTE W	188+60		LT/RT		106	1.1
ROUTE W	188+76	195+10	LT	683		6.8
ROUTE W	188+82	195+10	RT	634		6.3
			TOTALS	1492	209	17

			PAVE	MENT MARKING		
				4" WHITE STANDARD	4" YELLOW STANDARD	
				WATERBORNE PVMT	WATERBORNE PVMT	
				MARKING PAINT	MARKING PAINT	RUMBLE
				WITH TYPE P BEADS	WITH TYPE P BEADS	STRIP
LOCATION	STATION	STATION	SIDE	(LF)	(LF)	(STA)
ROUTE W	185+19.70	195+00.00		1961	1961	
ROUTE W	185+19.70	185+96.81	LT			7.7
ROUTE W	185+19.70	186+00.85	RT			8.1
ROUTE W	188+79.15	195+00	LT			62.1
ROUTE W	188+83.39	195+00	RT			61.7
		TOTALS		1961	1961	139.6



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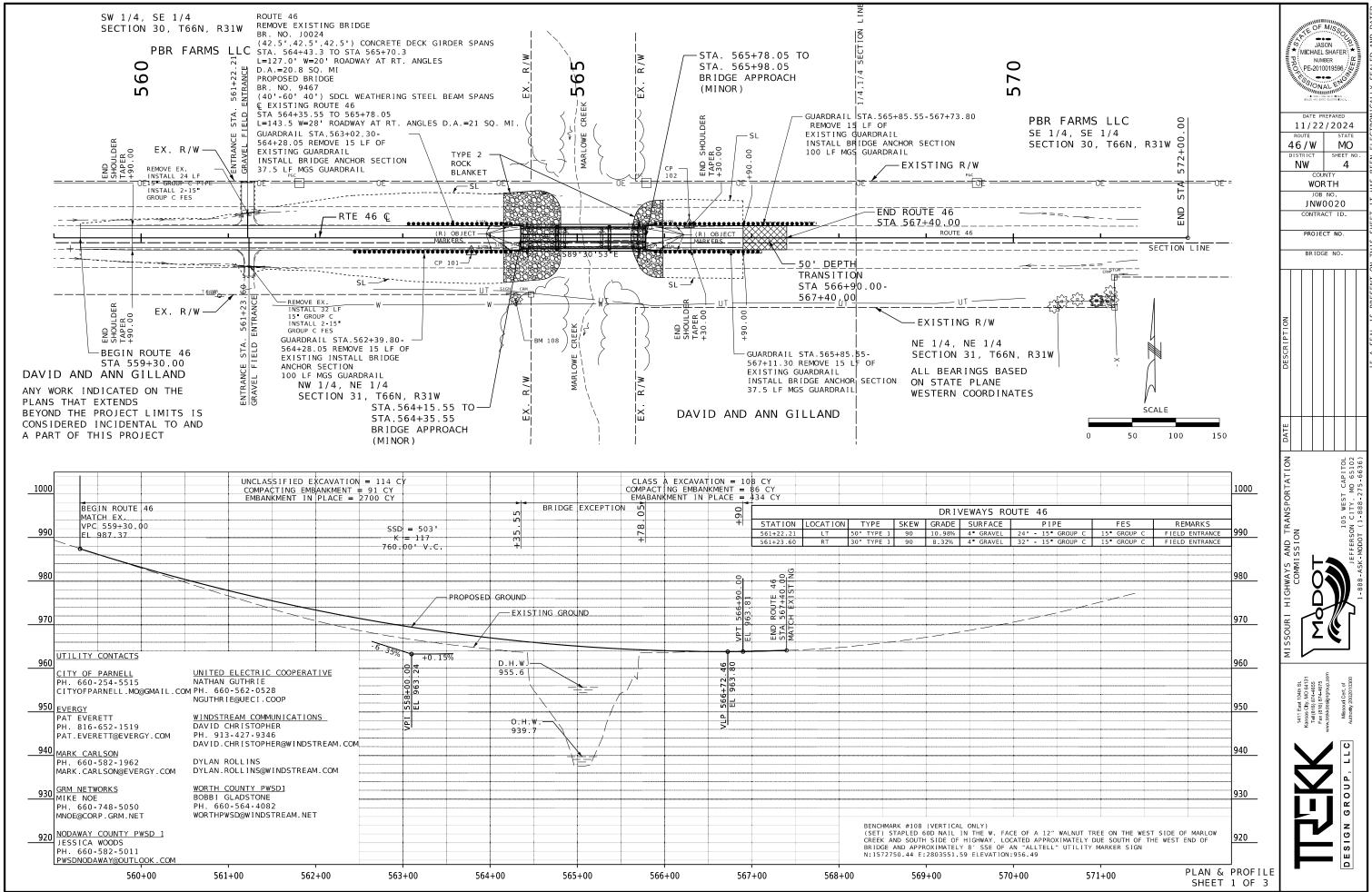


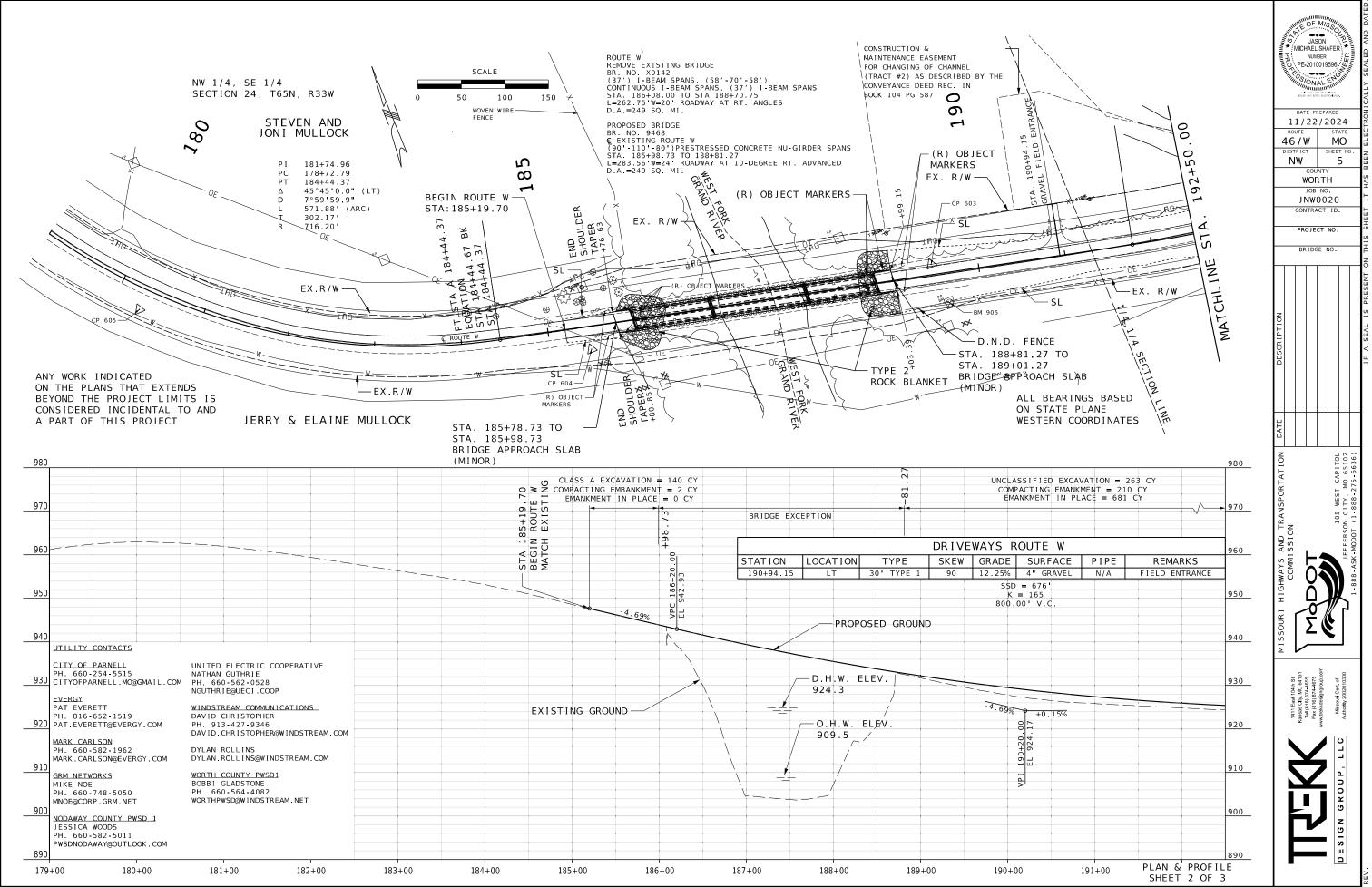
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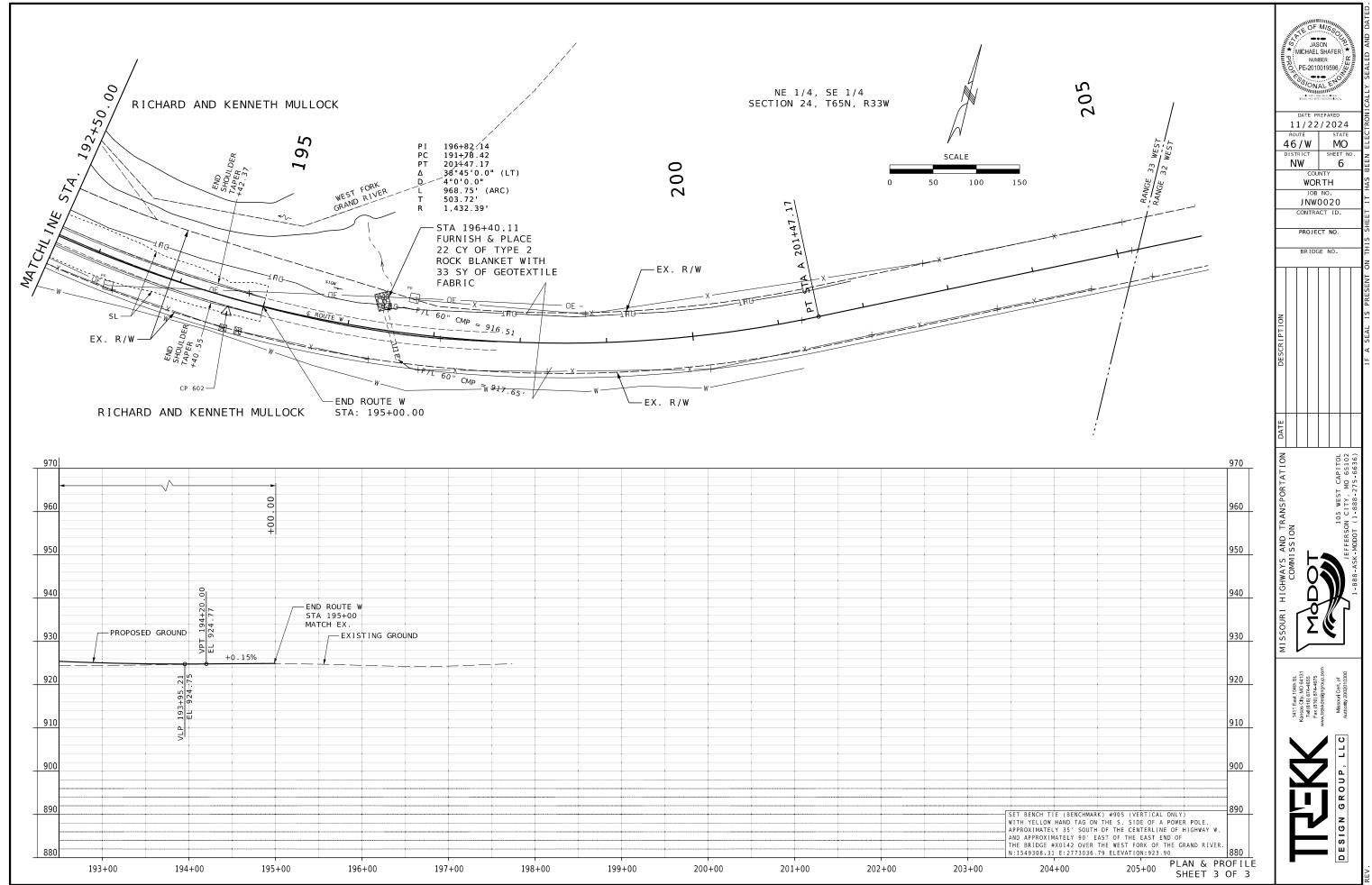


ROUTE W SUMMARY OF QUANTITIES SHEET 2 OF 3

TOTAL QTY TOTAL	SIGN	\Box				QTY	TOTAL S I	GN		$\overline{}$	EFFECTIVE: 07-01-2024		
SIZE AREA QTY AREA RELOCRELOC	l I		SIZE	AREA	QTY TOTAL		I	•	ITEM			ininii.	OF M/S
SIGN IN. SQ.FT. EACH SQ.FT. EACH SQ.FT.	DESCRIPTION	SIGN	I N	SQ.FT.	EACH SQ.FT.			DESCRIPTION	NUMBER	QTY	DESCRIPTION	MINITALE.	
WARNING SIGNS		1				JIDE SIG	NS		6122008		IMPACT ATTENUATOR 40 MPH (SAND BARRELS)	<u> ξ</u> ο .	JASON - 22 € CHAEL SHAFER: ★
WO1-1L 48X48 16.00	TURN (SYMBOL LEFT)	_		12.00				GORE EXIT	6122009		IMPACT ATTENUATOR 45 MPH (SAND BARRELS)	₽ ₽:	NUMBER :
WO1-1R 48X48 16.00 WO1-2L 48X48 16.00	TURN (SYMBOL RIGHT)	E05-2		12.00		++		EXIT OPEN	6122010		IMPACT ATTENUATOR SE MULL (SAND BARRELS)	5 M.	E-2010019596. 🕡
WO1-2L	CURVE (SYMBOL LEFT) CURVE (SYMBOL RIGHT)	_		12.00		+		ROAD WORK NEXT XX MILES	6122012		IMPACT ATTENUATOR 55 MPH (SAND BARRELS) IMPACT ATTENUATOR 60 MPH (SAND BARRELS)	THINIS IC	DONAL ENGINEER
WO1-3L 48X48 16.00	REVERSE TURN (SYMBOL LEFT)	_		8 00		+		END ROAD WORK	6122017	\rightarrow	IMPACT ATTENUATOR 65 MPH (SAND BARRELS)	148 S SE/LEO A	VINCHORDISE CENTE CON COL
WO1-3R 48X48 16.00	REVERSE TURN (SYMBOL RIGHT)		36X18			+ +		PILOT CAR FOLLOW ME	6122019	\rightarrow	IMPACT ATTENUATOR 70 MPH (SAND BARRELS)	D.4.T.5	ATE PREPARED
WO1-4L 48X48 16.00	REVERSE CURVE (SYMBOL LEFT)	GO20-4a						PILOT CAR IN USE WAIT & FOLLOW	6122020	\rightarrow	REPLACEMENT SAND BARREL		/22/2024
WO1-4R 48X48 16.00	REVERSE CURVE (SYMBOL RIGHT)	GO20-4a	18X12	1.50				PILOT CAR IN USE WAIT & FOLLOW	6122030		IMPACT ATTENUATOR (RELOCATION)	ROUTE	
WO1-4bL 48X48 16.00	DOUBLE ARROW REVERSE CURVE (SYMBOL LEFT)	GO20-5aP	36X24	6.00				WORK ZONE (PLAQUE)	6123001		TRUCK MOUNTED ATTENUATOR (TMA)	46/W	
WO1-4bR 48X48 16.00	DOUBLE ARROW REVERSE CURVE (SYMBOL RIGHT)			3.00				END DETOUR	6161008		ADVANCED WARNING RAIL SYSTEM	DISTRICT	l l
WO1-4cL 48X48 16.00	TRIPLE ARROW REVERSE CURVE (SYMBOL LEFT)			12.00				DETOUR (LEFT)	6161012		BUOYS (BOATS KEEP OUT)	NW	COUNTY 3
WO1-4cR 48X48 16.00 WO1-6 60X30 12.50	TRIPLE ARROW REVERSE CURVE (SYMBOL RIGHT) HORIZONTAL ARROW (SYMBOL)			12.00		+		DETOUR (RIGHT) STREET NAME (PLAQUE)	6161013 6161014		BUOYS (NO WAKE) SPECIAL SIGN ASSEMBLY (BOATS KEEP OUT)		WORTH
WO1-6a 72X36 18.00	HORIZ. ARROW (SYMBOL ON PERMANENT BARRICADE)			6.00		+		DETOUR ARROW (LEFT)	6161025		CHANNELIZER (TRIM LINE)		JOB NO.
WO1-7 60X30 12.50	DOUBLE HEAD HORIZONTAL ARROW (SYMBOL)	MO4 - 10R				+		DETOUR ARROW (RIGHT)	6161030		TYPE III MOVEABLE BARRICADE		NW0020
WO1-7a 72X36 18.00	DOUBLE HEAD HORIZ. ARROW (SYMBOL ON PERM. BARR.)			1	REGUL	LATORY S	SIGNS		6161033		DIRECTION INDICATOR BARRICADE	CON	ONTRACT ID.
WO1-8 18X24 3.00	CHEVRON (SYMBOL)	R1-1	48X48	13.25				STOP	6161040		FLASHING ARROW PANEL	PRO	ROJECT NO.
WO1-8a 30X36 7.50	CHEVRON (SYMBOL FOR DIVIDED HIGHWAYS)			. 6.93				YIELD	6161047		TYPE III OBJECT MARKER		
WO3-1 48X48 16.00	STOP AHEAD (SYMBOL)			9.00				TO ONCOMING TRAFFIC (PLAQUE)	6161055		SEQUENTIAL FLASHING WARNING LIGHT	BR	BRIDGE NO.
WO3-2 48X48 16.00	YIELD AHEAD (SYMBOL)	R1-3P		2.50		++		ALL WAY (PLAQUE)	6161070		TUBULAR MARKER		
WO3-3 48X48 16.00	SIGNAL AHEAD (SYMBOL)	R2-1		12.00		+		SPEED LIMIT XX	6161095		RADAR SPEED ADVISORY SYSTEM		
WO3-4 48X48 16.00 WO3-5 48X48 16.00	BE PREPARED TO STOP SPEED LIMIT AHEAD	R3-1 R3-2		16.00		+		NO RIGHT TURN (SYMBOL) NO LEFT TURN (SYMBOL)	6161096		CHANGEABLE MESSAGE SIGN, COMMISSION FURNISHED/RETAINED		
WO4-1L 48X48 16.00	MERGE (SYMBOL FROM LEFT)			9.00		+		NO TURNS	1 0101090		CHANGEABLE MESSAGE SIGN WITHOUT COMM.		
WO4-1E 48X48 16.00	MERGE (SYMBOL FROM RIGHT)			16.00		+		NO U-TURN (SYMBOL)	6161098A		INTERFACE - CONTRACTOR FURNISHED/RETAINED		
WO4-1aL 48X48 16.00	MERGE (LEFT)	R3-7L		6 25		+ +		LEFT LANE MUST TURN LEFT	1		CHANGEABLE MESSAGE SIGN WITH COMM.	011	
WO4-1aR 48X48 16.00	MERGE (RIGHT)	R3-7R	30X30	6.25				RIGHT LANE MUST TURN RIGHT	6161099	5	INTERFACE - CONTRACTOR FURNISHED/RETAINED	.d	
WO5-1 48X48 16.00	ROAD/BRIDGE/RAMP NARROWS	R4-1	36X48	12.00				DO NOT PASS	6162000A		WORK ZONE TRAFFIC SIGNAL SYSTEM	SCR	
WO5-3 48X48 16.00	ONE LANE BRIDGE	R4-2		12.00				PASS WITH CARE	6162002		TEMPORARY LONG-TERM RUMBLE STRIPS	DE	
WO5-5 48X48 16.00	NARROW LANES	R4-7a		12.00		++		KEEP RIGHT (HORIZONTAL ARROW)	_	,	TEMPORARY TRAFFIC BARRIER		
WO6-1 48X48 16.00	DIVIDED HIGHWAY (SYMBOL)	R4-8a		12.00				KEEP LEFT (HORIZONTAL ARROW)	6173600D		CONTRACTOR FURNISHED/RETAINED		
W06-2 48X48 16.00 W06-3 48X48 16.00	DIVIDED HIGHWAY END (SYMBOL) TWO WAY TRAFFIC (SYMBOL)	R5-1 R5-1a		6.25		+		DO NOT ENTER WRONG WAY	6173602B		TEMPORARY TRAFFIC BARRIER CONTRACTOR FURNISHED/COMMISSION RETAINED		
WO7-3a 30X24 5.00	NEXT XX MILES (PLAQUE)			6.75		++		ONE WAY ARROW (LEFT)	6174000A		TEMP. TRAFFIC BARRIER HEIGHT TRANSITION		++++
WO8-1 48X48 16.00	BUMP			6.75		+		ONE WAY ARROW (RIGHT)	6175010A		RELOCATING TEMPORARY TRAFFIC BARRIER	삗ᅵᅵ	
WO8-2 48X48 16.00	DIP			5.00				ONE WAY (LEFT)		\rightarrow	TEMPORARY TRAFFIC BARRIER	DA.	
WO8-3 48X48 16.00	PAVEMENT ENDS	R6-2R	24X30	5.00				ONE WAY (RIGHT)	6176000В		COMMISSION FURNISHED/RETAINED		\bot
WO8-4 48X48 16.00	SOFT SHOULDER	R9-9	24X12	2.00				SIDEWALK CLOSED			TEMP. TRAFFIC BARRIER HEIGHT TRANSITION	NO	-0L
WO8-5 48X48 16.00	SLIPPERY WHEN WET (SYMBOL)	_	1					SIDEWALK CLOSED AHEAD,	6177000B		COMMISSION FURNISHED/RETAINED	I –	P11
WO8-6 48X48 16.00	TRUCK CROSSING	R9-11L	24X18	3.00				(ARROW LEFT) CROSS HERE	6208064A		TEMPORARY RAISED PAVEMENT MARKER	ΥT	CAF MO 67
WO8-6c 48X48 16.00 WO8-7 36X36 9.00	TRUCK ENTRANCE	-	24X18	3.00				SIDEWALK CLOSED AHEAD,	9029400		TEMPORARY TRAFFIC SIGNALS	NO.	TS:
WO8-7 36X36 9.00 WO8-7a 36X36 9.00	LOOSE GRAVEL FRESH OIL / LOOSE GRAVEL	R9-11R R10-6		6.00		++		(ARROW RIGHT) CROSS HERE STOP HERE ON RED (45^ ARROW)	9029401	\longrightarrow	TEMPORARY TRAFFIC SIGNALS AND LIGHTING	NSF	M L
WO8-9 48X48 16.00	LOW SHOULDER	R11-2		_	2 20.00	,+		ROAD CLOSED	1	\rightarrow		RA	105 N CI
WO8-11 48X48 16.00	UNEVEN LANES	1		+		+		ROAD CLOSED XX MILES AHEAD	1			LONO	350
WO8-12 48X48 16.00	NO CENTER LINE	R11-3a	60X30	12.50	3 37.50	ر		LOCAL TRAFFIC ONLY				AND SSI	<u> </u>
WO8-15 48X48 16.00	GROOVED PAVEMENT	R11-4	60X30	12.50				ROAD CLOSED TO THRU TRAFFIC				S	
WO8-15P 30X24 5.00	MOTORCYCLE (PLAQUE)	CONST-3A						FINE SIGN				AY S	
WO8-17L 48X48 16.00	SHOULDER DROP-OFF (SYMBOL LEFT)	CONST-3X	56X12	4.67				SPEEDING/PASSING (PLATE)	Д			≩ŭ (
WO8-17R 48X48 16.00	SHOULDER DROP-OFF (SYMBOL RIGHT)	CONST	10726	12.00		LANEOUS	SIGNS	DOINT OF BRECENCE	4			HIG	ַ עו ה
W08-17P 30X24 5.00 W10-1 42RND. 9.62	SHOULDER DROP-OFF (PLAQUE) RAILROAD CROSSING	CONST-5				+		POINT OF PRESENCE POINT OF PRESENCE	-			I L	
W012-1 24X24 4.00	DOUBLE DOWN ARROW (SYMBOL)	CONST-8				+		WORK ZONE NO PHONE ZONE	-			I W	2 / L
WO12-2 48X48 16.00	LOW CLEARANCE (SYMBOL)	_		_	2 39.00	,+		WORK ZONE NO THONE ZONE	-			ر ا ورا	2 (%)
W012-2x 24X18 3.00	LOW CLEARANCE (PLAQUE)				2 39.00				1			153	
WO12-2a 84X24 14.00	OVERHEAD LOW CLEARANCE (FEET AND INCHES)	_			2 39.00				1				
WO12-4 120X60 50.00	LOW CLEARANCE XX FT XX IN XX MILES AHEAD	50D	36X78	19.50	3 58.50	J.					İ		
WO12-5 120X60 50.00	WIDTH RESTRICTION XX FT XX IN XX MILES AHEAD	50E	36X78	19.50	3 58.50	j						131	55 75 p.com
WO13-1 30X30 6.25	ADVISORY SPEED (PLAQUE)				3 58.50				_			1411 East 104th St. Kansas City, MO 6413 Tel (816) 874-4655	74.466 74.466 ngrou 2ert c
WO16-2 30X24 5.00	XXX FEET (PLAQUE)				1 19.50				4			East 1 Offty, 1	16) 87 16) 8 design souri (
WO16-3 30X24 5.00	X MILE (PLAQUE)	50H	36X/8	19.50	1 19.50	-			4			1411 E nsas Tel (8:	Tel (8: ax (8) trekk Miss
WO20-1	ROAD/BRIDGE/RAMP WORK AHEAD DETOUR AHEAD	1		+		+			-			Ka L	www.
WO20-2 48X48 16.00 2 32.00 WO20-3 48X48 16.00 5 80.00	ROAD CLOSED AHEAD	616-10	05		TOTAL	+	$\overline{}$		j				4 🕟
WO20-4 48X48 16.00 3 80.00	ONE LANE ROAD AHEAD	CONSTRU		N SIGN		_							
WO20-5 48X48 16.00	RIGHT/CENTER/LEFT LANE CLOSED AHEAD	616-10		_ 3.01	510		TOTAL						
WO20-5a 48X48 16.00	2 RIGHT/CENTER/LEFT LANES CLOSED AHEAD	RELOCAT		<u>I G</u> NS			0						
WO20-6a 48X48 16.00	RIGHT/CENTER/LEFT LANE CLOSED											7	
WO20-7a 48X48 16.00	FLAGGER (SYMBOL)	_											
WO21-2 36X36 9.00	FRESH OIL	_											_
WO21-5 48X48 16.00	SHOULDER WORK / SHOULDER WORK AHEAD	_											
WO22-1 48X48 16.00	BLASTING ZONE AHEAD	4								CIII	MMADY OF OUANITITIES		<u> </u>
WO22-2 42X36 10.50 WO22-3 42X36 10.50	TURN OFF 2-WAY RADIO AND PHONE END BLASTING ZONE	4								201	MMARY OF QUANTITIES		<u>-</u>
	I LEND BLASTING ZONE	1									CHEET 3 OF 3		
GO22-1 21X15 2.19	WET PAINT (ARROW PIVOTS)	4									SHEET 3 OF 3		







ROUTE 46 REFERENCE POINTS

GRID NORTH OF THE MISSOURI STATE PLANE COORDINATE

SYSTEM, WESTERN ZONE, NAD-83, NAVD-88

RECIPROCAL AVERAGE GRID FACTOR: 0.999901627

(SCALE FACTOR = 1.0000983827)

RM-2 PK MAG NAIL SET ALONG S. EDGE OF ASPHALT PAVEMENT

RM-1 PK MAG NAIL
SET ALONG S. EDGE
OF ASPHALT PAVEMENT

SET ALONG S. EDGE
OF ASPHALT PAVEMENT.

CONTROL PNT./PROJECT BENCHMARK #100(HORIZONTAL & VERTICAL)

(SET) 5/8" REBAR WITH ALUMINUM "TREKK CONTROL" CAP, ON THE SOUTH SIDE OF HIGHWAY 46,
WEST OF THE WEST END OF BRIDGE #J0024 OVER MARLOW CREEK; LOCATED APPROXIMATELY 31.5'
SOUTH OF THE SOUTHERN EDGE OF ASPHALT, AND APPROXIMATELY 570' WEST OF THE WEST END OF
THE BRIDGE DECK.

COORDINATES
NORTHING = 1,572,781.85
EASTING = 2,802,997.74
ELEVATION = 999.33

RM-3 EAST END
OF A GUARDRAIL
SECTION ON N.
SIDE OF HWY.

RM-2 EASTERN-MOST
DELINEATOR/CAUTION
SIGN ON N. SIDE OF HWY,
E. END OF BRIDGE.

RM-1 EASTERN-MOST DELINEATOR/CAUTION SIGN ON S. SIDE OF HWY, E. END OF BRIDGE.

CONTROL PNT. #102(HORIZONTAL ONLY)

(SET) 5/8" REBAR WITH YELLOW "CONTROL POINT" CAP, ON THE NORTH SIDE OF HIGHWAY 46, EAST OF THE EAST END OF BRIDGE #J0024 OVER MARLOW CREEK; LOCATED APPROXIMATELY 2.75' NORTH OF THE NORTHERN EDGE OF ASPHALT, AND APPROXIMATELY 58' EAST OF THE EAST END OF THE BRIDGE DECK.

COORDINATES
NORTHING = 1,572,834.88
EASTING = 2,803,751.63
ELEVATION = 962.95

RM-1 WESTERN-MOST DELINEATOR/CAUTION SIGN ON N. SIDE OF HWY, W. END OF BRIDGE

RM-2 WESTERN-MOST DELINEATOR/CAUTION SIGN ON S. SIDE OF HWY, W. END OF BRIDGE.

RM-3 "ALLTELL" UTILITY
MARKER SIGN

CONTROL PNT. #101(HORIZONTAL ONLY)

(SET) 5/8" REBAR WITH YELLOW "CONTROL POINT" CAP, ON THE SOUTH SIDE OF HIGHWAY 46, WEST OF THE WEST END OF BRIDGE #J0024 OVER MARLOW CREEK; LOCATED APPROXIMATELY 3' SOUTH OF THE SOUTHERN EDGE OF ASPHALT, AND APPROXIMATELY 65' WEST OF THE WEST END OF THE BRIDGE DECK.

COORDINATES
NORTHING = 1,572,805.88
EASTING = 2,803,501.43
ELEVATION = 963.98



11/22/2024

ROUTE STATE
46/W MO

7

WORTH

JOB NO.
JNW0020

CONTRACT ID.

NW

PROJECT NO.

BRIDGE NO.

URI HIGHWAYS AND TRANSPORTATION

COMMISSION

105 WEST CAPITOL

105 WEST CAPITOL

LEFFERSON CITY, MO 65102

1411 East 104th St. Kansas Ctty, MO 64131 Tel (816) 874-4655 Fax (816) 874-4675 www.trekkdesigngroup.com



REFERENCE POINT SHEET 1 OF 2

ROUTE W REFERENCE POINTS

GRID NORTH OF THE MISSOURI STATE PLANE COORDINATE

SYSTEM, WESTERN ZONE, NAD-83, NAVD-88

RECIPROCAL AVERAGE GRID FACTOR: 0.999901627

(SCALE FACTOR = 1.0000983827)

RM-2 A REFERENCE
COTTON SPINDLE
SET IN WESTERN
SIDE OF GRAVEL RD.

ON EAST SIDE OF GRAVEL RD.

RM-3 A REFERENCE
COTTON SPINDLE
SET IN EASTERN SIDE
OF GRAVEL RD.

CONTROL PNT./PROJECT BENCHMARK #100(HORIZONTAL & VERTICAL)
(SET) 5/8" REBAR WITH YELLOW "CONTROL POINT" CAP,
ALONG THE WEST SIDE OF COUNTY ROAD #87,
APPROXIMATELY 450' FEET SOUTH OF THE CENTERLINE OF
HIGHWAY W, AND APPROXIMATELY 5'
WEST OF THE WESTERN EDGE OF GRAVEL RD.

COORDINATES
NORTHING = = 1,549,090.82
EASTING = 2,774,527.89
ELEVATION = 930.34

RM-2 A REFERENCE
PK MAG NAIL
SET ALONG S. EDGE
OF HWY W.
PAVEMENT.

RM-1 A REFERENCE
PK MAG NAIL
SET ALONG S.
EDGE OF HWY. W
PAVEMENT.

RM-3 A REFERENCE
PK MAG NAIL
SET ALONG S.
EDGE OF HWY W.
PAVEMENT.

CONTROL PNT. #602 (HORIZONTAL ONLY)

(SET) 5/8" REBAR WITH YELLOW "CONTROL POINT" CAP,

ON THE SOUTH SIDE OF HIGHWAY W,

EAST OFTHE EAST END OF BRIDGE #X0142

OVER THE WEST FORK OF GRAND RIVER; LOCATED APPROXIMATELY

6' SOUTH OF THE SOUTHERN EDGE OF ASPHALT, AND APPROXIMATELY 600'

EAST OF THE EAST END OF

THE BRIDGE DECK ALONG THE WEST SIDE OF AN OLD FIELD ENTRANCE.

COORDINATES
NORTHING = 1,549,236.43
EASTING = 2,773,534.81
ELEVATION = 923.67

RM-3 TO A TELEPHONE PEDESTAL AT THE BASE OF WOOD UTILITY POLE.

RM-2 EASTERN-MOST
DELINEATOR/CAUTION
SIGN ON N. SIDE OF HWY
E. END OF BRIDGE

RM-1 EASTERN-MOST
DELINEATOR/CAUTION
SIGN ON S. SIDE OF HWY,
F. END OF BRIDGE

CONTROL PNT. #603 (HORIZONTAL ONLY)

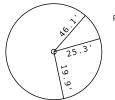
(SET) 5/8" REBAR WITH YELLOW "CONTROL POINT" CAP,
ON THE NORTH SIDE OF HIGHWAY W,
EAST OF THE EAST END OF BRIDGE #X0142

OVER THE WEST FORK OF THE GRAND RIVER; LOCATED

APPROXIMATELY 3' NORTH OF THE NORTHERN EDGE OF ASPHALT,
AND APPROXIMATELY 75' EAST OF
THE EAST END OF THE BRIDGE DECK.

COORDINATES
NORTHING = 1,549,357.58
EASTING = 2,773,033.50
ELEVATION = 928.57

RM-1 TO THE WESTERN-MOST DELINEATOR/CAUTION SIGN ON N. SIDE OF HWY, W. END OF BRIDGE



RM-2 TO THE WESTERN-MOST DELINEATOR/CAUTION SIGN ON S. SIDE OF HWY, W. END OF BRIDGE

RM-3 TO GUY ANCHOR

CONTROL PNT. #604 (HORIZONTAL ONLY)

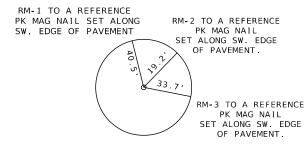
(SET) 5/8" REBAR WITH YELLOW "CONTROL POINT" CAP,
ON THE SOUTH SIDE OF HIGHWAY W,
WEST OF THE WEST END OF BRIDGE #X0142

OVER THE WEST FORK OF THE GRAND RIVER; LOCATED

APPROXIMATELY 16.25' SOUTH OF THE SOUTHERN EDGE OF ASPHALT,
AND APPROXIMATELY 61' WEST

OF THE WEST END OF THE BRIDGE DECK.

COORDINATES
NORTHING = 1,549,413.45
EASTING = 2,772,635.51
ELEVATION = 946.09



CONTROL PNT. #605 (HORIZONTAL ONLY)

(SET) 5/8" REBAR WITH YELLOW "CONTROL POINT" CAP,
ON THE SOUTHWESTERN SIDE OF HIGHWAY
W, WEST OF THE WEST END OF BRIDGE #X0142

OVER THE WEST FORK OF THE GRAND RIVER; LOCATED
APPROXIMATELY 18' SOUTHWEST OF THE SOUTHWESTERN
EDGE OF ASPHALT, AND APPROXIMATELY
580' WEST OF THE WEST END OF THE BRIDGE DECK.

COORDINATES
NORTHING = 1,549,650.85
EASTING = 2,772,171.51
ELEVATION = 960.17



TOTE PREPARED

11/22/2024

ROUTE STATE

46/W MO

DISTRICT SHEET NO.

NW 8

COUNTY

WORTH

JOB NO.
JNW0020
CONTRACT ID.

PROJECT NO

DESCRIPTION

BELDGE NO.

HIGHWAYS AND TRANSPORTATION COMMISSION

1411 East 104th St. Kansas City, MO 64131 Tel (816) 874-4655 Fax (816) 874-4675 www.trekkdesigngroup.com

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REFERENCE POINT SHEET 2 OF 2 ALL PROJECT COORDINATES HAVE BEEN PROJECTED FROM THE MISSOURI STATE PLANE COORDINATE (SPC) SYSTEM OF 1983 USING AN AVERAGE PROJECT PROJECTION (GRID TO GROUND) FACTOR. TO GET BACK TO STATE PLANE COORDINATES MULTIPY THE PROJECT COORDINATES BY THE AVERAGE GRID FACTOR AS SHOWN IN THE "REFERENCE CONTROL INFORMATION" PORTION OF THIS TABLE.

PROJECT COORDIN	ATE INFOR	MATION RTE 46		
COORDINATE SYSTEM	MOD. MO ST	ATE PLANE (GROUND)		
HORIZONTAL DATUM	NAD-83			
VERTICAL DATUM	NAVD-88			
GEOID MODEL	GEOID 18			
ELEVATIONS				
DETERMINED BY	GPS & LEVELS			
PROJECT PROJECTION	FACTOR	1.0000983827		

AL I GNMENTS

558+00.00

572+00.00

RTE 46

RTE 46

REFERENCE CONT	TR	OL INFORM	ATION
COORDINATE SYSTE	М	MISSOURI S	TATE PLANE
CONTROL STATION		CORS	
DESIGNATION	МО	DOT ALBANY	CORS ARP
CORS_ID	МО	AL	
PID	DM	4113	
LATITUDE	Ν4	0°15'20.754	115"
LONGITUDE	WO	94°17'58.90	379
NORTHING (M)	45	3884.8900	
EASTING (M)	86	7039.7900	
ZONE	(2	403) MO WES	ST
PROJECT AVERAGE	GR	ID FACTOR	0.999901627

EXAMPLE OF PROJECT COORDINATE TO S.P.C.

PROJECT NORTHING X AVERAGE GRID FACTOR
= STATE PLANE NORTHING
PROJECT EASTING X AVERAGE GRID FACTOR
= STATE PLANE EASTING

EXAMPLE: CONTROL POINT #100 N 1572781.85 x 0.999901627 = N 1572627.13 E 2802997.74 x 0.999901627 = E 2802722.00

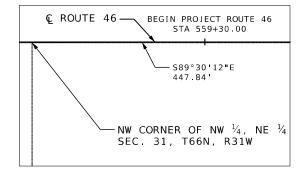
LINEAR UNIT CONVERSION

1 METER = 3.280833333 US SURVEY FEET (USFT)

				COC	ORDINATE POIN	T LISTING RT	TE 46	
				MODIFIED STATE PLANE (GROUND)				
			OFFSET	NORTHING	EASTING	ELEVATION		GPK
SHEET NO	STATION	LOCATION	(USFT)	(US SURVEY FT)	(US SURVEY FT)	(US SURVEY FT)	DESCRIPTION	POINT ID
PROJECT CO	NTROL POINTS							
4	558+74.98	RTE 46	44.07 RT	1,572,781.85	2,802,997.74	999.33	(SET) 5/8" REBAR WITH ALUMINUM "TREKK CONTROL" CAP	CP/BM#100
4	563+78.45	RTE 46	15.77 RT	1,572,805.88	2,803,501.43	963.98	(SET) 5/8" REBAR WITH YELLOW "CONTROL POINT" CAP	CP#101
4	564+29.07	RTE 46	70.79 RT	1,572,750.44	2,803,551.59	956.49	(SET) STAPLED 60D NAIL	BM#108
4	566+28.39	RTE 46	15.34 LT	1,572,834.88	2,803,751.63	962.95	(SET) 5/8" REBAR WITH YELLOW "CONTROL POINT" CAP	CP#102

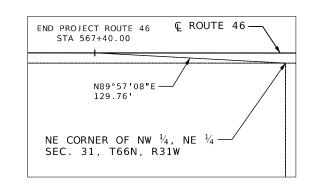
2,802,923.14

2,804,323.09



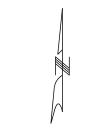
1,572,826.55

1,572,814.69



START

END



NOT TO SCALE

立

JASON

* MICHAEL SHAFER

NUMBER

O PE-2010019596

11/22/2024

WORTH

JOB NO.
JNW0020

CONTRACT ID.

PROJECT NO.

BRIDGE NO.

MO

9

46/W

NW

COORDINATE POINT SHEET 1 OF 2

009_CP_01_JNW0020_I50_RTE-46.dgn 7:46:16 AM 11/22/2024

ALL PROJECT COORDINATES HAVE BEEN PROJECTED FROM THE MISSOURI STATE PLANE COORDINATE (SPC) SYSTEM OF 1983 USING AN AVERAGE PROJECT PROJECTION (GRID TO GROUND) FACTOR. TO GET BACK TO STATE PLANE COORDINATES MULTIPY THE PROJECT COORDINATES BY THE AVERAGE GRID FACTOR AS SHOWN IN THE "REFERENCE CONTROL INFORMATION" PORTION OF THIS TABLE.

PROJECT COORD	INATE INFORMATION RTE W			
COORDINATE SYSTE	MOD. MO STATE PLANE (GROUND)			
HORIZONTAL DATUM	1 NAD-83			
VERTICAL DATUM	NAVD-88			
GEOID MODEL	GEOID 18			
ELEVATIONS				
DETERMINED BY	GPS/OPUS & LEVELS			
PROJECT PROJECTI	ON FACTOR 1.0000983827			
REFERENCE CON	TROL INFORMATION			
COORDINATE SYSTE	MISSOURI STATE PLANE			
CONTROL STATION	CORS			
DESIGNATION	MODOT ALBANY CORS ARP			
CORS_ID	MOAL			
PID	DM4113			
LATITUDE	N40°15'20.75415"			
LONGITUDE	W094°17'58.90379			
NORTHING (M)	453884.8900			

EXAMPLE OF PROJECT COORDINATE TO S.P.C.

(2403) MO WEST

867039.7900

PROJECT AVERAGE GRID FACTOR 0.999901627

PROJECT NORTHING X AVERAGE GRID FACTOR
= STATE PLANE NORTHING
PROJECT EASTING X AVERAGE GRID FACTOR
= STATE PLANE EASTING

EXAMPLE: CONTROL POINT #100 N 1549090.82 x 0.999901627 = N 1548938.43 E 2774527.89 x 0.999901627 = E 2774254.95

LINEAR UNIT CONVERSION

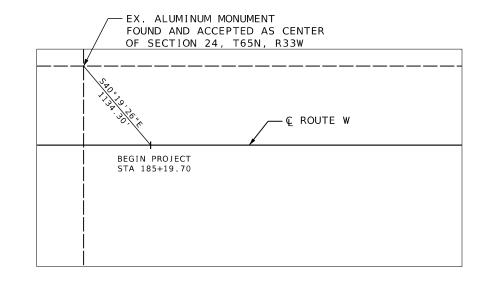
EASTING (M)

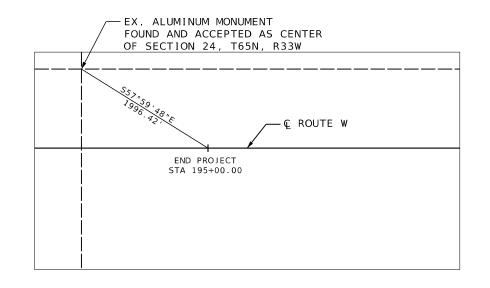
ZONE

1 METER = 3.280833333 US SURVEY FEET (USFT)

				CO	ORDINATE POIN	NT LISTING R	TE W	
	MODIFIED STATE PLANE (GROUND)							
			OFFSET	NORTHING	EASTING	ELEVATION		GPK
SHEET NO	STATION	LOCATION	(USFT)	(US SURVEY FT)	(US SURVEY FT)	(US SURVEY FT)	DESCRIPTION	POINT ID
PROJECT CO	NTROL POINTS)						
5	180+30.75	RTE W	24.27 RT	1,549,650.85	2,772,171.51	960.17	(SET) 5/8" REBAR WITH YELLOW "CONTROL POINT" CAP	CP#605
5	185+46.46	RTE W	26.54 RT	1,549,413.45	2,772,635.51	946.09	(SET) 5/8" REBAR WITH YELLOW "CONTROL POINT" CAP	CP#604
5	189+46.49	RTE W	12.04 LT	1,549,357.58	2,773,033.50	928.57	(SET) 5/8" REBAR WITH YELLOW "CONTROL POINT" CAP	CP#603
5	189+61.19	RTE W	35.10 RT	1,549,308.31	2,773,036.79	923.90	(SET) BENCHTIE WITH YELLOW HAND TAG	BM#905
6	194+60.77	RTE W	16.80 RT	1,549,236.43	2,773,534.81	923.67	(SET) 5/8" REBAR WITH YELLOW "CONTROL POINT" CAP	CP#602
NA	203+15.07	RTE W	408.89 RT	1,549,090.82	2,774,527.89	930.34	(SET) 5/8" REBAR WITH YELLOW "CONTROL POINT" CAP	CP/BM#100
AL I GNMENTS	,							
NA	176+86.17	RTE W	Ē	1,549,953.69	2,771,998.71		START	1
NA	178+72.79	RTE W	Œ	1,549,793.30	2,772,094.13		PC CURVE #1	2
5	181+74.96	RTE W	Ē	1,549,533.62	2,772,248.62		PI CURVE #1	3
5	184+44.37*	RTE W	Ē	1,549,463.08	2,772,542.44		PT CURVE #1	4
5	191+78.42	RTE W	Ē	1,549,291.73	2,773,256.21		PC CURVE #2	5
6	196+82.14	RTE W	Ē	1,549,174.14	2,773,746.02		PI CURVE #2	6
6	201+47.17	RTE W	Ē	1,549,389.02	2,774,201.61		PT CURVE #2	7
NA	213+49.79	RTE W	Œ	1,549,902.03	2,775,289.32		END	8

* THERE IS A STATION EQUATION HERE AT THE PT:184+44.67 (BK.) = 184+44.37 (AHD.)







NOT TO SCALE

JASON

* MICHAEL SHAFER

NUMBER

O PE-2010019596

11/22/2024

WORTH

JOB NO.
JNW0020

CONTRACT ID.

PROJECT NO.

BRIDGE NO.

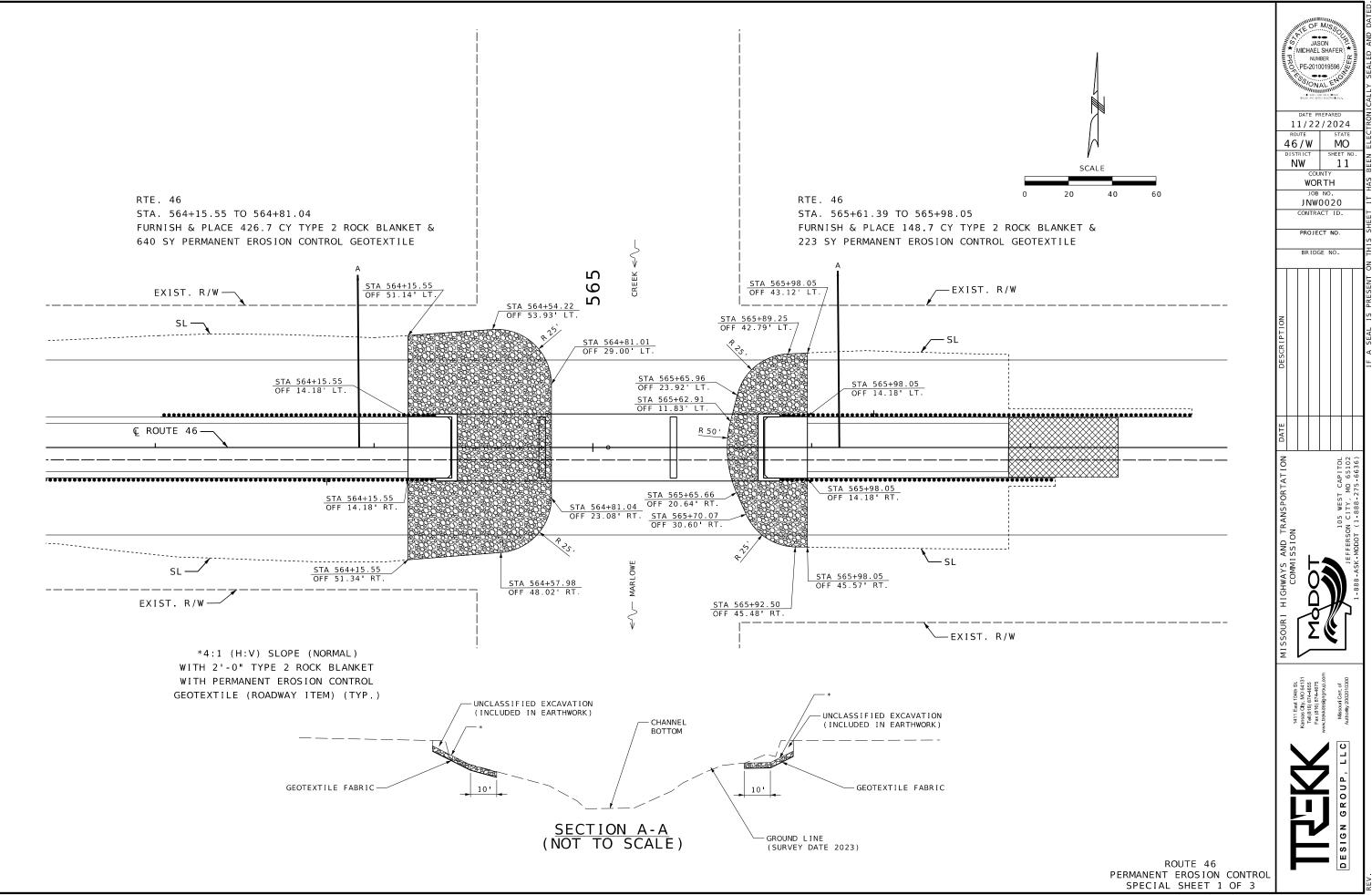
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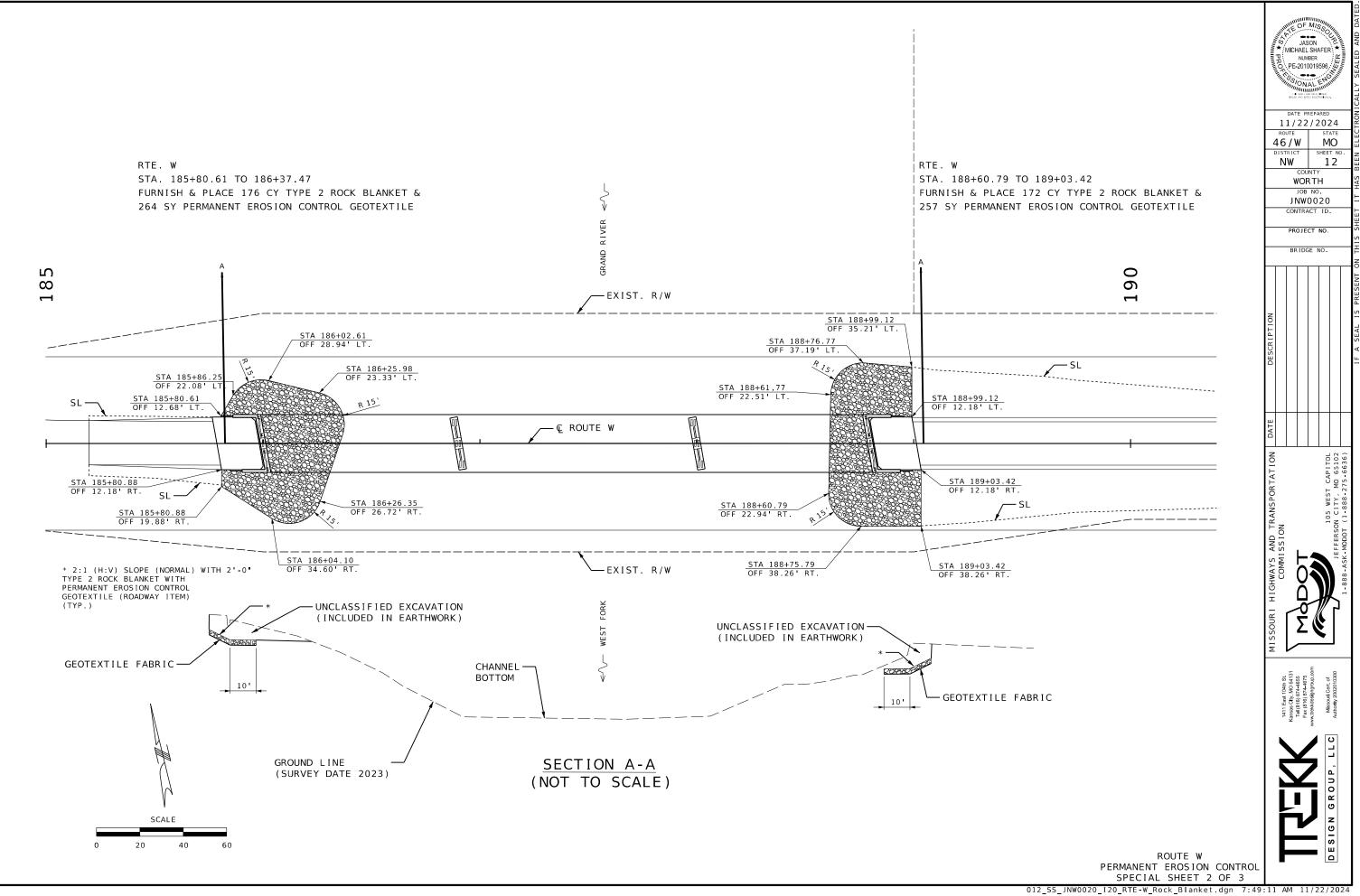
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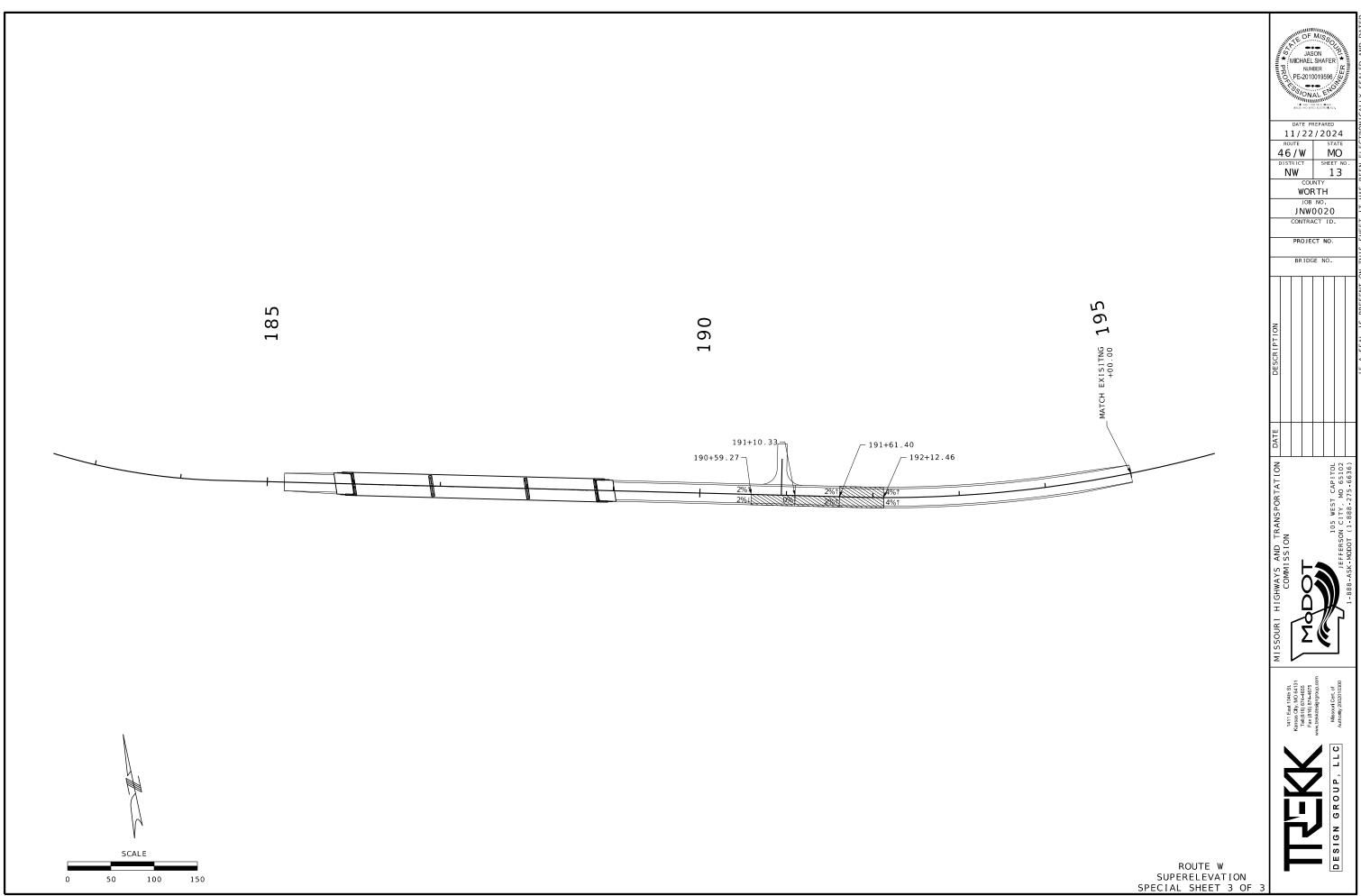
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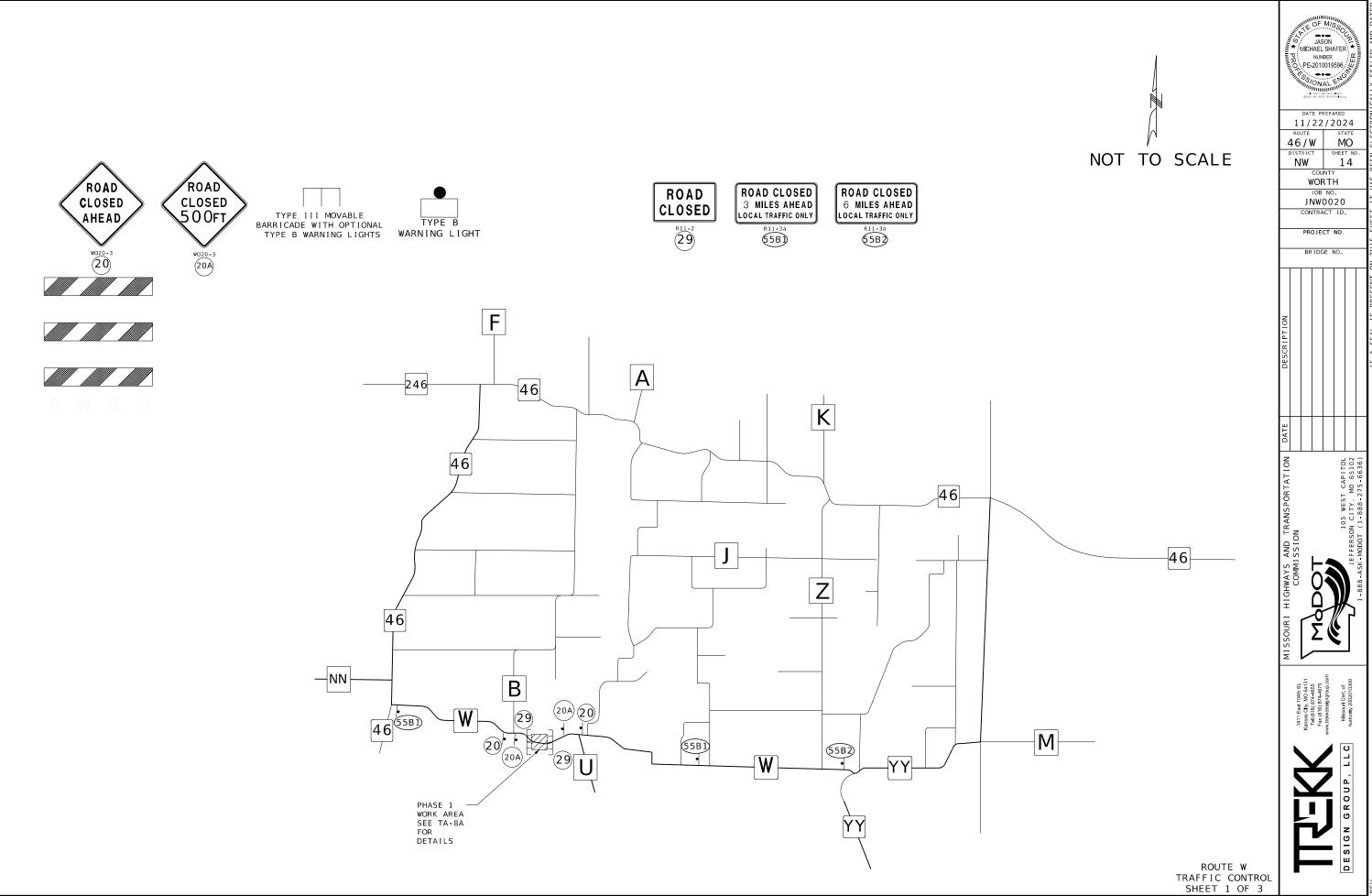
COORDINATE POINT SHEET 2 OF 2



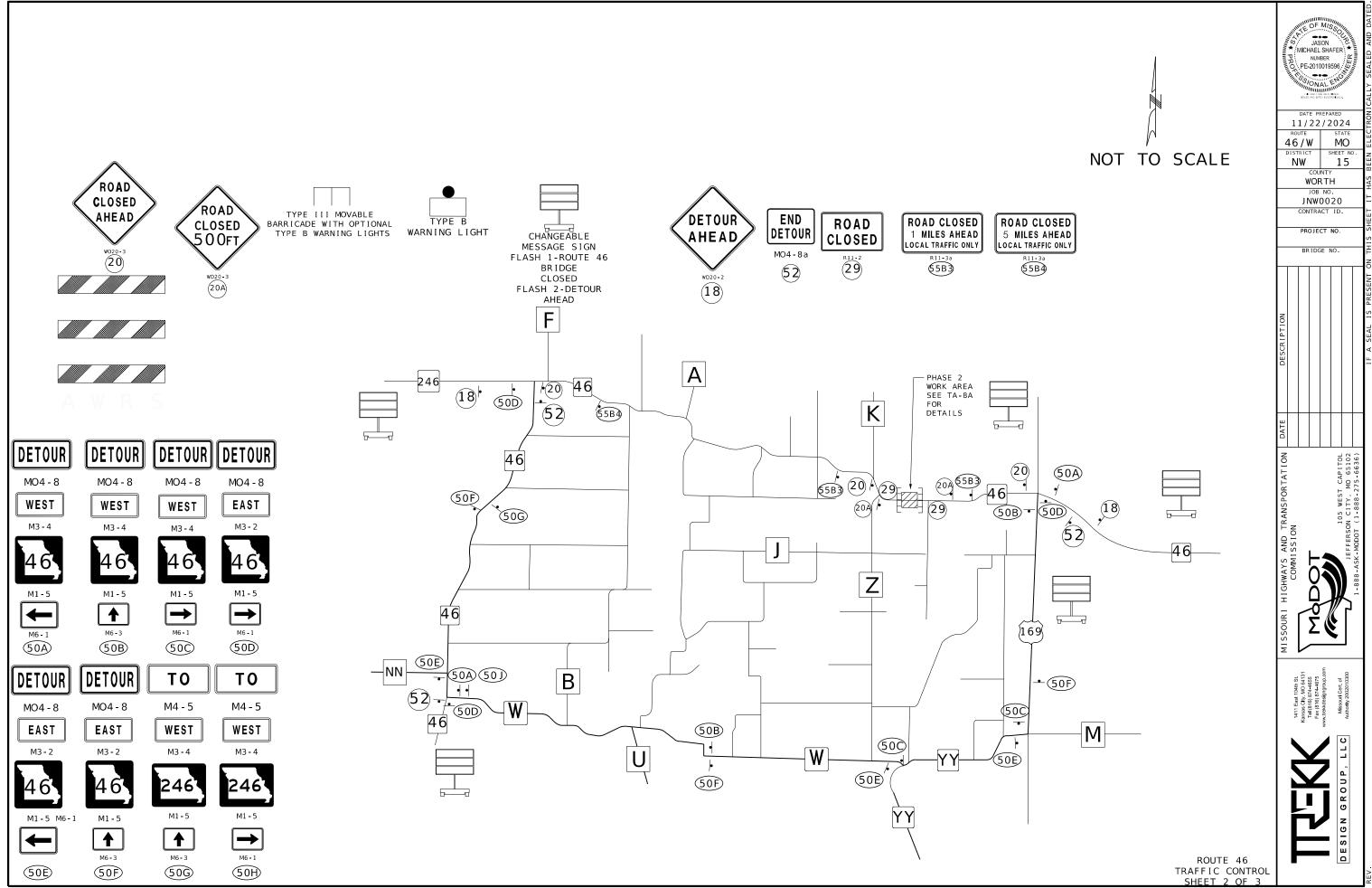




013_SS_JNW0020_Rte_W_Super_Elevation.dgn 7:49:14 AM 11/22/2024

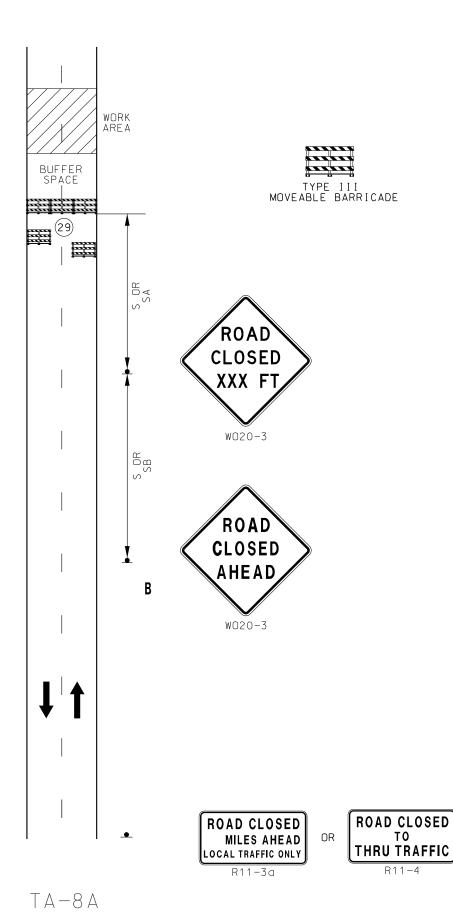


014_TC_01_JNW0020_RteW_Phase1.dgn 7:49:14 AM 11/22/2024



015_TC_02_JNW0020_Rte46_Phase2.dgn 7:49:15 AM 11/22/2024

ROAD CLOSED



NOTES:

WHEN A SIDE ROAD INTERSECTS THE HIGHWAY WITHIN THE TTC ZONE ADDITIONAL TTC DEVICES SHALL BE PLACED AS NEEDED.

"ROAD CLOSED" SIGN MAY BE PLACED 7-10 FEET BEHIND THE BARRICADES AND AT A SIGN HEIGHT APPROPRIATE TO THE TYPE OF ROADWAY. ONE BARRICADE SHOULD BE REQUIRED TO COMPLETELY CLOSE EACH 8-FEET OF PAVEMENT. PAVED SHOULDERS SHALL BE INCLUDED IN THE AREA TO BE CLOSED.

IF USED, THE "ROAD CLOSED XX MILES AHEAD LOCAL TRAFFIC ONLY" OR "ROAD CLOSED TO THRU TRAFFIC" SIGN SHOULD BE LOCATED AT THE FIRST STATE ROUTE OR, UPON THE DISCRETION OF THE ENGINEER, ANY OTHER INTERSECTION IN ADVANCE OF THE CLOSURE, ADDITIONAL BARRICADES MAY BE USED AND OFFSET TO FACILITATE ACCESS FOR WORK VEHICLES, LOCAL TRAFFIC, ETC.

TRAFFIC CONTROL SHOULD BE REMOVED AS SOON AS PRACTICAL AFTER CONDITION FOR THE CLOSURE NO LONGER EXISTS.

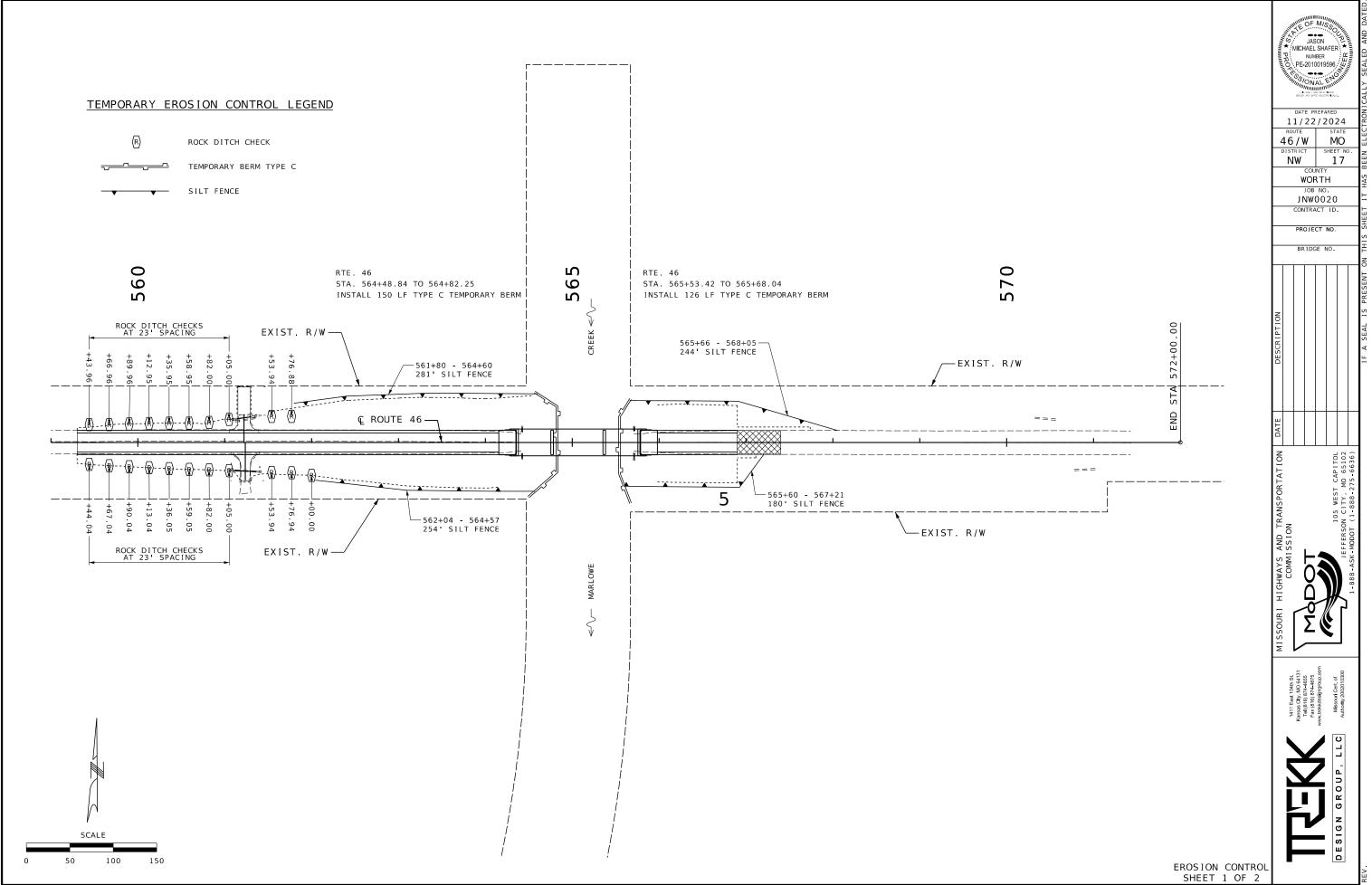
FOR LONG-TERM OPERATIONS, REFER TO EPG 616.6.2.2 FLAG AND ADVANCE WARNING RAIL SYSTEMS (AWRS).

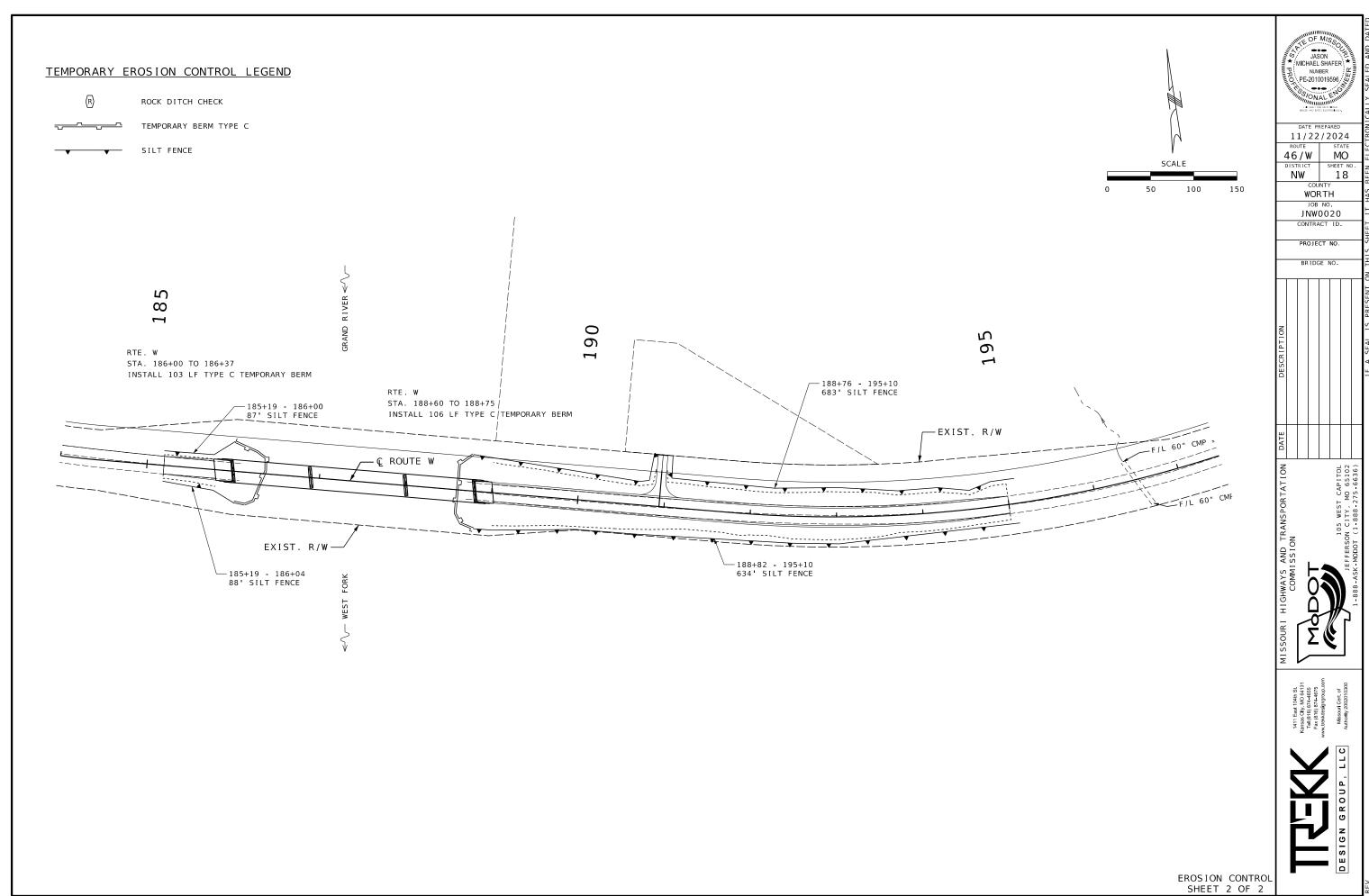
SUPPLEMENTAL WARNING METHODS MAY BE USED TO CALL ATTENTION TO THE WORK ZONE.

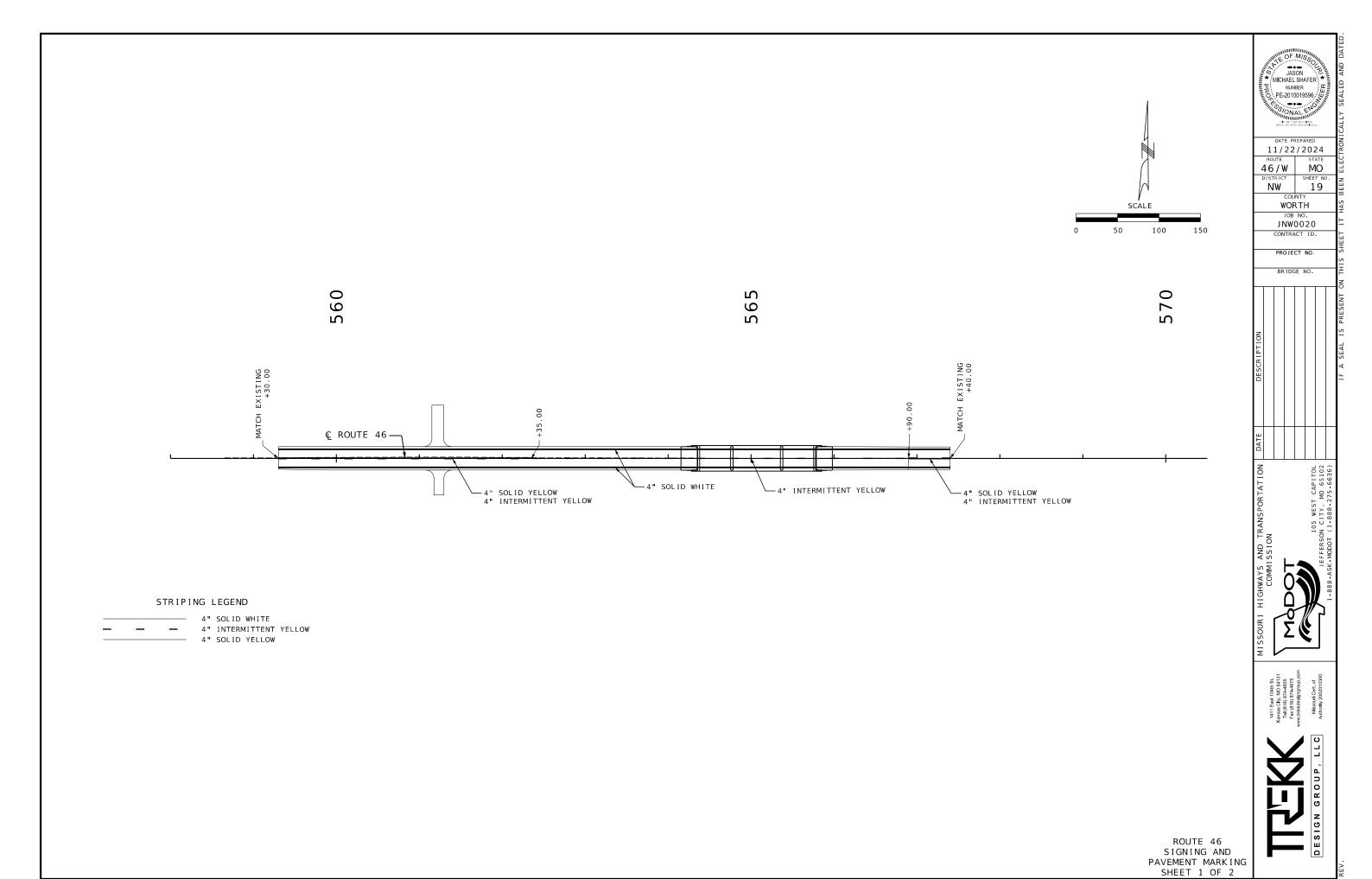
SEE STANDARD PLAN 616.10 FOR USE OF THE BARRICADES AND SIGNS.



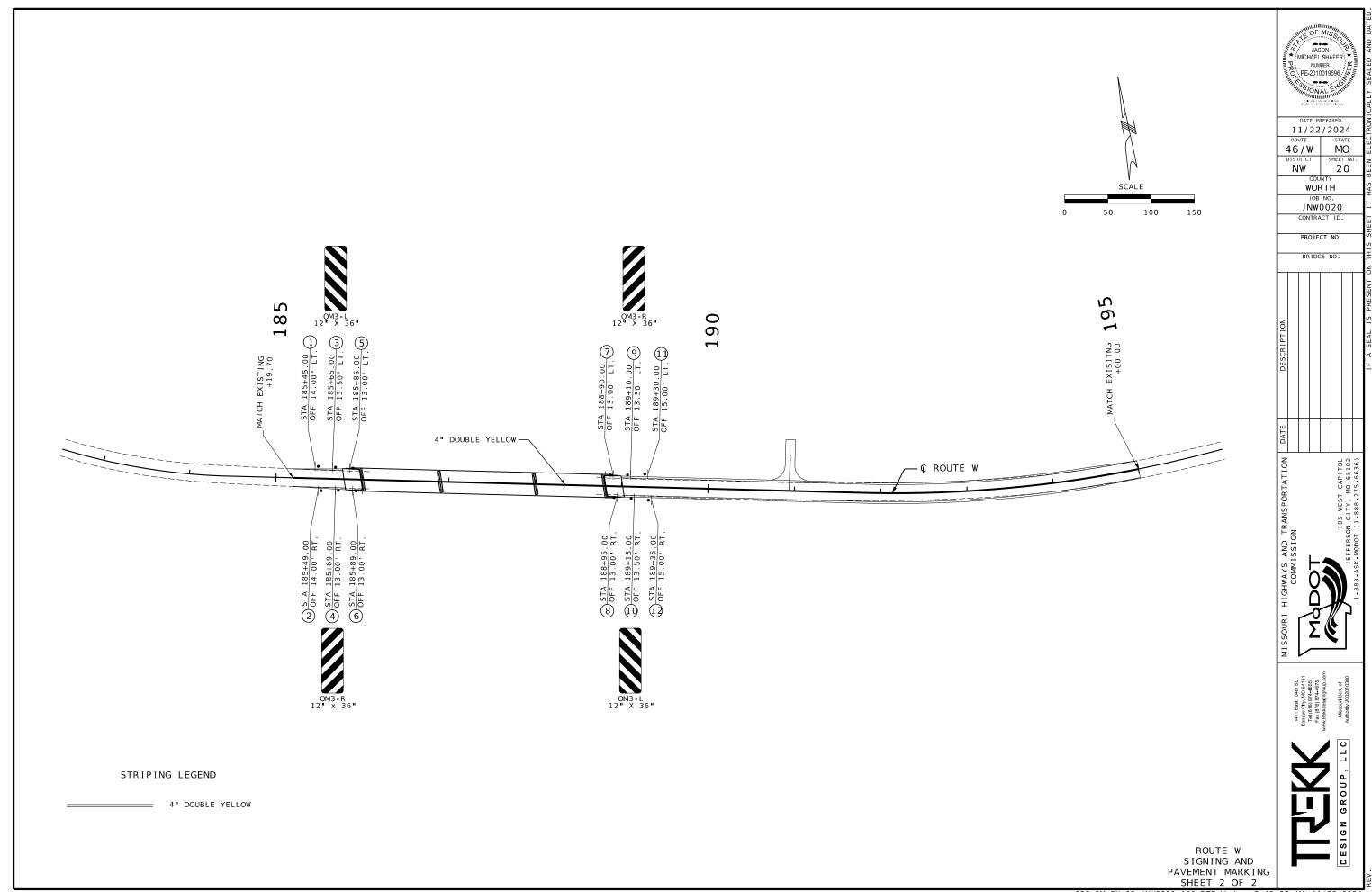
TRAFFIC CONTROL
SHEET 3 OF 3







019_PM_SN_01_JNW0020_I50_RTE-46.dgn 7:49:25 AM 11/22/2024



020_PM_SN_02_JNW0020_I50_RTE-W.dgn 7:49:27 AM 11/22/2024

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Estimated	Quantiti	es		
I t em		Substr.	Superstr.	Total
Class 1 Excavation	cu. yard	95		95
Removal of Bridges (J0024)	lump sum			1
Bridge Approach Slab (Minor)	sq. yard	-	126	126
Galvanized Structural Steel Piles (14 in.)	linear foot	1,376		1,376
Pile Wave Analysis	each	2		2
Pile Point Reinforcement	each	16		16
Class B Concrete (Substructure)	cu. yard	97.7		97.7
Slab on Steel	sq. yard		486	486
Type D Barrier	linear foot		315	315
Reinforcing Steel (Bridges)	pound	4,620		4,620
Temporary coating- Concrete Bents and Pier (Weathering Steel)	lump sum		1	1
Fabricated Structural Low Alloy Steel (I-Beam) A709, Grade 50 W	pound		69,990	69,990
Slab Drain	each		22	22
Vertical Drain at End Bent	each	2		2
Laminated Neoprene Bearing Pad (Tapered)	each		24	24

All concrete between the upper and lower construction joints in the end bents is included in the Estimated Quantities for Slab on Steel.

All reinforcement in the end bents is included in the Estimated Quantitites for Slab on Steel.

All reinforcement in the intermediate bent concrete diaphragms except reinforcement embedded in the beam cap is included in the Estimated Quantities for Slab on Steel

All concrete above the intermediate beam cap is included in the Estimated Quantities for Slab on Steel.

Sheet metal in intermediate concrete diaphragms is subsidiary to Slab on Steel.

Cost of L4x4 ASTM A709 Grade 36 HP pile anchors and 3/4-inch diameter ASTM F3125 Grade A325 Type 1 bolts, complete in place, will be considered completely covered by the contract untilprice for Galvanized Structural Steel Piles (14 in)

Estimated Ouantities for Slab on Steel

I t em	Total
Class B-2 Concrete cu. yard	153
Reinforcing Steel (Epoxy Coated) pound	51,410

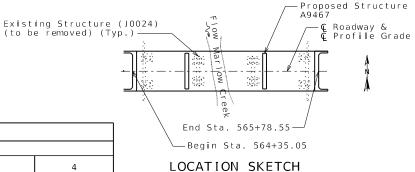
The table of Estimated Quantities for Slab on Steel represents the quantities used by the State in preparing the cost estimate for concrete slabs. The area of the concrete slab will be measured to the nearest square yard longitudinally from end of slab to end of slab and transversely from out to out of bridge slab (or with the horizontal dimensions as shown on the plan of slab). Payment for stay-in-place corrugated steel forms, conventional forms, all concrete and epoxy coated reinforcing steel will be considered completely covered by the contract unit price for the slab. Variations may be encountered in the estimated quantities but the variations cannot be used for an adjustment in the contract unit price

Method of forming the slab shall be as shown on the plans and in accordance with Sec 703. All hardware for forming the slab to be left in place as a permanent part of the structure shall be coated in accordance with ASTM A123 or ASTM B633 with a thickness class SC 4 and a finish type I, II or III.

Slab shall be cast-in-place with conventional forms or stay-in-place corrugated steel forms. Precast prestressed panels will not be permitted.

Bridge deck surface may be finished with a vibratory screed.

The contractor shall provide bracing necessary for lateral and torsional stability of the beams during construction of the concrete slab and remove the bracing after the slab has attained 75% design strength. Contractor shall not weld on or drill holes in the beams. The cost for furnishing, installing, and removing bracing will be considered completely covered by the contract unit price for Slab on Steel.



	Fou	nda	ation Dat	a								
		Bent Number										
Type	Design Data		1	2	3	4						
	Pile Type and Size		HP 14x73	HP 14x73	HP 14x73	HP 14x73						
	Numb e r	еa	4	4	4	4						
	Approximate Length Per Each	ft	88	86	85	85						
Load	Pile Point Reinforcement	еa	AII	AII	AII	All						
Bearing Pile	Min. Galvanized Penetration (Elev.)	ft	939.00	924.00	923.00	937.00						
	Est. Max. Scour Depth 500 (Elev.)	ft	-	934.00	935.00	-						
	Criteria for Min. Tip Penetration		Bear on Rock	Bear on Rock	Bear on Rock	Bear on Rock						
	Pile Driving Verification Method		DF	WEAP	WEAP	DF						
	Resistance Factor		0.4	0.5	0.5	0.4						
	Minimum Nominal Axial Compressive Resistance	kip	448	640	640	448						

DF = FHWA-modified Gates Dynamic Pile Formula WEAP = Wave Equation Analysis

Minimum Nominal Axial Compressive Resistance = <u>Maximum Factored Loads</u> (Side Resistance + Tip Resistance) Resistance Factors

Manufactured pile point reinforcement shall be used on all piles in this

HP Piles are anticipated to be driven to refusal on rock. Review all borings for depth of rock and restrict driving as appropriate to comply with hard rock driving criteria in accordance with Sec 702.

Pile point reinforcement need not be galvanized. Shop drawings will not be required for pile point reinforcement.

Drainage Area = 21 sq. mi. Design Flood Frequency = 50 yr. Design Flood Discharge = 6,500 cfs Design Flood (D.F.) Elevation = 955.6 ft. Base Flood (100-Year) Base Flood Elevation = 956.5 Base Flood Discharge = 7,500 cfs. Estimated Backwater = 0.1 ft Average Velocity thru Opening = 8.5 ft./p Freeboard (50-year) Freeboard = 5.3 ft. Roadway Overtopping Overtopping Flood Discharge = N/A

Overtopping Flood Frequency = >500-yr 500-yr Flood Elevation = 958.5

Hydrologic Data

General Notes:

Design Specifications:

2020 AASHTO LRFD Bridge Design Specification (9th Ed)

Seismic Design Category A (Seismic Details) Design earthquake response spectral acceleration coefficient at 1.0 second

period, SD1 = 0.104g Acceleration Coefficient (Effective peak ground acceleration

coefficient), As = 0.064g

Design Loading: Vehičular = HĬ 93

Future Wearing Surface = 35 lb/sf

Earth = 120 lb/cf, Equivalent Fluid Pressure 45 lb/cf Superstructure: Simply-Supported, Non-Composite for dead load.

Continuous Composite for live load.

Design Unit Stresses:

Class B Concrete (Substructure) Class B-2 Concrete (Superstructure, except Barrier)
Class B-1 Concrete (Barrier)
Reinforcing Steel (ASTM A615 Grade 60)
Structural Steel HP Pile (ASTM A709 Grade 50) f'c = 4,000 psi f'c = 4,000 psify = 60,000 psify = 50,000 psi

Neoprene bearing pads shall be 60 durometer and shall be in accordance with Sec 716

Fabricated Steel Connections:

Field connections shall be made with 3/4-inch diameter ASTM F3125 Grade A325 Type 3 bolts and 13/16-inch diameter holes, except as noted.

Joint Filler: All joint filler shall be in accordance with Sec 1057 for preformed sponge rubber expansion and partition joint filler, except as noted.

Structural Steel Protective Coating:

Protective Coating: System G in accordance with Sec 1080.

Prime Coat: The cost of the inorganic zinc prime coat will be considered completely covered by the contract unit price for the fabricated

Field Coats: The color of the field coats shall be Brown (Federal Standard #30045). The cost of the intermediate and finish field coats will be considered completely covered by the contract unit price for the fabricated

At the option of the contractor, the intermediate and finish field coats may be applied in the shop. The contractor shall exercise extreme care during all phases of loading, hauling, handling, erection and pouring of the slab to minimize damage and shall be fully responsible for all repairs and cleaning of the coating systems as required by the engineer.

Concrete Protective Coatings:

Temporary coating for concrete bents and piers (weathering steel) shall be applied on all concrete surfaces above the ground line or low water elevation on all abutments and intermediate bents in accordance with Sec 711.

Reinforcing Steel:

Minimum clearance to reinforcing steel shall be 11/2", unless

Minimum clearance between galvanized piles and uncoated (plain) reinforcin steel including bar supports shall be 1 1/2". Nylon, PVC, or polyethylene spacers shall be used to maintain clearance. Nylon cable ties shall be use to bind the spacers to the reinforcement.

Traffic Handling:

Structure to be closed to traffic during construction. See roadway plans for traffic control.

High strength bolts, nuts and washers will be sampled for quality assurance as specified in Sec 106

JEFF A. GARDNER NUMBER PE-2016019369

12/3/2024 46 MO SHEET NO BR 2

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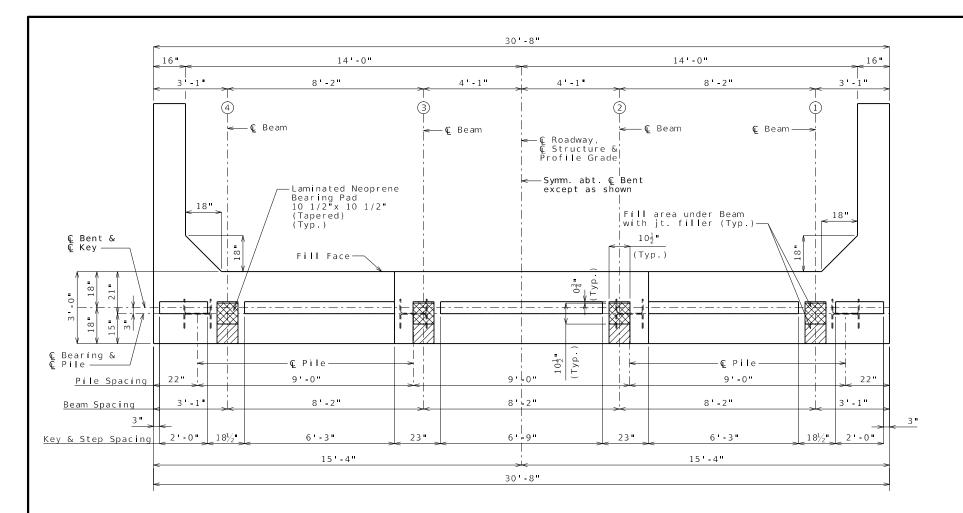
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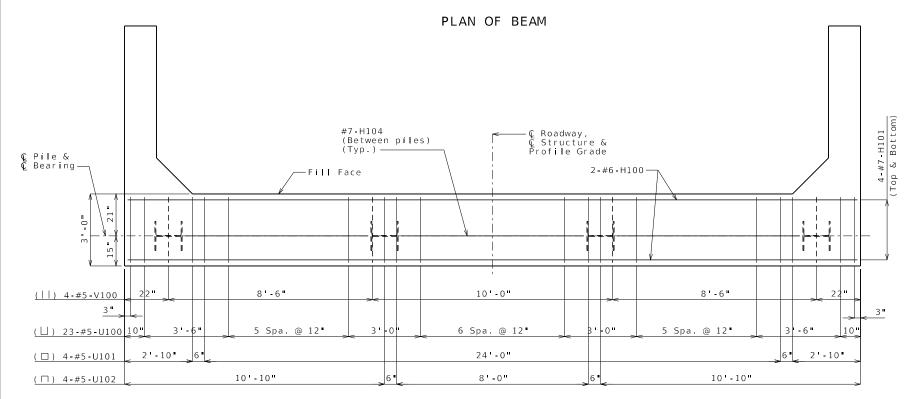
PROJECT NO

BRIDGE NO A9467



GENERAL NOTES AND QUANTITIES





PLAN OF BEAM SHOWING REINFORCEMENT

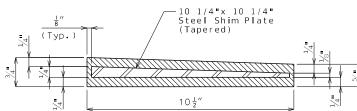
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DETAILS OF END BENT NO. 1

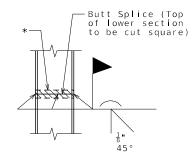
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Sheet No. 3 of 30

Ahead Station>



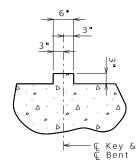
SECTION THRU LAMINATED NEOPRENE BEARING PAD



STEEL PILE SPLICE

(If required)

* Galvanizing material shall be omitted or removed one inch clear of weld locations in accordance with Sec 702.



SECTION THRU KEY

Substructure Quantity Table	for Bent N	lo. 1
I t em		Quantity
Class 1 Excavation	cu. yard	35
Galvanized Structural Steel Piles (14 in.)	linear foot	352
Pile Point Reinforcement	each	4
Class B Concrete (Substructure)	cu. yard	13.7

These quantities are included in the Estimated Quantities table on Sheet No. 2.

General Notes:

The concrete diaphragm at the end bents shall be poured a minimum of 12 hours before the slab is poured.

Work this sheet with Sheets No. 4 & 5.

All U bars and pairs of V bars shall be placed parallel to centerline of roadway.

Reinforcing steel shall be shifted to clear piles. U bars shall clear piles by at least 1 1/2 inches.



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12/3/2024

ROUTE STATE

46 MO

BR 3 COUNTY WORTH JOB NO.

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JNW0020 CONTRACT ID.

PROJECT NO.

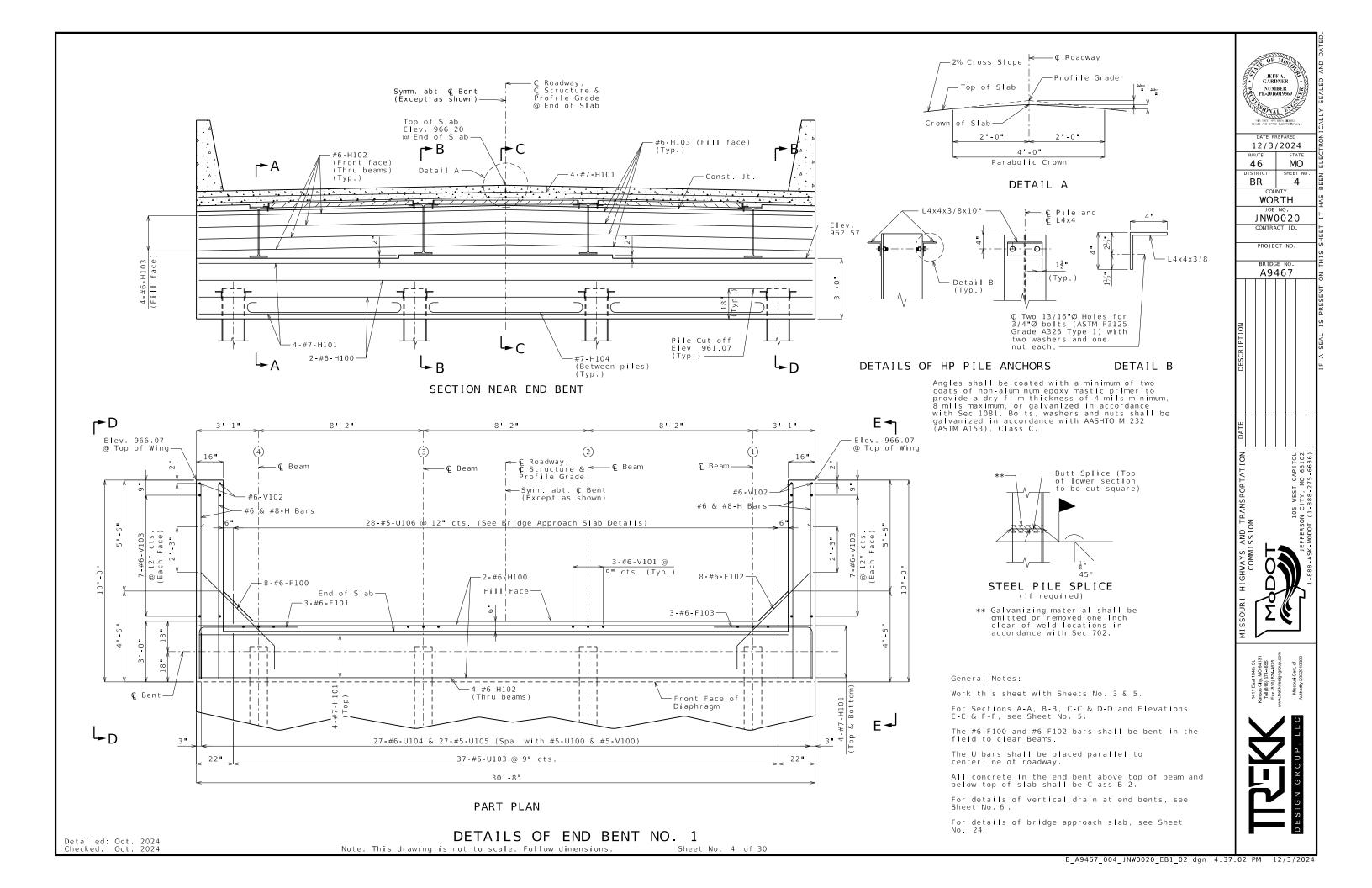
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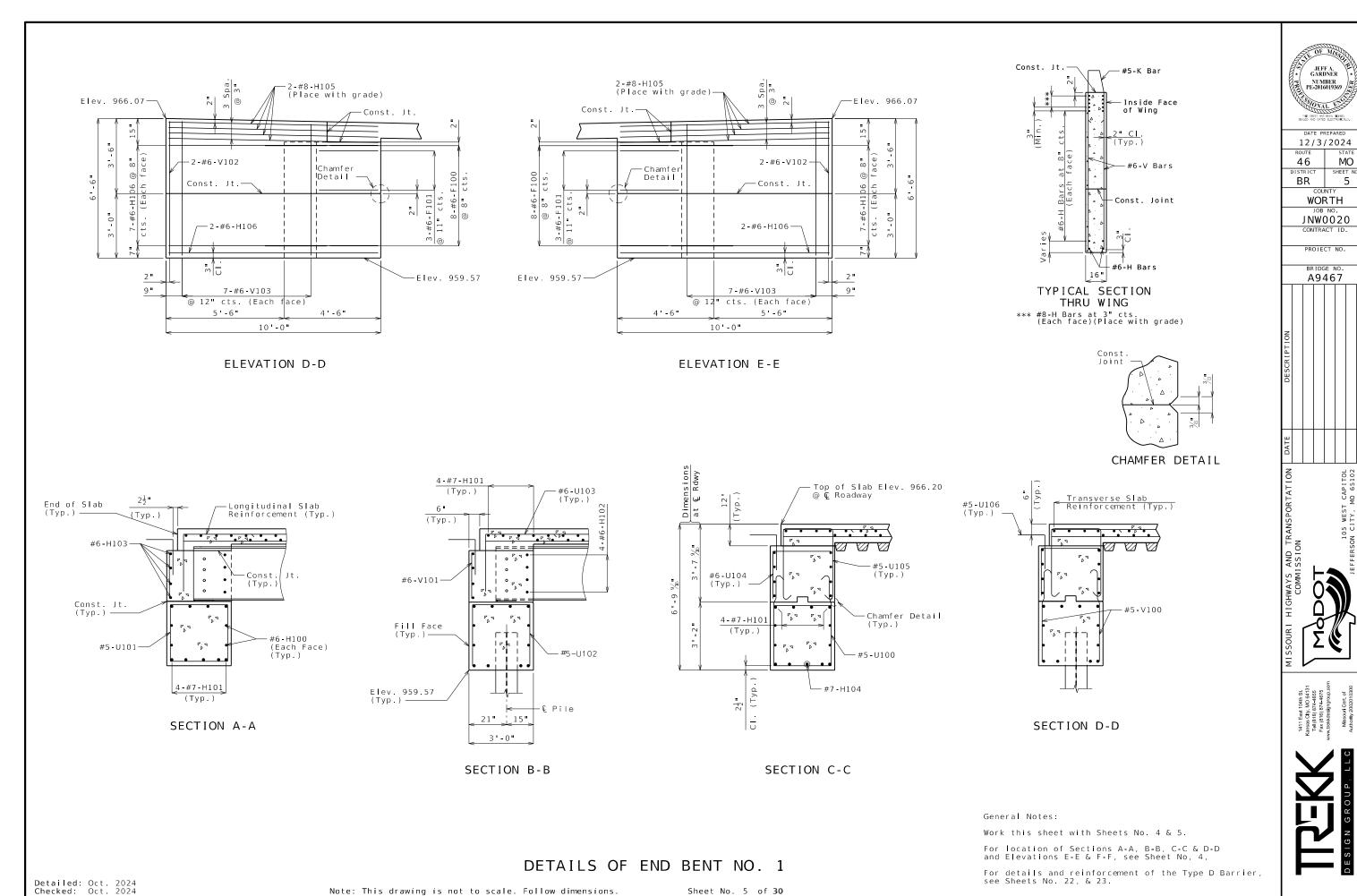
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LEFFERSON CITY, MO 65102

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— Coupler

Coupler

- Perforated

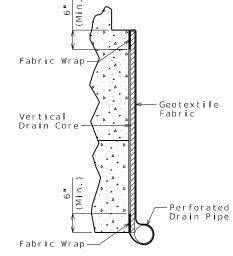
Drain Pipe

(Typ.)

- Unperforated

Drain Pipe

— Unperforated Drain Pipe



PART SECTION A-A (Section thru wing similar)

General Notes:

percent.

All drain pipe shall be sloped 1 to 2

Drain pipe may be either 6-inch diameter corrugated metallic-coated steel pipe underdrain, 4-inch diameter corrugated polyvinyl chloride (PVC) drain pipe, or 4inch diameter corrugated polyethylene (PE)

Drain pipe shall be placed at fill face of end bent and inside face of wings. The pipe shall slope to lowest grade of ground line, also missing the lower beam of end bent by a minimum of 1 1/2 inches.

Perforated pipe shall be placed at fill face side and inside face of wings at the bottom of end bent and plain pipe shall be used where the vertical drain ends to the exit at ground line.

Ground Line Elbow Cut coupler flush with ground line ELEVATION OF WING Perforated Drain Pipe Unperforated Drain Pipe Part Plan

- Cut coupler flush

with ground line-

ELEVATION OF WING

Unperforated

Cut coupler

to slope of ground line

Drain Pipe-

DETAIL A

Rodent Screen-

Detailed: Oct. 2024 Checked: Oct. 2024

OPTIONAL TURNED DRAIN

(Use only when straight drain is not practical.)

VERTICAL DRAIN AT END BENTS

(Squared end bent shown, skewed end bent similar)

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 6 of 30

Perforated Drain Pipe

- Perforated

Сар –

Drain Pipe

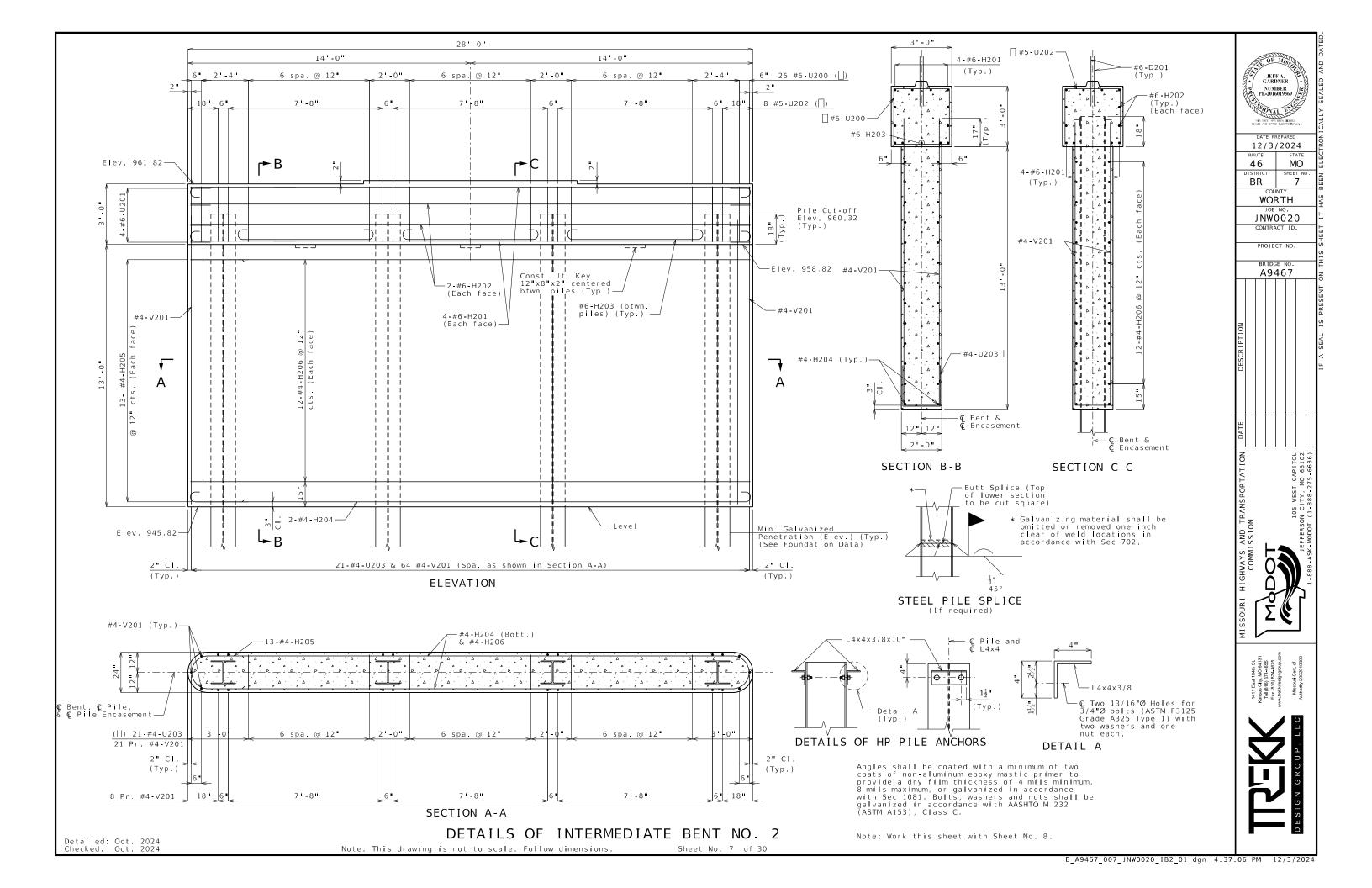
ELEVATION OF END BENT

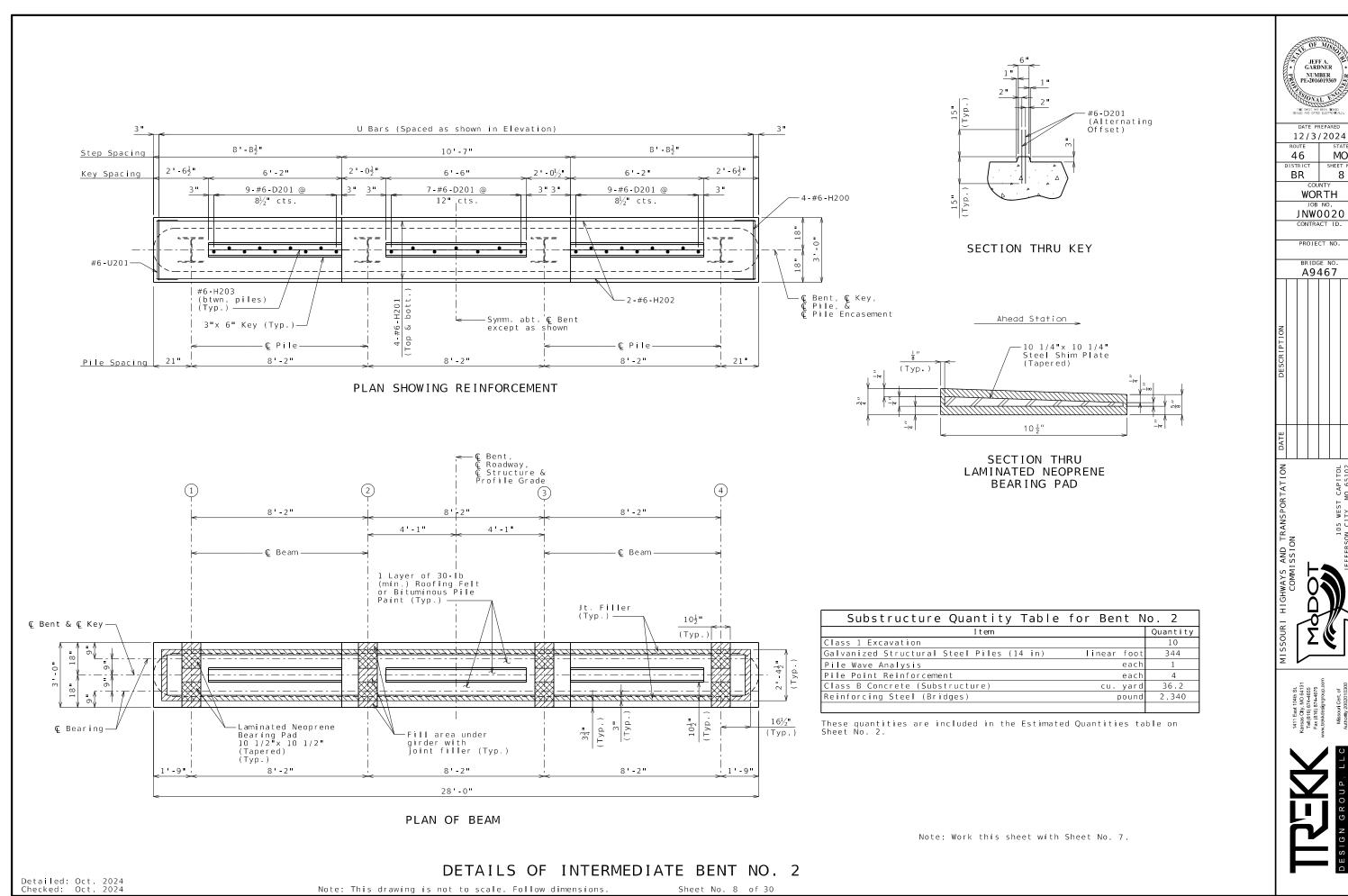
Geotextile

Fabric (Typ.)

PLAN OF END BENT

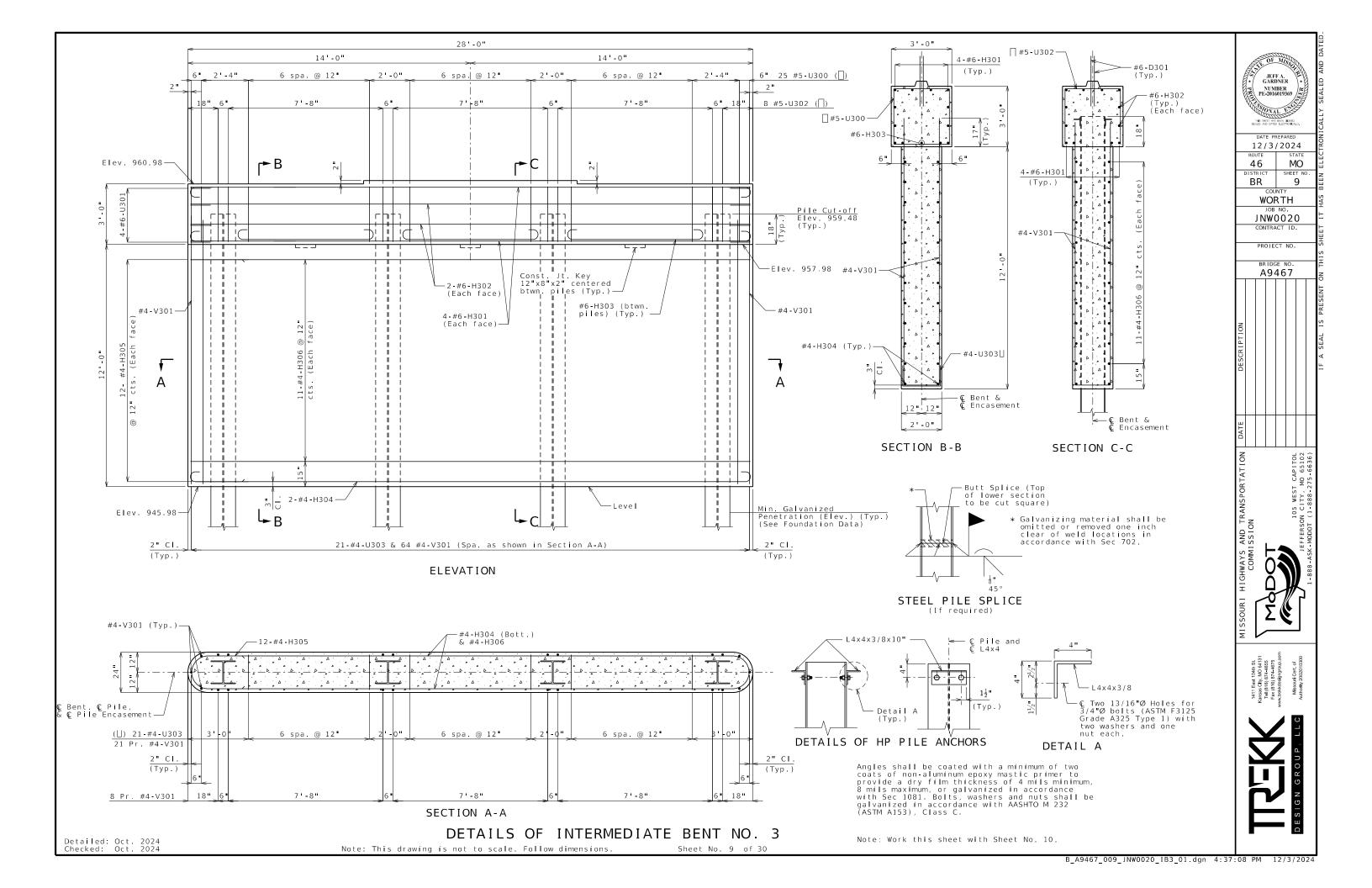
JEFF A. GARDNER NUMBER PE-2016019369 12/3/2024 46 MO SHEET NO BR 6 WORTH JNW0020 PROJECT NO. A9467

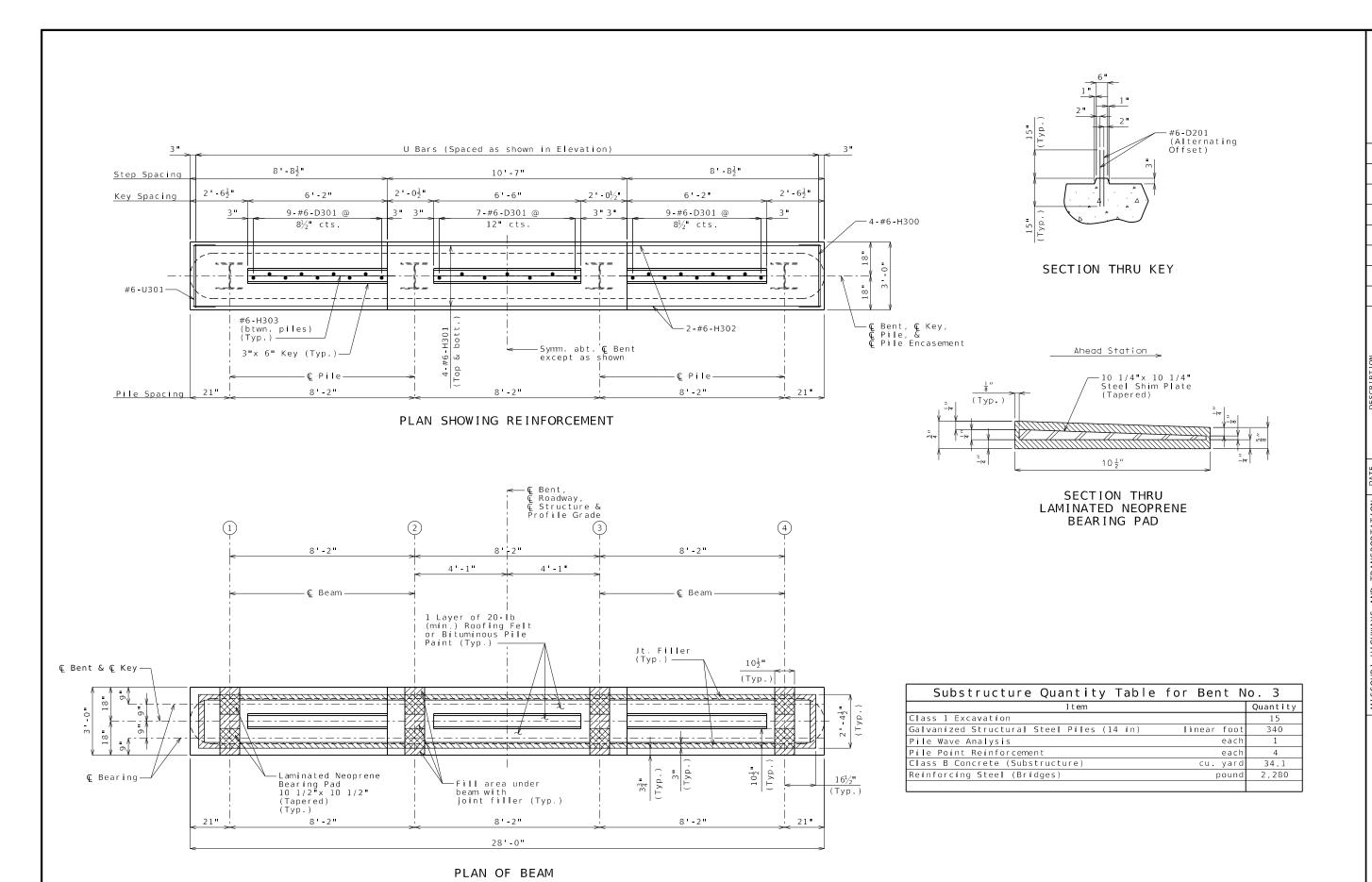




MO SHEET NO

8





Note: Work this sheet with Sheet No. 9.

DETAILS OF INTERMEDIATE BENT NO. 3

Detailed: Oct. 2024 Checked: Oct. 2024 JEFF A. GARDNER NUMBER PE-2016019369

12/3/2024

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PROJECT NO.

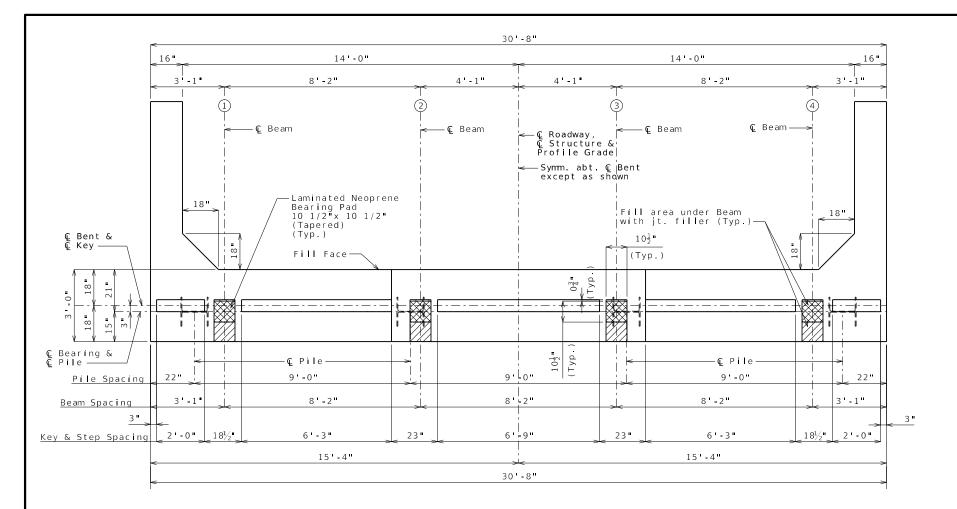
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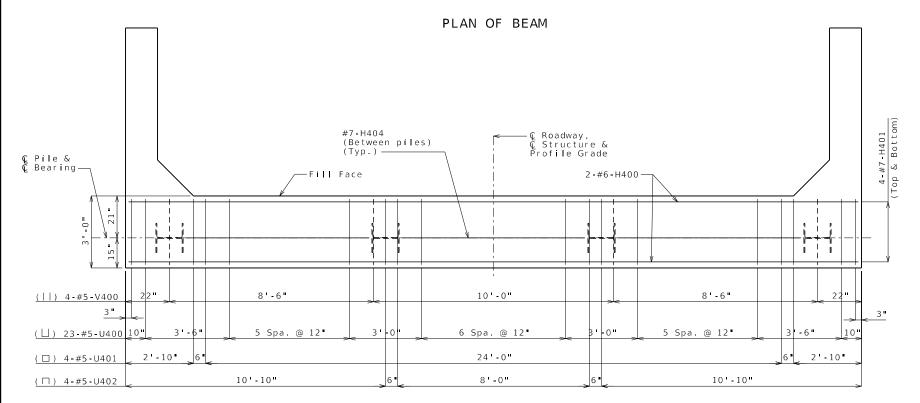
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PLAN OF BEAM SHOWING REINFORCEMENT

Keys not shown for clarity

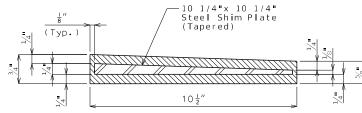
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DETAILS OF END BENT NO. 4

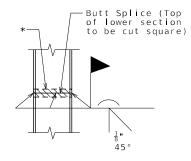
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Sheet No. 11 of 30

Ahead Station>



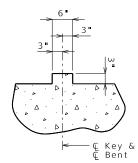
SECTION THRU LAMINATED NEOPRENE BEARING PAD



STEEL PILE SPLICE

(If required)

* Galvanizing material shall be omitted or removed one inch clear of weld locations in accordance with Sec 702.



SECTION THRU KEY

Substructure Quantity Table	for Bent N	o. 4
I t em		Quantity
Class B Concrete (Substructure)	cu. yard	13.7
Galvanized Structural Steel Piles (14 in)	linear foot	340
Pile Point Reinforcement	each	4
Class 1 Excavation	cu. yard	35

These quantities are included in the Estimated Quantities table on Sheet No. 2.

General Notes:

The concrete diaphragm at the end bents shall be poured a minimum of 12 hours before the slab is poured.

Work this sheet with Sheets No. 12 & 13.

All U bars and pairs of V bars shall be placed parallel to centerline of roadway.

Reinforcing steel shall be shifted to clear piles. U bars shall clear piles by at least 1 1/2 inches.



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BR 11 WORTH

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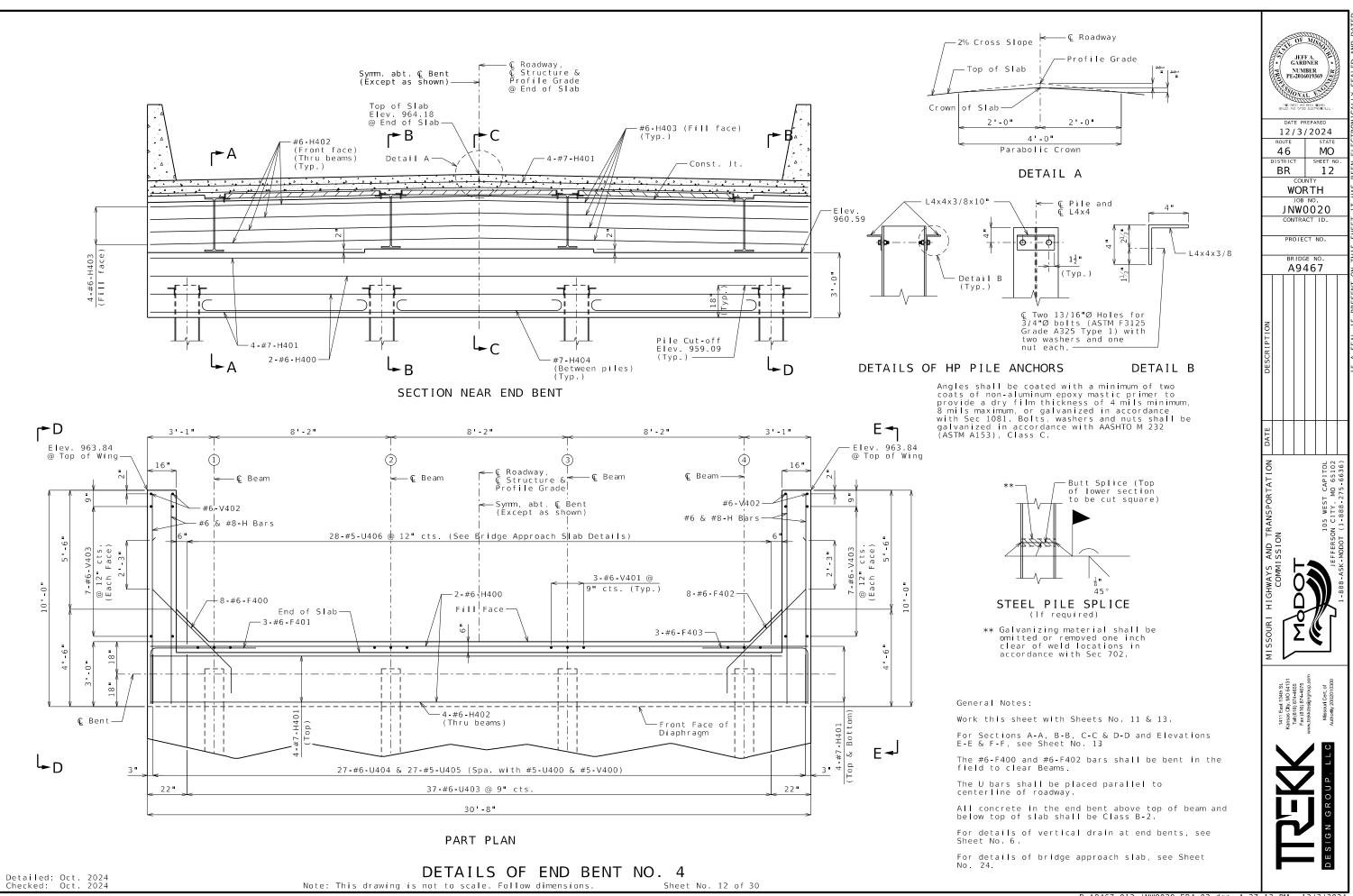
COMMISSION

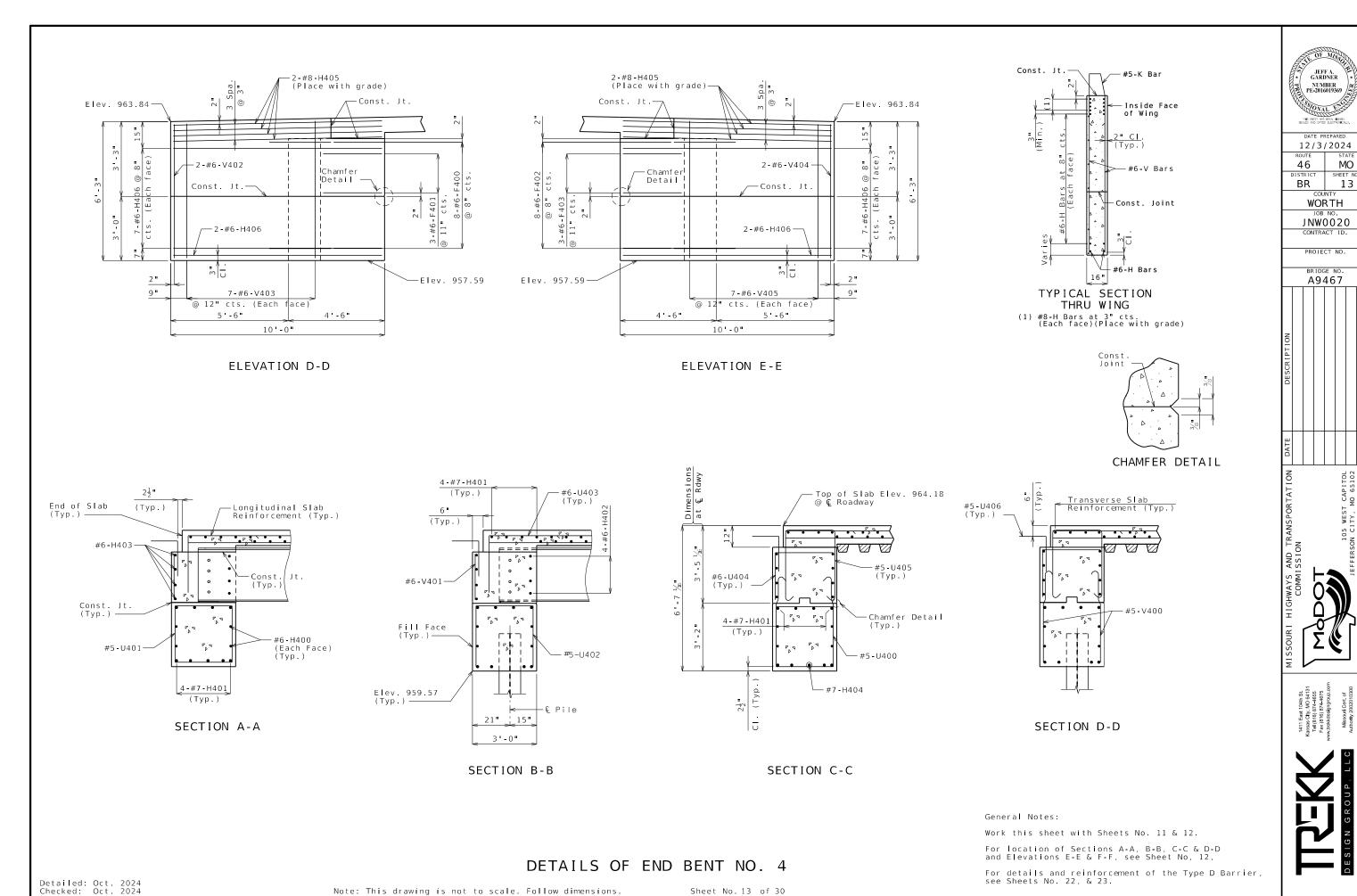
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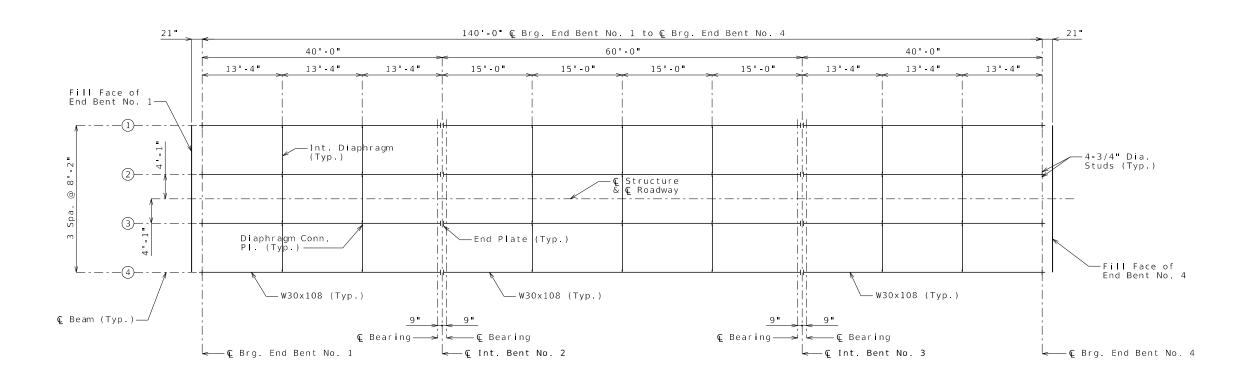
1411 East 104th St. Kansas City, MO 64131 Tel (816) 874-4655 Fax (816) 874-4675 ww.trekkdesigngroup.com





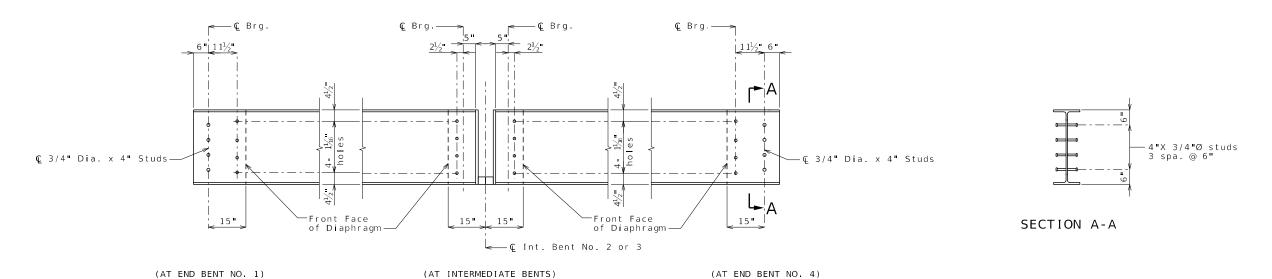


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PLAN OF STRUCTURAL STEEL

SPAN (2-3)



DETAILS AT END OF BEAMS

FRAMING PLAN

Notes:

All dimensions are horizontal from ${\mathbb C}$ Bearing to ${\mathbb C}$ Bearing.

SPAN (3-4)

For details of intermediate diaphragms, connection plates and end plates see Sheet No. 16.

All structural steel shall be ASTM A709 Grade 50W and shall not be galvanized. Weight of all structural steel is included in the weight of Fabricated Structural Low Alloy Steel (I-Beam) A709, Grade 50W. JEFF A. GARDNER NUMBER PE-2016019369

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12/3/2024

ROUTE STATE

46 MO

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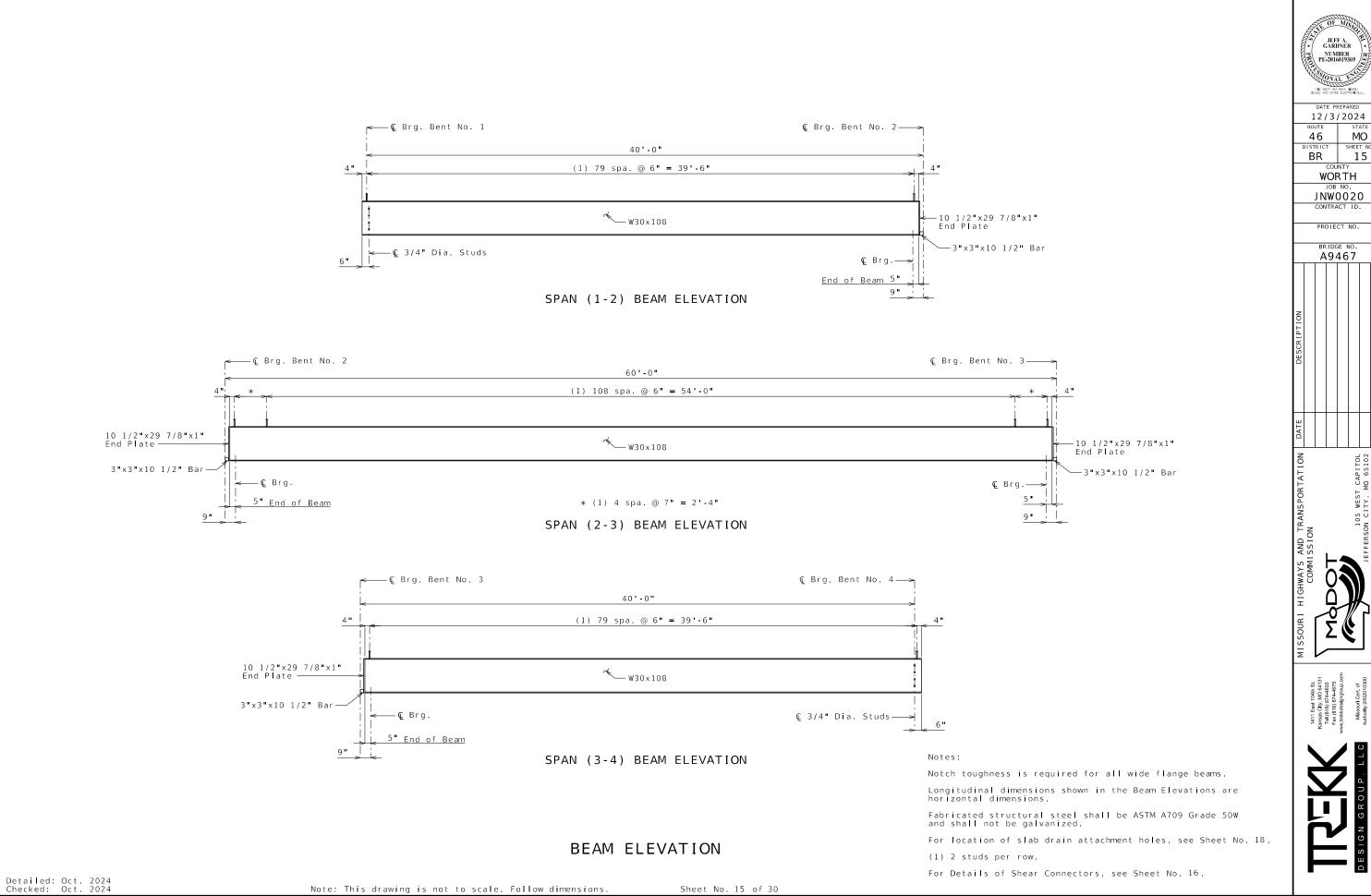
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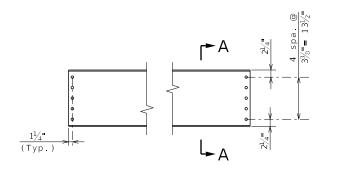


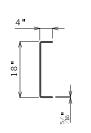
Detailed: Oct. 2024 Checked: Oct. 2024 SPAN (1-2)



12/3/2024 MO







INTERMEDIATE DIAPHRAGM

SECTION A-A

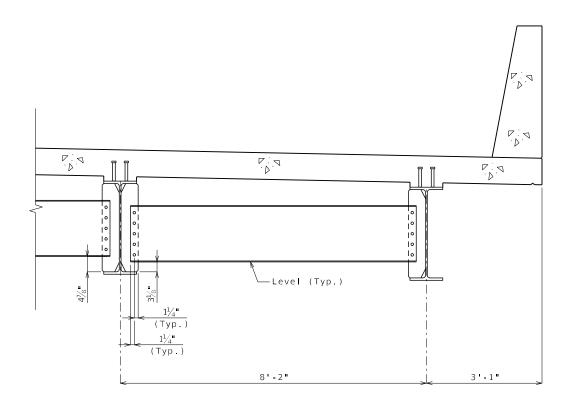
Notes:

Detailed: Oct. 2024 Checked: Oct. 2024 All bolted connections shall be 7/8"Ø ASTM F3125 Type 3 bolts in 15/16"Ø.

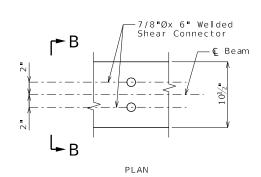
At the contractor's option, holes in the diaphragm plate of non slab bearing diaphragms may be made 3/16" larger than the nominal diameter of the bolt. A hardened washer shall be used under the bolt head and nut when this option is used. Holes in the girder diaphragm connection plate or transverse web stiffener shall be standard size.

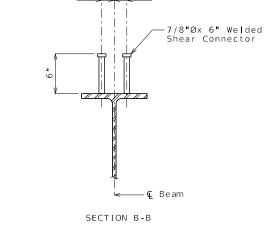
All structural steel shall be ASTM A709 Grade 50W and shall not be galvanized.

Weight of all structural steel is included in the weight of Fabricated Structural Low Alloy Steel (I-Beam) A709, Grade 50W.



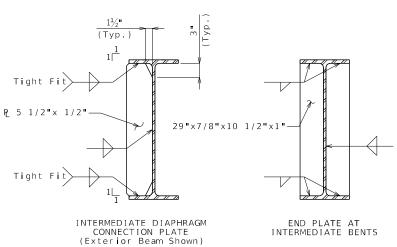




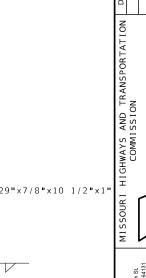


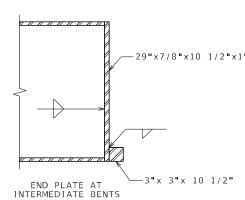
DETAILS OF SHEAR CONNECTORS

Weight of 2,549 pounds of shear connectors for the beams is included in the weight of Fabricated Structural Low Allow Steel (I-Beam) A709, Grade 50 W. Shear connectors shall be in accordance with Sec 712, 1037, and 1080.











JEFF A. GARDNER NUMBER PE-2016019369

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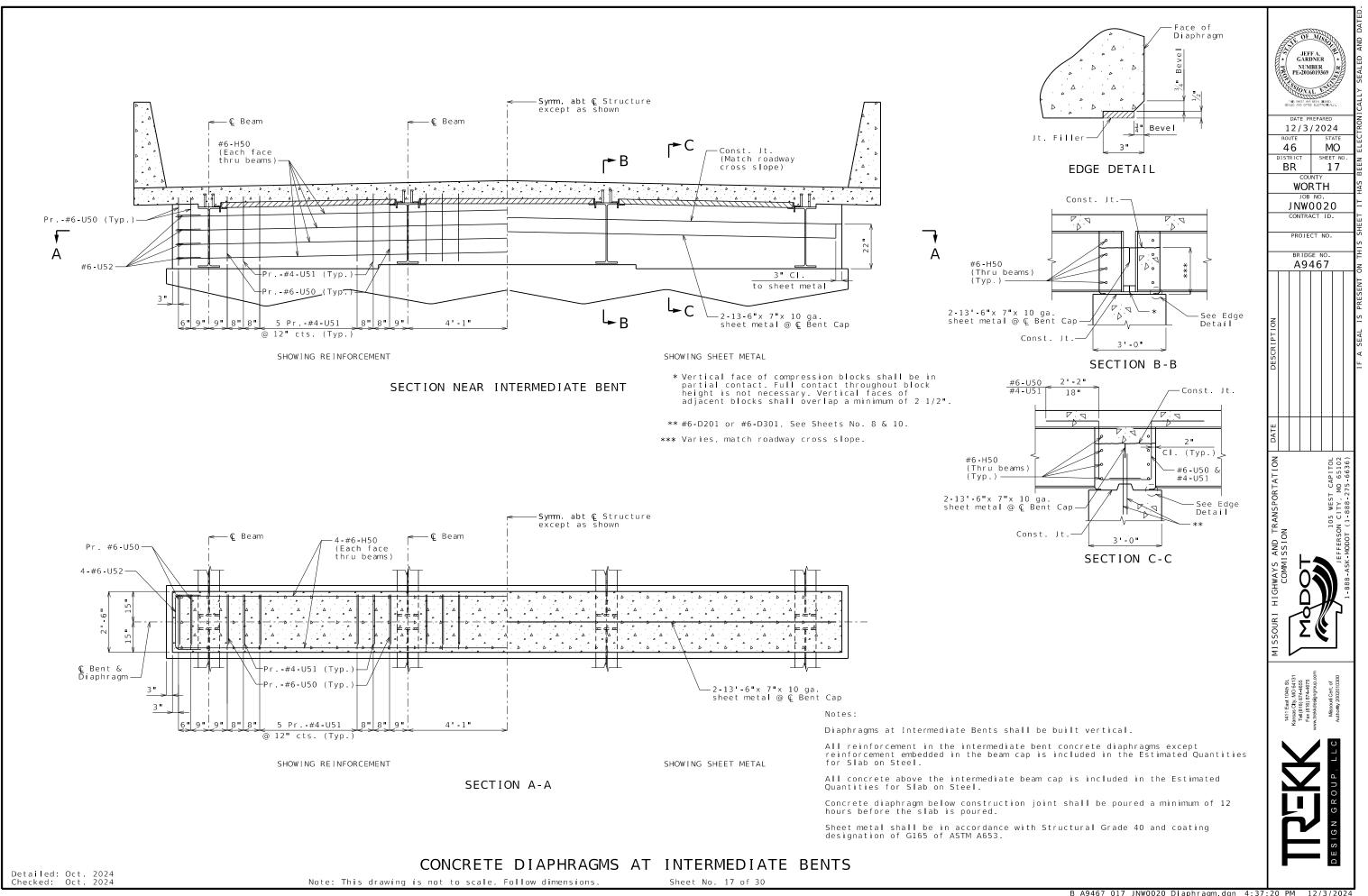
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STEEL DETAILS

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 16 of 30

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Contractor shall have the option to construct either steel or FRP slab drains.

Slab drain bracket assembly shall be ASTM

Locate drains in slab by dimensions shown

The bracket assembly shall be galvanized

All bolts, hardened washers, lock washers and nuts shall be galvanized in accordance with AASHTO M 232 (ASTM A153), Class C,

All 1/2-inch diameter bolts shall be ASTM

slab drains and the bracket assembly.

attachment shall be located on the plate Beam shop drawings.

The galvanized surfaces of drain support brackets shall be prepared according to the coating manufacturer's recommendation and field coated with a gray epoxy-mastic primer (non-aluminum) within a distance of 6 inches from the point of connection to

1/4-inch welded sheets of ASTM A709 Grade 36 steel or from 1/4-inch structural steel

Outside dimensions of drains are 8" x 4".

Drains shall be machine filament-wound thermosetting resin tubing meeting the requirements of ASTM D2996 with the

Shape of drains shall be rectangular with outside nominal dimensions of 8° x 4".

Minimum reinforced wall thickness shall be

The resin used shall be ultraviolet (UV) resistant and/or have UV inhibitors mixed throughout. Drains may have an exterior coating for additional UV resistance.

(Federal Standard 26373). The color shall be uniform throughout the resin and any

The combination of materials used in the manufacture of the drains shall be tested for UV resistance in accordance with ASTM D4329 Cycle A. The representative material shall withstand at least 500 hours of testing with only minor discoloration and without any physical deterioration. The contractor shall furnish the results of the required ultraviolet testing prior to

At the contractor's option, drains may be field cut. The method of cutting FRP slab drain shall be as recommended by the manufacturer to ensure a smooth, chip free



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BR 18 WORTH

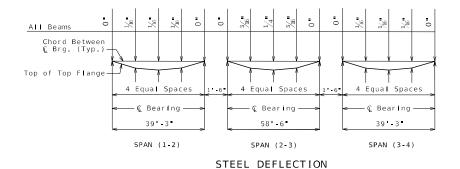
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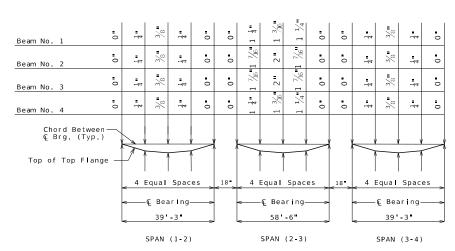
PROJECT NO

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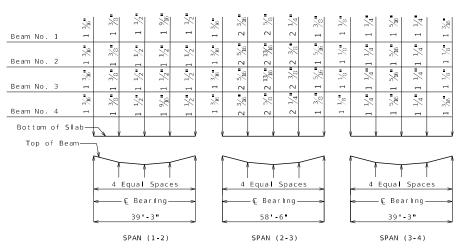
Sheet No. 18 of 30





DEAD LOAD DEFLECTION

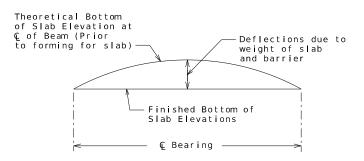
15% of dead load deflection is due to the weight of structural steel. Dead load deflection includes weight of structural steel, concrete slab, and barrier.



THEORETICAL SLAB HAUNCHING DIAGRAM (ESTIMATED AT 90 DAYS)

If beam camber is different from that shown in the camber diagram, in order to maintain minimum slab thickness, an adjustment of the slab haunches, an increase in slab thickness or a ralse In grade uniformly throughout the structure shall be necessary. No payment will be made for additional labor or materials required for variation in haunching, slab thickness or grade adjustment.

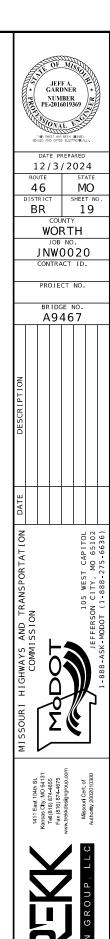
Concrete in the slab haunches is included in the Estimated Quantities for Slab on Concrete Beam.

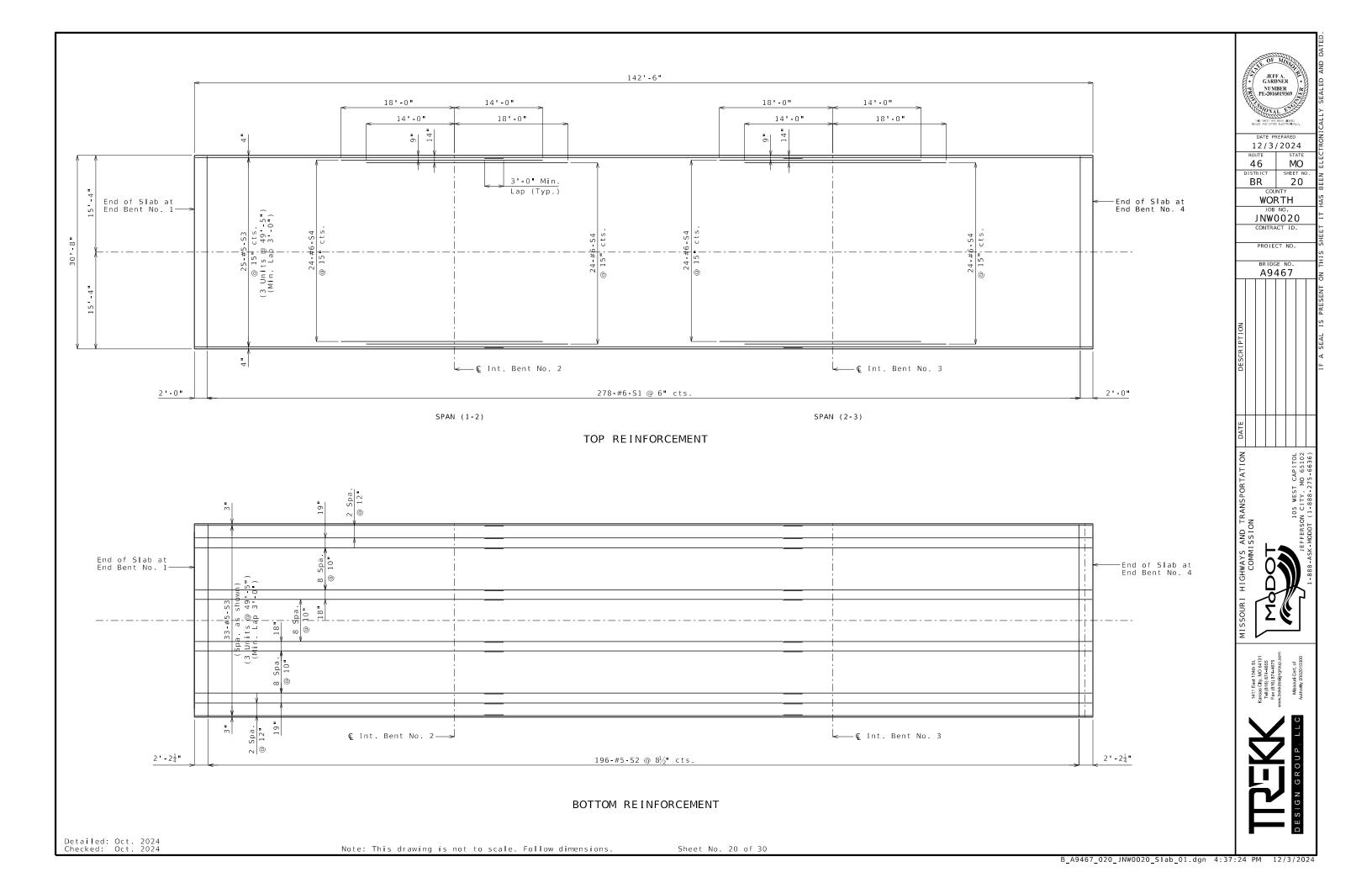


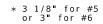
TYPICAL SLAB ELEVATIONS DIAGRAM

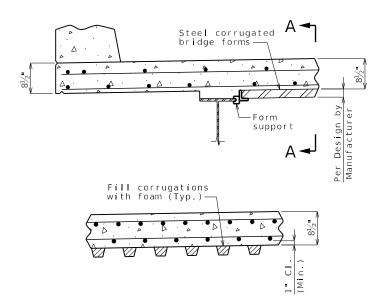
	Theoretical Bottom of Slab Elevations at Centerline of Beam (Prior to forming for slab)														
Beam															
Number	€ Brg.	. 25	.50	. 75	€ Brg.	© Brg.	. 25	.50	. 75	€ Brg.	€ Brg.	. 25	.50	. 75	© Brg.
1	965.22	965.04	964.86	964.68	964.49	964.47	964.32	964.14	963.91	963.64	963.62	963.53	963.44	963.34	963.23
2	965.38	965.21	965.03	964.84	964.66	964.63	964.50	964.08	964.08	963.80	963.79	963.70	963.60	963.50	963.40
3	965.38	965.21	965.03	964.84	964.66	964.63	964.50	964.08	964.08	963.80	963.79	963.70	963.60	963.50	963.40
4	965.22	965.04	964.86	964.68	964.49	964.47	964.32	964.14	963.91	963.64	963.62	963.53	963.44	963.34	963.23

Elevations are based on a constant slab thickness of 8 1/2" and include allowance for theoretical dead load deflections due to weight of slab (including corrugated steel form) and barrier.









OPTIONAL STAY-IN-PLACE FORM DETAILS

Stay-In-Place Forms:

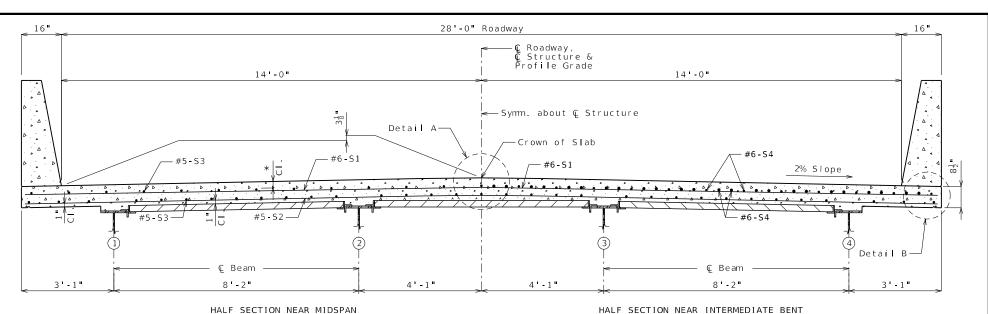
Corrugated steel forms, supports, closure elements and accessories shall be in accordance with grade requirement and coating designation G165 of ASTM A653. Complete shop drawings of the permanent steel deck forms shall be required in accordance with Sec 1080.

Corrugations of stay-in-place forms shall be filled with an expanded polystyrene material. The polystyrene material shall be placed in the forms with an adhesive in accordance with the manufacturer's recommendations.

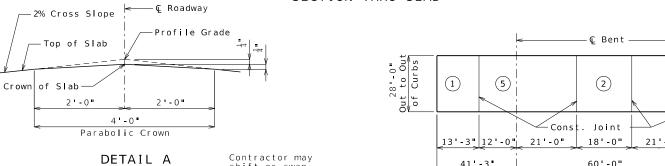
Form sheets shall not rest directly on the top of beam flanges. Sheets shall be securely fastened to form supports with a minimum bearing length of one inch on each end. Form supports shall be placed in direct contact with the flange. Welding on or drilling holes in the beam flanges will not be permitted. All steel fabrication and construction shall be in accordance with Sec 1080 and 712. Certified field welders will not be required for welding of the form supports. be required for welding of the form supports.

The design of stay-in-place corrugated steel forms is per manufacturer which shall be in accordance with Sec 703 for false work and forms. Maximum actual weight of corrugated steel forms allowed shall be 4 psf assumed for beam loading.

The contractor shall provide a method of preventing the direct contact of the stay in place forms and connection components with uncoated weathering steel members that is approved by the engineer.

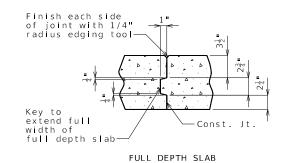


SECTION THRU SLAB



DETAIL A	shift or swap
Const. Jt. Contra may sh bar as needed tie R2 in bar Groove (Typ.)	bars as needed to tie R3 bar in barrier (4" min bar ift spacing)

DETAIL B OPTIONAL SHIFTING TOP BARS AT BARRIER



SLAB CONSTRUCTION JOINT

SLAB DETAILS

	41 -3		60'-0"		41	- 3 "	
	SPAN (1-2)	S	SPAN (2-	3)	SPAN	(3-4)	
	Se	quence	of P	ours			e of Pour ls./Hr.
		Dire	ction			With Retarder	No Retarder
Basic Sequence	1 2		3 Directi	4 on	5	25	25
	s to the basic s cordance with Se		nce are	subject	to the app	roval of th	ne
Alternate A	1		+ 2		+ 3	25	25
Pours	End to 5	1 t	o 4	2 t	o End		
Alternate B	1 + 5 +	2		4 + 3		25	25
Pours	End to 4	l		2 to E	nd	_ ²⁵	25
Alternate C		1 + 5 + 2	2 + 4 +	3	25	30	

(4)

(3)

End to End The contractor shall pour and satisfactorily finish the slab pours at the rate given. Retarder, if used, shall be an approved type and retard the set of concrete to $\bar{2}.5$ hours.

SLAB POURING SEQUENCE

Notes:

For reinforcement of barrier not shown, see Sheets No. 22 & 23.

For Theoretical Bottom of Slab Elevations, Girder Camber Diagram and Theoretical Slab Haunching Diagram, see Sheet No. 19.

For Plan of Slab Showing Reinforcement, see Sheet No. 20.



JEFF A. GARDNER

NUMBER PE-2016019369

12/3/2024

WORTH JNW0020

PROJECT NO.

A9467

MO SHEET NO

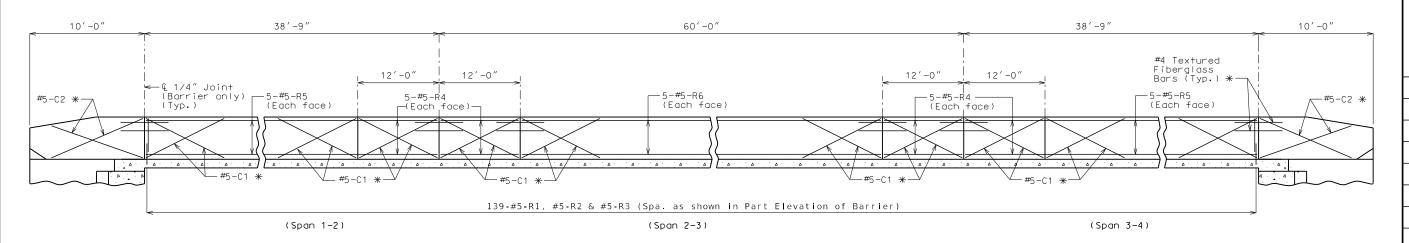
21

46

BR

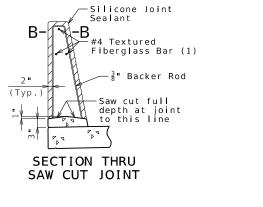
Detailed: Oct. 2024 Checked: Oct. 2024

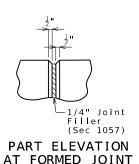
39

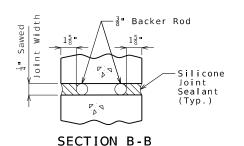


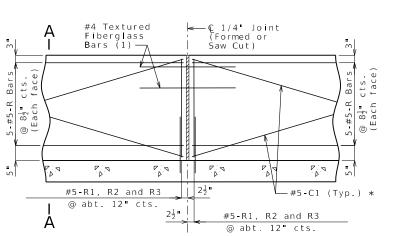
ELEVATION OF BARRIER

(Left barrier shown, right barrier similar)
Longitudinal dimensions are horizontal.





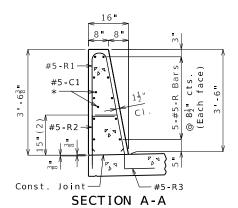




PART ELEVATION OF BARRIER

(1) Four feet long, centered on joint, slip-formed option only

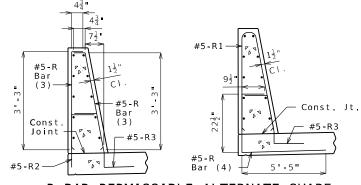
Detailed: Oct. 2024 Checked: Oct. 2024



Use a minimum lap of 3'-1" for #5 horizontal barrier bars.

The cross-sectional area above the slab is 3.52 square feet.

(2) To top of bar



R-BAR PERMISSIBLE ALTERNATE SHAPE

- (3) The R1 bar may be separated into two bars as shown, at the contractor's option, only when slip forming is not used. (All dimensions are out to out.)
- (4) The R2 bar and #5 bottom transverse slab bar in cantilever (prestressed panels only) combination may be furnished as one bar as shown, at the contractor's option.

General Notes:

st Slip-formed option only.

Conventional forming or slip forming may be used. Saw cut joints may be used with conventional forming.

Top of barrier shall be built parallel to grade and barrier joints (except at end bents) normal to grade.

All exposed edges of barrier shall have either a 1/2-inch radius or a 3/8-inch bevel, unless otherwise noted.

Payment for all concrete and reinforcement, complete in place, will be considered completely covered by the contract unit price for Type D Barrier per linear foot.

Concrete in barrier shall be Class B-1.

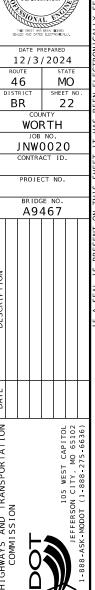
Measurement of barrier is to the nearest linear foot for each structure, measured along the outside top of slab from end of wing to end of wing.

Concrete traffic barrier delineators shall be placed on top of the barrier as shown on Missouri Standard Plan 617.10 and in accordance with Sec 617. Delineators on bridges with two-lane, two-way traffic shall have retroreflective sheeting on both sides. Concrete traffic barrier delineators will be considered completely covered by the contract unit price for Type D Barrier.

Joint sealant and backer rods shall be in accordance with Sec 717 for silicone joint sealant for saw cut and formed joints.

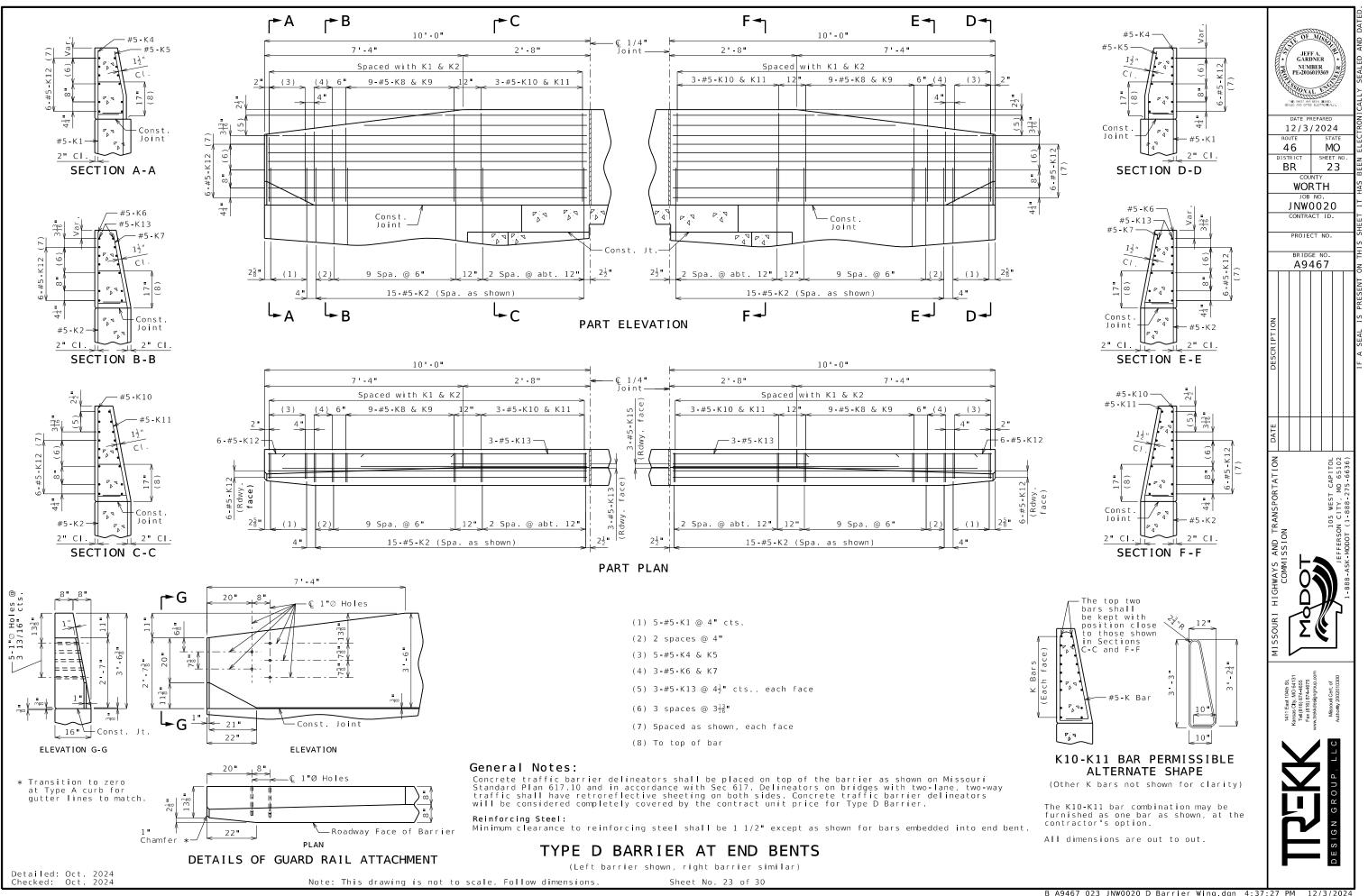
For slip-formed option, both sides of barrier shall have a vertically broomed finish and the top shall have a transversely broomed finish.

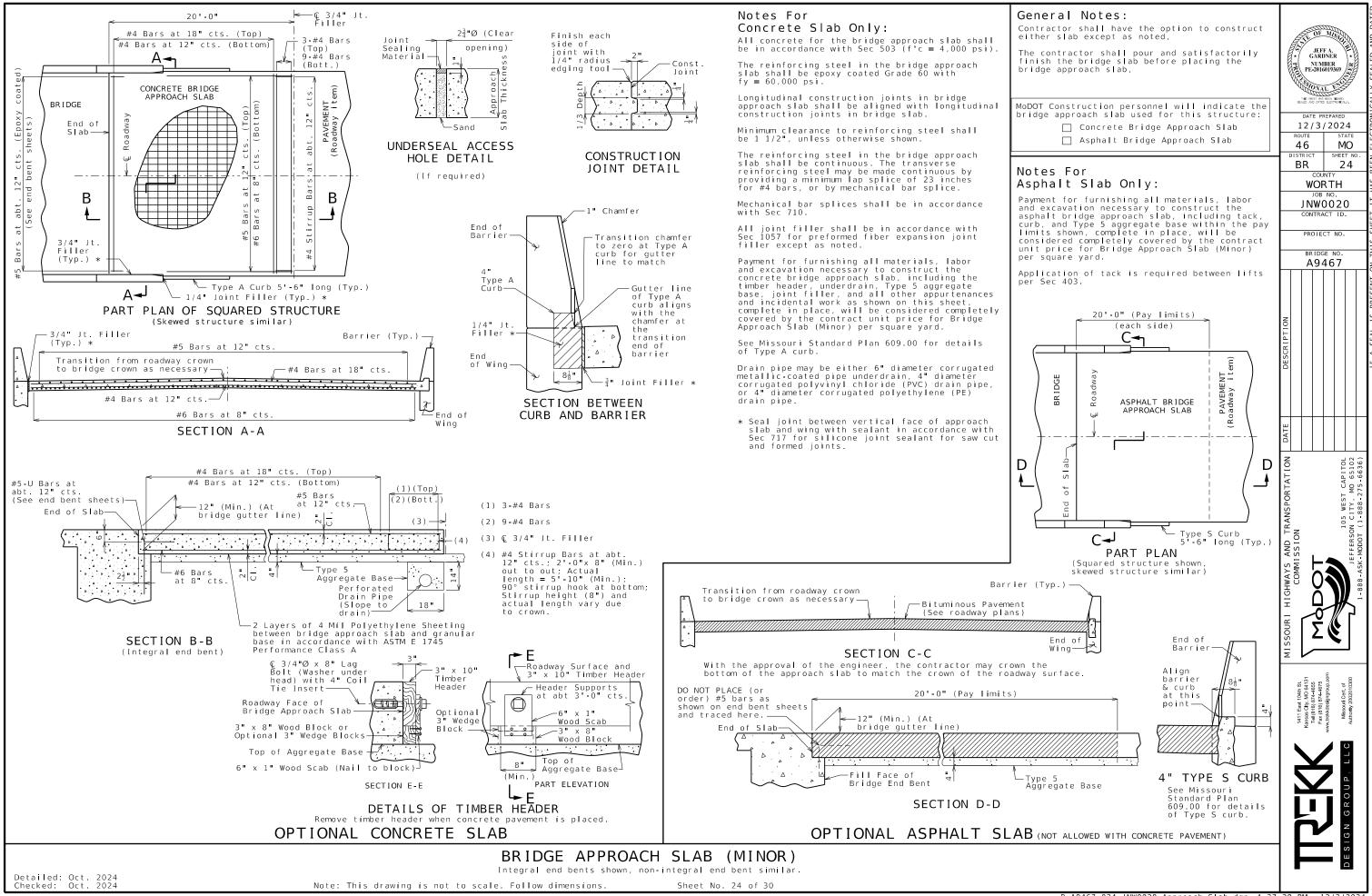
Plastic waterstop shall not be used with saw cut joints.

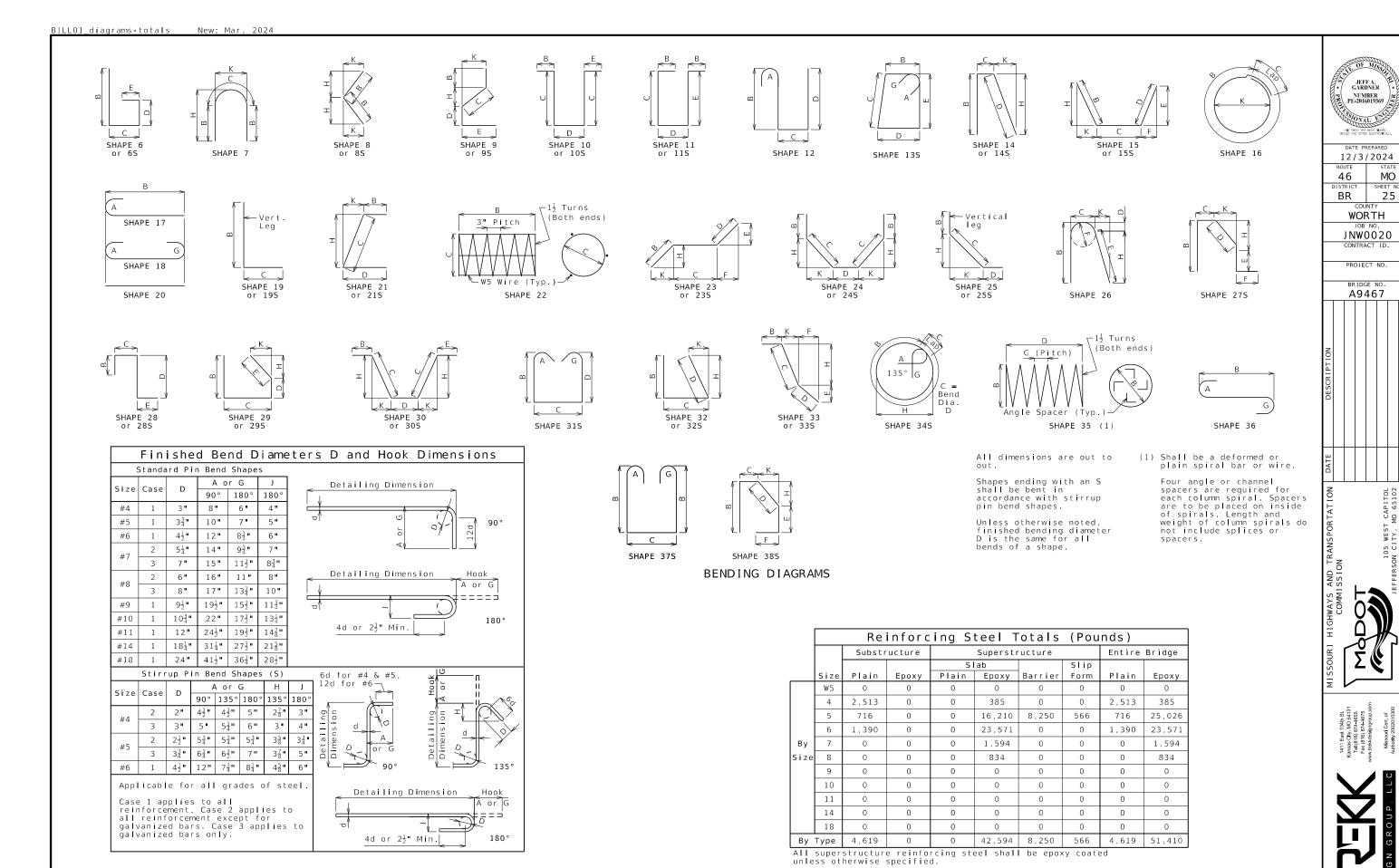




TYPE D BARRIER Sheet No. 22 of 30







BENDING DIAGRAMS AND REINFORCING STEEL TOTALS

Detailed: Oct. 2024 Checked: Oct. 2024 BILLO2 data New: Mar. 2024

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					П																			
8	6 H201	BEAM		18		27	8.000													29		29	0	34
4	6 H202	BEAM		20		27	8.000													27		27	8	16
3	6 H203	C-2010 40800	1	18	-	6	8.000													8		8	0	;
2	4 H204		\perp	18	-	26	0.000	_												27		27	0	:
26	4 H205		\perp	7	-	3	0.000	2	5.750							3	9.500		19.000		_	8	6	14
24	4 H206	BEAM	+	20		26	0.000													26	0	26	0	41
25	E 11200	DEAM	+	40		2	0.000	2	0.000	2	8.000	2	9.000							4.4	44	11	2	29
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8	5 U202		+	10	Н			2	8.000	_	8.000									8	_	7	9	
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25	6 D301	BEAM KEY		20		2	6.000													2	6	2	6	9
8	6 H301	BEAM		18		27	8.000													29	0	29	0	34
4	6 H302	BEAM		20		27	8.000													27	8	27	8	16
3	6 H303	BEAM		18		6	8.000													8	0	8	0	
2	4 H304	10.000 S.0000.		18		26	0.000													27	_	27	0	:
24	4 H305			7	-	3	0.000	2	5.750							3	9.500		19.000	-	_	8	6	13
24	4 H306	BEAM		20		26	0.000													26	0	26	0	4
100000			_		Ш																			
25	5 U300		+	13		2	8.000	2	8.000		8.000	2	8.000			_				11	_	11	3	29
8	6 U301		+	10	Ш			_	12.000	_	6.750					\vdash				4	_	4	3	5
8	5 U302		+	10	Н			2	8.000	2	8.000					\vdash				8 5		7	9	
21	4 U303	BEAM	+	10	Н			2	0.000		20.000			_		\vdash				5	- 8	5 5	ь	-
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04	4 7 30 1	COLUMN	+	20	Н	13	2.000									\vdash				13		13		- 30
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		INT. BENT DIAPHR																						
16	6 H50	DIAPHRAGM	E	_		27	2.000													27	2	27	2	65
40	6 U50	DIAPHRAGM	E	28	П			2	2.000	2	6.000	2	2.000							6		6	6	39
96		DIAPHRAGM	Е						18.000		6.000		2.000							6		6	0	38
16	6 U52	DIAPHRAGM	E	10					12.000	2	2.000									3	2	2	10	(
			1																			_		
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	Bill of Reinforcing Steel																							
														ensions							Nom.	Ac	tual	
N	o. Siz	re/			Code	s		В		С		D		E		F		Н	K	7	Length	Le	ngth	Weight
Re	q. Ma	ark	LOCATION	C	SH	V	ft	in.	ft	in.	ft	in.	ft	in.	ft	in.	ft	in. f	i i	n. ft	t in.	ft	in.	lb
			END BENT 1																					
10	6	F100	WING BRACE	Е	23		2	3.000	5	1.750	1	2.000		10.000		10.000	1	7.000 1	7.00	00 8	3 7	8	6	204
6	6	F101	DIA PHRA GM	Е	6		4	10.000	2	8.500										7	7	7	5	67
				П																				
4	6	H100	BEAM	Е	20		30	5.000												3	30 5	30	5	183
1:	2 7	H101	BEAM & DIAPHRAGM	Е	20		30	5.000												3	30 5	30	5	746
4	6	H102	DIAPHRAGM	Е	20		30	5.000												3	5 5	30	5	183
4	6	H103	DIAPHRAGM	Е	20		30	5.000												3	5 5	30	5	183
3	7	H104	DIAPHRAGM	Е	18		6	8.000												8	3	8	3	51
10	8 6	H105	WING	Е	20		9	9.000												9	9	9	9	417
3:	2 6	H106	WING	Е	20		9	9.000												9	9	9	9	469
2:	3 5	U100	BEAM	Е	31		4	5.000	2	9.000	4	5.000								1	2 6	12	4	296
4	5	U101	BEAM	Е	13		2	9.000	2	8.000	2	9.000	2	8.000						1	1 9	11	6	48
4	5	U102	BEAM	Е	10				2	10.000	2	9.000								8	5	8	2	34
3	7 6	U103	BEAM	Е	19		2	11.000	4	6.500										7	6	7	4	408
2	7 6	U104	BEAM	Е	19		2	4.500	2	9.000										5	5 1	4	11	199
2	7 6	U105	DIAPHRAGM	Е	31		2	10.750	2	3.000	2	10.750								9	0	8	9	355
2	3 5	U106	DIAPHRAGM	Е	19		2	0.000	1	3.000										3	3 3	3	2	92
				П		П																		
8	5	V100	BEAM	Е	17		4	5.000												5	5 0	5	0	42
1:	2 6	V101	BEAM	Е	20		2	4.500												2	2 4	2	4	42
4	6	V102	WING	Е	20		6	3.000												6	3	6	3	38
2	3 6	V103	WING	Е	20	٧	6	1.250												6	3 1	6	1	258
			INC = 0.125 INCH	П			6	2.750												6	5 2	6	2	
				П																				
			END BENT 4																					
10	6	F400	WING BRACE	Е	23		2	3.000	5	1.750	1	2.000		10.000		10.000	1	7.000 1	7.00	00 8	3 7	8	6	204
6	6	F401	DIAPHRAGM	Е	6		4	10.000	2	8.500										7	7	7	5	67
				П																				
4	6	H400	BEAM	Е	20		30	5.000												3	50 5	30	5	183
13	2 7	H401	BEAM & DIAPHRAGM	Е	20		30	5.000												3	5 5	30	5	746
4	6	H402	DIAPHRAGM	Е	20		30	5.000												3	5 5	30	5	183
4	6	H403	DIAPHRAGM	Е	20		30	5.000												_		30	5	183
3		H404	DIAPHRAGM	Е	18		6	8.000												8	100	8	3	51
10	8 8	H405	WING	Е	20		9	9.000												9		9	9	417
3:	2 6	H406	WING	Е	20		9	9.000												9	9	9	9	469
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2:	_	_	BEAM	Е	31		4	5.000	_	9.000	-	5.000	_							_		12	4	296
4	5	U401	BEAM	Е	13		2	9.000	2	8.000	_	9.000	2	8.000						1		11	6	48
4			BEAM	Е	10				2	10.000	-	9.000								8			2	34
3	_	_	BEAM	Е	19		2	11.000	4	6.500										7		_	4	408
2			BEAM	Е	19		2	2.000	_	9.000	_									4		_	9	193
2	_		DIAPHRACM	Ε	31		2	8.250	_	3.000	_	8.250								8		8	4	338
2	3 5	U406	DIAPHRAGM	Е	19		2	0.000	1	3.000										3	3 3	3	2	92
				Ш		\perp							Ш							\perp				
8	_	-	BEAM	Е	17	\perp	4	5.000												5		5	0	42
1:	_	_	BEAM	Е	20		2	2.000												2		2	2	39
4		V402		Ε	20		6	0.000												6		6	0	36
2	3 6	V103	WING	Е	20		6	0.500												6	0	6	0	252

Nominal lengths are based on out to out dimensions shown in bending diagrams and are listed to the nearest inch for fabricator's use. Actual lengths are measured along centerline bar to the nearest inch. Weights are based on actual lengths.

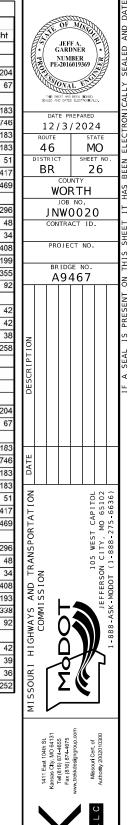
For bending diagrams and steel reinforcing totals, see Sheet No. 25.

All bars shall be Grade 60.

Codes: C = Required coatings, where E = Epoxy Coated and <math>G = Galvanized.

SH = Required shape, see bending diagrams.

V = Sets of varied bars and number of bars of each length. Bar dimensions vary in equal increments between dimensions shown on this line and the following line and the actual length dimension shown on this line and the following line vary by the specified increment.





BILL02 data New: Mar. 2024

								Вi	II c) f	Rei	n f	orci	nç	St	еe	l							
												Dim	ensions							N	lom.	A	ctual	
No.	Size/			Code	s		В		С		D		Е		F		Н		K		ength	Le	ngth	Weight
Req.	Mark	Location	С	SH	V	ft	in.	ft	in.	ft	in.	ft	in.	ft	in.	ft	in.	ft	in.	ft	in.	ft	in.	lb
		SLAB														-				_				
278	6 S1	SLAB	Е	20	Π	30	5.000													30	5	30	5	12701
196	5 S2	SLAB	Е	20	Т	30	5.000													30	5	30	5	6218
174	5 S3	SLAB	Е	20		49	5.000													49	5	49	5	8968
96	6 S4	SLAB	Е	20		32	0.000													32	0	32	0	4614
		TYPE D BARRIER			_															_				
278	5 R1	BARRIER	Е	26	Π	3	3.000		5.500	3	3.625					3	3.000		6.750	6	10	6	9	1957
278	5 R2	BARRIER	Е	19	Г		20.500		9.500											2	6	2	5	701
278	5 R3	BARRIER	Е	27					9.500		15.250		5.000		12.000		15.000		3.000	3	6	3	4	967
80	5 R4	BARRIER	Е	20		11	9.000													11	9	11	9	980
40	5 R5	BARRIER	E	20		26	5.000													26	5	26	5	1102
20	5 R6	BARRIER	Е	20		35	9.000													35	9	35	9	746
20	5 K1	BARRIER	Е	27	П	3	8.000		9.250		5.375	3	2.750				5.250		1.000	8	0	7	11	165
60	5 K2	BARRIER	Е	27	Т	3	8.000		9.250		14.500	2	5.750				14.250		2.750	8	2	7	11	495
					Τ																			
20	5 K4	BARRIER	Е	19	V	2	4.250		10.000											3	2	3	1	66
		INC. = 0.500 INCH				2	6.250		10.000											3	4	3	3	
20	5 K5	BARRIER	Е	14	V		8.250		9.500		18.500						4.000		18.000	3	0	2	11	63
		INC. = 0.500 INCH			Г		8.250		9.500		20.500						4.500		20.000	3	2	3	1	
12	5 K6	BARRIER	Е	19	V	2	6.750		10.000											3	5	3	4	42
		INC. = 0.500 INCH			Г	2	7.750		10.000											3	6	3	5	
12	5 K7	BARRIER	Е	21	V	2	6.625		10.000							2	6.000		6.250	3	5	3	3	41
		INC. = 0.500 INCH	Т		Т	2	7.625		10.000							2	7.000		6.500	3	6	3	4	
36	5 K8	BARRIER	E	19	V	2	8.500		10.000											3	7	3	5	138
		INC. = 0.750 INCH				3	2.500		10.000											4	1	3	11	
12	5 K9	BARRIER	Е	21	V	2	8.500		10.000							2	7.750		6.750	3	7	3	5	46
		INC. = 0.750 INCH				3	2.500		10.000							3	1.750		7.750	4	1	3	11	
12	5 K10	BARRIER	Е	19	Г	3	3.000		10.000											4	1	4	0	50
12	5 K11	BARRIER	Е	21	Т	3	3.000		10.000							3	2.250		7.750	4	1	3	11	49
48	5 K12	BARRIER	Е	20	Т	9	9.000													9	9	9	9	488
24	5 K13	BARRIER	Г	20	V	3	2.000													3	2	3	2	154
		INC. = 36.000 INCH				9	2.000													9	2	9	2	
		SLIP FORM OPTION	I																					
40	5 C1	BARRIER	Е	20		12	0.000													12	0	12	0	501
8	5 C2	BARRIER	Е	20		7	9.000													7	9	7	9	65

Nominal lengths are based on out to out dimensions shown in bending diagrams and are listed to the nearest inch for fabricator's use. Actual lengths are measured along centerline bar to the nearest inch. Weights are based on actual lengths.

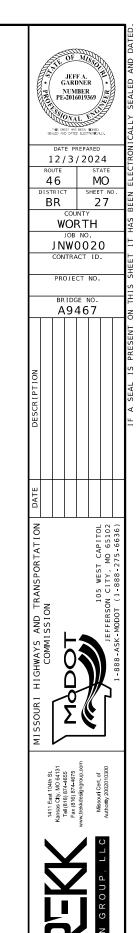
For bending diagrams and steel reinforcing totals, see Sheet No. 25.

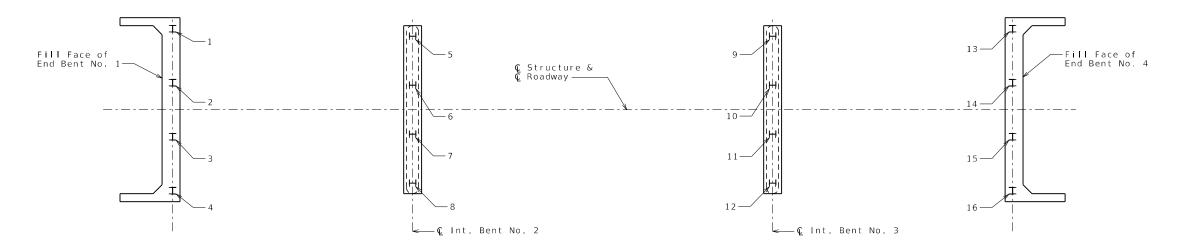
All bars shall be Grade 60.

Codes: C = Required coatings, where E = Epoxy Coated and <math>G = Galvanized.

SH = Required shape, see bending diagrams.

V = Sets of varied bars and number of bars of each length. Bar dimensions vary in equal increments between dimensions shown on this line and the following line and the actual length dimension shown on this line and the following line vary by the specified increment.





PART PLAN SHOWING PILE NUMBERING FOR RECORDING AS-BUILT PILE DATA

		As-Bu	ilt Pile Data
Pile No.	Length in Place (ft)	Computed Nominal Axial Compressive Resistance (kips)	Rema r k s
			End Bent No. 1
1			
2			
3			
4			
			Int. Book No. 2
_			Int. Bent No. 2
5			
6			
7			
8			

		As-Bu	ilt Pile Data
Pile No.	Length in Place (ft)	Computed Nominal Axial Compressive Resistance (kips)	Remarks
			Int. Bent No. 3
9			
10			
11			
12			
			End Bent No. 4
13			End Bent NO. 4
14			
15			
16			

Note: Indicate in remarks column:
A. Pile type and grade
B. Batter C. Driven to practical refusal

This sheet to be completed by MoDOT construction personnel.

12/3/2024 STATE MO
SHEET NO.
28 46 DISTRICT WORTH JOB NO.
JNW0020
CONTRACT ID. PROJECT NO. BRIDGE NO.

Missouri Department of Transportation

BORING NO. B-103 Page 1 of 2

	Construction and Materials	
ob No.: NW0020 A9467	County: Worth	Route: _46
Design: _A9467	Skew: Square	Location: 2 Miles West of Grant City
Bent: _1	Logged By: Ricardo Todd	Operator: Robert Wadlow
Station:	Northing: 1572811.2	Date of Work: 06/05/24-06/05/24
Offset:	Easting: 2803559.9	Depth to Water: _20.1
Elevation: 963.5	Requested Northing: _1572807.2	Depth Hole Open: _39.7
Requested Station: 564+36.8	Requested Easting: _2803559.8	Time Change: 1 hour
Requested Offset: 14 R	Equipment: Acker Soil XLS Split-Spoon Sa	mpler, NQ

Requested Elevation: _____ Location Note: Offset slightly north of requested location

rill N	o : G	0.100					n of requested location					
<u>Drill No.: G-9462</u> Han			ciency:	819	%	D	rilling Method: _	Hollow Stem Auge	er			
(t)	Graphic	Description	Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests	Index Tests			
-		0.0-0.4' ASPHALT 0.4-1.1' CRUSHED AGGREGATE BASE 1.1-9.8' Dark gray, LEAN CLAY trace gravel, medium stiff, moist	960		67	3-3-3 (8)		PP = 0.75 tsf	MC = 31.8% Y _{set} = 119 pcf ⁽¹⁾			
- - -		9.8-15.1' Gray, LEAN CLAY scattered gravel, very stiff, moist 15.1-19.8' Gray, LEAN CLAY trace gravel, stiff moist	950	X	73	3-7-8 (20) 3-3-4 (9)		PP = 2.75 tsf PP = 1.25 tsf	MC = 15.7% Y _{sat} = 136 pcf ¹⁾ LL = 28 PL = 16 MC = 25.1% Y _{sat} = 125 pcf ¹⁾ LL = 33			
20 - -		19.8-29.3' Gray, LEAN CLAY trace sand, soft, moist	940		73	1-1-1 (3)		PP = 0.50 tsf	MC = 25.1% y _{sat} = 125 pcf ⁽¹⁾ LL = 25 PL = 17			
30 -		29.3-35.1' Grayish tan, LEAN CLAY trace gravel, and fine sand, medium stiff, moist	930	X	67	1-1-2 (4) 1-3-3 (8)		PP = 0.50 tsf PP = 0.75 tsf	MC = 27.4% y _{sat} = 123 pcf ⁽¹⁾ MC = 23.4% y _{sat} = 127 pcf ⁽¹⁾ LL = 39 PL = 16			
- - 40		35.1-69.8' Brown mottled gray, LEAN CLAY scattered gravel, stiff, moist			67	1-3-4 (9) 1-4-5		PP = 1.00 tsf PP = 1.25 tsf	MC = 17.1% y _{sat} = 134 pcf ⁽¹⁾ 11 = 33 PL = 16 MC = 21.4% y _{sat} = 129 pcf ⁽¹⁾			
- - - 50			920	\times	67	1-4-5 (12)		PP = 1.25 tsf	MC = 21.6% Y _{sat} = 129 pcf ⁽¹⁾			
- - -			910	X	73	2-4-6 (14) 2-4-6 (14)		PP = 1.50 tsf PP = 1.75 tsf	MC = 20.0% y _{sat} = 130 pcf ⁽¹⁾ LL = 34 PL = 16 MC = 21.2% y _{sat} = 129 pcf ⁽¹⁾			
	0	0 10 20 30 40 50	0.0-0.4' ASPHALT 0.4-1.1' CRUSHED AGGREGATE BASE 1.1-9.8' Dark gray, LEAN CLAY trace gravel, medium stiff, moist 9.8-15.1' Gray, LEAN CLAY scattered gravel, very stiff, moist 15.1-19.8' Gray, LEAN CLAY trace gravel, stiff, moist 19.8-29.3' Gray, LEAN CLAY trace sand, soft, moist 20 29.3-35.1' Grayish tan, LEAN CLAY trace gravel, and fine sand, medium stiff, moist 35.1-69.8' Brown mottled gray, LEAN CLAY scattered gravel, stiff, moist	0 0.0-0.4' ASPHALT 0.4-1.1' CRUSHED AGGREGATE BASE 1.1-9.8' Dark gray, LEAN CLAY trace gravel, medium stiff, moist 9.8-15.1' Gray, LEAN CLAY scattered gravel, very stiff, moist 950 15.1-19.8' Gray, LEAN CLAY trace gravel, stiff, moist 19.8-29.3' Gray, LEAN CLAY trace sand, soft, moist 20 29.3-35.1' Grayish tan, LEAN CLAY trace gravel, and fine sand, medium stiff, moist 930 35.1-69.8' Brown mottled gray, LEAN CLAY scattered gravel, stiff, moist 920 920	0.0-0.4' ASPHALT 0.4-1.1' CRUSHED AGGREGATE BASE 1.1-9.8' Dark gray, LEAN CLAY trace gravel, medium stiff, moist 9.8-15.1' Gray, LEAN CLAY scattered gravel, very stiff, moist 950 15.1-19.8' Gray, LEAN CLAY trace gravel, stiff, moist 19.8-29.3' Gray, LEAN CLAY trace sand, soft, moist 20 29.3-35.1' Grayish tan, LEAN CLAY trace gravel, and fine sand, medium stiff, moist 35.1-69.8' Brown mottled gray, LEAN CLAY scattered gravel, stiff, moist 930 35.1-69.8' Brown mottled gray, LEAN CLAY scattered gravel, stiff, moist	0 0.0-0.4' ASPHALT 0.4-1.1' CRUSHED AGGREGATE BASE 1.1-9.8' Dark gray, LEAN CLAY trace gravel, medium stiff, moist 9.8-15.1' Gray, LEAN CLAY scattered gravel, very stiff, moist 950 15.1-19.8' Gray, LEAN CLAY trace gravel, stiff, moist 19.8-29.3' Gray, LEAN CLAY trace sand, soft, moist 19.8-29.3' Gray, LEAN CLAY trace sand, soft, moist 20 29.3-35.1' Grayish tan, LEAN CLAY trace gravel, and fine sand, medium stiff, moist 35.1-69.8' Brown mottled gray, LEAN CLAY scattered gravel, stiff, moist 67 920 67 910 73	0 0-0.4' ASPHALT 0.4-1.1' CRUSHED AGGREGATE BASE 1.1-9.8' Dark gray, LEAN CLAY trace gravel, medium stiff, moist 9.8-15.1' Gray, LEAN CLAY scattered gravel, very stiff, moist 9.8-15.1' Gray, LEAN CLAY trace gravel, stiff, moist 9.8-15.1' Gray, LEAN CLAY trace gravel, stiff, moist 9.8-29.3' Gray, LEAN CLAY trace sand, soft, moist 940 20 9.8-15.1' Grayish tan, LEAN CLAY trace gravel, stiff, moist 940 67 1-1-12 (4) 33 35.1-69.8' Brown mottled gray, LEAN CLAY scattered gravel, stiff, moist 920 67 1-4-5 (12) 910 73 2-4-6 (14) 910	0 0-0 4' ASPHALT 0 4-1.1' CRUSHED AGGREGATE BASE 1.1-9.8' Dark gray, LEAN CLAY trace gravel, medium stiff, moist 9 8-15.1' Gray, LEAN CLAY scattered gravel, very stiff, moist 9 50 15 1-19.8' Gray, LEAN CLAY trace gravel, stiff, moist 9 19 8-29.3' Gray, LEAN CLAY trace sand, soft, moist 9 19 8-29.3' Gray, LEAN CLAY trace sand, soft, moist 20 21 19 8-29.3' Gray, LEAN CLAY trace sand, soft, moist 9 19 8-29.3' Gray, LEAN CLAY trace gravel, stiff, moist 9 10 67 1.1-12 (4) 10 67 1.3-3 (8) 9 10 67 1.3-4 (9) 10 67 1.4-5 (12) 9 10 67 1.4-5 (12) 9 10 67 1.4-5 (12) 9 10 73 2.4-6 (14)	0 0-0-3 'ASPHALT			

N₅₀ = (Em/60)Nm N₅₀ - Corrected N value for standard 60% SPT efficiency; Em - Measured hammer efficiency in percent; Nm - Observed N-value; (1) = Assumed, (2) = Actual Coordinate System: Modified U.S. State Plane 1983 Coordinate Zone: Missouri West Coordinate Proj. Factor: 1.0000983827

Coordinate Datum: NAD 83 (CONUS) Coordinate Units: U.S. Survey Feet

E * Persons using this information are cautioned that the materials shown are determined by the equipment noted and accuracy of the "log of materials" is limited thereby and by judgement of the operator. THIS INFORMATION IS FOR DESIGN PURPOSES ONLY.

(Continued Next Page)

Missouri Department of Transportation Construction and Materials

BORING NO. B-103 Page 2 of 2

Job No.: NW0020 A9467	County: Worth	Route: _46
Design: _A9467	Skew: Square	Location: 2 Miles West of Grant City
Bent: _1	Logged By: Ricardo Todd	Operator: Robert Wadlow
Station:	Northing: _1572811.2	Date of Work: 06/05/24-06/05/24
Offset:	Easting: 2803559.9	Depth to Water: 20.1
Elevation: 963.5	Requested Northing: 1572807.2	Depth Hole Open: 39.7
Requested Station: 564+36.8	Requested Easting: 2803559.8	Time Change: 1 hour
Requested Offset: 14 R	Equipment: Acker Soil XLS ,Split-Spoon Sam	pler, NQ
Requested Elevation:	Location Note: Offset slightly north of request	ed location

Requ	ested E	Elevation: Location No	te: Of	set s	lightly no	orth of requested	location		
Drill l	No. : <u>G</u>	-9462 Hammer Effi	ciency	819	%	D	rilling Method: _	Hollow Stem Auge	er
Depth (ft)	Graphic	Description	Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N _{En})	Shear Data	Field Tests	Index Tests
60	409	35.1-69.8' Brown mottled gray, LEAN CLAY scattered gravel, stiff, moist (continued)	900	× ×	67	4-5-8 (18) 3-5-8 (18)		PP = 2.00 tsf PP = 2.00 tsf	MC = 19.3% y _{sw} = 131 pcf MC = 19.5% y _{sw} = 131 pcf LL = 34 PL = 14
70		69.8-86.6' Black mottled gray, LEAN CLAY scattered gravel, very stiff, moist	890		67	3-5-11 (22) 3-6-10 (22)		PP = 2.75 tsf PP = 3.00 tsf	MC = 19.0% Y _{sat} = 132 pcf LL = 33 PL = 14 MC = 19.0% Y _{sat} = 132 pcf
90		86.6-90.1' Limestone, highly weathered	880		73	3-6-11 (23) 4-7-11 (24)		PP = 3.25 tsf PP = 3.50 tsf	MC = 17.9% Y _{sot} = 133 pcf MC = 17.1% Y _{sot} = 134 pcf LL = 35 PL = 14
		90.1-91.1' Limestone, whitish gray, medium bedded, strong rock, slightly weathered, fine grained 91.1-93.5' Shale, olive tan, thinly laminated, weak rock 93.5-94.9' Limestone, whitish gray, thin bedded, strong rock, slightly weathered, fine grained grained	870		100 100 (62) 100	37/0.1', 10/0'	Qu Test Results UCS = 749 ksf MC = 0%		
		94.9-98.3' Shale, gray, thinly laminated, weak rock 98.3-99.4' Shale, black, thinly laminated, weak rock 99.4-100.1' Limestone, whitish gray, thin bedded, strong rock, slightly weathered, fine grained Bottom of borehole at 100.1 feet.					pvi		

N_{so} = (Em/60)Nm N_{so} - Corrected N value for standard 60% SPT efficiency; Em - Measured hammer efficiency in percent; Nm - Observed N-value; (1) = Assumed, (2) = Actual Coordinate System: Modified U.S. State Plane 1983 Coordinate Zone: Missouri West Coordinate Proj. Factor: _1.0000983827

 Coordinate System:
 Modified U.S. State Plane 1983
 Coordinate Zone:
 Missouri West

 Coordinate Datum:
 NAD 83 (CONUS)
 Coordinate Units:
 U.S. Survey Feet

*Persons using this information are cautioned that the materials shown are determined by the equipment noted and accuracy of the "log of materials" is limited thereby and by judgement of the operator. THIS INFORMATION IS FOR DESIGN PURPOSES ONLY.

BORING DATA

Note: For locations of borings, see Sheet No. 1.

Detailed: Oct. 2024 Checked: Oct. 2024

Note: This drawing is not to scale. Follow dimensions. Sheet No. 29 of 30

NIMBER PE-2016019369 STATE

ROUTE STATE 46 MO DISTRICT SHEET NO. BR 29 COUNTY

JOB NO.
JNW0020
CONTRACT ID

PROJECT NO.

BRIDGE NO. A9467						
	DESCRIPTION					
	DATE					
	NOI				TOL	5102

MISSOURI HIGHWAYS AND TRANSPORTY
COMMISSION

MADOT

105 WEST CA

LEFFERSON CITY, MO

1411 East 104th St.
Kansas City, MO 64131
Tel (816) 874-4655
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Missouri Department of Transportation **Construction and Materials**

BORING NO. B-401 Page 1 of 2

Job No.: NW0020 A9467 County: Worth Design: <u>A9467</u> Skew: Square Location: 2 Miles West of Grant City Operator: Josh Starkey Bent: 4 Logged By: Matthew Kistler Northing: 1572832.9 Date of Work: 06/05/24-06/05/24 Offset: Easting: 2803705 Depth to Water: 19.5 Elevation: 962.9 Requested Northing: 1572833.9 Depth Hole Open: Requested Station: 565+76.8 Requested Easting: 2803700.0 Time Change: At Time of Drilling Requested Offset: 14 L Equipment: CME 55 LC ,Grab Sample, Split-Spoon Sampler, NQ

Location Note: Offset off roadway

Requested Elevation: Drill No.: 360485-PPI Hammer Efficiency: 84.7% Drilling Method: Hollow Stem Auger Depth (ft) ROD Description 0.0-2.2' GRAVEL, loose, dry, well graded, (Shoulder) 2.2-7.0' Brown, LEAN CLAY, soft, moist 27 PP = 0.25 tsf γ_{sat} = 126 pcf⁽¹ (4) 7.0-19.2' Dark gray, LEAN CLAY, medium 2-3-5 10 27 PP = 0.75 tsf (11) $\gamma_{rot} = 127 \text{ pcf}^{(1)}$ 950 2-2-2 53 PP = 0.50 tsf $\gamma_{sat} = 125 \text{ pcf}^{(1)}$ 1-2-1 PP = 0.25 tsf 53 19.2-24.1' Dark gray, LEAN CLAY, soft, moist **γ** sat = 123 pcf¹¹ LL = 26 PL = 16 23.8-88.5' switched to mud rotary after SPT at . 100 Sieve # % Passing 3/8" 100.0 #4 99.0 #10 97.0 #16 94.5 23.5' 24.1-28.0' Dark brownish tan, SAND with clay, very loose, wet 87 28.0-38.0' Gray, CLAYEY SAND, loose, wet #40 50 0 #50 26.9 #100 13.8 #200 12.2 Sieve Analysis ieve # % Passir 3/8" 100.0 #4 99.6 #10 98.8 3-2-2 73 (6) 38.0-48.0' Gray, SAND, medium dense, wet #16 98.8 #40 98.0 #50 96.7 47 #100 79 7 920 40 (16)Sieve Analysis Sieve # % Passing 3/4" 100.0 3/8" 97.8 48.0-53.1' Gray, SAND with clay, medium 40 dense to dense, moist (20) #4 93.9 53.1-85.5' Dark gray, LEAN CLAY trace fine gravel, stiff to very stiff, moist, (GLACIAL TILL) PP = 2.00 tsf 73 (16)#40 37.6 #50 21.2 100 3-5-6

N₅₀ = (Em/60)Nm N₆₀ - Corrected N value for standard 60% SPT efficiency; Em - Measured hammer efficiency in percent; Nm - Observed N-value; (1) = Assumed, (2) = Actual Coordinate System: Modified U.S. State Plane 1983 Coordinate Zone: Missouri West Coordinate Proj. Factor: 1.0000983827 Coordinate Datum: NAD 83 (CONUS) Coordinate Units: U.S. Survey Feet

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(Continued Next Page)

Missouri Department of Transportation Construction and Materials

BORING NO. B-401 Page 2 of 2

	Construction and materials				
bb No. : NW0020 A9467	County: Worth	Route: _46			
esign: _A9467	Skew: Square	Location: 2 Miles West of Grant City			
ent: 4	Logged By: Matthew Kistler	Operator: Josh Starkey			
ation:	Northing: 1572832.9	Date of Work: 06/05/24-06/05/24			
ffset:	Easting: _2803705	Depth to Water: 19.5			
evation: 962.9	Requested Northing: 1572833.9	Depth Hole Open:			
equested Station: 565+76.8	Requested Easting: 2803700.0	Time Change: At Time of Drilling			
equested Offset: 14 L	Equipment: CME 55 LC ,Grab Sample, Split-S	Spoon Sampler, NQ			
equested Elevation:	Location Note: Offset off roadway				
equested Elevation:	Location Note: Offset off roadway				

Requested Elevation: Location Note: Offset off roadway									
Drill No	o.: _3	60485-PPI Hammer Eff	iciency	<u>/: 84</u>	.7%	D	rilling Method: _	Hollow Stem Auge	er
Depth (ft)	Graphic	Description	Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests	Index Tests
60	,,,,,					(46)			= 135 not ⁽¹⁾
		53.1-85.5' Dark gray, LEAN CLAY trace fine gravel, stiff to very stiff, moist, (GLACIAL TILL) (continued)	900	×	100	3-5-7 (17)		PP = 1.50 tsf	y _{sot} = 135 pct ^(γ) MC = 20.2% y _{sot} = 130 pcf ^(γ) LL = 35 PL = 15 MC = 20.0% y _{sot} = 130 pcf ^(γ)
70			-	\times	100	4-5-7 (17)		PP = 1.75 tsf	MC = 19.8% y _{sat} = 131 pcf ⁽¹⁾
			890						
			-	X	100	4-5-7 (17)		PP = 1.75 tsf	MC = 19.5% y _{sal} = 131 pcf ⁽¹⁾
80			-	\times	100	4-6-8 (20)		PP = 2.25 tsf	MC = 18.3% y _{sat} = 132 pcf ⁽¹⁾
			880						
		•	-	\times	100	5-8-10 (25)		PP = 2.00 tsf	MC = 17.2% y _{sat} = 134 pcf ⁽¹⁾
	· VI.	85.5-88.5' Limestone, highly weathered	-						
90		88.5-90.5' Limestone, light whitish gray, thin bedded, strong rock, slightly weathered, fine grained g 90.5-93.1' Shale, brownish tan, thinly laminated, weak rock, moderately weathered 93.1-93.5' Limestone, light whitish gray, thin bedded, strong rock, slightly weathered, fine grained	870		100 (22)	24/0.0', 10/0'	Qu Test Results UCS = 696 ksf MC = 1.2%		
80 80 90 90 PO		Bottom of borehole at 93.5 feet.							
N _{so} = (En	m/60\N	Im N ₆₀ - Corrected N value for standard 60% SPT efficiency	: Fm - M	leasur	ed hamm	er efficiency in perce	nt: Nm - Observed N	l-value: (1) = Assume	ed. (2) = Actual

Coordinate Proj. Factor: 1.0000983827

Coordinate System: Modified U.S. State Plane 1983 Coordinate Zone: Missouri West Coordinate Units: U.S. Survey Feet Coordinate Datum: NAD 83 (CONUS)

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BORING DATA

Note: For locations of borings, see Sheet No. 1.

Detailed: Oct. 2024 Checked: Oct. 2024

Note: This drawing is not to scale. Follow dimensions. Sheet No. 30 of 30

JEFF A. GARDNER NUMBER PE-2016019369

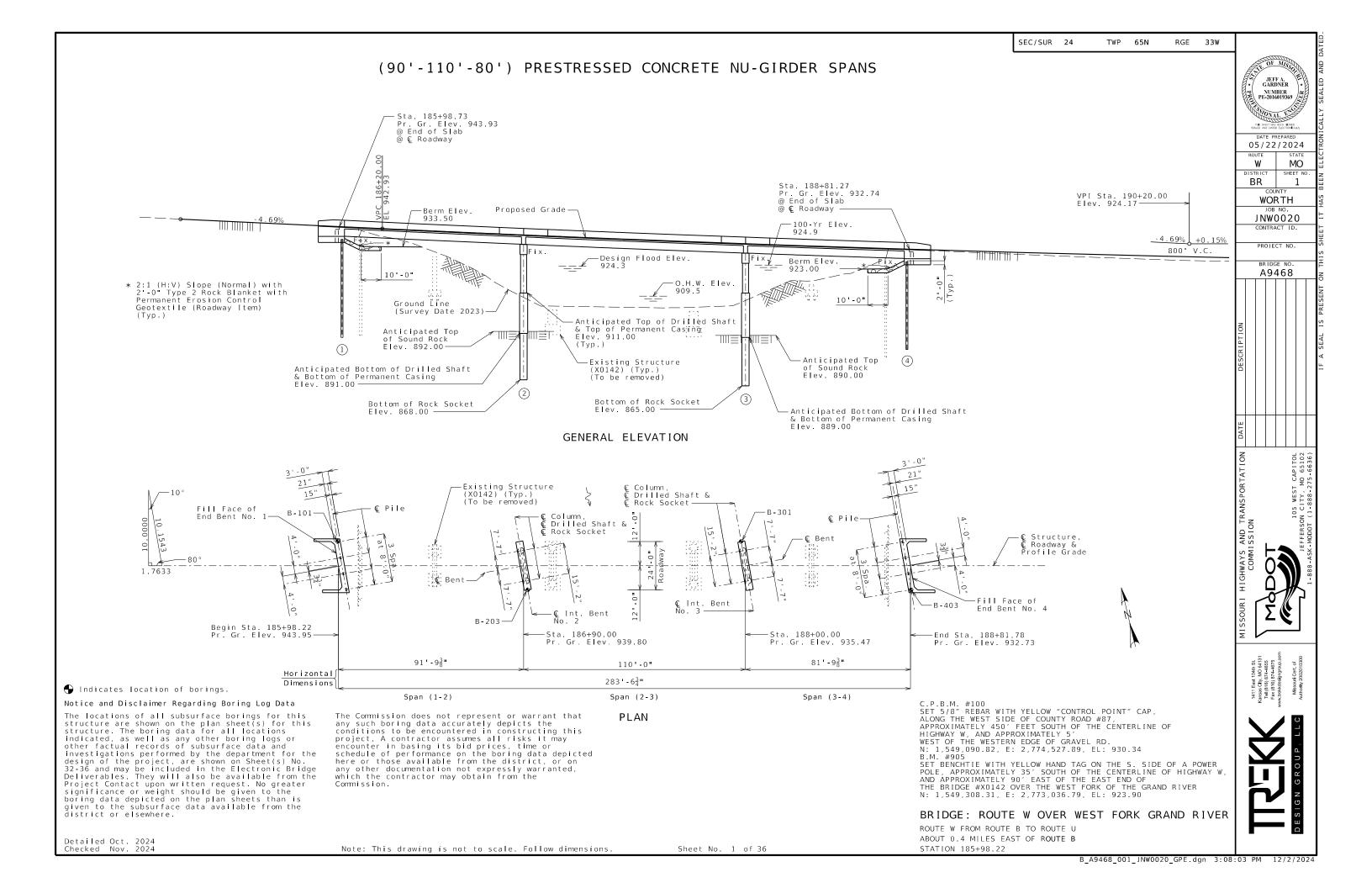
12/3/2024 46 MO SHEET NO BR 30

WORTH

JNW0020

PROJECT NO A9467





Estimated Quantities				
I t em		Substr.	Superstr.	Total
Class 1 Excavation	cu. yard	80	-	80
Removal of Bridges (X0142)	lump sum	-	-	1
Bridge Approach Slab (Minor)	sq. yard	-	109	109
Drilled Shaft (4ft. Oin. Dia.)	linear foot	80.0	-	80.0
Rock Sockets (3ft. 6in. Dia.)	linear foot	98.0	-	98.0
Video Camera Inspection	each	4	-	4
Foundation Inspection Holes	linear foot	138.0	-	138.0
Sonic Logging Test	each	4	-	4
Galvanized Structural Steel Piles (12 in.)	linear foot	372	-	372
Dynamic Pile Testing	each	2	-	2
Pre-Bore for Piling	linear foot	190	-	190
Pile Point Reinforcement	each	8	-	8
Class B Concrete (Substructure)	cu. yard	102.6	-	102.6
Type D Barrier	linear foot	-	607	607
Slab on Concrete NU-Girder	sq. yard	-	837	837
NU-43 Prestressed Concrete NU-Girder	linear foot	-	840	840
Reinforcing Steel (Bridges)	pound	33,250	-	33,250
Vertical Drain at End Bent	each	-	-	2
Laminated Neoprene Bearing Pad	each	-	12	12
Laminated Neoprene Bearing Pad (Tapered)	each	-	6	6

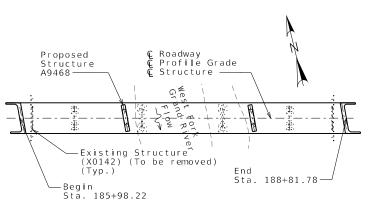
All concrete above the construction joint in the end bents is included in the Estimated Quantities for Slab on Concrete NU-Girder.

All reinforcement in the end bents is included in the Estimated Quantitites for Slab on Concrete NU-Girder.

All reinforcement in the intermediate bent concrete diaphragms except reinforcement embedded in the beam cap is included in the Estimated Quantities for Slab on Concrete NU-Girder.

All concrete above the intermediate beam cap is included in the Estimated Quantities for Slab on Concrete NU-Girder.

Hydrologic Data
Drainage Area = 249 sq. mi.
Design Flood Frequency = 50 yr.
Design Flood Discharge = 18,500 cfs
Design Flood (D.F.) Elevation = 924.3 ft.
Base Flood (100-Year)
Base Flood Elevation = 924.9
Base Flood Discharge = 21,300 cfs.
Estimated Backwater = 0.2 ft.
Average Velocity thru Opening = 4.1 ft./p
Freeboard (50-year)
Freeboard = 5.7 ft.
Roadway Overtopping
Overtopping Flood Discharge = 12,800 cfs.
Overtopping Flood Frequency = 12-yr
Overtopping Flood Elevation = 922.7 ft.



LOCATION SKETCH

	Founda	ation Dat	t a		
			Bent 1	Numb e r	
Type	Design Data	1	2	3	4
	Pile Type and Size	HP 12x53	-	-	HP 12x53
	Number ea	4	-	-	4
	Approximate Length Per Each ft	49	=	-	44
Load	Pile Point Reinforcement ea	All	=	-	AII
Bearing Pile	Min. Galvanized Penetration (Elev.) ft	Full Length	=	-	Full Length
1116	Pile Driving Verification Method	DT	-	-	DT
	Resistance Factor	0.65	-	-	0.65
	Minimum Nominal Axial Compressive Resistance kip	358	<u>-</u>	-	341
	Number ea	-	2	2	-
	Foundation Material	-	Weak Rock	Weak Rock	-
	Elevation Range ft	-	888 - 878	886-874	-
	Minimum Nominal Axial Compressive Resistance (Side Resistance) ksf	-	4.0	4.0	-
	Foundation Material	-	Strong Rock	Strong Rock	-
Rock Socket	Elevation Range ft	-	878-855	874-861	-
Socket	Minimum Nominal Axial Compressive Resistance (Side Resistance) ksf	-	12.9	12.9	-
	Minimum Nominal Axial Compressive Resistance (Tip Resistance) ksf	_	34.0	39.2	-

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	-		
		ticipated to be driven to refusal on rock. Review all	
		ck and restrict driving as appropriate to comply with	11
ur	iving criteri	a in accordance with Sec 702.	

Load Bearing Pile: DT = Dynamic Testing Minimum Nominal Axial Compressive Resistance = Maximum Factored Loads

Detailed Oct. 2024

Minimum Nominal Axial Compressive Resistance (Side Resistance + Top Resistance) = Maximum Factured Loads/Resistance Factors.

Prebore for piles at Bent No. 1 to elevation 889.00.

Manufactured pile point reinforcement shall be used on all piles in this structure.

borings hard řock

All piles shall be galvanized down to the minimum galvanized penetration (elevation).

Pile point reinforcement need not be galvanized. Shop drawings will not be required for pile point reinforcement.

Thickness of permanent steel casing shall be in accordance with Sec 701.

Sonic logging testing shall be performed on all drilled shafts and rock sockets.

Note: This drawing is not to scale. Follow dimensions. Sheet No. 2 of 36

General Notes:

Design Specifications: 2020 AASHTO LRFD Bridge Design Specification (9th Ed) Seismic Design Category = A (Nonseismic) Design earthquake response spectral acceleration coefficient at 1.0 second period, SD1 = 0.104g

Acceleration Coefficient (effective peak ground acceleration coefficient),

Design Loading:

Design Loading:
Vehicular = HL-93
Future Wearing Surface = 35 lb/sf
Earth = 120 lb/cf, Equivalent Fluid Pressure 45 lb/cf
Superstructure: Simply-Supported, Non-Composite for dead load.
Continuous Composite for live load.

Design Unit Stresses:

Class B Concrete (Substructure, except Drilled Shaft & Rock Sockets) f'c = 3,000 psiClass B-2 Concrete (Drilled Shafts & Rock Sockets)
Class B-2 Concrete (Superstructure, except
Prestressed Girders and Barrier) f'c = 4,000 psif'c = 4,000 psi f'c = 4,000 psi Class B-1 Concrete (Barrier) Reinforcing Steel (ASTM A615 Grade 60) Structural Steel HP Pile (ASTM A709 Grade 50) fy = 60,000 psify = 50,000 psiFor NU-Girders, see Sheets No. 14 thru 19.

Neoprene Pads:

Neoprene Bearing Pads shall be 60 durometer and shall be in accordance with Sec 716.

Joint Filler:

All joint filler shall be in accordance with Sec 1057 fo preformed sponge rubber expansion and partition joint filler, except as noted.

Reinforcing Steel:

Minimum clearance to reinforcing steel shall be 11/2", unless otherwise shown

Class B-2 Concrete

Reinforcing Steel (Epoxy Coated)

Structure to be closed to traffic during construction. Traffic to be maintained on other routes during construction. See Roadway plans for traffic control.

Estimated Ouantities for Slab on Concrete NU-Girder I t em

The table of Estimated Quantities for NU-Girder represents the quantities used by the State in preparing the cost estimate for concrete slabs. The area of the concrete slab will be measured to the nearest square yard longitudinally from end of slab to end of slab and transversely from out to out of bridge slab (or with the transversery from out to out of bridge stab (of with the horizontal dimensions as shown on the plan of slab). Payment for stay-in-place corrugated steel forms, conventional forms, all concrete and epoxy coated reinforcing steel will be considered completely covered by the contract unit price for the slab.

Variations may be encountered in the estimated quantities but the variations cannot be used for an adjustment in the contract unit

variations cannot be used for an adjustment in the contract unit

Method of forming the slab shall be as shown on the plans and in accordance with Sec 703. All hardware for forming the slab to be left in place as a permanent part of the structure shall be oated in accordance with ASTM A123 or ASTM B633 with a thickness lass SC 4 and a finish type I, II or III.

slab shall be cast-in-place with conventional forms or stay-in-place corrugated steel forms. Precast prestressed panels will not be permitted.

JEFF A. GARDNER NUMBER PE-2016019369

11/26/2024 W MO SHEET NO BR 2

WORTH

JNW0020 CONTRACT ID.

PROJECT NO

BRIDGE NO A9468

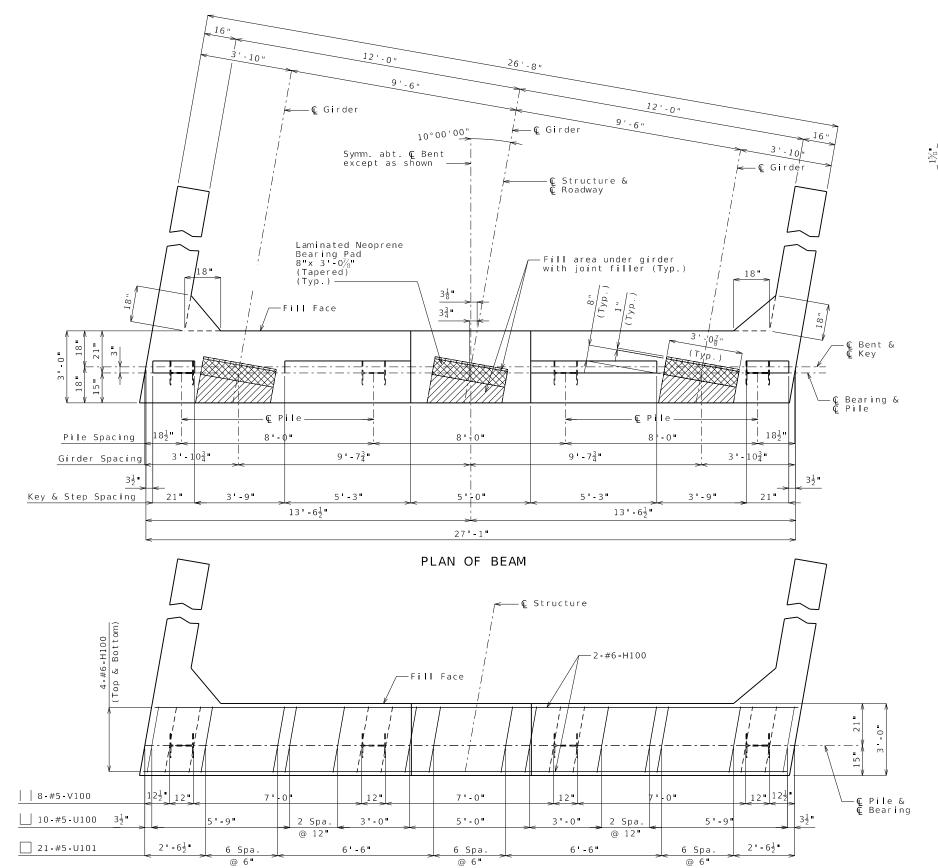


GENERAL NOTES AND QUANTITIES

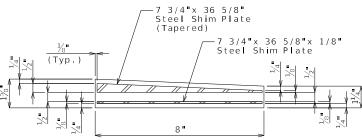
Total

262

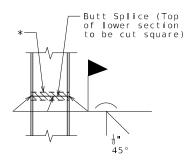
cu. yard



Ahead Station_



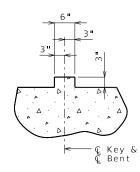
SECTION THRU LAMINATED NEOPRENE BEARING PAD



STEEL PILE SPLICE

(If required)

* Galvanizing material shall be omitted or removed one inch clear of weld locations in accordance with Sec 702.



SECTION THRU KEY

General Notes:

Work this sheet with Sheets No. 4 & 5.

All U bars and pairs of V bars shall be placed parallel to centerline of roadway.

Reinforcing steel shall be shifted to clear piles. U bars shall clear piles by at least 1 1/2".



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DATE PREPARED					
11/26/2024					
OUTE	STATE	Ľ			
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STRICT	SHEET NO.	z			
BR	3	Ц Ц 2			
COUNTY					
WORTH					
JOB	NO.	ľ			

JNW0020

PROJECT NO





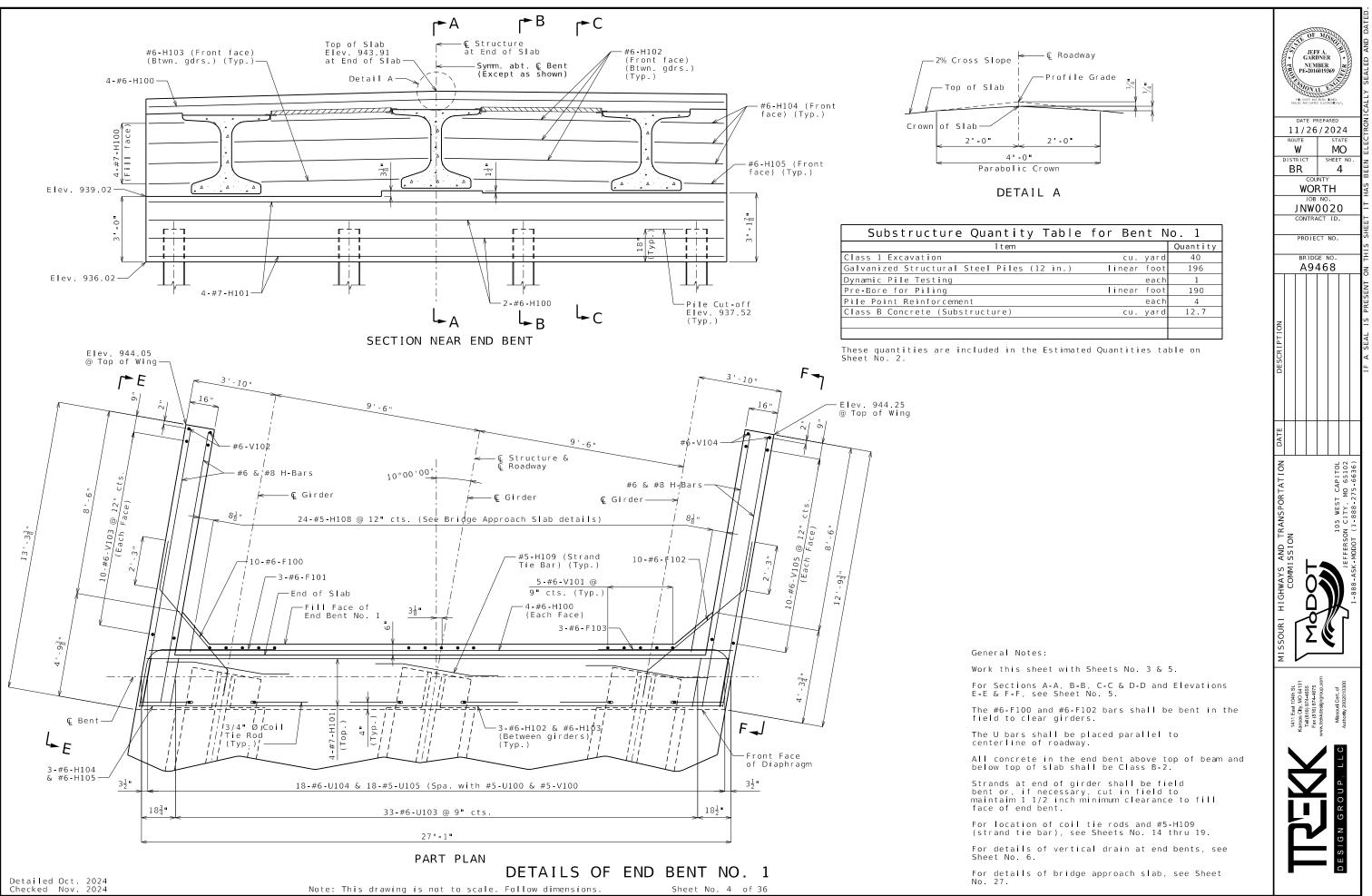


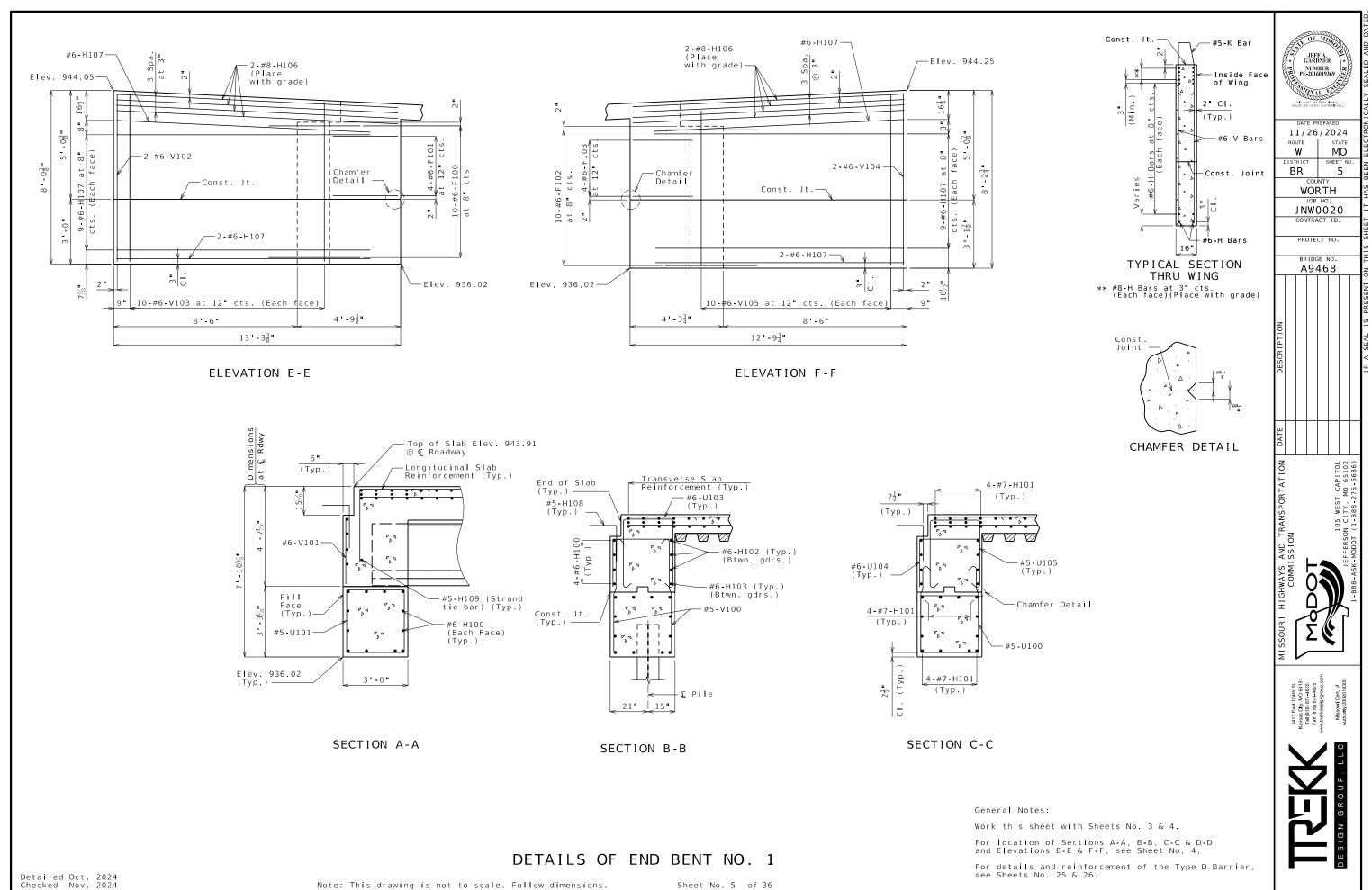
PLAN OF BEAM SHOWING REINFORCEMENT

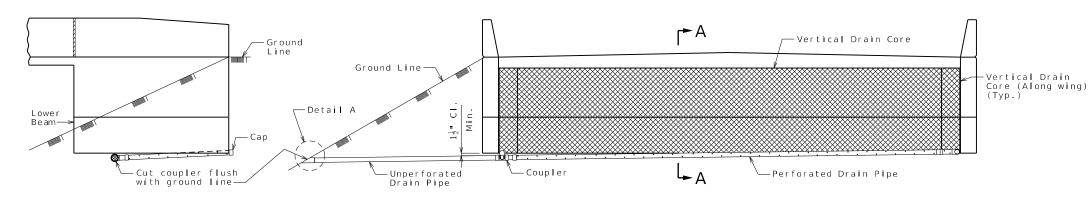
Keys not shown for clarity

Detailed Oct. 2024 Checked Nov. 2024

DETAILS OF END BENT NO. 1 Sheet No. 3 of 36



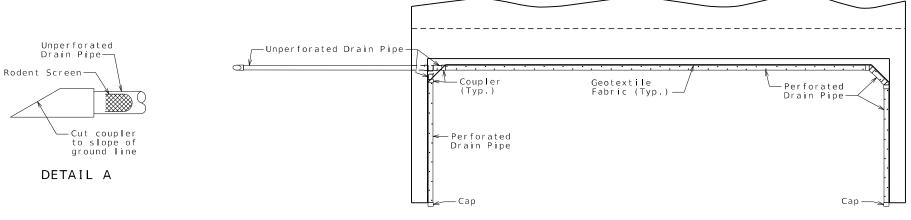




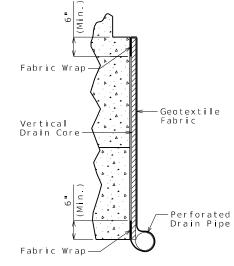
ELEVATION OF WING

Detailed Oct. 2024 Checked Nov. 2024

ELEVATION OF END BENT



PLAN OF END BENT



PART SECTION A-A (Section thru wing similar)

General Notes:

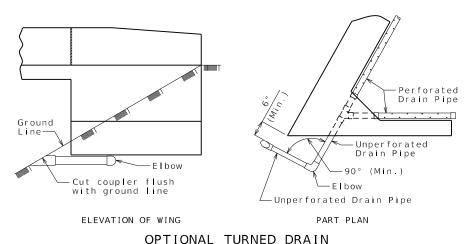
percent.

All drain pipe shall be sloped 1 to 2

Drain pipe may be either 6-inch diameter corrugated metallic-coated steel pipe underdrain, 4-inch diameter corrugated polyvinyl chloride (PVC) drain pipe, or 4-inch diameter corrugated polyethylene (PE)

Drain pipe shall be placed at fill face of end bent and inside face of wings. The pipe shall slope to lowest grade of ground line, also missing the lower beam of end bent by a minimum of 1 1/2 inches.

Perforated pipe shall be placed at fill face side and inside face of wings at the bottom of end bent and plain pipe shall be used where the vertical drain ends to the exit at ground line.



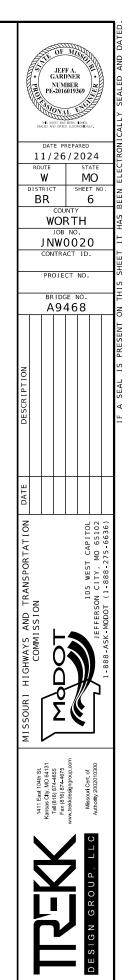
(Use only when straight drain is not practical.)

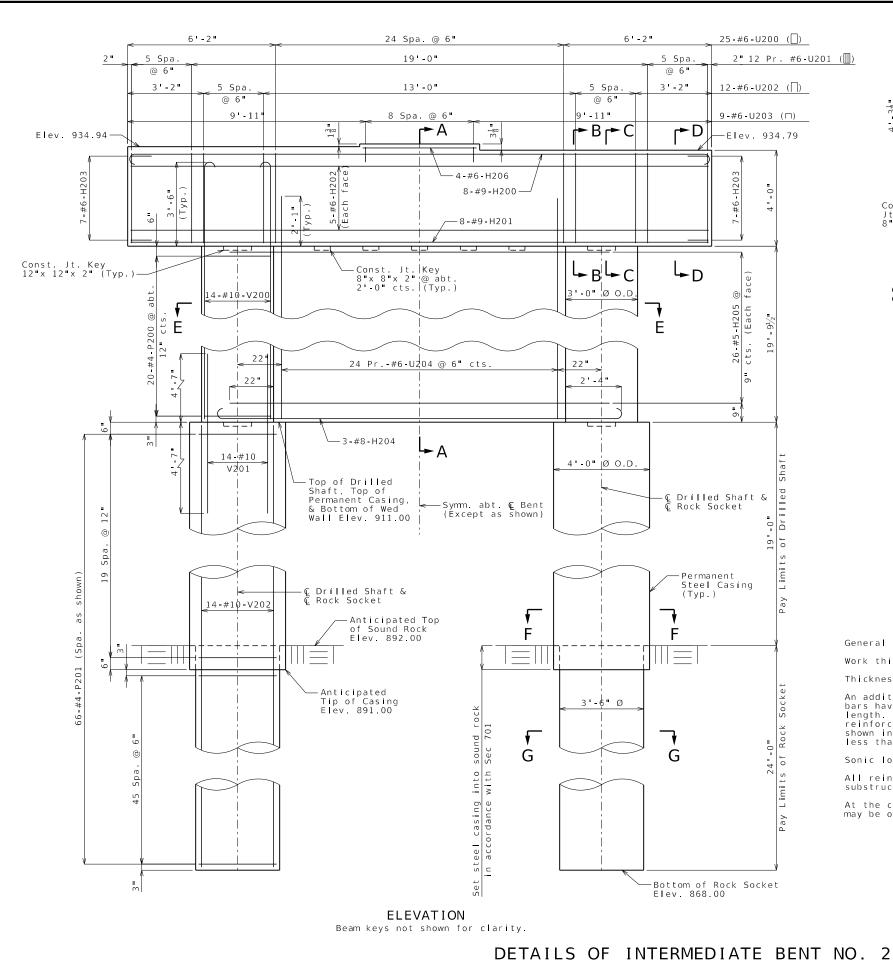
VERTICAL DRAIN AT END BENTS

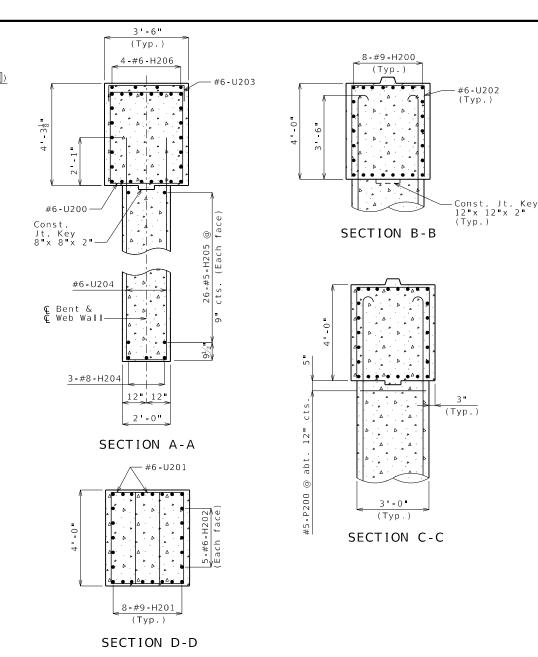
(Squared end bent shown, skewed end bent similar)

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 6 of 36







General Notes:

Work this sheet with Sheet No. 8.

Thickness of permanent steel casing shall be in accordance with Sec 701.

An additional 4 feet has been added to V-bar lengths and additional 18-#4-P201 bars have been added for possible change in drilled shaft or rock socket length. The additional V-bar length shall be cut off or included in the reinforcement lap if not required. The P bars shall be spaced similarly to that shown in Elevation, if required, or a lesser spacing if not required but not less than 6-inch centers.

Sonic logging testing shall be performed on all drilled shafts and rock sockets.

All reinforcement in drilled shafts and rock sockets is included in the substructure quantities.

At the contractor's option, the hooks of vertical bars embedded in the beam cap may be oriented inward or outward.

Substructure Quantity Tabl	le for Bent N	o. 2
I t em		Quantity
Drilled Shaft (4ft. Oin. Dia.)	linear foot	38
Rock Sockets (3ft. 6in. Dia.)	linear foot	48
Video Camera Inspection	each	2
Foundation Inspection Holes	linear foot	68
Sonic Logging Test	each	2
Class B Concrete (Substructure)	cu. yard	41.8
Reinforcing Steel (Bridges)	pound	16,940

These quantities are included in the Estimated Quantities table on Sheet No. 2.

Detailed Oct. 2024 Checked Nov. 2024

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 7 of 36

JEFF A. GARDNER

NUMBER PE-2016019369

11/26/2024

WORTH

JNW0020

PROJECT NO

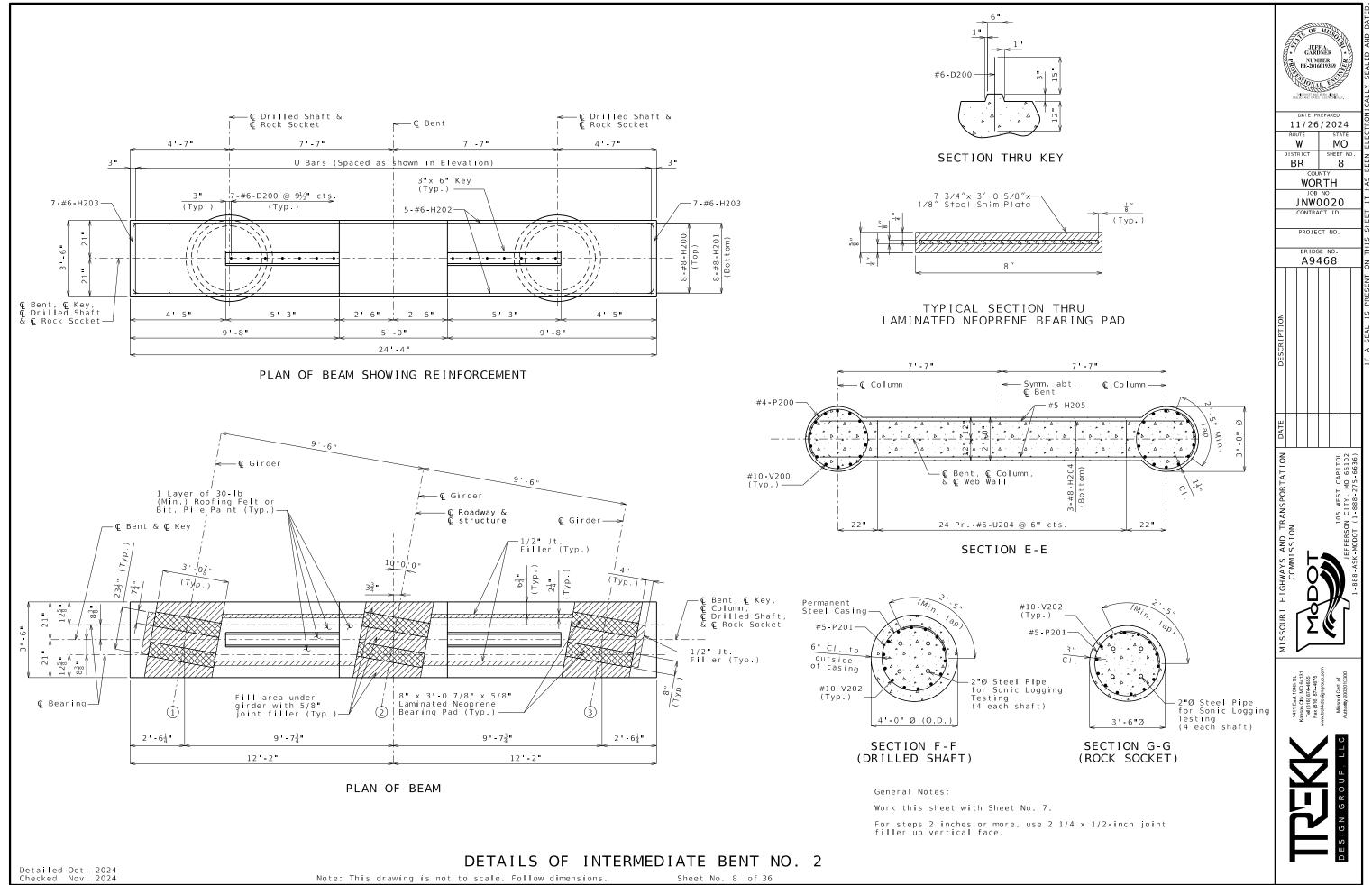
BRIDGE NO A9468

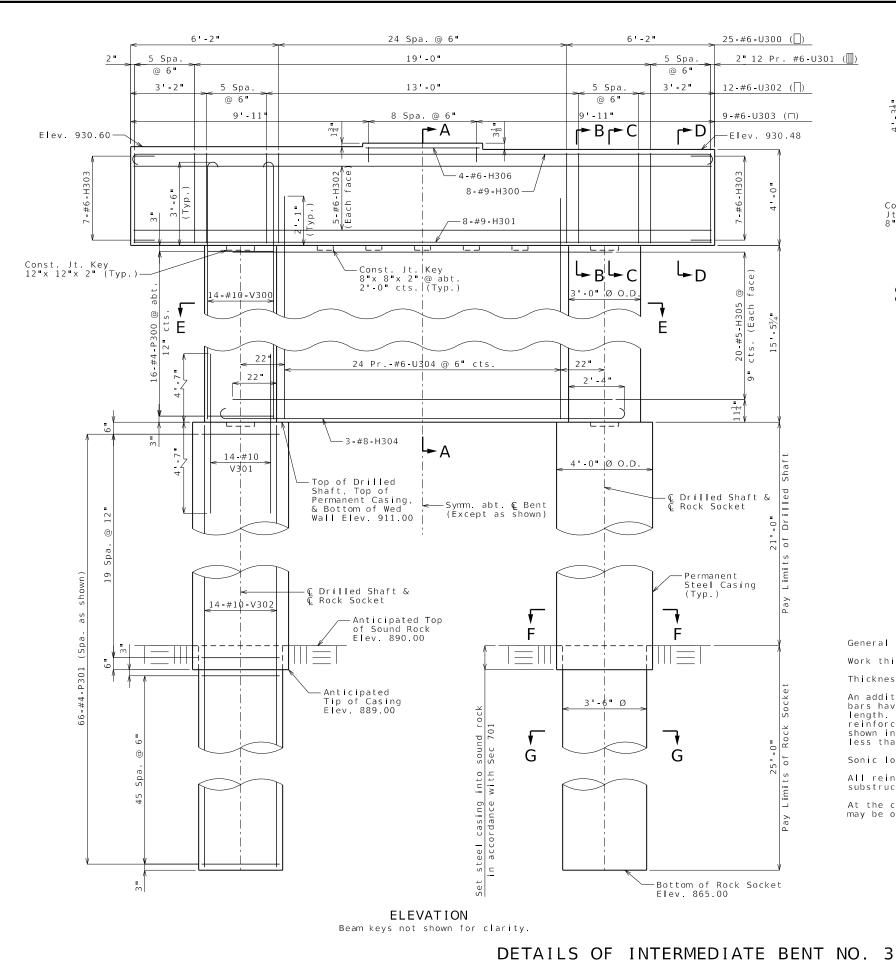
MO SHEET NO

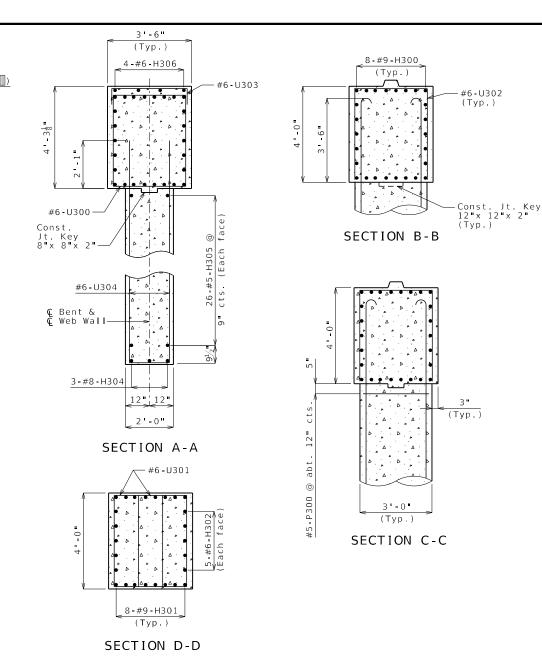
7

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BR







General Notes:

Work this sheet with Sheet No. 10.

Thickness of permanent steel casing shall be in accordance with Sec 701.

An additional 4 feet has been added to V-bar lengths and additional 18-#4-P301 bars have been added for possible change in drilled shaft or rock socket length. The additional V-bar length shall be cut off or included in the reinforcement lap if not required. The P bars shall be spaced similarly to that shown in Elevation, if required, or a lesser spacing if not required but not less than 6-inch centers.

Sonic logging testing shall be performed on all drilled shafts and rock sockets.

All reinforcement in drilled shafts and rock sockets is included in the substructure quantities.

At the contractor's option, the hooks of vertical bars embedded in the beam cap may be oriented inward or outward.

Substructure Quantity Table	e for Bent N	o. 3
I t em		Quantity
Drilled Shaft (4ft. Oin. Dia.)	linear foot	42
Rock Sockets (3ft. 6in. Dia.)	linear foot	50
Video Camera Inspection	each	2
Foundation Inspection Holes	linear foot	70
Sonic Logging Test	each	2
Class B Concrete (Substructure)	cu. yard	35.5
Reinforcing Steel (Bridges)	pound	16,310

These quantities are included in the Estimated Quantities table on Sheet No. 2.

Detailed Oct. 2024 Checked Nov. 2024

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 9 of 36

JEFF A. GARDNER

NUMBER PE-2016019369

11/26/2024

WORTH

JNW0020

PROJECT NO

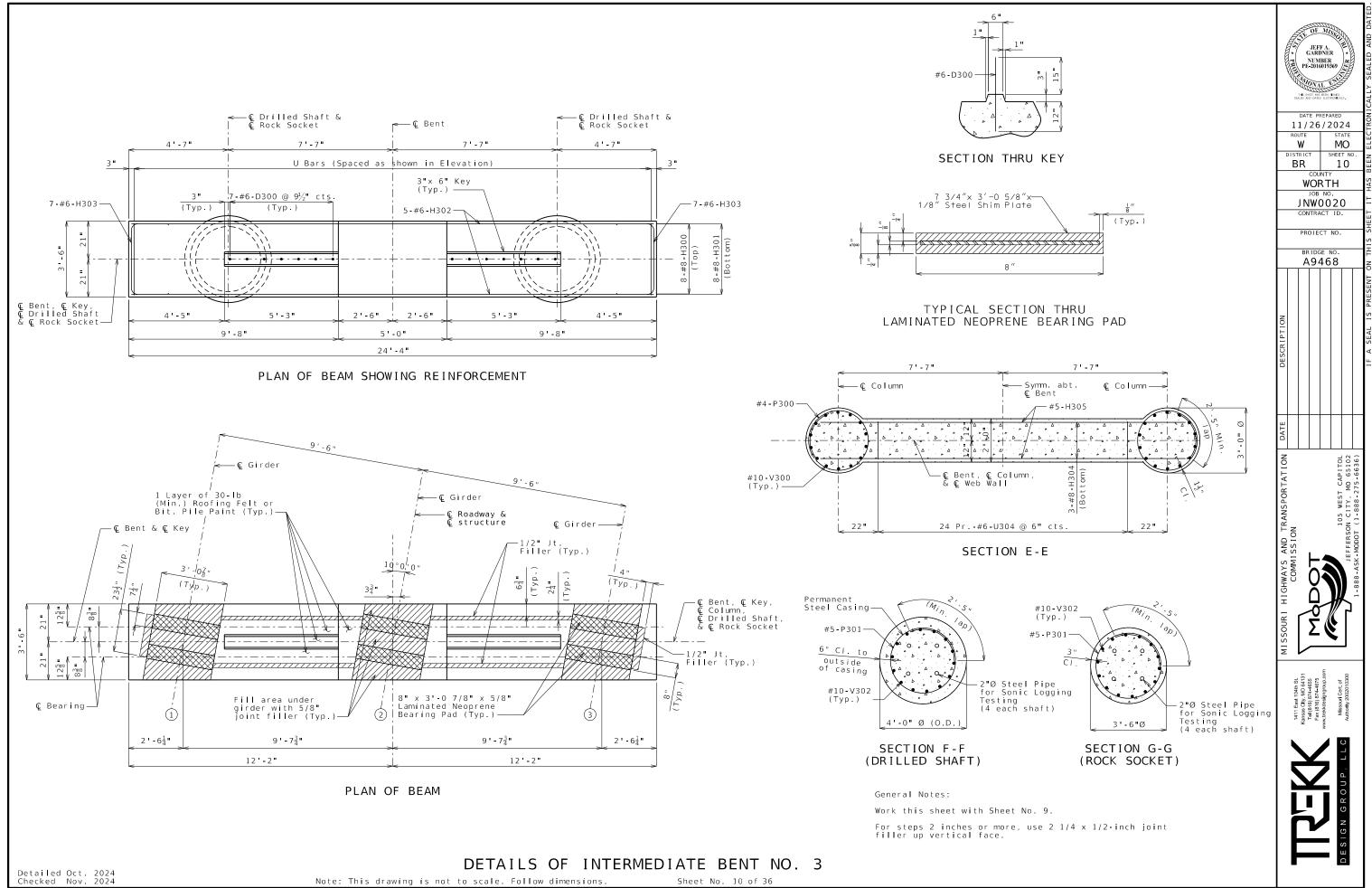
BRIDGE NO A9468

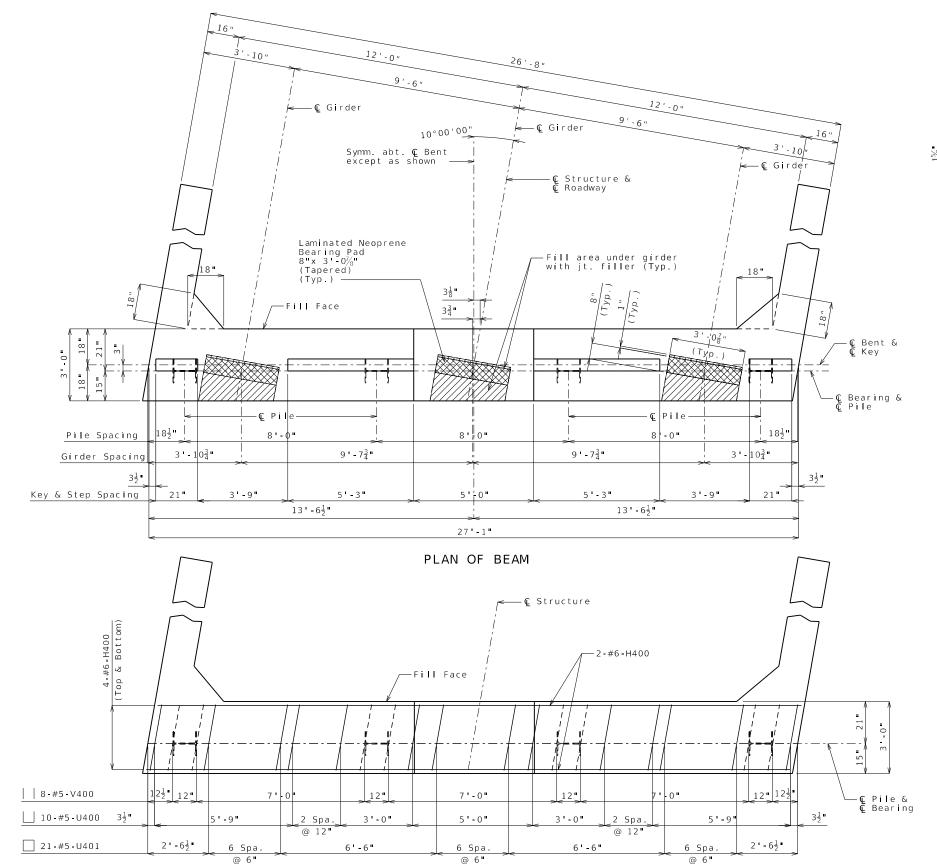
MO SHEET NO

9

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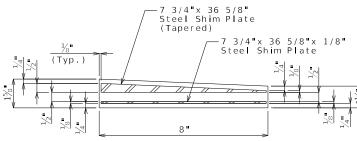
PLAN OF BEAM SHOWING REINFORCEMENT Keys not shown for clarity

Detailed Oct. 2024 Checked Nov. 2024 DETAILS OF END BENT NO. 4

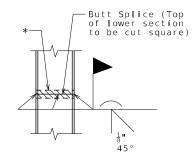
Note: This drawing is not to scale. Follow dimensions.

Sheet No. 11 of 36

Ahead Station>



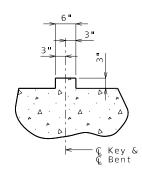
SECTION THRU LAMINATED NEOPRENE BEARING PAD



STEEL PILE SPLICE

(If required)

* Galvanizing material shall be omitted or removed one inch clear of weld locations in accordance with Sec 702.



SECTION THRU KEY

General Notes:

Work this sheet with Sheets No. 12 & 13.

All U bars and pairs of V bars shall be placed parallel to centerline of roadway.

Reinforcing steel shall be shifted to clear piles. U bars shall clear piles by at least 1 1/2".



DATE PREPARED				
11/26/2024				
ROUTE STATE				
W MO				
DISTRICT	SHEET NO.			
BR 11				
COUNTY				
WORTH				

WORTH

JOB NO.
JNW0020

PROJECT NO.

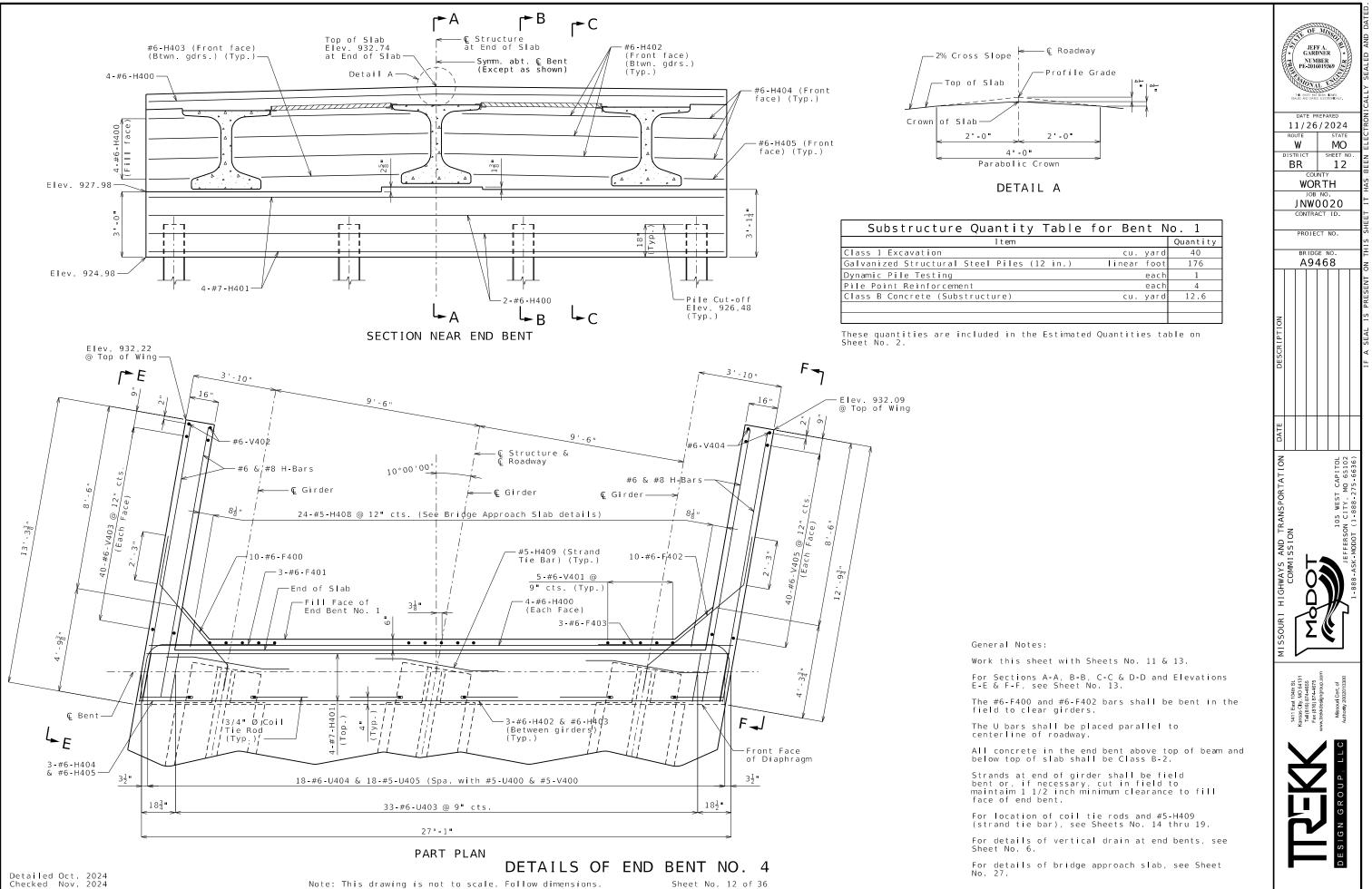
BRIDGE NO.
A9468

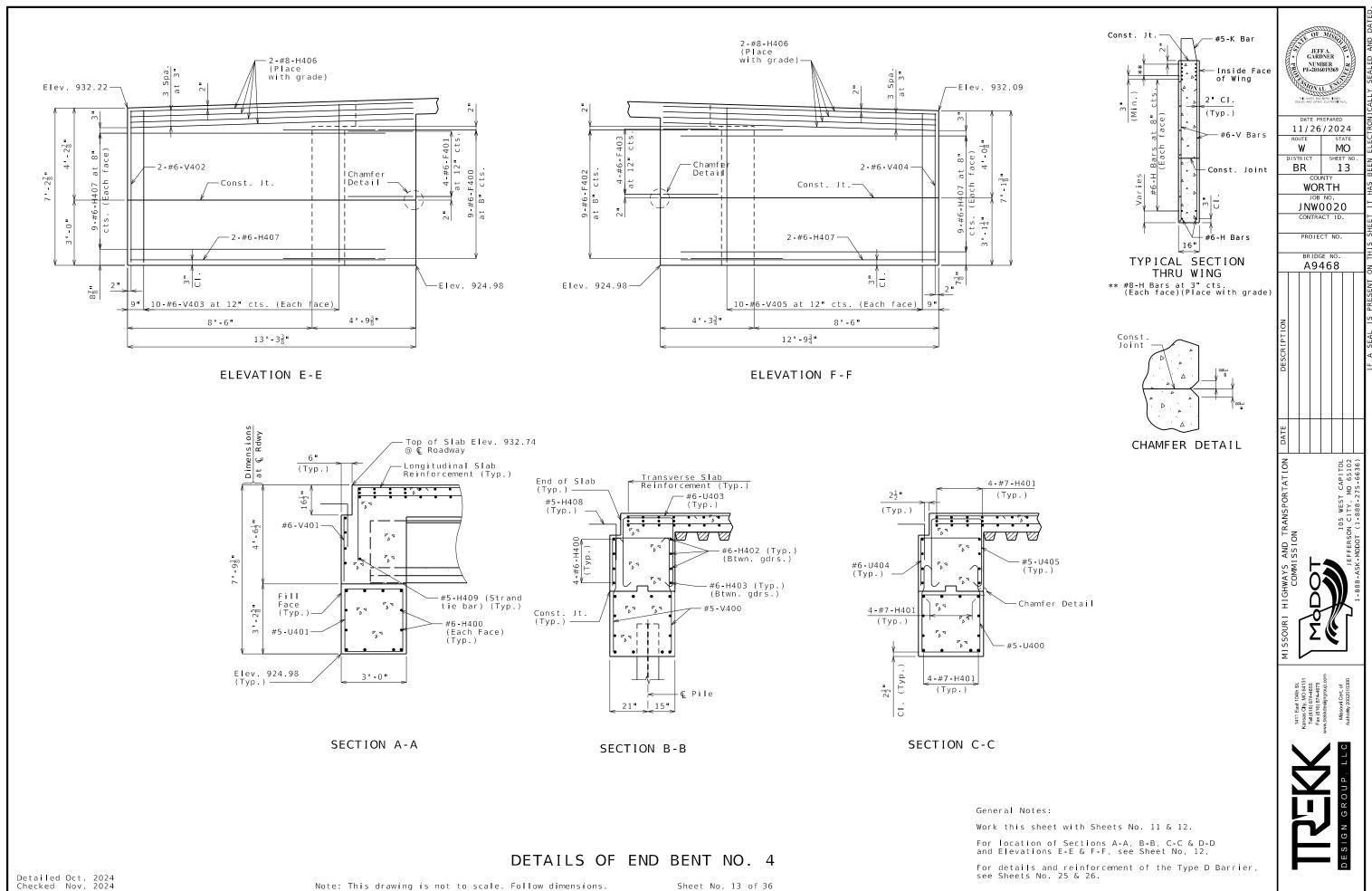
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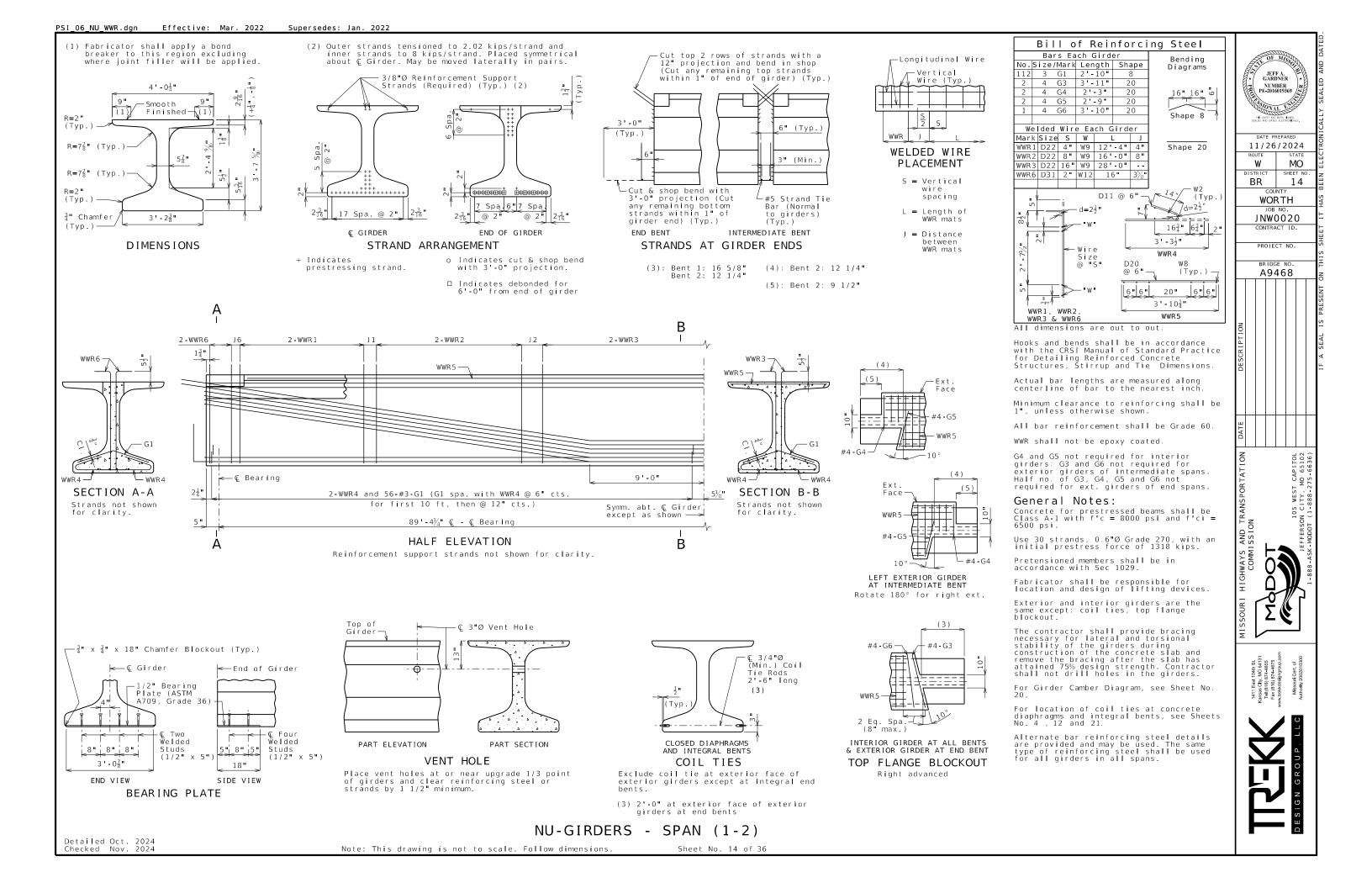
105 WEST CAPITOL
LEFERSON CITY, MO 65102

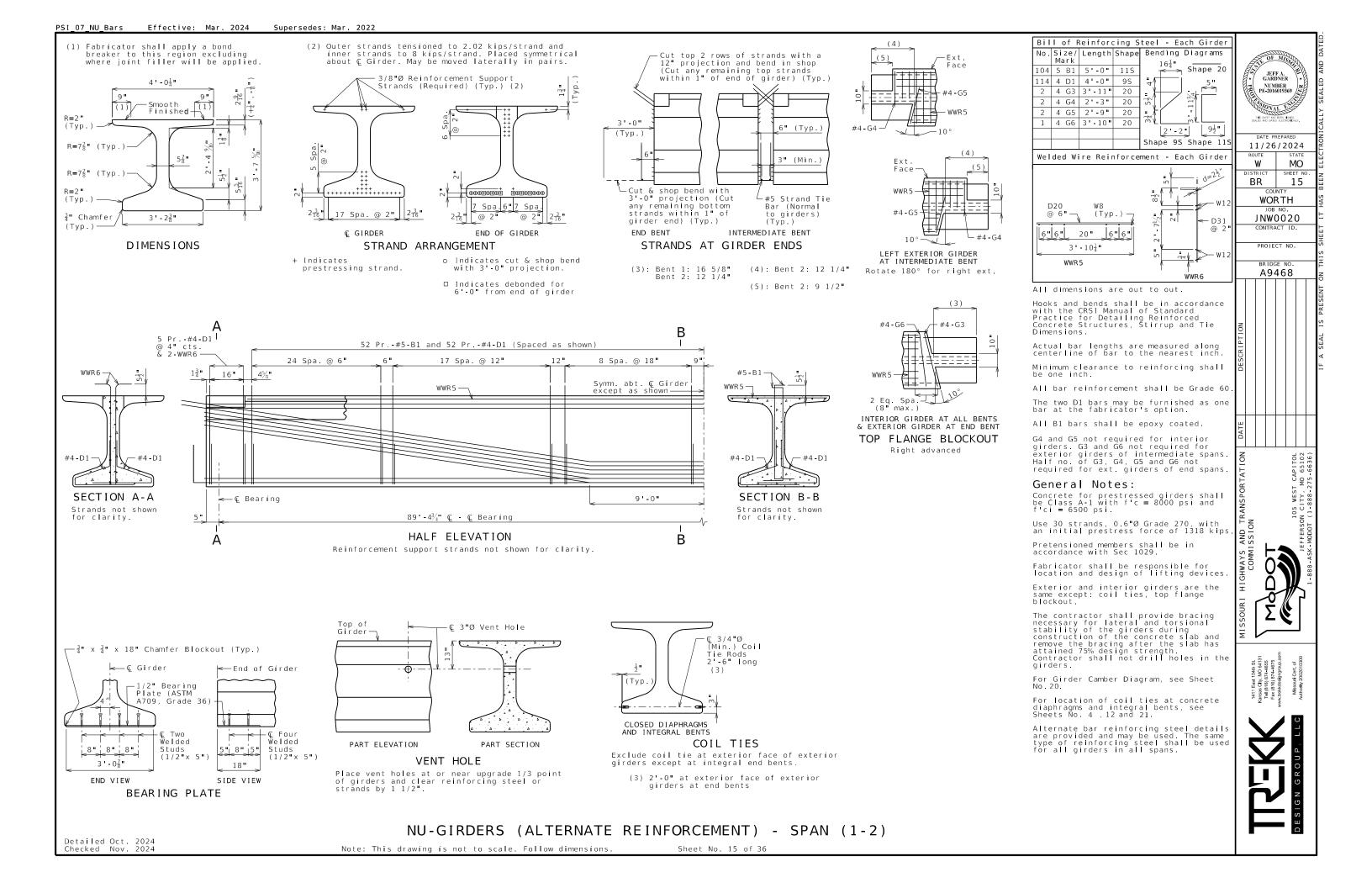
1411 East 104th St.
4ansas Clty, MO 64131
(1616) 874-4655
Fax (816) 874-4675
w.trekkdesigngroup.com
Missouri Cert. of

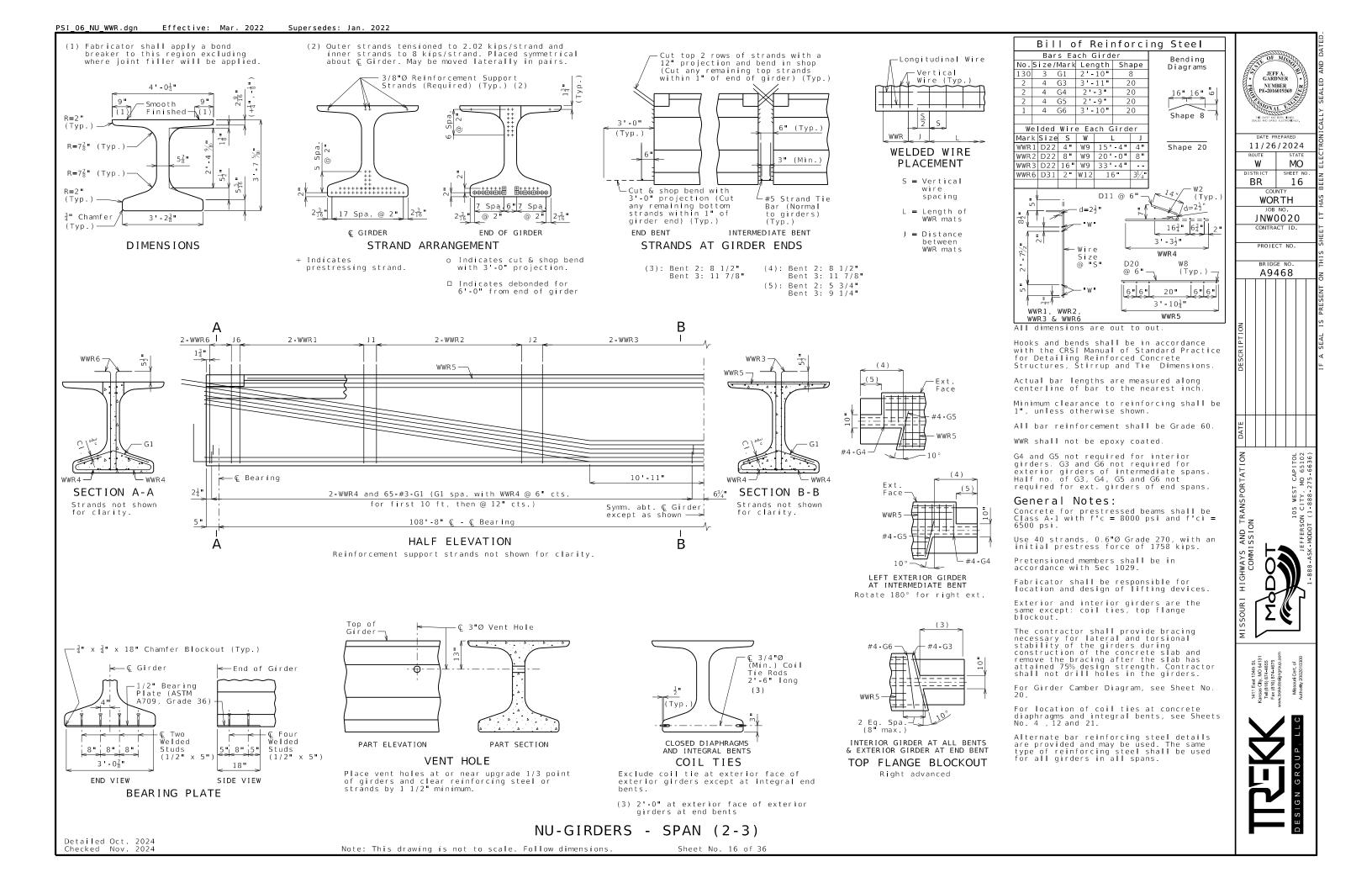


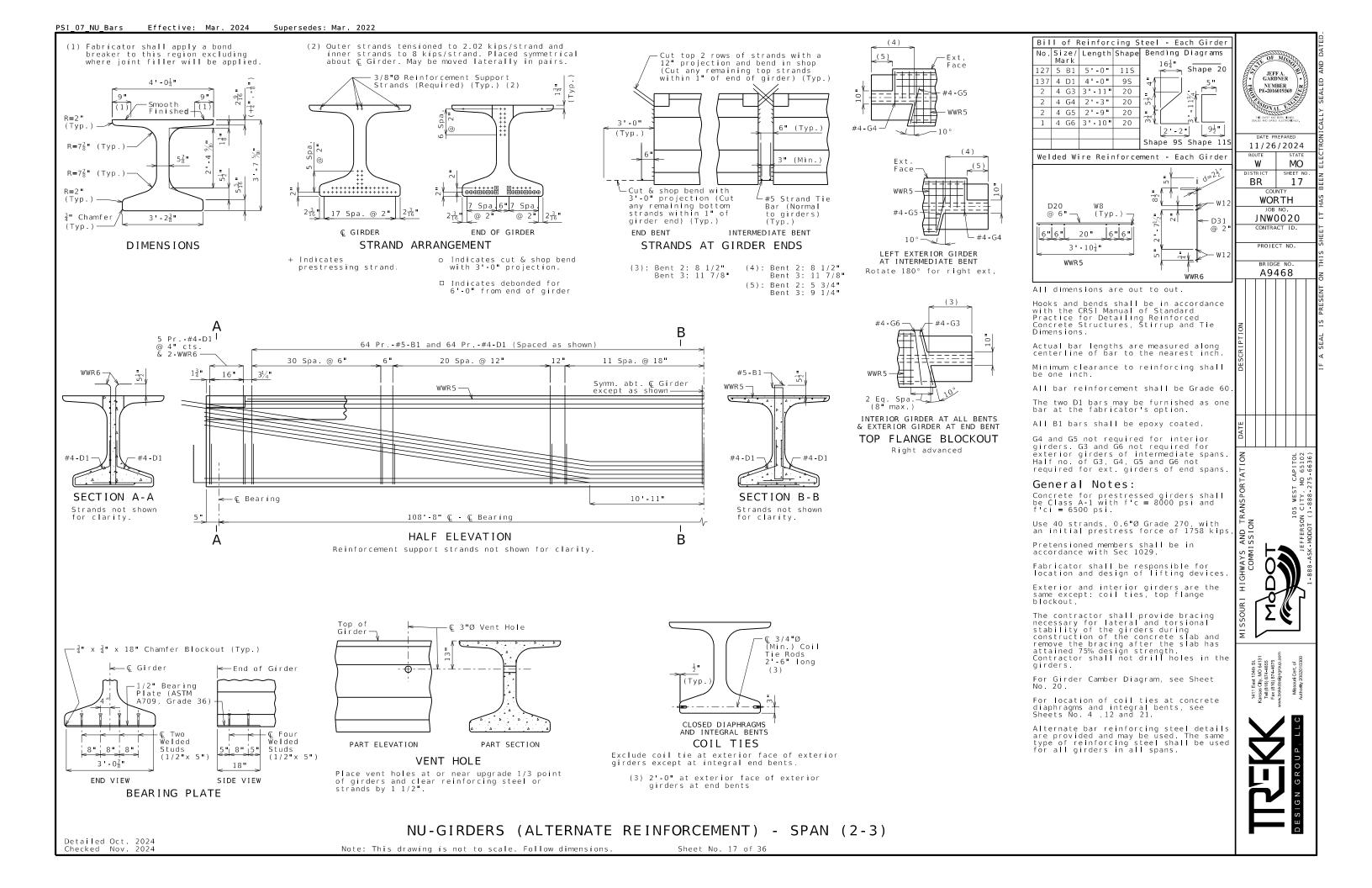


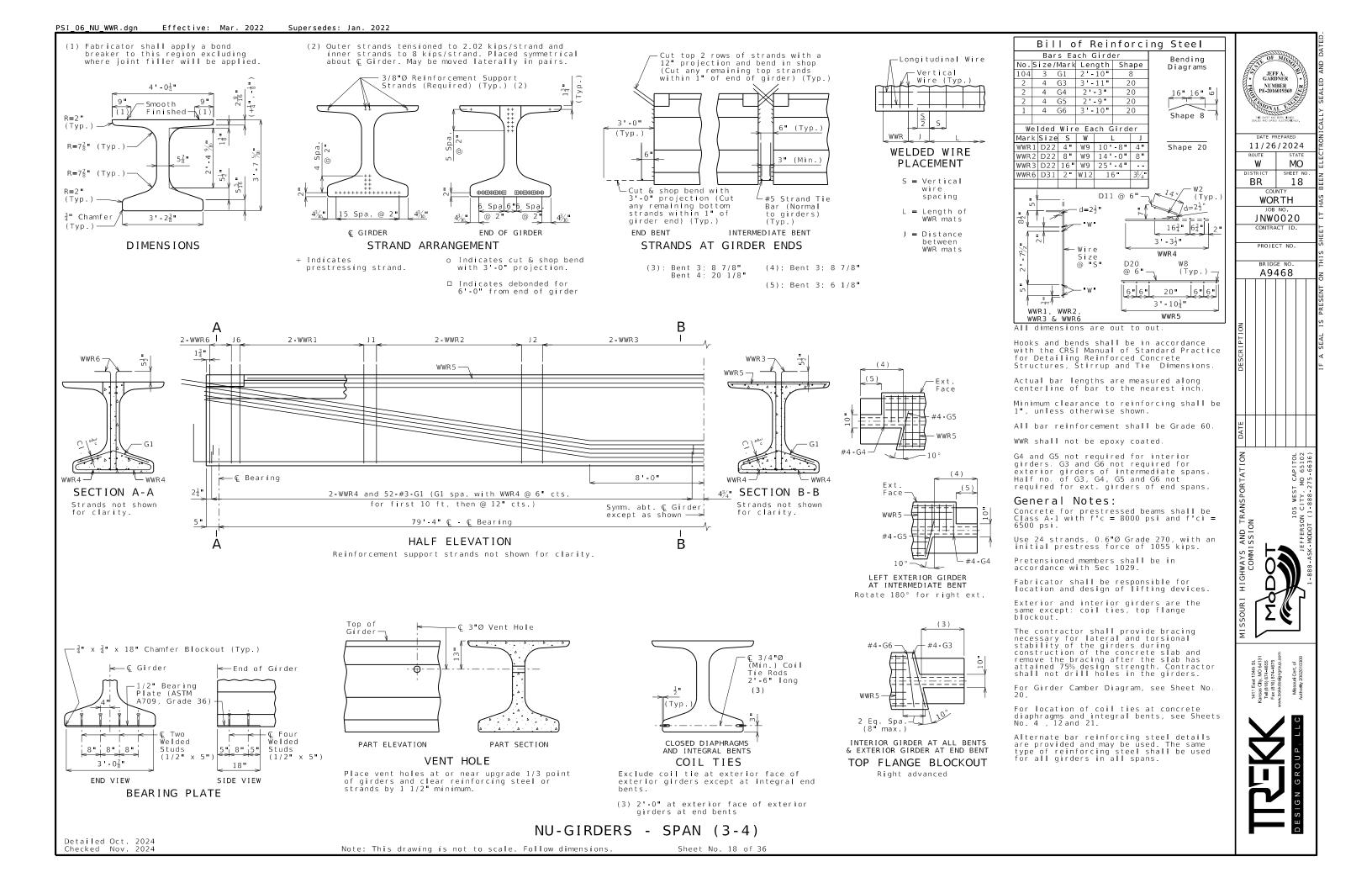


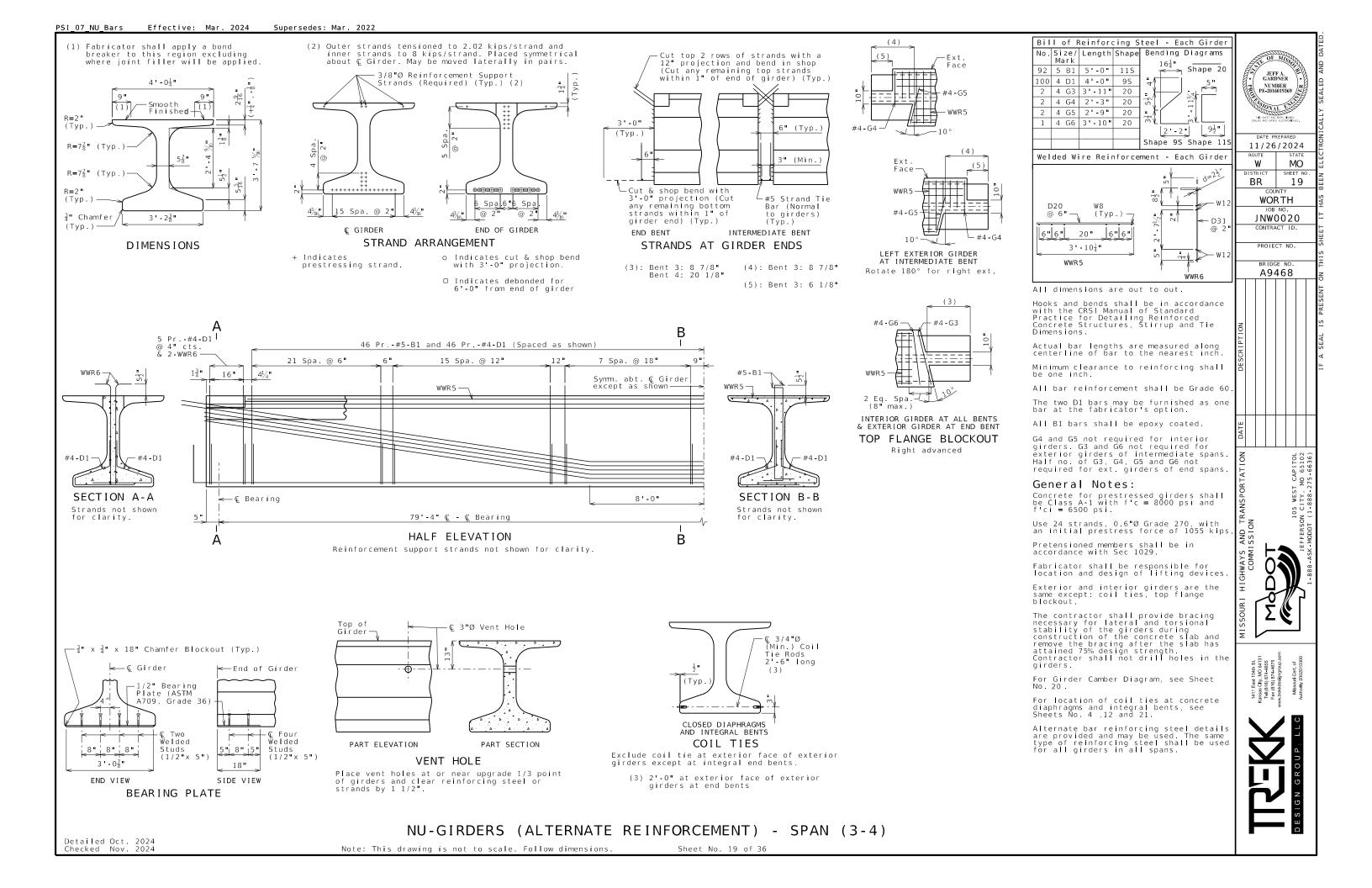


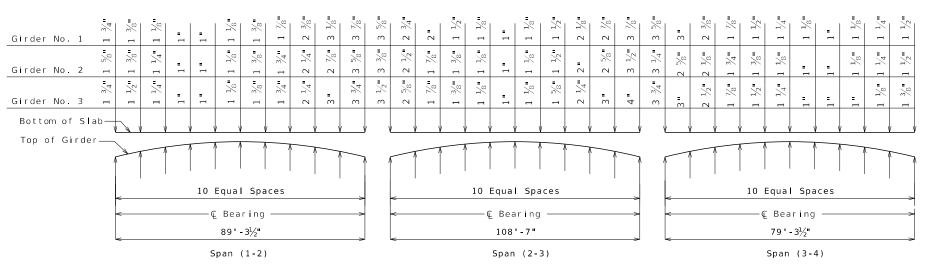


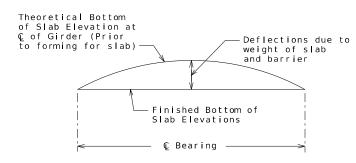












TYPICAL SLAB ELEVATIONS DIAGRAM

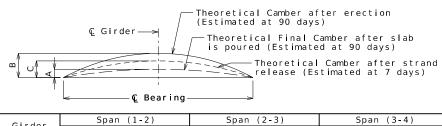
THEORETICAL SLAB HAUNCHING DIAGRAM (ESTIMATED AT 90 DAYS)

If girder camber is different from that shown in the camber diagram, in order to maintain minimum slab thickness, an adjustment of the slab haunches, an increase in slab thickness or a raise in grade uniformly throughout the structure shall be necessary. No payment will be made for additional labor or materials required for variation in haunching, slab thickness or grade adjustment.

Concrete in the slab haunches is included in the Estimated Quantities for Slab on Concrete NU-Girder.

The								Center ed at			der
Girder				Span (1-2) (89)'-3½" ©	Brg Ç	Brg.)			
Number	€ Brg.	.10	. 20	. 30	. 40	. 50	. 60	.70	.80	.90	€ Brg.
1	942.89	942.47	942.05	941.63	941.22	940.81	940.40	940.00	939.61	939.22	938.84
2	943.14	942.72	942.30	941.88	941.46	941.05	940.65	940.25	939.85	939.46	939.08
3	943.05	942.63	942.21	941.79	941.37	940.96	940.55	940.15	939.76	939.37	938.98
Girder				Span (2-3) (10)8'-7" €	Brg - ©	Brg.)			
Number	€ Brg.	.10	. 20	. 30	. 40	. 50	. 60	.70	.80	.90	€ Brg.
1	938.81	938.35	937.90	937.45	937.01	936.58	936.15	935.73	935.32	934.92	934.52
2	939.06	938.59	938.14	937.69	937.25	936.81	936.38	935.96	935.55	935.15	934.75
3	938.96	938.49	938.04	937.59	937.14	936.71	936.28	935.86	935.44	935.04	934.64
Girder				Span (3-4) (79)'-3½" ©	Brg (£	Brg.)			
Number	€ Brg.	. 10	. 20	. 30	. 40	. 50	. 60	. 70	.80	. 90	€ Brg.
1	934.50	934.22	933.94	933.66	933.39	933.12	932.85	932.59	932.33	932.08	931.83
2	934.73	934.45	934.16	933.89	933.61	933.34	933.08	932.82	932.56	932.30	932.05
3	934.62	934.34	934.05	933.77	933.50	933.23	932.96	932.70	932.44	932.19	931.93

Elevations are based on a constant slab thickness of 8 1/2" and include allowance for theoretical dead load deflections due to weight of slab (including precast panel) and barrier.



Girder	S	pan (1-2	2)	S	pan (2-3	3)	Span (3-4)				
Girder	Α	В	С	Α	В	С	Α	В	С		
Exterior	1"	21/2"	13/8"	15/8"	43/4"	13/4"	3/4"	15/8"	1 "		
Interior	½ "	-/2	-/0	11/4"	74	-/4	5/8"	-78	_		

GIRDER CAMBER DIAGRAM

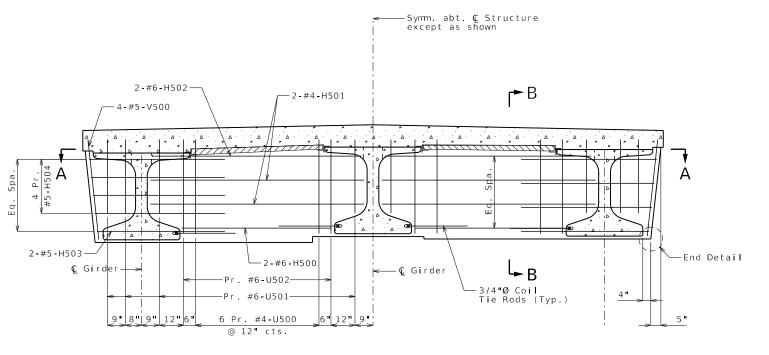
Conversion Factors for Girder Camber (Estimated at 90 days):

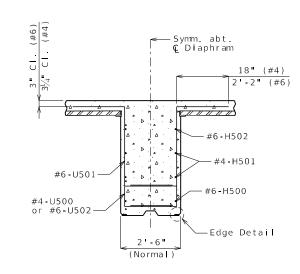
0.1 pt. = 0.314 x 0.5 pt. 0.2 pt. = 0.593 x 0.5 pt. 0.3 pt. = 0.813 x 0.5 pt. 0.4 pt. = 0.952 x 0.5 pt.



1411 East 104th St. Kansas Clty, MO 64131 Tel (816) 874-4655 Fax (816) 874-4675 www.trekkdestgingroup.com



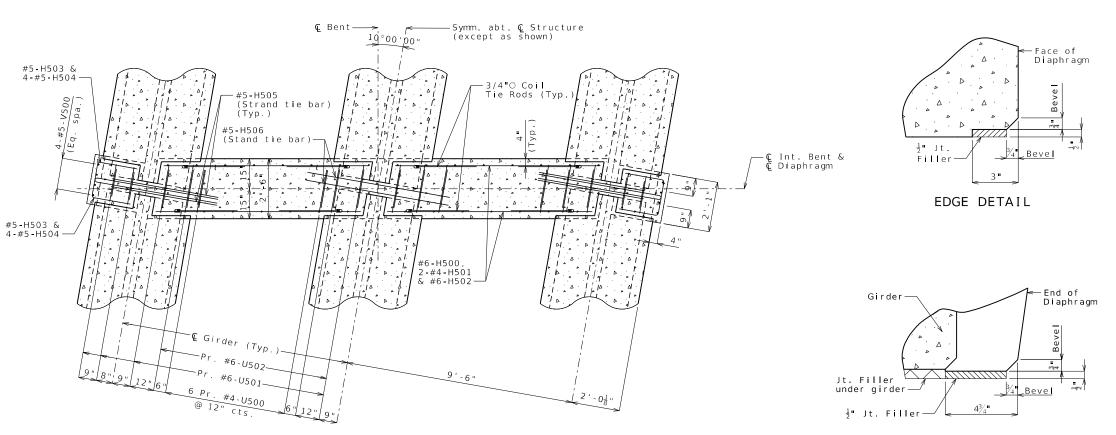




SECTION B-B

SECTION NEAR INTERMEDIATE BENT Normal to © Structure

SECTION A-A



END DETAIL

CONCRETE DIAPHRAGMS AT INTERMEDIATE BENT NO. 2 & 3

11/26/2024

WORTH

JNW0020

PROJECT NO.

BRIDGE NO

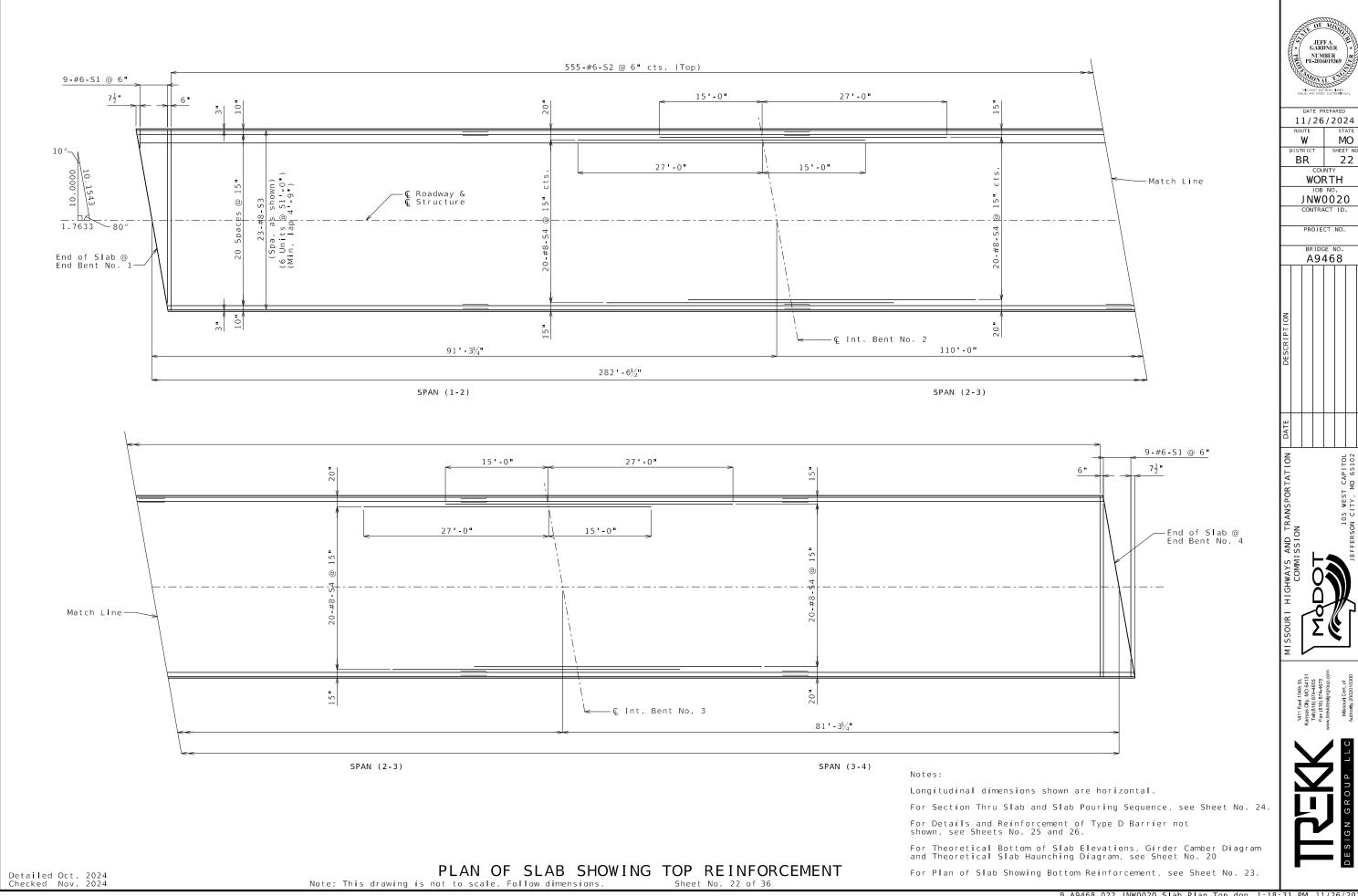
A9468

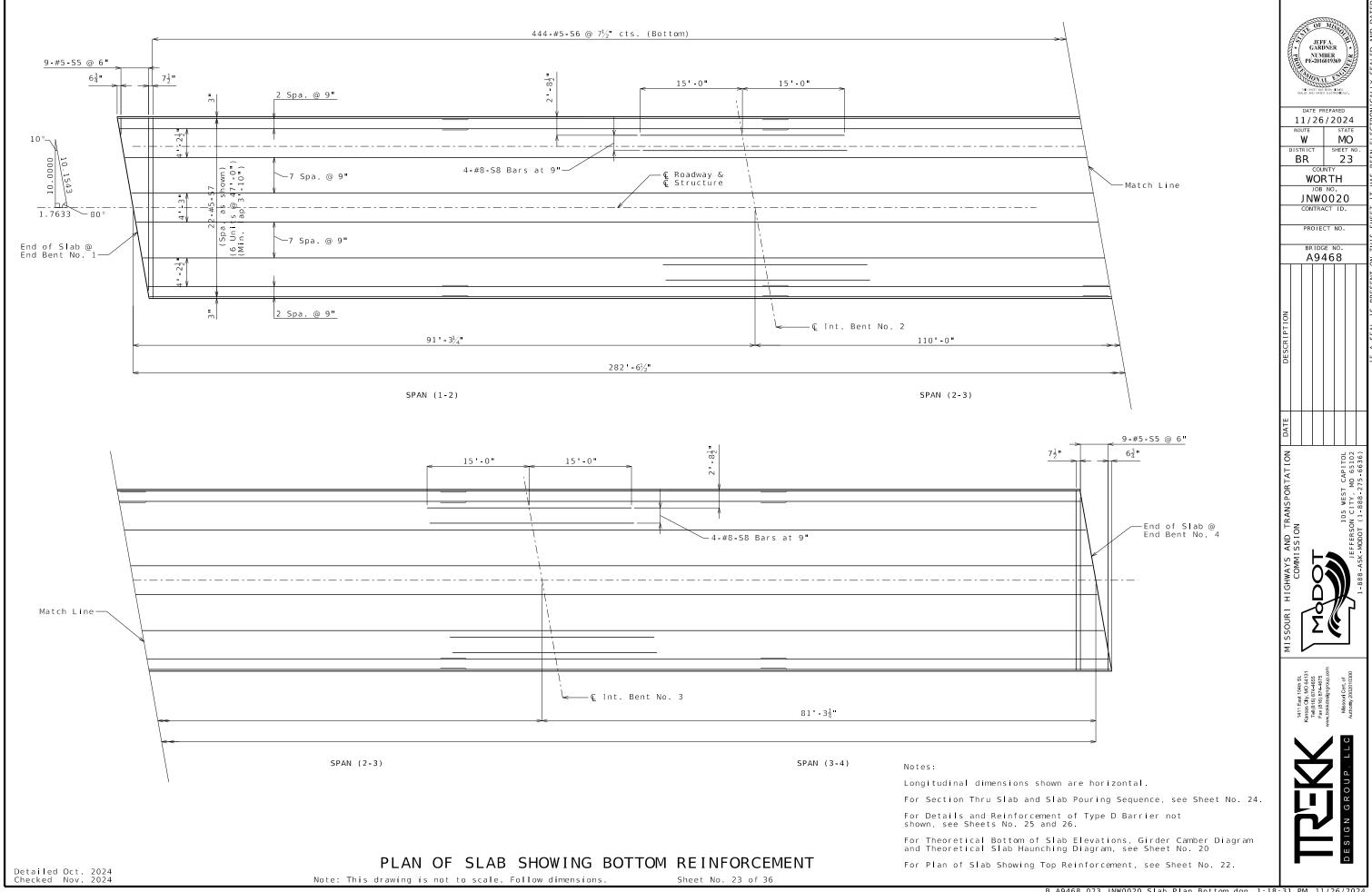
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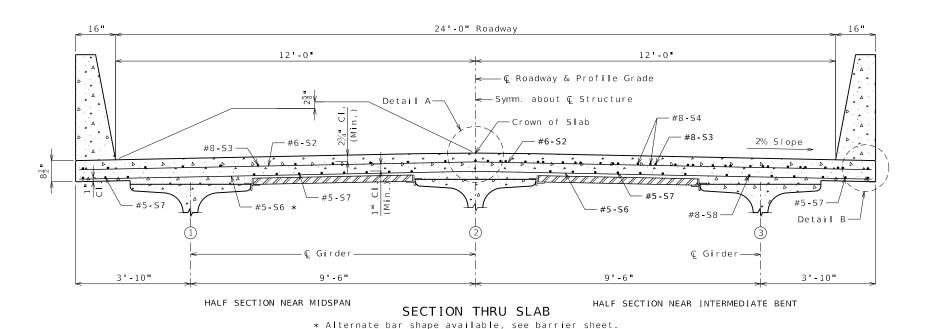
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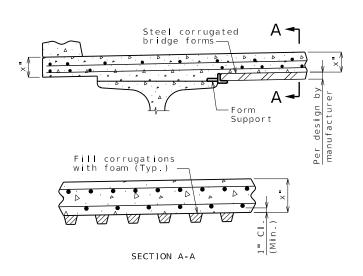
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OPTIONAL STAY-IN-PLACE FORM DETAILS

Stay-In-Place Forms:

Corrugated steel forms, supports, closure elements and accessories shall be in accordance with grade requirement and coating designation G165 of ASTM A653. Complete shop drawings of the permanent steel deck forms shall be required in accordance with Sec 1080.

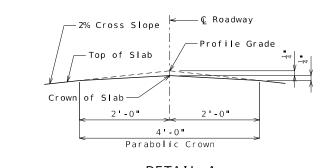
Corrugations of stay-in-place forms shall be filled with an expanded polystyrene material. The polystyrene material shall be placed in the forms with an adhesive in accordance with the manufacturer's recommendations.

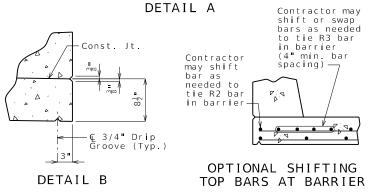
Form sheets shall not rest directly on the top of girder flanges. Sheets shall be securely fastened to form supports with a minimum bearing length of one inch on each end. Form supports shall be placed in direct contact with the flange.

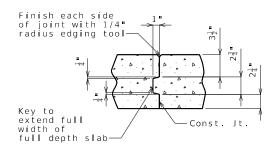
Drilling holes in the girder flanges will not be permitted.

All steel fabrication and construction shall be in accordance with Sec 1080 and 712. Certified field welders will not be required for welding of the form supports.

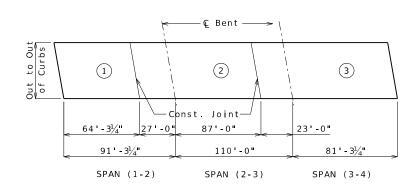
The design of stay-in-place corrugated steel forms is per manufacturer which shall be in accordance with Sec 703 for false work and forms. Maximum actual weight of corrugated steel forms allowed shall be 4 psf assumed for girder loading.







FULL DEPTH SLAB SLAB CONSTRUCTION JOINT



	Se	urs	Min. Rate of Pour Cu. Yds./Hr.			
			With Retarder			
Basic	1	2	3	25		
Sequence		!	25			
Alternate pour engineer in ac	s to the basic s cordance with Se	kip sequence are succ 703.	ubject to the app	roval of the		
Alternate A	1	+ 2	3	37		
Pours	End	2 to End] 37			
Alternate B		1 + 2 + 3		59		
Pours		39				

The contractor shall pour and satisfactorily finish the slab pours at the rate given. Retarder, if used, shall be an approved type and retard the set of concrete to 2.5 hours.

SLAB POURING SEOUENCE

Notes:

For reinforcement of barrier not shown, see Sheets No. 25 & 26.

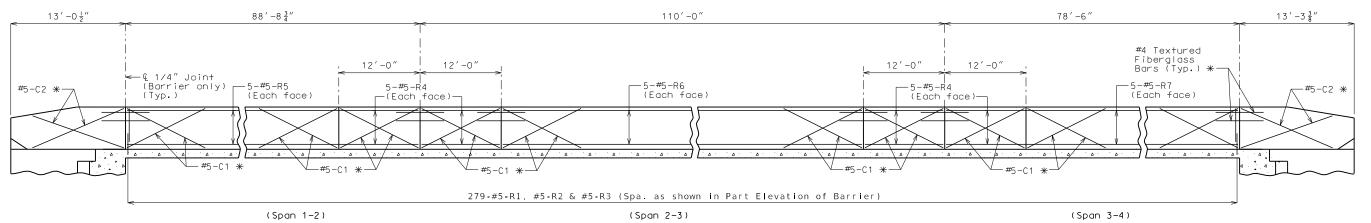
For Theoretical Bottom of Slab Elevations, Girder Camber Diagram and Theoretical Slab Haunching Diagram, see Sheet No. 20.

For Plan of Slab Showing Reinforcement, see Sheets No. 22 & 23.

SLAB DETAILS

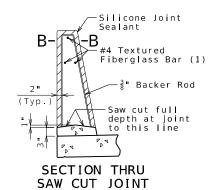
Detailed Oct. 2024 Checked Nov. 2024

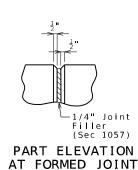
JEFF A. GARDNER

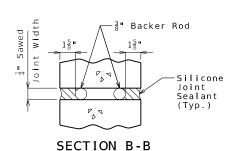


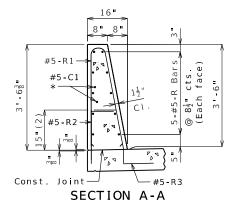
ELEVATION OF BARRIER

(Left barrier shown, right barrier similar) Longitudinal dimensions are horizontal.











Use a minimum lap of 3'-1" for #5 horizontal barrier bars.

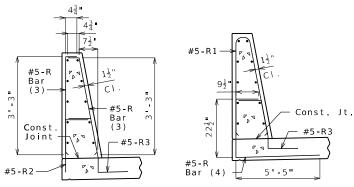
The cross-sectional area above the slab is 3.52 square feet.

(2) To top of bar

Note: This drawing is not to scale. Follow dimensions.

Q 1/4" Joint (Barrier only) (Typ.) Q Int. Roadway Face of Barrier

PART PLAN SHOWING JOINT LOCATION



R-BAR PERMISSIBLE ALTERNATE SHAPE

- (3) The R1 bar may be separated into two bars as shown, at the contractor's option, only when slip forming is not used. (All dimensions are out to out.)
- (4) The R2 bar and #5 bottom transverse slab bar in cantilever (prestressed panels only) combination may be furnished as one bar as shown, at the contractor's option.

General Notes:

* Slip-formed option only.

Conventional forming or slip forming may be used. Saw cut joints may be used with conventional forming.

Top of barrier shall be built parallel to grade and barrier joints (except at end bents) normal to grade.

All exposed edges of barrier shall have either a 1/2-inch radius or a 3/8-inch bevel, unless otherwise noted.

Payment for all concrete and reinforcement, complete in place, will be considered completely covered by the contract unit price for Type D Barrier per linear foot.

Concrete in barrier shall be Class B-1.

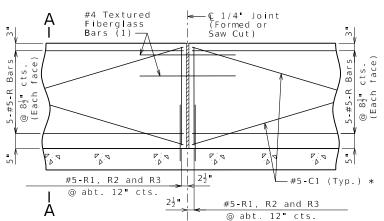
Measurement of barrier is to the nearest linear foot for each structure, measured along the outside top of slab from end of wing to end of wing.

Concrete traffic barrier delineators shall be placed on top of the barrier as shown on Missouri Standard Plan 617.10 and in accordance with Sec 617. Delineators on bridges with two-lane, two-way traffic shall have retroreflective sheeting on both sides. Concrete traffic barrier delineators will be considered completely covered by the contract unit price for Type D Barrier.

Joint sealant and backer rods shall be in accordance with Sec 717 for silicone joint sealant for saw cut and formed joints.

For slip-formed option, both sides of barrier shall have a vertically broomed finish and the top shall have a transversely broomed finish.

Plastic waterstop shall not be used with saw cut joints.



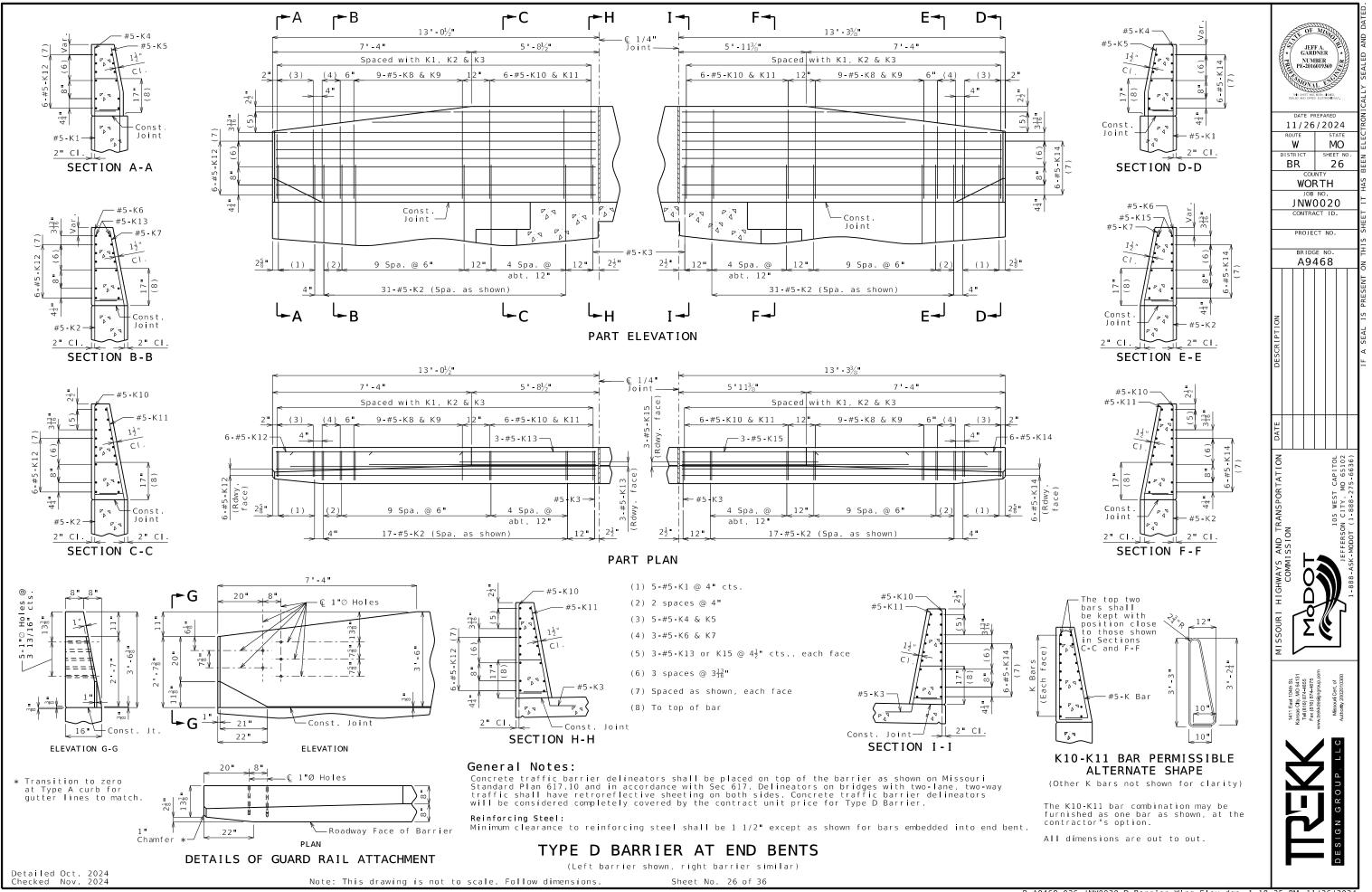
PART ELEVATION OF BARRIER

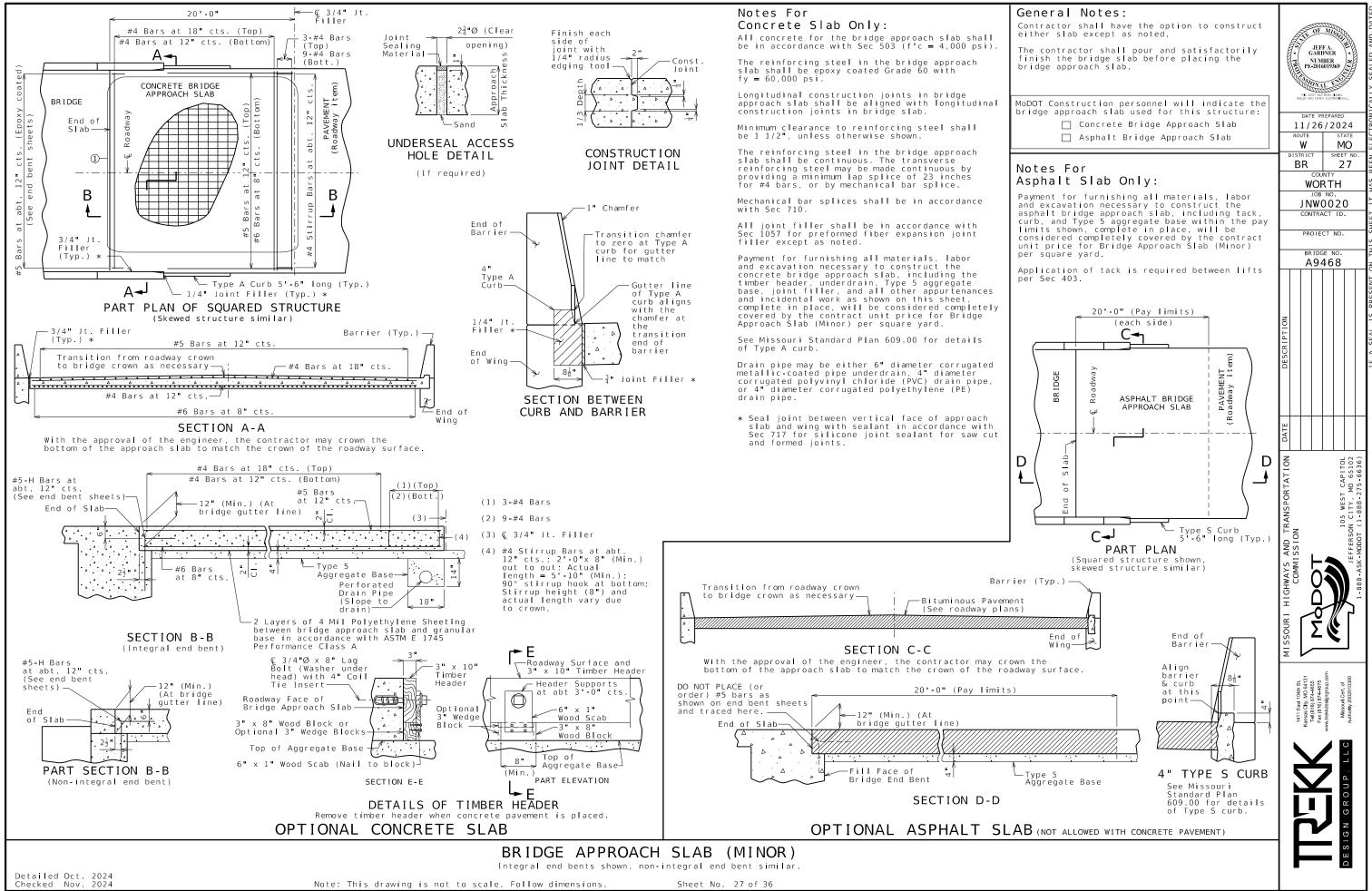
(1) Four feet long, centered on joint, slip-formed option only

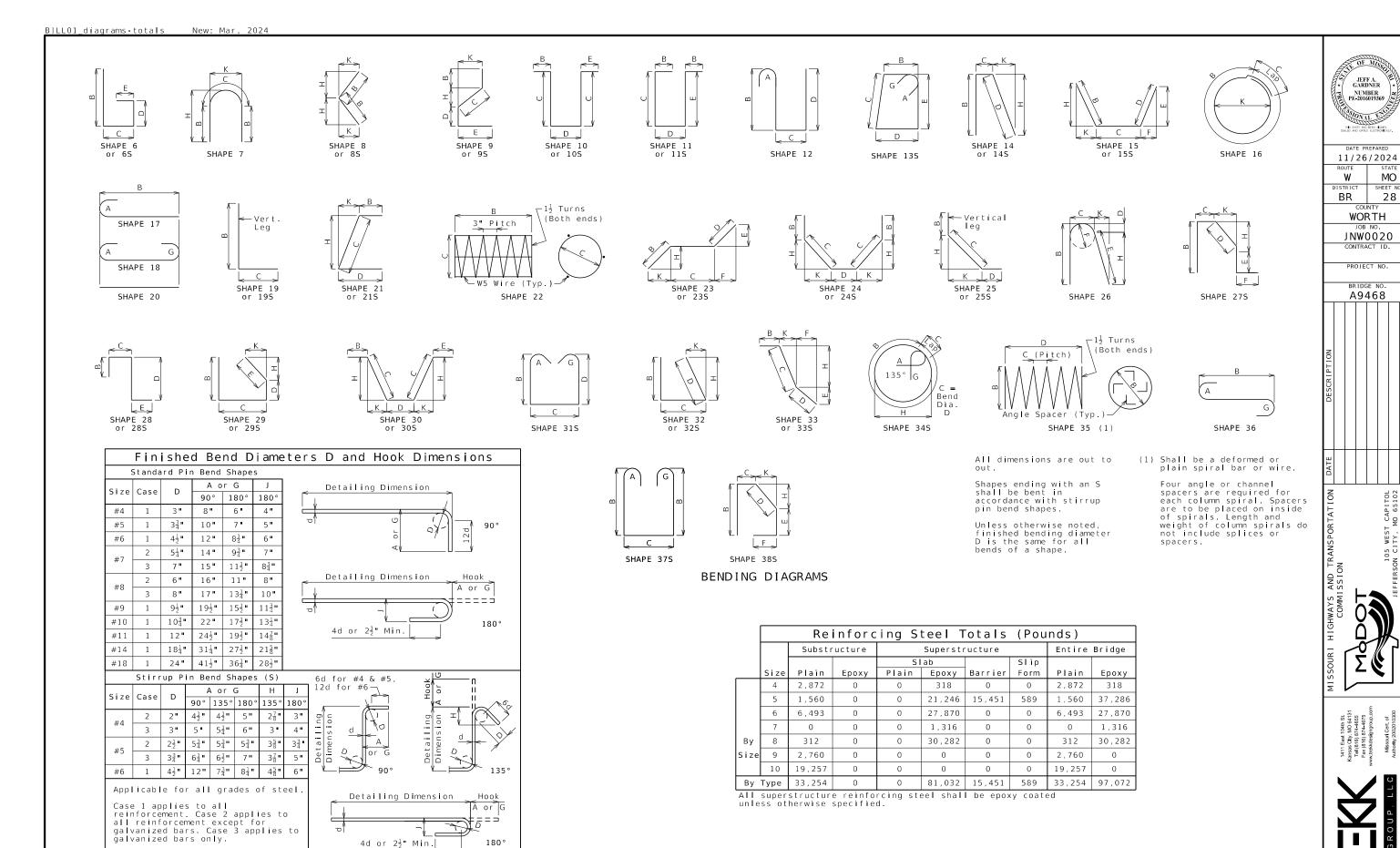
Detailed Oct. 2024 Checked Nov. 2024 TYPE D BARRIER

Sheet No. 25 of 36









BILLO2 data New: Mar. 2024

			Е	Bill o	f Reinf	forcir	ng Ste	e I									Bill of	Reir	nforci	ing Stee	 е I					
						mensions			Nom	. Actua	I								Dimensions				Nom.	Actual	ı	
No. Size/	Codes	Е	3	С	D	E	F	Н	K Lengt	h Lengti	n Weight	No.	Size/		Codes	В	С	D	E	F	Н	K	Length	Length	h Weigh	ht
Req. Mark Location	C SH	V ft	in. 1	ft in.	ft in. ft	in. ft	t in. ft	in. ft	in. ft	in. ft	in. Ib	Req.	Mark	LOCATION	C SH	V ft in.	ft in. ft	in.	ft in.	ft in ft	in. f	ft in.	ft in.	. ft i	in. Ib	
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												6	430 04 300 00 000	DIAPHRAGM	E 20	8 10.500							8 10	0 8	10	80
14 6 D200 BEAM KEY	20	2	6.000						2	6 2	6 53	2	761 62 63	DIAPHRAGM	E 20	6 1.500							6 1	6	1	18
												6		DIAPHRAGM	E 20	3 4.500							3 4	1 3	4	30
8 9 H200 BEAM	18	_	1.000						26	8 26	8 725			DIAPHRAGM	E 20	2 0.000								0 2	0	85
8 9 H201 BEAM	20	_	1.000			+			24	1 24	1 655	-	8 H106		E 20	12 6.750								6 12		534
10 6 H202 BEAM 14 6 H203 BEAM	20	24	1.000	40.000	4 500	\rightarrow			5	1 24	1 362		6 H107	an Manager	E 20	11 9.500									10 7	711
	10	16 1	0.000	12.000	3 1.500	+-+	-		19	2 4 5 19	5 156		17 17 18 18 18 18	DIAPHRAGM STRAND TIE	E 19	2 0.000	1 3.000 3 2.500 1	3.000	2.500	1 2.750	2.500 1	1 2.750		3 3 9 5	2	19
3 8 H204 WEB WALL 52 5 H205 WEB WALL	20		0.000				-		15	10 15	10 859	_	5 H109	STRAIND HE	E 23	1 3.000	3 2.500 1	3.000	2.500	1 2.750	2.500	2.750	5 8	15	-	-10
4 5 H206 BEAM	20	0.000	9.000			+			15	9 4	9 20	10	5 U100	DEAM	E 31	4 6.125	2 9.500 4	6.125			+		12 9	9 12	7 .	131
4 3 1/200 BEAW	20	+	3.000		- - - - - - - - - - 	+			4	3 4	3 20	21	5 U101	CATA NAME	E 13	2 9.500	2 8.000 2	9.500	2 8.000		-		202	0 11		252
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28 10 V201 SHAFT	20		2.000			+++			9	2 9	2 1104	-		DIAPHRAGM	E 19	3 1.625	2 9.500				+ +			1 5		155
28 10 V202 SOCKET/SHAFT	20	_	9.000						46	9 46	9 5633			DIAPHRAGM	E 31	3 11.000	2 3.375 3	11.000			_					203
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24 6 U201 BEAM	13		2.000	3 9.000	2 2.000 3	9.000			13	1 12	7 454			DIAPHRAGM	E 20	3 0.625								0 3	0	68
12 6 U202 BEAM	10			3 9.000	3 3.000				10	9 10	5 188		6 V102	\$40-20, 2000 W. W. 11320139	E 20	7 9.375							7 9	9 7	9	23
9 6 U203 BEAM	10			12.000	3 3.000				5	3 4	11 66	20	6 V103	WING	E 20	V 7 3.750							7 :	3 7	3 2	224
24 6 U204 WEB WALL	10		- 2	21 9.000	21.000				45	3 45	0 1622			INC = 0.500 INCH		7 8.750							7 ε	3 7	8	
												2	6 V104	WING	E 20	7 11.750							7 11	1 7	11	24
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						+																		+		
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	1					+					. 700	16		DIAPHRAGM	E 20	8 6.500								6 8	6	91
8 9 H300 BEAM 8 9 H301 BEAM	18	24							26	8 26	8 725 1 655			DIAPHRAGM DIAPHRAGM	E 20	8 6.500 4 5.000	10 500						-	6 8	6 1	102
	20	24	1.000			+-+			24	1 24	1 362			DIAPHRAGM	E 6	4 5.000 4 10.000	10.500 10.500							3 5 0 6	2	225
10 6 H302 BEAM 14 6 H303 BEAM	10	124	1.000	12.000	3 1.500	+-+			5	2 4	10 102			STRAND TIE	E 20	4 5.500	10.500				-			6 4		38
3 8 H304 WEB WALL	18	16 1	0,000	12.000	3 1.500	+	+		19	5 19	5 156	-	201 01.100.000.00	STRAND TIE	E 20	5 8.500								9 5	0	24
40 5 H305 WEB WALL	20		0.000			+			15	10 15	10 661	1	0 1.000	O II VIII D II L		0.000										-
4 5 H306 BEAM	20	-	9.000			+			4	9 4	9 20	48	4 U500	DIAPHRAGM	E 28		1 11.000 3	9.500	1 6.000				7 :	3 7	1 7	227
	+ + +											32		DIAPHRAGM	E 28		1 11.000 3	0.500					7 :	26	- 25	328
28 10 V300 COLUMN	17	18 1	0.000			+			20	4 20	4 2450	1 —		DIA PHRA GM	E 28		1 11.000 3	9.500					7 11	_		182
28 10 V301 SHAFT	20	_	2.000						9	2 9	2 1104	_														
28 10 V302 SOCKET/SHAFT	20	49	9.000						49	9 49	9 5994	16	5 V500	DIAPHRAGM	E 20	4 1.000							4	1 4	1	68
25 6 U300 BEAM	13		3.000		3 3.000 3	9.000			15	3 14	9 554	4		END BENT 4												
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12 6 U302 BEAM	10			3 9.000					10	9 10	5 188	-	44 1000 740-00	WING BRACE	E 23	2 3.000		2.000	10.750	9.000 1				3 8		110
9 6 U303 BEAM	10	\perp		12.000					5		11 66	4	2 2 2 2 2	DIAPHRAGM	E 6	2 8.750				2	8.250	5.750		3 8		49
24 6 U304 WEB WALL	10			17 6.000	21.000	+			36	9 36	5 1313			WING BRACE	E 23	2 3.000		2.000	10.750		8.750 1			2 9		123
10 1000 000 000	1	-	7 7			+			0.005		4 416	4	6 F403	DIAPHRAGM	E 6	2 8.750	5 5.750			2	8.250	5.750	8 3	3 8	0	48
16 4 P300 COLUMN	16		7.750			+		2	9.000 11	1 11	1 118	-	6 11400	DEAM & DIA CUDA CU	E 20	26 40.000					+		26 1	0 26	10	161
158 4 P301 SOCKET/SHAFT	16	10	2.500 2	2 5.000		+		2	9.000 12	8 12	8 1337	-		BEAM & DIAPHRAGM BEAM & DIAPHRAGM	E 20	26 10.000					+					658
SUPERSTRUCTURE												12 6	0 7 1 1 1 1	DIAPHRAGM	E 20	26 10.000 8 10.500			_		+				10000	80
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			1	I			T					6		DIAPHRAGM	E 20	3 4.500					+			4 3	1.5	30
10 6 F100 WNG BRACE	E 23	2	3.000 4	4 10.500	1 2.000	10.750	9.000 1	8.750 1	5.250 8	3 8	2 123	-	12) 100000000000000000000000000000000000	DIAPHRAGM	E 20	2 0.000					+		282	0 2		85
4 6 F101 DIAPHRAGM	E 6		8.750		. 2.000	10.700	2.000 1	8.250	5.750 8	3 8	1 49	-	8 H406	TOTAL TRANSPORT OF THE PROPERTY.	E 20	12 6.750					+++			6 12		534
10 6 F102 WNG BRACE	E 23		3.000		1 2.000	10.750	9.000 1	8.750 1	5.250 9	2 9	1 136	I ⊢—	6 H407		E 20	11 9.500					+					640
4 6 F103 DIA PHRAGM	E 6		8.750			15.755	2.000	8.250	5.750 8	3 8	0 48	24		DIAPHRAGM	E 19	2 0.000	1 3.000				+					79
						+ +						3		STRAND TIE	E 23	1 3.000		3.000	2.500	1 2.750	2.500 1	1 2.750		9 5		18
													- 11.50			. 5.550		3.000	2.000	2.700				1-		

All bars shall be Grade 60.

Codes: C = Required coatings, where E = Epoxy Coated and <math>G = Galvanized.

SH = Required shape, see bending diagrams.

V = Sets of varied bars and number of bars of each length. Bar dimensions vary in equal increments between dimensions shown on this line and the following line and the actual length dimension shown on this line and the following line vary by the specified increment.

11/26/2024 W MO SHEET NO BR 29 WORTH JNW0020 PROJECT NO. BRIDGE NO A9468 HIGHWAYS AND TRANSPORTATION COMMISSION



Nominal lengths are based on out to out dimensions shown in bending diagrams and are listed to the nearest inch for fabricator's use. Actual lengths are measured along centerline bar to the nearest inch. Weights are based on actual lengths.

For bending diagrams and steel reinforcing totals, see Sheet No. _. Detailed Oct. 2024 Checked Nov. 2024

BILL OF REINFORCING STEEL

Sheet No. 29 of 36

Note: This drawing is not to scale. Follow dimensions.

BILLO2 data New: Mar. 2024

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						Dimensi			<u></u>	N	om. Ac	tual							_			Dimensions				Nom.	Actual		OF MISS
No.	Size/	Codes	В	С	D	Е		F	Н	K Le	100	ngth	Weight	No.	Size/		Cod	les	В	С	D	E	F	Н	K	Length	1	Weight	
Req.	Mark Location	C SH \	ft in.	ft in.	ft in.	. ft	in. ft	in. ft	in. f		in. ft	in.		Req.	Mark	LOCATION	CSF		ft in ft	in. ft	in.	ft in.	ft in	n. ft in.	. ft i		n. ft ir		JEFF A. GARDNER
10	5 U400 BEAM	E 31	4 5.625	9.500	4 5.625	5				12	8 12	6	130			SLIP FORM OPTION													NUMBER PE-201601936
21	5 U401 BEAM	E 13	2 9.500	2 8.000	9.500	2 8.	.000			11	10 11	6	252	40	5 C1	BARRIER	E 20	0	12 0.000							12	0 12	0 501	
33	6 U403 DIAPHRAGM	E 19	3 3.500	7.000	,					7	10 7	9	384	8	5 C2	BARRIER	E 20	0	10 6.000							10	6 10	6 88	
18	6 U404 DIA PHRA GM	E 19	2 11.000	9.500	,					5	8 5	6	149																THIS SHEET HAS BEEN SIGN SEALED AND DATED ELECTHOR
18	5 U405 DIA PHRA GM	E 31	3 10.000	2 3.375	3 10.000					10	11 10	8	200																0.75 005040
																		\top											DATE PREPAR 11/26/20
16	5 V400 BEAM & DIAPHRAGM	E 17	4 5.625	,						5	1 5	1	85	5				\top			$\overline{}$			+					ROUTE S
15	6 V401 DIAPHRAGM	E 20	2 11.000							2	11 2	11	66	5															11 w
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18	5 S5 SLAB	E 20 \								2	4 2	4		 			\vdash	+						+					
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444	5 S6 SLAB	E 20							_	26	5 26	5						-							+				8
444								-				7		<u> </u>										+				+	
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24	5 K14 BARRIER		13 0.375							13	0 13	0	325	5															1411 East 104th St. Kansas Cty, MO 6413 Tel (816) 874-4655 Fax (816) 874-4675 www.trekkdesigngrup.co.
										6	5 6	5	118	3				\neg						T					1 1 2 2 2 2 2
12	5 K15 BARRIER INC. = 36.000 INCH		6 5.750 12 5.750								5 12		110	1 🗕															= 88 ° × ¥

Nominal lengths are based on out to out dimensions shown in bending diagrams and are listed to the nearest inch for fabricator's use. Actual lengths are measured along centerline bar to the nearest inch. Weights are based on actual lengths.

All bars shall be Grade 60.

Sheet No. 30 of 36

Codes: C = Required coatings, where E = Epoxy Coated and <math>G = Galvanized.

SH = Required shape, see bending diagrams.

V = Sets of varied bars and number of bars of each length. Bar dimensions vary in equal increments between dimensions shown on this line and the following line and the actual length dimension shown on this line and the following line vary by the specified increment.

For bending diagrams and steel reinforcing totals, see Sheet No. _. Detailed Oct. 2024 Checked Nov. 2024

BILL OF REINFORCING STEEL Note: This drawing is not to scale. Follow dimensions.



PART PLAN SHOWING PILE & DRILLED SHAFT NUMBERING FOR RECORDING AS-BUILT PILE DATA & AS-BUILT DRILLED SHAFT DATA

	As-Built Pile Data								
Pile No.	Length in Place (ft)	PDA Nom. Axial Compressive Resistance (kips)	PDA End of Drive Blow Count (blows/in.)	Actual End of Drive Blow Count (blows/in.)	R ema r k s				
					End Bent No. 1				
1									
2									
3									
4									
					End Bent No. 4				
5									
6									
7									
8									

	As-Built Drilled Shaft Data										
Shaft No.	Top of Sound Rock (Elev.)	Tip of Casing (Elev.)	Bottom of Rock Socket (Elev.)	Remarks							
				Intermediate Bent No. 2							
1											
2											
				Intermediate Bent No. 3							
3											
4											

o t	е	1								
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Note:
Indicate in remarks column:
A. Pile type and grade
B. Batter
C. Driven to practical refusal
D. PDA test pile
E. Minimum tip elevation controlled
(Use when actual blow count is less than PDA blow count due
to minimum tip elevation requirement. A plus sign (+) shall
be placed after the PDA nominal axial compressive resistance
value indicating actual value is higher than PDA value.)

This sheet to be completed by MoDOT construction personnel.



DATE PREPARED									
11/26/2024									
ROUTE	STATE								
W	W MO								
DISTRICT	SHEET NO.								
BR	31								
COUNTY									
WORTH									

JNW0020

PROJECT NO.

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	DESCRIPTION				
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/lissouri	Department of	Transportation
Co	netruction and	Materials

BORING NO. B-101 Page 1 of 2

	Construction and materials	
bb No. : NW0020-A9468	County: Worth	Route: W
esign: <u>A9468</u>	Skew: 10 RA	Location: Over W. Fork Grand River
ent: _1	Logged By: Matthew Kistler	Operator: Josh Starkey
tation:	Northing: 1549438.9	Date of Work: 05/29/24-05/29/24
ffset:	Easting: 2772694.5	Depth to Water: 40.1
evation: 943.8	Requested Northing:	Depth Hole Open:
equested Station:	Requested Easting:	Time Change: At Time of Drilling
equested Offset:	Equipment: PPI CME 55S/N ,Split-Spoon Sa	mpler, NQ

Requested Elevation: Location Note: On Stake Hammer Efficiency: 78% Drill No.: Rig #408095 Drilling Method: Hollow Stem Auger Depth (ff) REC (RQD) Description Field 0.0-4.1' Brown, LEAN CLAY, medium stiff, moist 2-3-6 (12) 93 PP = 1.75 tsf **y**_{sat} = 134 pcf¹¹ LL = 37 PL = 16 4.1-6.0' Brown and gray, LEAN CLAY with sand, stiff, moist 6.0-8.8' Dark brown, LEAN CLAY, stiff, moist 2-3-5 (10) 8.8-13.6' Light brown, LEAN CLAY trace fine 87 PP = 1.50 tsf γ_{sat} = 127 pcf⁽¹⁾ LL = 41 PL = 19 10 gravel, moist Sieve Analysis Sieve # % Passing 3/4" 100.0 3/8" 96.8 #4 94.0 #10 90.0 #16 87.0 13.6-20.0' Gray and tan, SAND, medium 4-4-6 67 (13) dense, moist 14.9-20.0' scattered Iron Manganese #40 73.6 #50 48.7 #100 7.9 925 6-6-9 0 #200 46 20.0-27.6' Dark gray, LEAN CLAY trace fine gravel, stiff, moist, (Glacial Till) 920 73 PP = 3.25 tsf **y**_{sat} = 127 pcf¹¹ LL = 49 PL = 21 (17)T_20151118.GDT - 7/31/24 14:56 - Z. 27.6-30.8' COBBLES, dense, Limestone 915 Cobbles and Boulders 30-15-23 33 30.8-53.2' Dark gray, LEAN CLAY trace fine gravel, very stiff, moist, (Glacial Till) 910 9-6-7 (17) 93

N₅₀ = (Em/60)Nm N₆₀ - Corrected N value for standard 60% SPT efficiency; Em - Measured hammer efficiency in percent; Nm - Observed N-value; (1) = Assumed, (2) = Actual

Coordinate System: Modified U.S. State Plane 1983 Coordinate Zone: Missouri West Coordinate Proj. Factor: 1.0000983827

Coordinate Datum: NAD 83 (CONUS) Coordinate Units: U.S. Survey Feet

* Persons using this information are cautioned that the materials shown are determined by the equipment noted and accuracy of the "log of materials" is limited thereby and by judgement of the operator. THIS INFORMATION IS FOR DESIGN PURPOSES ONLY.

(Continued Next Page)

Missouri Department of Transportation Construction and Materials

BORING NO. B-101 Page 2 of 2

Job No.: NW0020-A9468	County: Worth	Route: W				
Design: _A9468	Skew: 10 RA	Location: Over W. Fork Grand River				
Bent: 1	Logged By: Matthew Kistler	Operator: _Josh Starkey				
Station:	Northing: 1549438.9	Date of Work: 05/29/24-05/29/24				
Offset:	Easting: 2772694.5	Depth to Water: 40.1				
Elevation: 943.8	Requested Northing:	Depth Hole Open:				
Requested Station:	Requested Easting:	Time Change: At Time of Drilling				
Requested Offset:	Equipment: PPI CME 55S/N ,Split-Spoon Sampler, NQ					
Requested Elevation:	Location Note: On Stake					

Drill No.: R	Rig #408095 Hammer Eff	ciency:	789	%	D	rilling Method: _	Hollow Stem Auge	er
Depth (ft) Graphic	Description	Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests	Index Tests
35	30.8-53.2' Dark gray, LEAN CLAY trace fine gravel, very stiff, moist, (Glacial Till) (continued)	905		93	4-5-7 (16)		PP = 2.50 tsf	MC = 22.1% y _{sat} = 128 pcf ⁽¹⁾
45	*	900		100	4-6-11 (22)		PP = 2.50 tsf	MC = 21.1% y sal = 129 pcf ⁽¹⁾
50		895		100	5-8-5 (17)		PP = 2.25 tsf	MC = 20.7% Y sal = 130 pcf ⁽¹⁾
55	53.2-54.8' Shale, dark gray, highly weathered 54.8-67.4' Shale, dark gray, very weak rock, moderately weathered	890		100	48-50/0.3'	Qu Test Results UCS = 15.0 ksf MC = 10.8%	PP = 9.00 tsf	
50 60 65		885		100 (48)		γ moist = 142.3 pcf Qu Test Results UCS = 15.6 ksf MC = 11.1% γ moist = 133.9 pcf	PP = 5.00 tsf	
65	62.8-67.8' water used to extrude core.	880		100 (36)		Qu Test Results UCS = 4.37 ksf MC = 18.2% Y moist = 133.1 pcf		
	67.4-67.8' Fossiliferous Limestone, light gray slightly weathered Bottom of borehole at 67.8 feet.							

N_{so} = (Em/60)Nm N₆₀ - Corrected N value for standard 60% SPT efficiency; Em - Measured hammer efficiency in percent; Nm - Observed N-value; (1) = Assumed, (2) = Actual

 Coordinate System:
 Modified U.S. State Plane 1983
 Coordinate Zone:
 Missouri West
 Coordinate Proj. Factor:
 1.0000983827

 Coordinate Datum:
 NAD 83 (CONUS)
 Coordinate Units:
 U.S. Survey Feet

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BORING DATA

Note: For locations of borings, see Sheet No. 1.

Detailed Oct. 2024 Checked Nov. 2024

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 32 of 36

JEFF A.
NIMBER
PF-2016019369

DATE PREPARED

11/26/2024

ROUTE STATE
W MO
DISTRICT SHEET NO.
BR 32

COUNTY WORTH JOB NO.

JNW0020 CONTRACT ID.

BRIDGE NO.
A9468

TOOMISSION

COMMISSION

105 WEST CAPITOL

1411 East 104th St. ansas City, MO 64131 Tel (816) 874-4655 Fax (816) 874-4675 w.trekkdesigngroup.com



Missouri Department of Transportation Construction and Materials

BORING NO. B-203 Page 1 of 3

Job No.: NW0020-A9468 County: Worth Route: W Design: A9468 Skew: 10 RA Location: Over W. Fork Grand River Bent: 2 Logged By: Ricardo Todd Operator: Bradley Wadlow Northing: 1549396.6 Date of Work: 06/04/24-06/04/24 Easting: 2772780.7 Offset: Depth to Water: 30.0 Elevation: 940.1 Requested Northing: 1549393.6 Depth Hole Open: Requested Station: Requested Easting: 2772780.5 Time Change: At Time of Drilling Requested Offset: Equipment: Acker Soil XLS ,Split-Spoon Sampler, NQ

Location Note: Offset to avoid bridge cross member

Requested Elevation: Hammer Efficiency: 81% Drill No.: G-9462 Drilling Method: Casing Advancer Depth (ff) Description 0.0-0.1' ASPHALT 0.1-0.6' CONCRETE 0.6-30.0' Air 935 930 925 920 915 30.0-31.0' Light tan black, SAND, very loose, 31.0-34.1' BOULDERS

34.1-40.1' Gray, SAND, very loose, wet N₅₀ = (Em/60)Nm N₆₀ - Corrected N value for standard 60% SPT efficiency; Em - Measured hammer efficiency in percent; Nm - Observed N-value; (1) = Assumed, (2) = Actual Coordinate System: Modified U.S. State Plane 1983 Coordinate Zone: Missouri West Coordinate Proj. Factor: 1.0000983827

Coordinate Datum: NAD 83 (CONUS) Coordinate Units: U.S. Survey Feet

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(Continued Next Page)

Missouri Department of Transportation **Construction and Materials**

BORING NO. B-203 Page 2 of 3

ob No.: NW0020-A9468	County: Worth	Route: W			
esign: _A9468	Skew: 10 RA	Location: Over W. Fork Grand River			
ent: _2	Logged By: Ricardo Todd	Operator: Bradley Wadlow			
tation:	Northing: 1549396.6	Date of Work: 06/04/24-06/04/24			
ffset:	Easting: 2772780.7	Depth to Water: 30.0			
levation: 940.1	Requested Northing: 1549393.6	Depth Hole Open:			
equested Station:	Requested Easting: 2772780.5	Time Change: At Time of Drilling			
equested Offset:	Equipment: Acker Soil XLS ,Split-Spoon Sampler, NQ				

Location Notes Officet to avoid bridge gross member

Reque	ested E	levation: Location Not	te: Off	set to	avoid b	ridge cross mem	ber		
Drill N	lo.: <u>G</u>	-9462 Hammer Effi	ciency	81	%	D	rilling Method:	Casing Advancer	
Depth (ft)	Graphic	Description	Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests	Index Tests
35	2,30.007	34.1-40.1' Gray, SAND, very loose, wet	905			5-1-1			
 		(continued)	- - - -	X	73	5-1-1 (3)			
40	11111	40.1-45.7' Gray, LEAN CLAY scattered gravel,	900			3-7-8			MC = 21.4%
 		very stiff, moist	- - - -	X	73	(20)		PP = 2.25 tsf	MC = 21.4% Y sat = 129 pcf ⁽¹⁾ LL = 36 PL = 19
45		į	895	$\overline{}$	73	5-8-13		PP = 7.50 tsf	MC = 16.4%
-		45.7-55.1' Shale, gray, very weak rock, highly weathered	† -		13	(28)		FF = 7.50 tsi	y sat = 135 pcf ⁽¹
		Weathered	† -	П	100_/	37/0.1', 10/0'	ĺ	PP = 7.50 tsf	
50			890	Ш	13 (0)				
 - 55			885		0 (0)				
_		55.1-60.1 Shale, gray, very weak rock,		П	100	37/0.1', 10/0'	Qu Test Results UCS = 2.54 ksf	PP = 7.50 tsf	
 - 60		moderately weathered	880		66 (12)		MC = 12.9% MC = 12.9%		
		60.1-85.1' Calcareous Shale, gray, medium strong rock, moderately weathered to slightly weathered			64 (32)		Qu Test Results UCS = 226 ksf MC = 7.3% y moist = 151.1 pcf		
65			875		100 (60)		Qu Test Results UCS = 122 ksf MC = 7.3%		
70			-		(60)		UCS = 193 ksf MC = 6.1% y moist = 154.4		

🗓 N_{so} = (Em/60)Nm N_{so} - Corrected N value for standard 60% SPT efficiency; Em - Measured hammer efficiency in percent; Nm - Observed N-value; (1) = Assumed, (2) = Actual

Coordinate Proj. Factor: 1.0000983827 Coordinate System: Modified U.S. State Plane 1983 Coordinate Zone: Missouri West

Coordinate Datum: NAD 83 (CONUS) Coordinate Units: U.S. Survey Feet

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(Continued Next Page)

BORING DATA



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11/26/2024					
STATE	ı				
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WORTH

JNW0020

PROJECT NO

BRIDGE NO. A9468										
DESCRIPTION										
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BORING NO. B-203 Page 3 of 3

	Construction and materials					
ob No. : NW0020-A9468	County: Worth	Route: _W				
esign: A9468	Skew: _10 RA	Location: Over W. Fork Grand River				
ent: _2	Logged By: Ricardo Todd	Operator: Bradley Wadlow				
tation:	Northing: 1549396.6	Date of Work: 06/04/24-06/04/24				
ffset:	Easting: 2772780.7	Depth to Water: 30.0				
levation: 940.1	Requested Northing: 1549393.6	Depth Hole Open:				
equested Station:	Requested Easting: 2772780.5	Time Change: At Time of Drilling				
equested Offset:	Equipment: Acker Soil XLS Split-Spoon Sampler, NQ					

Requ	ested I	Elevation: Location Not	te: Off	set to	avoid b	ridge cross mem	ber		
Drill N	lo.: _@	9-9462 Hammer Effi	ciency:	81	%	Dı	rilling Method: _	Casing Advancer	
Depth (ft)	Graphic	Description	Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests	Index Tests
75		60.1-85.1' Calcareous Shale, gray, medium strong rock, moderately weathered to slightly weathered <i>(continued)</i>	865		100 (88)		pcf Qu Test Results UCS = 334 ksf MC = 4.8%		
85 - 85 - 85 - 85 - 85 - 85 - 85 - 85 -		Bottom of borehole at 85.1 feet.	855				CUS = 153 ksf MC = 8.1% MC = 8.1% MC = 149.7 pot = 149.7 pot = 113 ksf MC = 8.9% MC = 8.9% MC = 8.9% MC = 8.9% MC = 8.9% MC = 8.9% MC = 8.9% MC = 8.9% MC = 9.1% MC = 9.1% MC = 9.1% MC = 9.1% MC = 9.1% MC = 9.1% MC = 9.1%		

N_{so} = (Em/60)Nm N_{so} - Corrected N value for standard 60% SP⊤ efficiency; Em - Measured hammer efficiency in percent; Nm - Observed N-value; (1) = Assumed, (2) = Actual Coordinate System: Modified U.S. State Plane 1983 Coordinate Zone: Missouri West Coordinate Proj. Factor: 1.0000983827

Coordinate Datum: NAD 83 (CONUS) Coordinate Units: U.S. Survey Feet

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Missouri Department of Transportation Construction and Materials

BORING NO. B-301 Page 1 of 3

	Concentration and materials					
ob No.: NW0020-A9468	County: Worth	Route: W				
esign: _A9468	Skew: 10 RA	Location: Over W. Fork Grand River				
ent: _3	Logged By: Matthew Kistler	Operator: Josh Starkey				
tation:	Northing: 1549388.2	Date of Work: 06/11/24-06/11/24				
ffset:	Easting: _2772888.8	Depth to Water: 31.2				
levation: 935.8	Requested Northing: 1549392.2	Depth Hole Open:				
equested Station:	Requested Easting: 2772889.0	Time Change: At Time of Drilling				
equested Offset:	Equipment: PPI CME 55LC ,Split-Spoon Sar	npler, NQ				
equested Elevation:	Location Note: Offset to avoid bridge cross member					

Requ	ested	Elevation: Lo	ocation Note	: <u>Off</u>	set to	avoid b	ridge cross mem	ber		
Drill No.: Rig #360485			ammer Effici	iency:	84.	7%	Dı	rilling Method: _	Casing Advancer	
O Depth (ft)	Graphic	Description		Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N _{so.})	Shear Data	Field Tests	Index Tests
5 5 10 10 10 10 10 10 10 10 10 10 10 10 10		0.0-0.1' ASPHALT 0.1-0.6' CONCRETE 0.6-28.0' Air		935 930 925 925 920 915 910						
30		28.0-31.2' Dark brownish gray, LEAl soft to medium stiff, moist ☑ 30.5' switched to mud rotary after sp. 31.2-42.0' Dark gray, SAND, very lo fine grained, poorly graded	ot _	905	X	20	2-0-0 (0)			
35			Ť		\forall	40	2-1-1			

Coordinate System: Modified U.S. State Plane 1983 Coordinate Zone: Missouri West Coordinate Proj. Factor: 1.0000983827
Coordinate Datum: NAD 83 (CONUS) Coordinate Units: U.S. Survey Feet

y N₅₀ = (Em/60)Nm N₆₀ - Corrected N value for standard 60% SPT efficiency; Em - Measured hammer efficiency in percent; Nm - Observed N-value; (1) = Assumed, (2) = Actual

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COUNTY
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JOB NO.

JNW0020 CONTRACT ID.

PROJECT NO.

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A9468

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COMMISSION

COMMISSION

105 WEST CAP

LEFFERSON CITY, MO 6

1411 East 104th St. Kansas City, MO 64131 Tel (816) 874-4655 Fax (816) 874-4675 wv.trekkdesigngroup.com



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Missouri Department of Transportation **Construction and Materials**

BORING NO. B-301 Page 2 of 3

Job No.: NW0020-A9468 County: Worth Route: W Design: <u>A9468</u> Skew: 10 RA Location: Over W. Fork Grand River Bent: 3 Logged By: Matthew Kistler Operator: Josh Starkey Northing: 1549388.2 Date of Work: 06/11/24-06/11/24 Easting: 2772888.8 Offset: Depth to Water: 31.2 Elevation: 935.8 Requested Northing: 1549392.2 Depth Hole Open: Requested Station: Requested Easting: 2772889.0 Time Change: At Time of Drilling Equipment: PPI CME 55LC ,Split-Spoon Sampler, NQ Requested Offset: Requested Elevation: Location Note: Offset to avoid bridge cross member

	Drill No.: Rig #360485 Hammer Efficiency: 84.7% Drilling Method: Casing Advancer								
Depth (ft)	Graphic	Description	Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N _{co})	Shear Data	Field Tests	Index Tests
_ 35 		31.2-42.0' Dark gray, SAND, very loose, wet, fine grained, poorly graded (continued)	900	> <		(3)			
40			895	X	20	3-2-1 (4)			
		42.0-44.5' Dark gray, LEAN CLAY, stiff, (completely weathered shale)	† - -		93	31-37/0.3'		PP = 9.00 tsf	
55 65 70		44.5-44.8' Shale, dark gray, thinly laminated, moderately weathered 44.8-59.4' Shale, dark gray, very weak rock, slightly weathered	890		100 (94)		Qu Test Results UCS = 8.89 ksf MC = 13% y moist = 140 pcf Qu Test Results UCS = 15.1 ksf		
50			885		100 (46)		MC = 11.1% MC = 11.1% MC = 140.2 pcf Qu Test Results UCS = 4.46 ksf MC = 12.2% moist = 138.6 pcf		
55			880		100 (52)		Qu Test Results UCS = 4.09 ksf MC = 17.9% 7 moist = 133.1 pcf		
65		59.4-60.1' Limestone, light gray, thin bedded, slightly weathered 60.1-61.9' Shale, dark gray, very weak rock, slightly weathered 61.9-74.8' Calcareous Shale, light gray, medium bedded, weak rock to medium strong rock	875		100 (76)		Qu Test Results UCS = 50.8 ksf MC = 9.9% moist = 145.7 pcf Qu Test Results UCS = 71.7 ksf		
70			870		100 (96)		MC = 7.6% Y moist = 148.3 pcf Qu Test Results UCS = 135 ksf MC = 6.6% Y moist = 153 pcf		

N₅₀ = (Em/60)Nm N₅₀ - Corrected N value for standard 60% SPT efficiency; Em - Measured hammer efficiency in percent; Nm - Observed N-value; (1) = Assumed, (2) = Actual Coordinate Proj. Factor: 1.0000983827 Coordinate System: Modified U.S. State Plane 1983 Coordinate Zone: Missouri West Coordinate Units: U.S. Survey Feet Coordinate Datum: NAD 83 (CONUS)

* Persons using this information are cautioned that the materials shown are determined by the equipment noted and accuracy of the "log of materials" is limited thereby and by judgement of the operator. THIS INFORMATION IS FOR DESIGN PURPOSES ONLY.

(Continued Next Page)

Missouri Department of Transportation **Construction and Materials**

BORING NO. B-301 Page 3 of 3

ob No.: NW0020-A9468	County: Worth	Route: W
Design: <u>A9468</u>	Skew: 10 RA	Location: Over W. Fork Grand River
Sent: <u>3</u>	Logged By: Matthew Kistler	Operator: Josh Starkey
station:	Northing: 1549388.2	Date of Work: 06/11/24-06/11/24
Offset:	Easting: _2772888.8	Depth to Water: 31.2
levation: 935.8	Requested Northing: 1549392.2	Depth Hole Open:
Requested Station:	Requested Easting: 2772889.0	Time Change: At Time of Drilling
Requested Offset:	Equipment: PPI CME 55LC ,Split-Spoon Sar	npler, NQ
Requested Elevation:	Location Note: Offset to avoid bridge cross m	ember

		Station.	Requested E	asung.		2003.0		ine changeAt	Time or Drilling	
Requested Offset: Equipment: PPI CME 55					SLC ,Sp	olit-Spoon Sample	er, NQ			
Requested Elevation: Location Note: Offset to avoid bridge cross member										
Drill No.: Rig #360485 Hammer Efficiency: 84.7% Drilling Method: Casing Advancer										
Depth		Description		Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests	Index Tests
-		61.9-74.8' Calcareous Shale, li medium bedded, weak rock to rock (continued)		865		100 (96)		Qu Test Results UCS = 125 ksf MC = 6.8% γ moist = 151.1 pcf		
TRICOCT_ZOLOTICO COLT. TO 1124 14:00 * L. L. L. L. L. L. L. L. L. L. L. L. L.		Bottom of borehole at 7	4.8 feet.							

N₅₀ = (Em/60)Nm N₅₀ - Corrected N value for standard 60% SPT efficiency; Em - Measured hammer efficiency in percent; Nm - Observed N-value; (1) = Assumed, (2) = Actual Coordinate Proj. Factor: 1.0000983827 Coordinate System: Modified U.S. State Plane 1983 Coordinate Zone: Missouri West

Coordinate Datum: NAD 83 (CONUS) Coordinate Units: U.S. Survey Feet

* Persons using this information are cautioned that the materials shown are determined by the equipment noted and accuracy of the "log of materials" is limited thereby and by judgement of the operator. THIS INFORMATION IS FOR DESIGN PURPOSES ONLY.

BORING DATA

Note: For locations of borings, see Sheet No. 1.

Detailed Oct. 2024 Checked Nov. 2024

Note: This drawing is not to scale. Follow dimensions. Sheet No. 35 of 36

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DATE PREPARED							
11/26/2024							
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WORTH

JNW0020

PROJECT NO

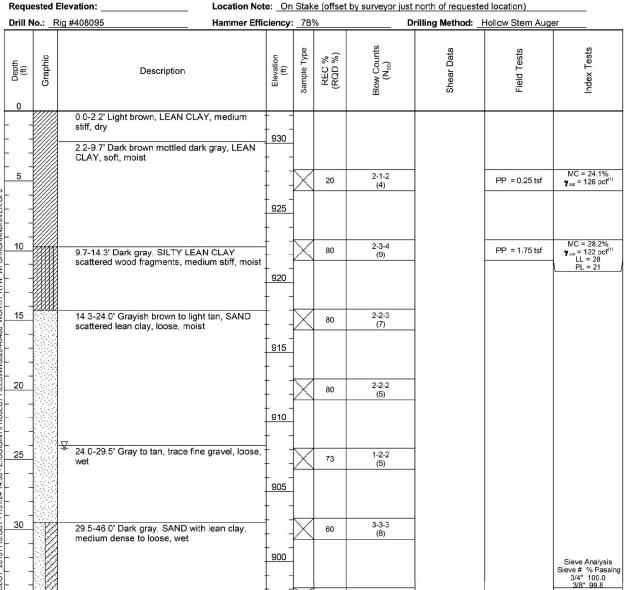
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DESCRIPTION					
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ATION			APITOL	65102	



lissouri	Department of	Transportation	
Col	nstruction and	Materials	

BORING NO. B-403 Page 1 of 2

	Construction and materials			
Job No.: NW0020-A9468	County: Worth	Route: W		
Design: _A9468	Skew: _10 RA	Location: Over W. Fork Grand River		
Bent: 4	Logged By: Matthew Kistler	Operator: Josh Starkey		
Station:	Northing: _1549351.2	Date of Work: 05/30/24-06/04/24		
Offset:	Easting: _2772965.4	Depth to Water: 24.2		
Elevation: 932.3	Requested Northing: 1549349.2	Depth Hole Open:		
Requested Station:	Requested Easting: 2772965.3	Time Change: At Time of Drilling		
Requested Offset:	Equipment: PPI CME 55S/N ,Split-Spoon Sa	ampler, NQ		
Requested Elevation:	Location Note: On Stake (offset by surveyor just north of requested location)			



0-1-2 N₅₀ = (Em/60)Nm N₆₀ - Corrected N value for standard 60% SPT efficiency; Em - Measured hammer efficiency in percent; Nm - Observed N-value; (1) = Assumed, (2) = Actual Coordinate System: Modified U.S. State Plane 1983 Coordinate Zone: Missouri West Coordinate Proj. Factor: 1.0000983827

Coordinate Datum: NAD 83 (CONUS) Coordinate Units: U.S. Survey Feet * Persons using this information are cautioned that the materials shown are determined by the equipment noted and accuracy of the "log of materials" is limited thereby and

by judgement of the operator. THIS INFORMATION IS FOR DESIGN PURPOSES ONLY.

Missouri Department of Transportation **Construction and Materials**

BORING NO. B-403 Page 2 of 2

Job No.: NW0020-A9468	County: Worth	Route: _W
Design: <u>A9468</u>	Skew: 10 RA	Location: Over W. Fork Grand River
Bent: _4	Logged By: Matthew Kistler	Operator: Josh Starkey
Station:	Northing: 1549351.2	Date of Work: 05/30/24-06/04/24
Offset:	Easting: _2772965.4	Depth to Water: 24.2
Elevation: 932.3	Requested Northing: 1549349.2	Depth Hole Open:
Requested Station:	Requested Easting: 2772965.3	Time Change: At Time of Drilling
Requested Offset:	Equipment: PPI CME 55S/N Split-Spoon Sa	impler, NQ
Requested Elevation:	Location Note: On Stake (offset by surveyor j	iust north of requested location)

		Elevation: Location Not ig #408095 Hammer Effic	ciency:	789	%	t by surveyor just north of requested location) Drilling Method: Hollow Stem Auger			
(t) (t)	Graphic	Description	Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests	Index Tests
40		29.5-46.0' Dark gray, SAND with lean clay, medium dense to loose, wet (continued) 39.2' sand heaved before SPT could be performed 40.0' switch to mud rotary	895	X	93	4-3-4 (9)			#10 96.9 #16 94.7 #40 57.2 #50 28.6 #100 10.9 #200 9.0
50		46.0-48.9' Shale, dark gray, completely weathered to highly weathered 48.9-54.0' Shale, dark gray, very weak rock, moderately weathered to slightly weathered 54.0-55.1' Fossiliferous Limestone, light gray, thin bedded, slightly weathered	885		93 100 (65) 94 (42)	24-36/0.3', 10/0'	Qu Test Results UCS = 6.42 ksf MC = 12.8% Y most = 140 pcf Qu Test Results UCS = 6.30 ksf MC = 14.4% Y most = 137.5 pcf	PP = 9.00 tsf PP = 9.00 tsf	
60		55.1-60.1' Shale, dark gray, weak rock, slightly weathered Bottom of borehole at 60.1 feet.	875		100 (90)		Qu Test Results UCS = 65.4 ksf MC = 10.6% Y moist = 145.6 pcf		

 N_{so} = (Em/60)Nm N_{eo} - Corrected N value for standard 60% SPT efficiency; Em - Measured hammer efficiency in percent; Nm - Observed N-value; (1) = Assumed, (2) = Actual

Coordinate Proj. Factor: 1.0000983827 Coordinate System: Modified U.S. State Plane 1983 Coordinate Zone: Missouri West Coordinate Datum: NAD 83 (CONUS) Coordinate Units: U.S. Survey Feet

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(Continued Next Page)

BORING DATA

Note: For locations of borings, see Sheet No. 1.

Detailed Oct. 2024 Checked Nov. 2024

Note: This drawing is not to scale. Follow dimensions. Sheet No. 36 of 36 JEFF A. GARDNER

11/26/2024 W MO SHEET NO BR 36

WORTH

JNW0020

PROJECT NO

BRIDGE NO. A9468									
DESCRIPTION									
DATE									
NC					0	0.2			

