DESIGN DESIGNATION

MELVILLE ROAD/COUNTY ROAD 127 A.A.D.T. - 2025 = 2.182A.A.D.T. - 2045 = 2.411D.H.V. = 50%/50% T = 11.00%V = 35 M.P.H D = 7.43%

FUNCTIONAL CLASSIFICATION: MELVILLE RD. - MAJOR
CRD 127/NORTON RD. - LOCAL - MAJOR COLLECTOR

NORMAL RIGHT OF WAY

CONVENTIONAL SYMBOLS

(USED IN PLANS) EXISTING NEW BUILDINGS AND STRUCTURES [=== GUARD RAIL GUARD CABLE 0000 •••• 0000 •••• CONCRETE RIGHT-OF-WAY MARKER STEEL RIGHT-OF-WAY MARKER \circ LOCATION SURVEY MARKER UTILITIES FIBER OPTICS -OTV--UTV--OT--UT--OE--UE-OVERHEAD CABLE TV -01V- UNDERGROUND CABLE TV -UTV--0T-OVERHEAD TELEPHONE UNDERGROUND TELEPHONE UT -OE--UE-S OVERHEAD POWER UNDERGROUND POWER SANITARY SEWER -ss-STORM SEWER GAS ---G---WATER SAN HYD WW D MANHOLE FIRE HYDRANT WATER VALVE WATER METER °' DROP INLET DITCH BLOCK = GROUND MOUNTED SIGN LIGHT POLE H-FRAME POWER POLE \Box TELEPHONE PEDESTAL Δ FFNCF CHAIN LINK WOVEN WIRE GATE POST \boxtimes BM \otimes BENCHMARK REMOVE (R) (UIP) USE IN PLACE (ATG) ADJUST TO GRADE (DND) DO NOT DISTURB BY OTHERS (BO) (TBA) TO BE ABANDONED GRADING LIMITS -----

NOTE: DASHED OR OPEN SYMBOLS INDICATE EXISTING FEATURES

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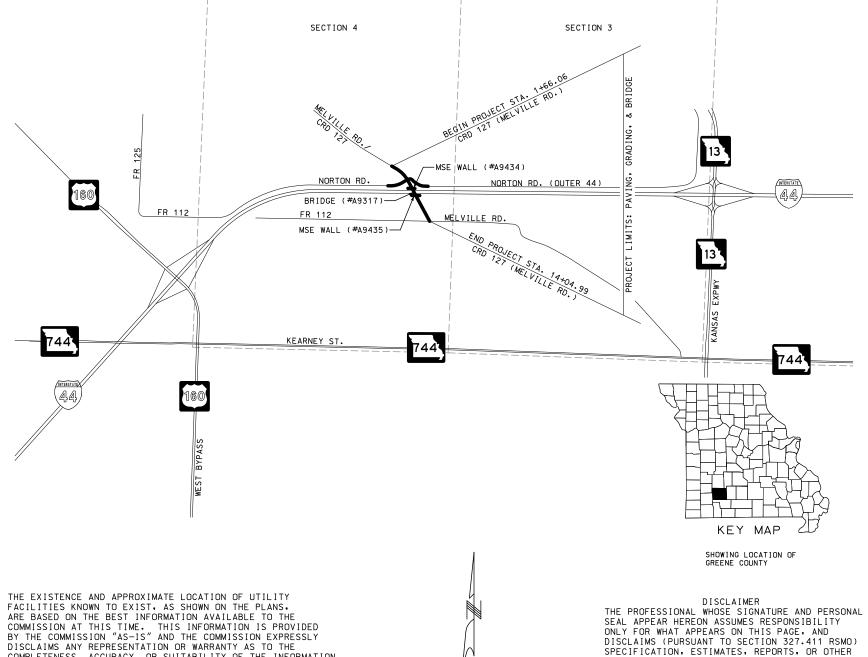
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MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION PLANS FOR PROPOSED STATE HIGHWAY

GREENE COUNTY SECTION 04 T29N R22W

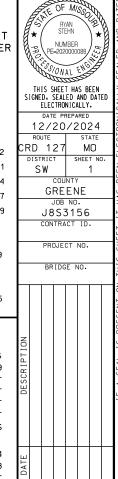


INDEX OF SHEETS

	SHEET
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QUANTITIES (QU) (10 SHEETS)	3
PLAN SHEETS (PS)	4-8
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BRIDGE DRAWINGS (B)	
A9317	1-39
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CROSS SECTIONS (XS)	1-45

LENGTH OF PROJECT

ı		INOULUI
	MELVILLE RD.	STA. 1+66.06
ı	BEGINNING OF PROJECT	
ı	END OF PROJECT	STA. 14+04.99
ı	APPARENT LENGTH	1,238.93 FEET
ı	EQUATIONS AND EXCEPTIONS:	0.00 FEET
ı	TOTAL PROJECT CORRECTIONS	0.00 FEET
ı	NET LENGTH OF PROJECT	1,238.93 FEET
ı	STATE LENGTH	0.235 MILES
ı	NORTON RD.	
ı	BEGINNING OF PROJECT	STA. 874+76.64
ı	END OF PROJECT	STA. 883+58.98
ı	APPARENT LENGTH	882.34 FEET
ı	EQUATIONS AND EXCEPTIONS:	0.00 FEET
ı	TOTAL PROJECT CORRECTIONS	0.00 FEET
ı	NET LENGTH OF PROJECT	882.34 FEET
ı	STATE LENGTH	0.167 MILES
ı		



URPHY & TILLY FINDALE STREET MO 65807 (417)

FOR INFORMATION ONLY ESTIMATED DISTURBED ACRES 3.557 ACRES

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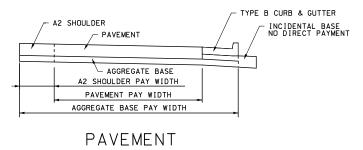
THE PROJECT TO WHICH THIS PAGE REFERS.

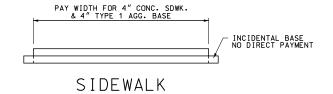
MELVILLE RD.									
OPTIONAL PA	VEMENT	ASPHALT FACTORS							
OPTION A - HMA	OPTION B - PCCP		COMBINED FACTOR						
2" BP-1 W/ PG64-22 8" BB W/ PG64-22 6" TYPE 1 OR 5 AGG. BASE	8" PCCP 15' JOINTS 1.25" DOWELS 6" TYPE 1 OR 5 AGG. BASE	BP-1 W/ PG64-22 BB W/ PG64-22	1.990 TONS/CY 2.000 TONS/CY						

	NORTON	RD.			
OPTIONAL PA	AVEMENT	ASPHALT FACTORS			
OPTION A - HMA	OPTION B - PCCP		COMBINED FACTOR		
2" BP-1 W/ PG64-22 8" BB W/ PG64-22 6" TYPE 1 OR 5 AGG. BASE	8" PCCP 15' JOINTS 1.25" DOWELS 6" TYPE 1 OR 5 AGG. BASE	BP-1 W/ PG64-22 BB W/ PG64-22	1.990 TONS/CY 2.000 TONS/CY		

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ITENDED TO BE USED FOR ANY PART OR PARTS OF
E PROJECT TO WHICH THIS PAGE REFERS.

TACK COAT APPLICATION RATES									
SURFACE TYPE	UNDILUTED	20% DILUTED							
NEW ASPHALT SURFACES	0.05 GAL/SY	0.06 GAL/SY							
EXISTING ASPHALT OR CONCRETE SURFACES	0.08 GAL/SY	0.10 GAL/SY							
COLDMILLED ASPHALT OR CONCRETE SURFACES	0.10 GAL/SY	0.13 GAL/SY							





NOTES:

- 1. SEE PLAN SHEETS FOR SAWCUT LOCATIONS. NO DIRECT PAY FOR PAVEMENT SAWCUTS.
- 2. WHEN CONCRETE OPTIONAL PAVEMENT IS USED, CONTRAST PAVEMENT MARKINGS ARE REQUIRED. NO DIRECT PAYMENT WILL BE MADE FOR CONTRAST MARKINGS.
- 3. THE THINNER PAVEMENT OPTION OF EACH OPTIONAL PAIR WAS USED IN THE COMPUTATION OF EARTHWORK FOR THIS PROJECT.

MELVILLE RD./NORTON RD.

TYPICAL SECTIONS SHEET 1 OF 12 THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY.

DATE PREPARED

12/20/2024

ROUTE

CRD 127 MO

DISTRICT SHEET NO.

SW 2

COUNTY

GREENE

JOB NO.

J 8 S 3 1 5 6

CONTRACT ID.

BRIDGE NO.

H

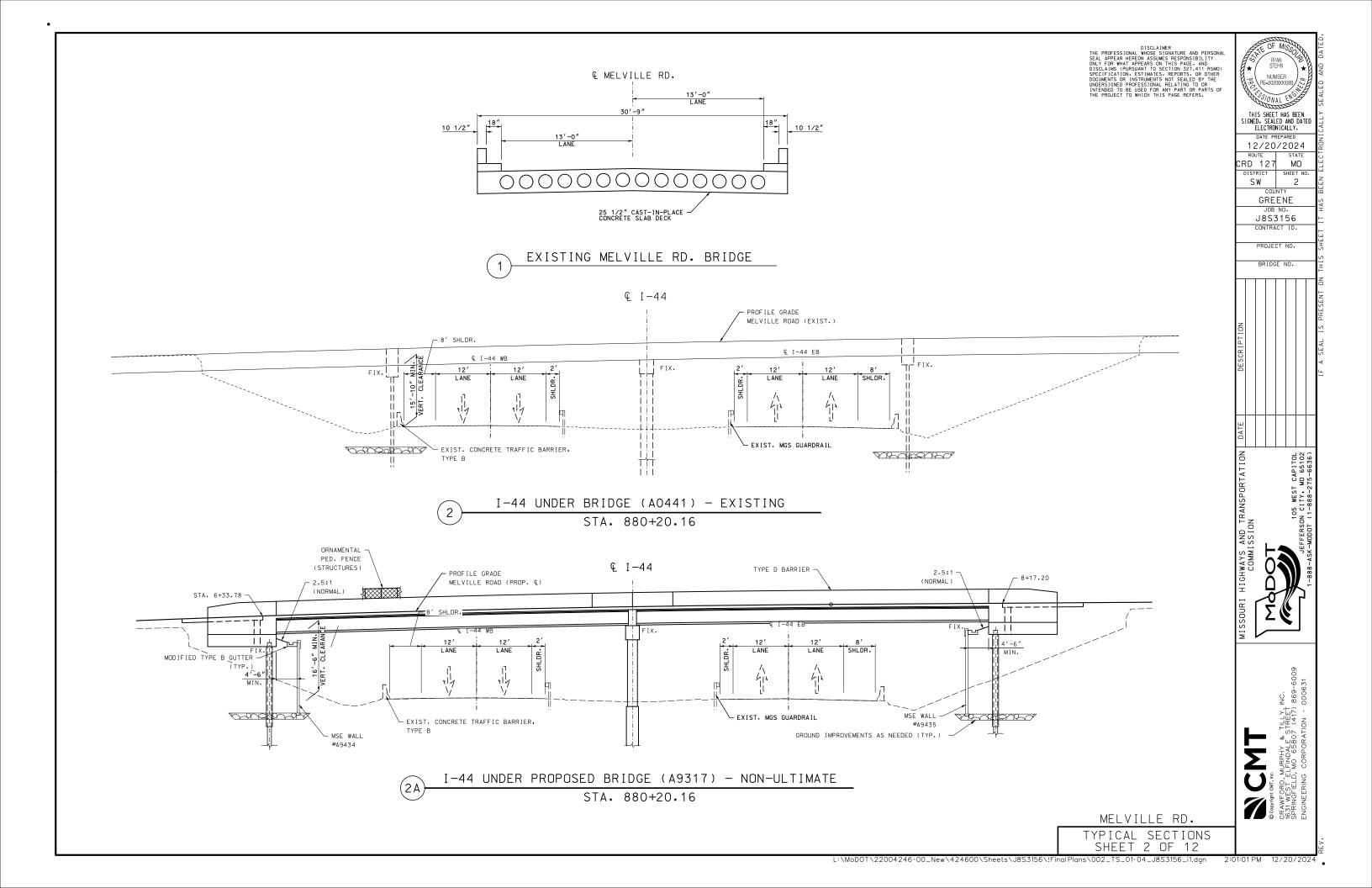
SSION

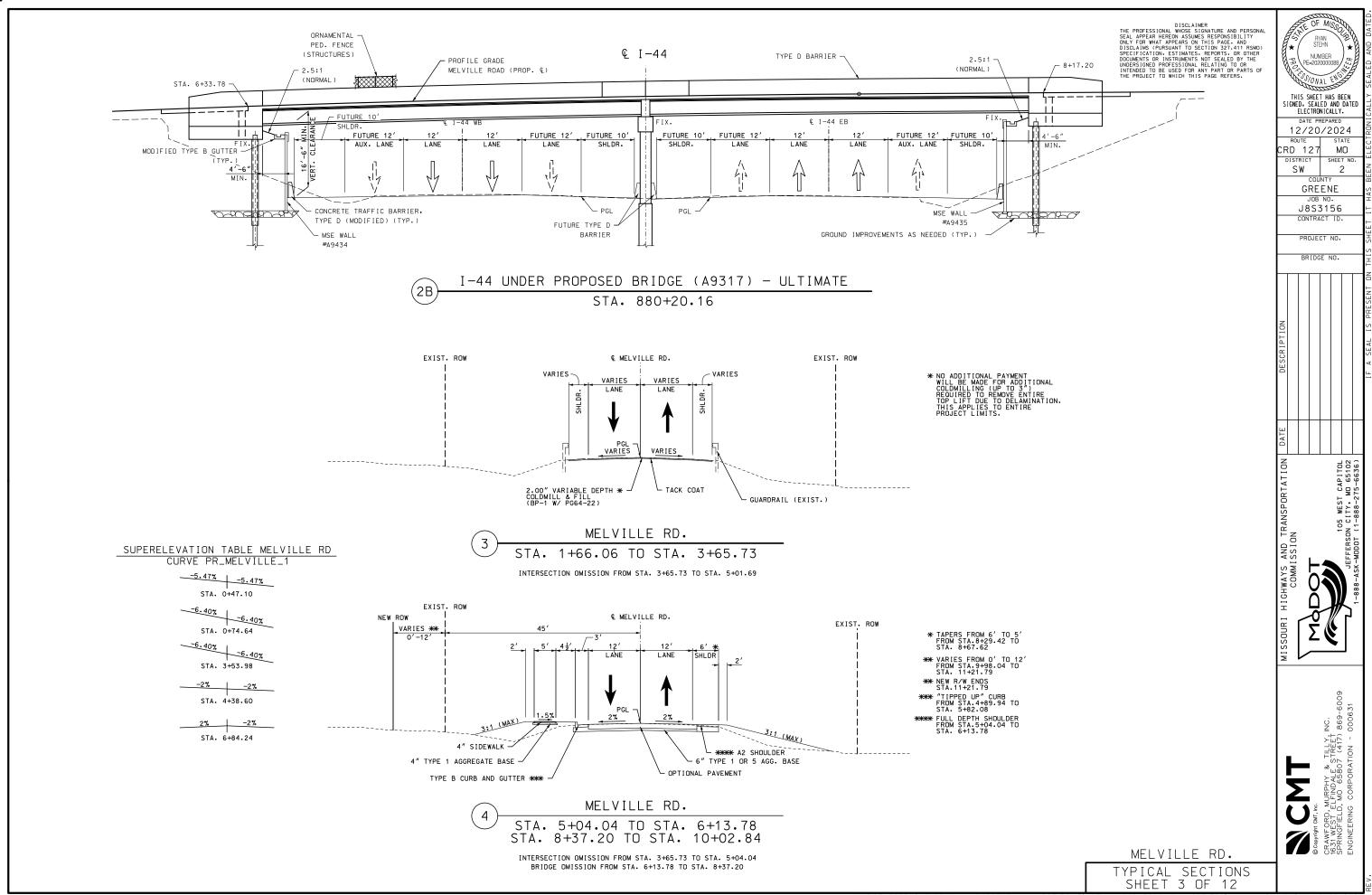
105 WEST CAPITOL

FERSON CITY, MO 65102

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PRINGFIELD, MO 65807 (417) 869-6009
NGINEERING CORPORATION - 0000631





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12/20/2024

RD 127 MO

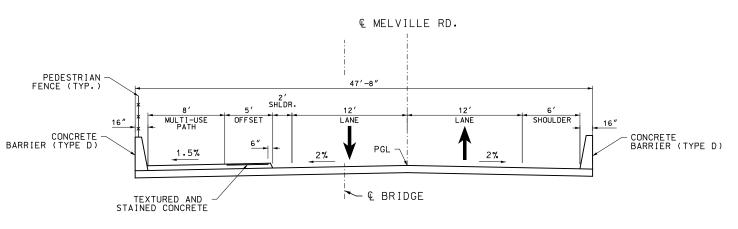
GREENE J8S3156 CONTRACT ID.

PROJECT NO. BRIDGE NO.

SHEET NO

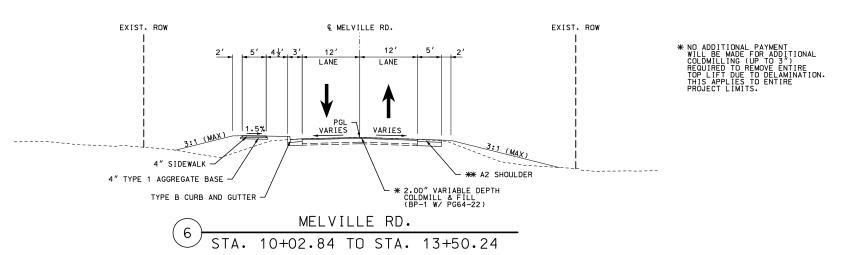
DISTRICT

SW

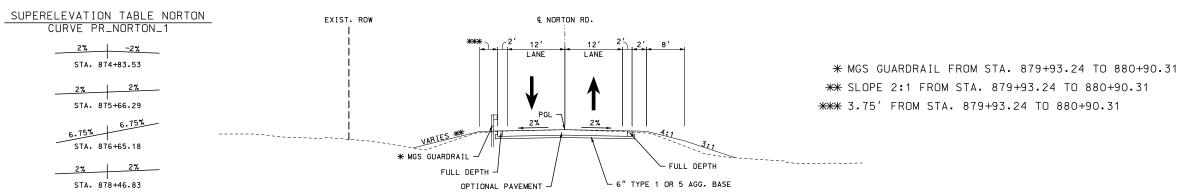


MELVILLE RD. BRIDGE STA. 6+33.21 TO STA. 8+17.77

BRIDGE APPROACH SLAB OMISSION FROM STA. 6+13.78 TO STA. 6+33.21 AND FROM STA. 8+17.77 TO STA. 8+37.20



INTERSECTION OMISSION FROM STA. 13+50.24 TO STA. 14+04.99

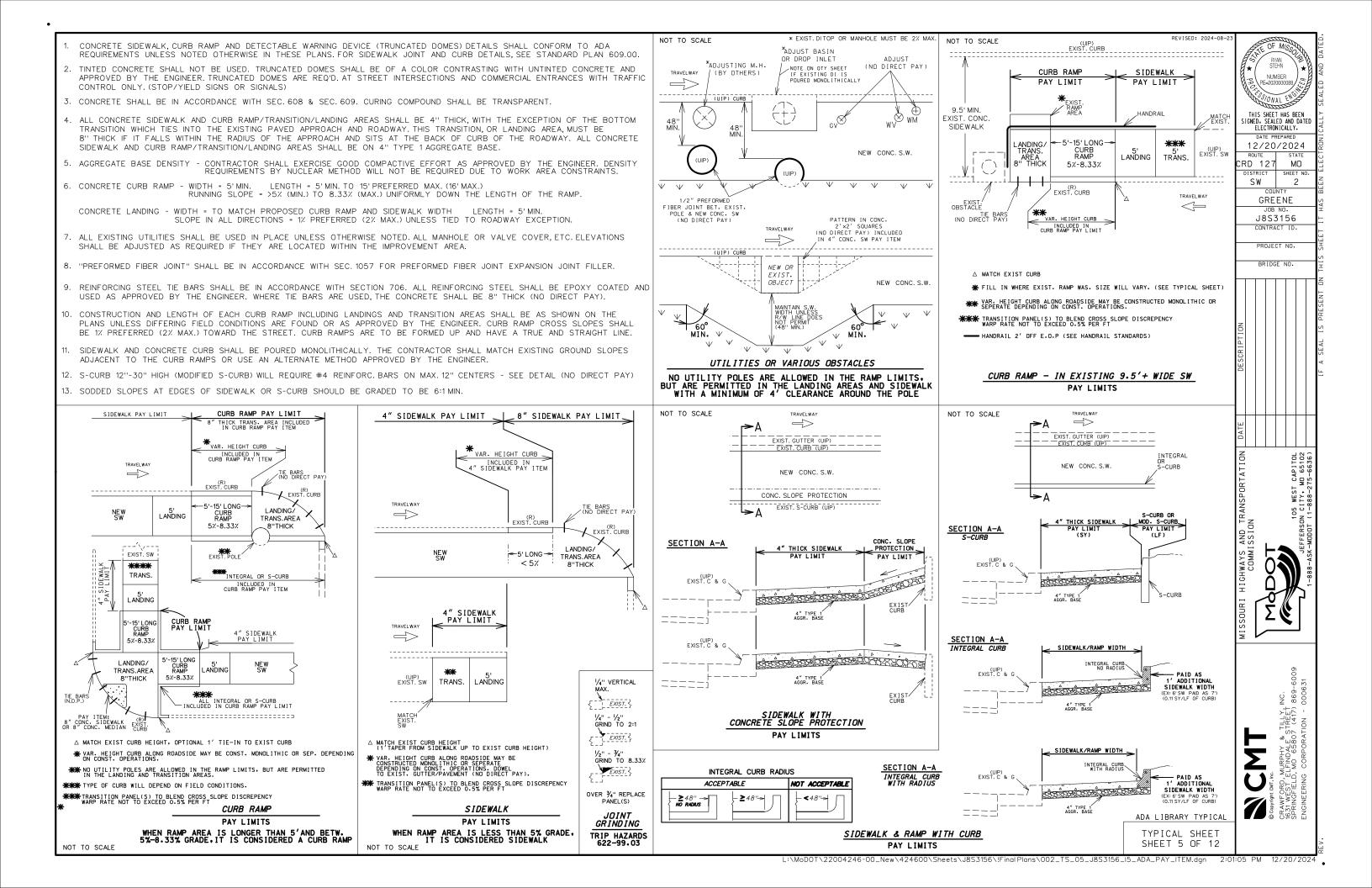


NORTON RD. STA. 874+76.64 TO STA. 883+58.98

INTERSECTION OMISSION FROM STA. 878+43.30 TO STA. 879+93.24

MELVILLE RD./NORTON RD

TYPICAL SECTIONS SHEET 4 OF 12



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T'- IS TO BE 8" THICK
AT ALL PAVED APPROACHES
AND STREETS IF WITHIN THE
RADIUS. PAID FOR AS
CONCRETE CURB RAMP

T - TRANSITION TO EXISTING SW OR PAVED APPROACH (LANDING PREFERED AT PAVED APPROACH IF CONDITIONS ALLOW)

T IS STILL NEEDED EVEN IF THE EXISTING SIDEWALK IS THE SAME WIDTH AS THE NEW SIDEWALK.(IT WILL BLEND THE EXIST XSLOPE INTO THE NEW LANDING XSLOPE)

– LANDING

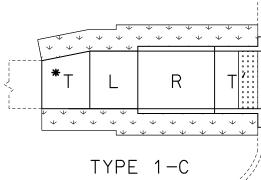
R - RAMP

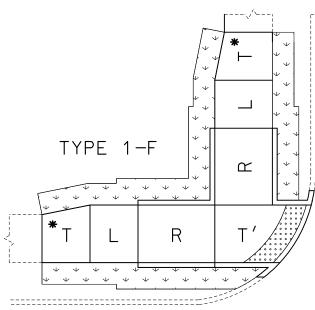
- CURB RAMP PAY LIMITS

- TRUNCATED DOME LOCATION IF DIST. IS < 5' OR >5'

TRUNCATED DOMES ARE USED ONLY AT STREETS & ENTRANCES UTILIZING YIELD / STOP SIGNS OR SIGNALS

QUANTITIES ARE CALCULATED BY AREA AND MAY NOT MATCH EXACT FIELD CONDITIONS.





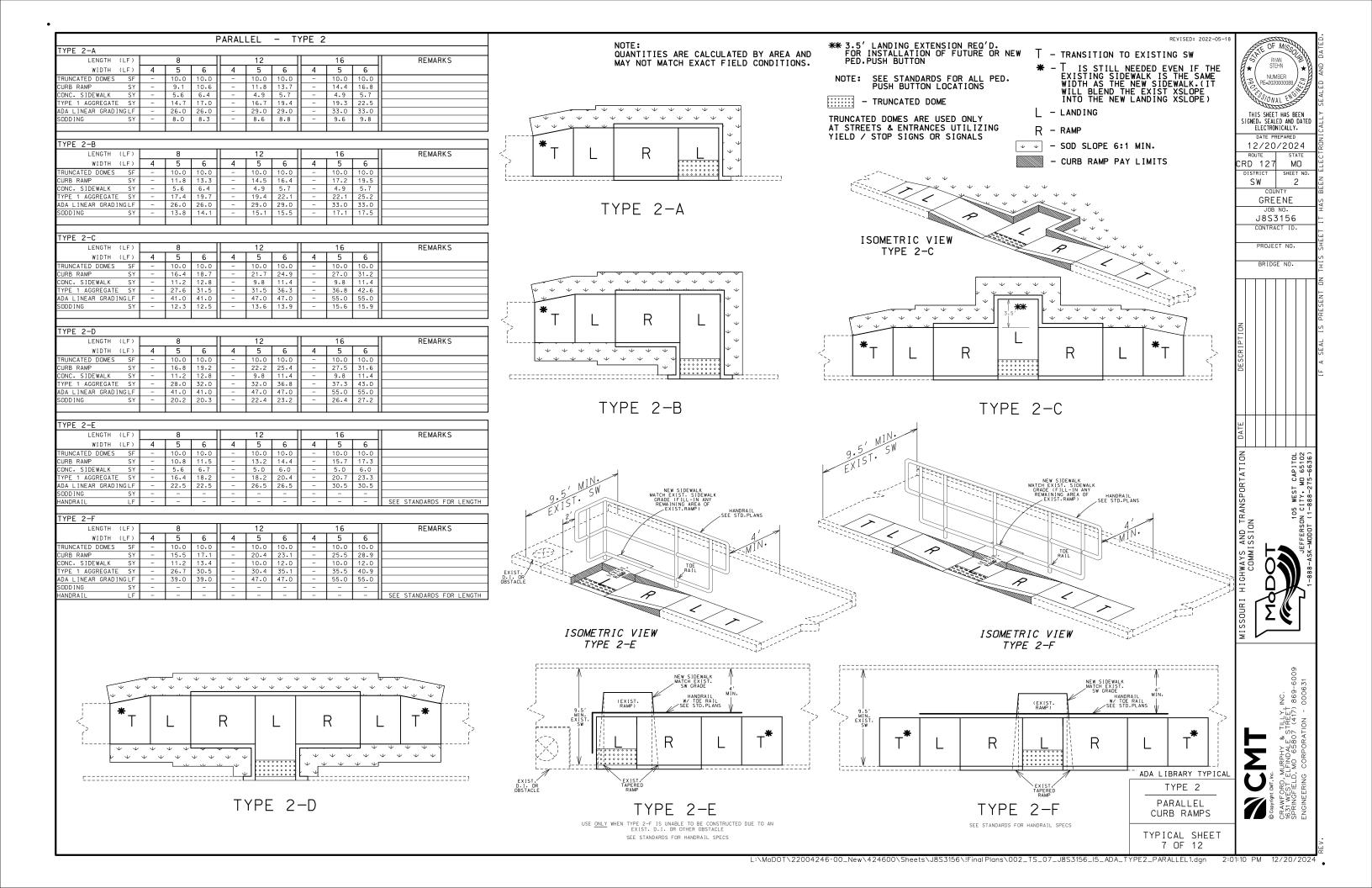
ADA LIBRARY TYPICAL

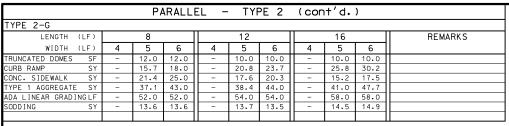
TYPE 1

PERPENDICULAR CURB RAMPS

TYPICAL SHEET 6 OF 12

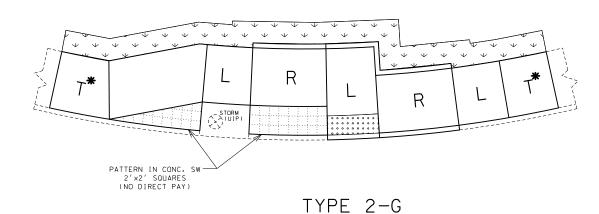
REVISED: 2022-05-18 RYAN STEHN NUMBER PE-202000008 THIS SHEET HAS BEEN SIGNED, SEALED AND DATE ELECTRONICALLY. 12/20/2024 RD 127 MO DISTRICT SHEET NO. SW 2 GREENE J8S3156 CONTRACT ID. PROJECT NO. BRIDGE NO. T CAPITOL MO 65102 D, MURPHY & TILLY, INC. F ELFINDALE STREET LD, MO 65807 (417) 869-6 ING CORPORATION - 00063





TYPE 2-H											
LENGTH (LF)		8			12			16		REMARKS	
WIDTH (LF)	4	5	6	4	5	6	4	5	6		
TRUNCATED DOMES SF	-	12.0	12.0	-	10.0	10.0	-	10.0	10.0		
CURB RAMP SY	-	15.7	18.0	-	21.0	24.5	-	26.9	31.4		
CONC. SIDEWALK SY	-	13.2	16.0	-	12.0	14.2	-	12.0	14.4		
TYPE 1 AGGREGATE SY	-	28.9	34.0	-	33.0	38.6	-	38.9	45.8		
ADA LINEAR GRADINGLF	-	42.0	42.0	-	49.0	49.0	-	55.0	55.0		
SODDING SY	-	11.1	11.2	-	12.5	12.5	-	14.5	14.9		

ISOMETRIC VIEW TYPE 2-G



T*
- IS STILL NEEDED EVEN IF THE
EXISTING SIDEWALK IS THE SAME
WIDTH AS THE NEW SIDEWALK.(IT
WILL BLEND THE EXIST XSLOPE
INTO THE NEW LANDING XSLOPE)

T - TRANSITION TO EXISTING SW

L - LANDING

R - RAMP

→ → - SOD SLOPE 6:1 MIN.

- TRUNCATED DOME LOCATION IF DIST. IS <5' OR > 5'

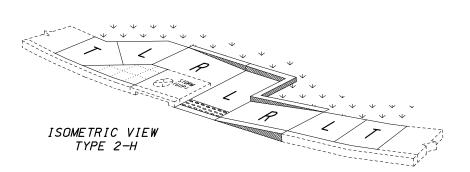
TRUNCATED DOMES ARE USED ONLY AT STREETS & ENTRANCES UTILIZING YIELD / STOP SIGNS OR SIGNALS

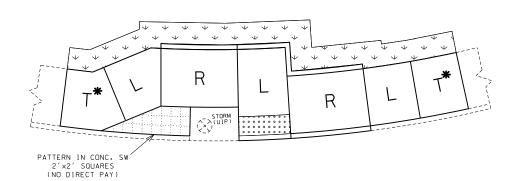
- CURB RAMP PAY LIMITS

- INCLUDED IN 4" CONC. SIDEWALK PAY LIMIT

REVISED: 2022-05-1

QUANTITIES ARE CALCULATED BY AREA AND MAY NOT MATCH EXACT FIELD CONDITIONS.





TYPE 2-H



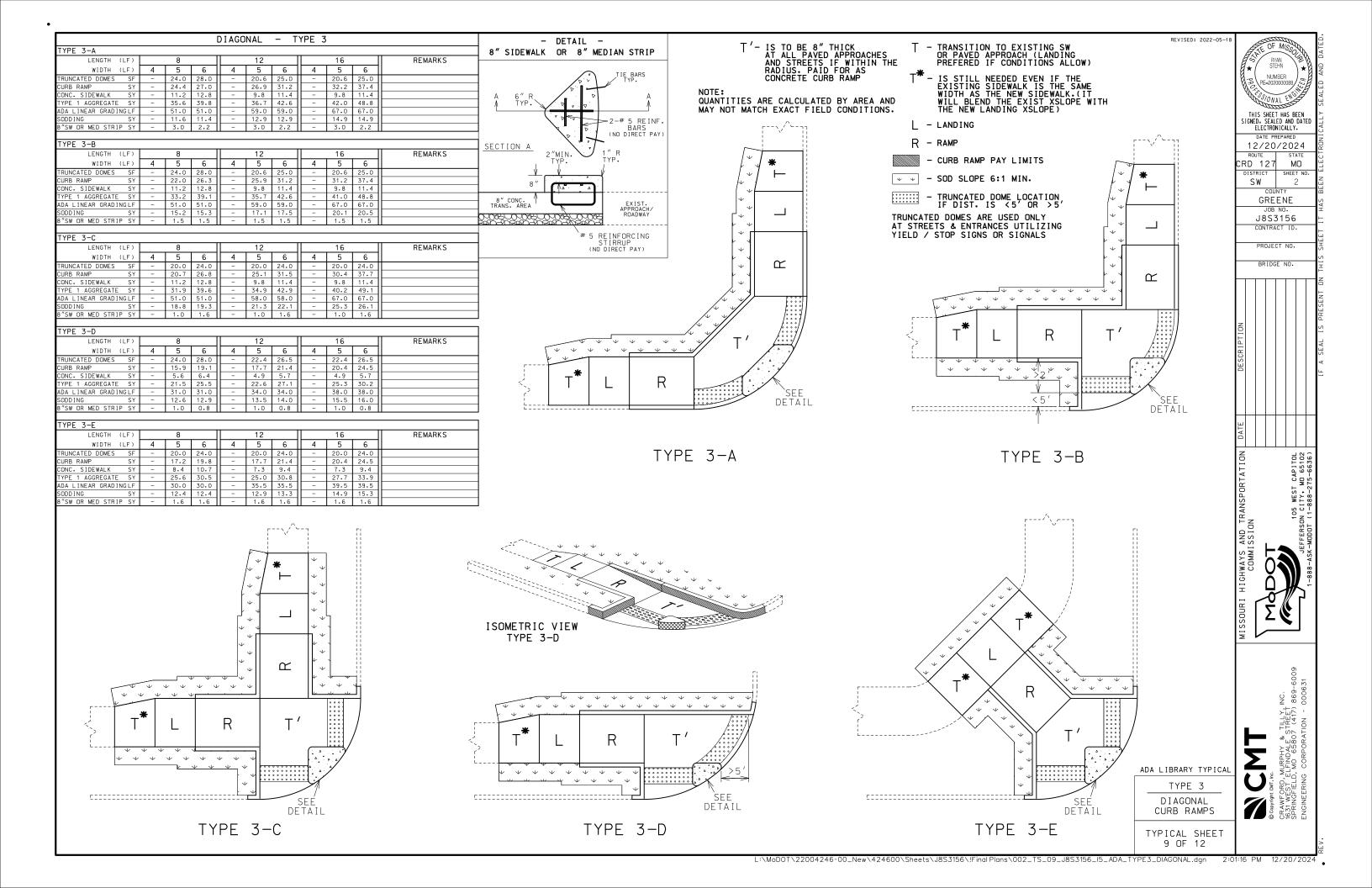
NUMBER

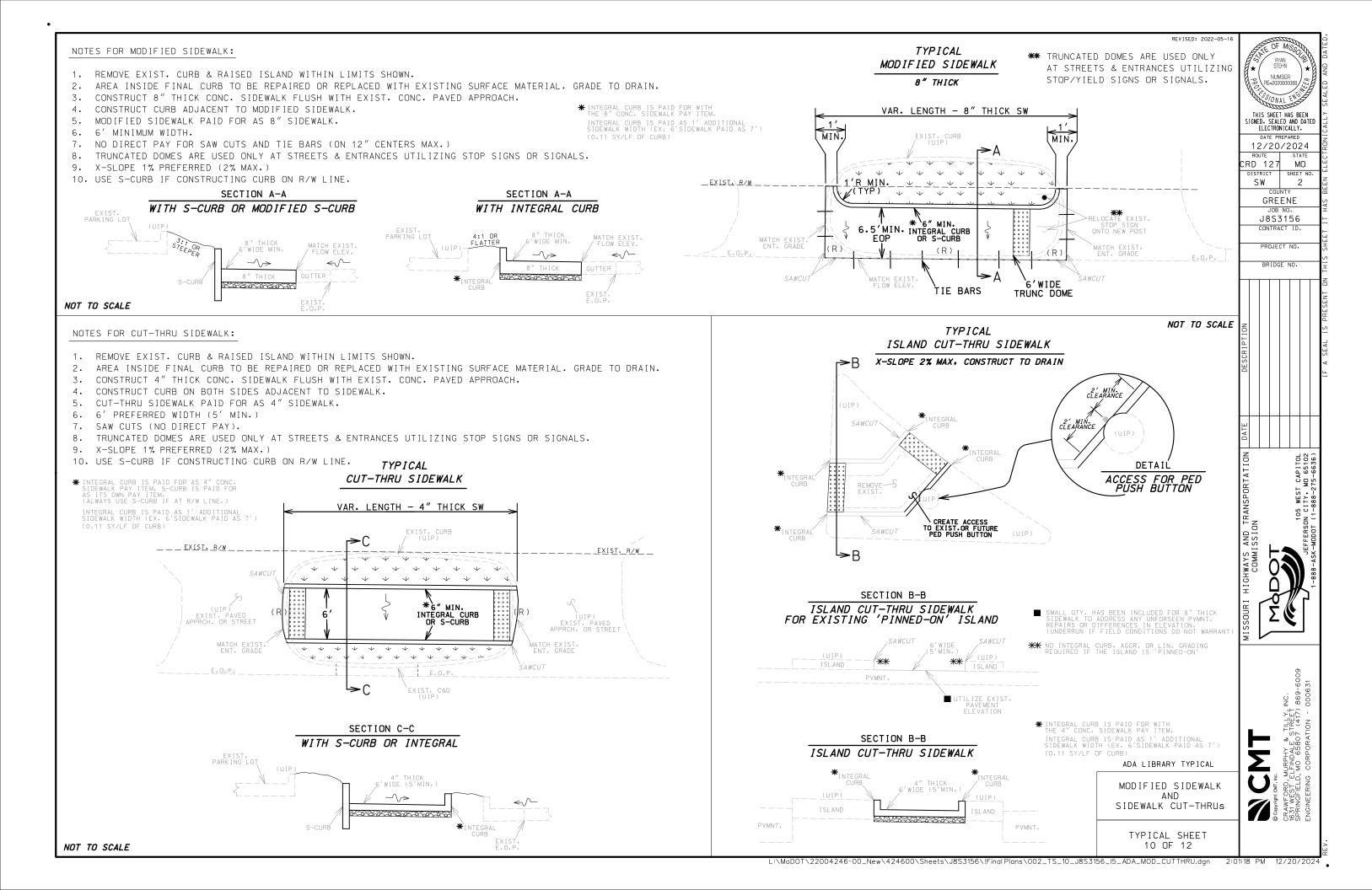
ADA LIBRARY TYPICAL

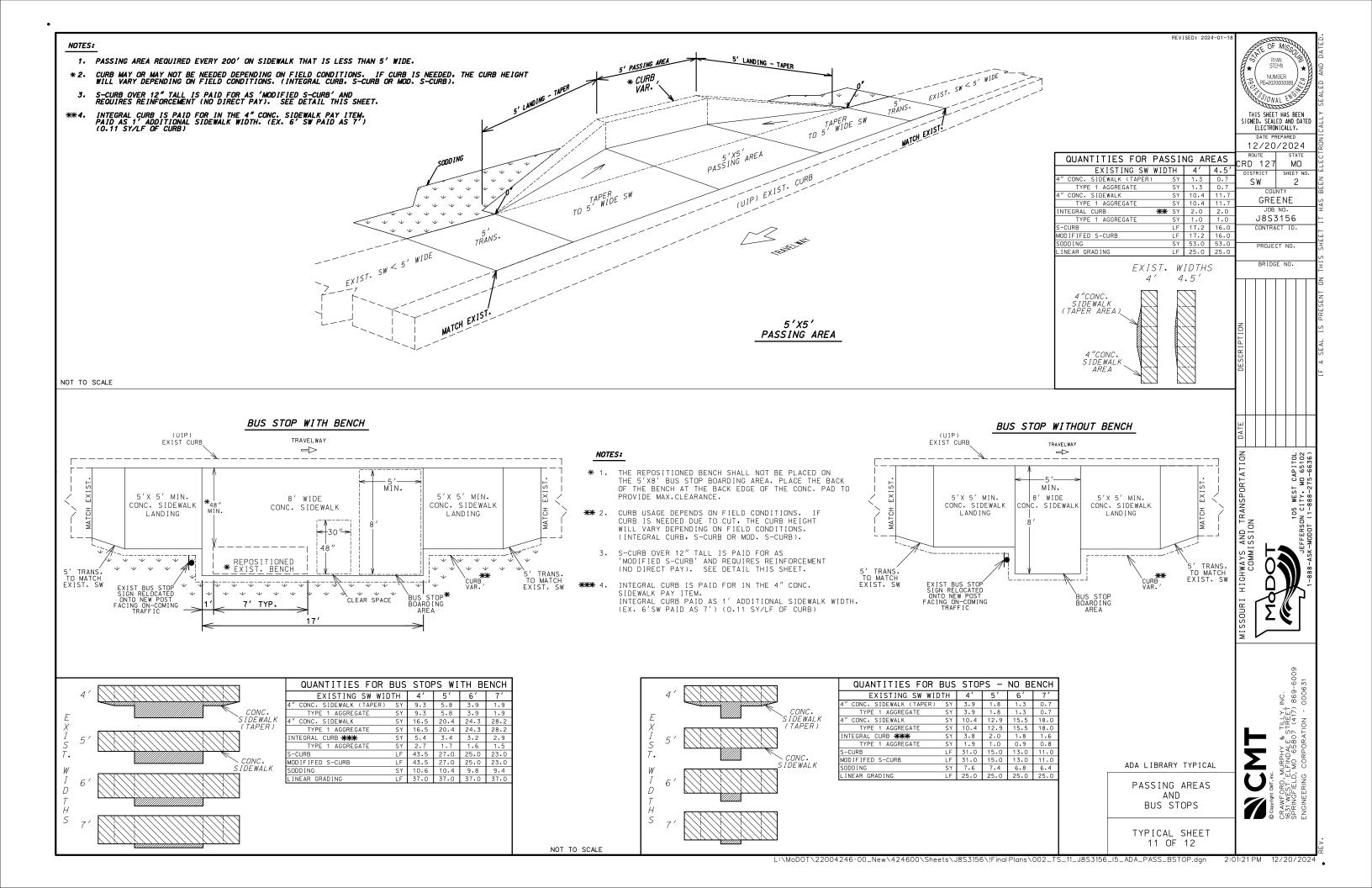
TYPE 2

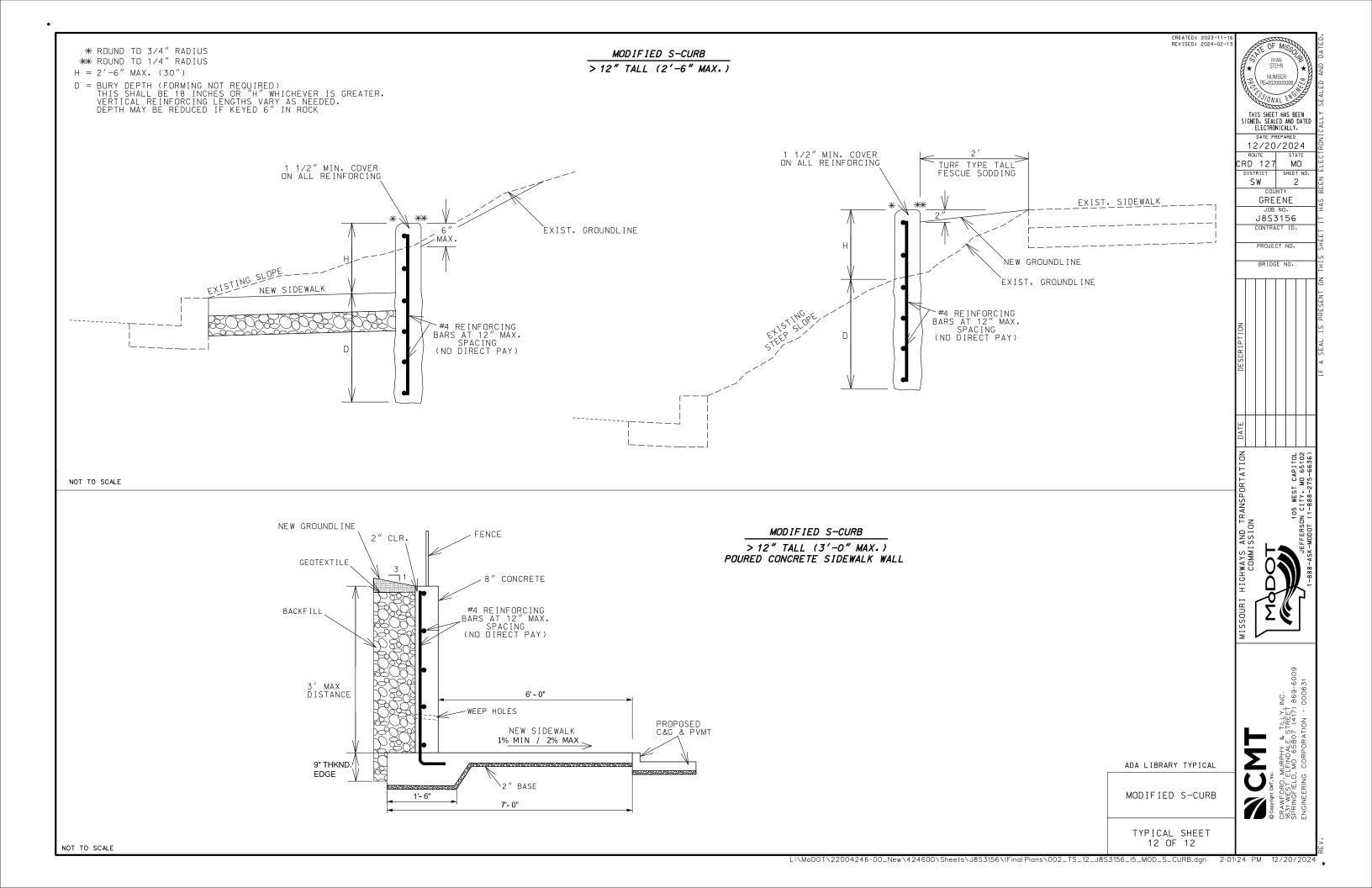
PARALLEL CURB RAMPS

TYPICAL SHEET 8 OF 12









					OF IMPRO			
PLAN SHEET	FROM STATION	TO STATION	BASELINE	OFFSET	DESCRIPTION	UNITS	QUANTITY	REMARKS
4 - 5	3+41.47	4+25.06	MELVILLE	LT.	GUARDRAIL	LF	86.0	
4 - 5	3+65.73	4+88.54	MELVILLE	RT	GUARDRAIL	LF	124.0	
4 - 6	3+65.73	10+02.84	MELVILLE	CL	PAVEMENT	SY	1219.0	
5	5+05	5.53	MELVILLE	LT.	LIGHT POLE	EACH	1.0	
5	5+08	3.02	MELVILLE	LT.	SIGN	EACH	1.0	
5	5+49	9.60	MELVILLE	RT.	SIGN	EACH	1.0	
5	5+58.20	6+05.07	MELVILLE	RT.	GUARDRAIL	LF	48.0	
5	5+91.57	6+19.85	MELVILLE	LT.	GUARDRAIL	LF	29.0	
5	6+03	3.08	MELVILLE	LT.	SIGN	EACH	1.0	
5	10+0	0.00	MELVILLE	LT.	18" PIPE	LF	55.0	
5	10+0	0.00	MELVILLE	LT.	18" FES	EACH	2.0	
5	876+29.94	880+50.20	I -44	RT.	GUARDRAIL	LF	315.0	
5	879+12.66	881+12.66	I -44	LT.	GUARDCABLE	LF	200.0	
5	879+2	26.93	I -44	CL	18 IN. FES	EACH	1.0	
5	879+26.93	881+09.41	I -44	CL	18 IN. PIPE	LF	183.0	
5	879+93.69	1014+28.24	I -44	LT.	GUARDRAIL	LF	415.0	
5	881+09.41		I-44	CL	18 IN. FES	EACH	1.0	
5	8+42.30	8+69.62	MELVILLE	RT.	GUARDRAIL	LF	28.0	
5	8+58.32	9+34.80	MELVILLE	LT.	GUARDRAIL	LF	77.0	
5	8+59.11		MELVILLE	LT.	SIGN	EACH	1.0	
7 - 8	874+76.64 883+58.98		NORTON	CL	PAVEMENT	SY	2783.0	
7	7 875+75.41		NORTON	LT.	SIGN	EACH	1.0	
7	876+23.41	876+42.82	NORTON	LT.	FENCE	LF	39.0	
				1 LUM	P SUM			

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RSONAL RYAN NUMBER PE-202000088								
1/28/2025 ROUTE STATE CRD 127 MO DISTRICT SHEET NO. SW 3 COUNTY GREENE JOB NO. J8S3156 CONTRACT ID. PROJECT NO.	TY RSMD) THER THE	THI SIGNE	PE-	ON HEE	A L S	S BI	EN DATE	9
ROUTE STATE CRD 127 MO DISTRICT SHEET NO. SW 3 COUNTY GREENE JOB NO. J8S3156 CONTRACT ID.		1						
CRD 127 MO DISTRICT SHEET NO. SW 3 COUNTY GREENE JOB NO. J8S3156 CONTRACT ID. PROJECT NO.				. 0 /				
SW 3 COUNTY GREENE JOB NO. J8S3156 CONTRACT ID. PROJECT NO.				27				
COUNTY GREENE JOB NO. J8S3156 CONTRACT ID. PROJECT NO.		DIST	RIC	Т	S	HEE	T NO	٥.
GREENE JOB NO. J853156 CONTRACT ID. PROJECT NO.		S	W				3	
J8S3156 CONTRACT ID. PROJECT NO.			GI	٦E	E١	۱E		
PROJECT NO.								
			CON	TRA	CT	ID	•	
BRIDGE NO.			PRO	JJE:	СТ	NO.		
			BR	IDG	E N	۱0۰		

CLEARING AND GRUBBING											
SHEET	FROM TO STATION STATION	ROADWAY	LOCATION	CLEARING AND GRUBBING (ACRE)	REMARKS						
6	10+04.75	MELVILLE	48.57′ LT	0.01	SINGLE TREE, PREVIOUSLY TOPPED						
6	10+54.19	MELVILLE	42.10' LT	0.01	SINGLE TREE, PREVIOUSLY TOPPED						
6	10+83.24 MELVILLE		44.13′ LT	0.01	SINGLE TREE, PREVIOUSLY TOPPED						
6	11+00.35	MELVILLE	43.47′ LT	0.01	SINGLE TREE, PREVIOUSLY TOPPED						
7	7 874+93.48		27.40' LT	0.01	SINGLE TREE, PREVIOUSLY TOPPED						
7 876+26.11		NORTON	47.80′ LT	0.01	SINGLE TREE, PREVIOUSLY TOPPED						
			TOTAL	1							

MOBILIZATION

1 LUMP SUM

CONTRACTOR FURNISHED SURVEYING AND STAKING
1 LUMP SUM

SAWCUTS FOR PAVEMENT REMOVAL

ALL SAWCUTS NOT SHOWN ARE INCIDENTAL TO REMOVAL OF IMPROVEMENTS

ADDITIONAL MOBILIZATION FOR SEEDING

1 EACH

PLAN SHEET	STATION	ROUTE	LOCATION	CULVERT CLEANOUT (EA)	REMARKS
7	876+76	NORTON	RT	1	
			TOTAL	1	

SUMMARY OF QUANTITIES 1 OF 10

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THE PROJECT TO WHICH THIS PAGE REFERS.

	EARTHWORK											
ROADWAY	CUT	EARTH CUT	ROCK CUT	FILL	EXCAVATION TO BE USED IN EMBANKMENT (CUYD)	UNCLASSIFIED	EMBANK MENT IN PLACE (CUYD)	COMPACTING EMBANKMENT (CUYD)	COMPACTING IN CUT (STA)	REMARKS		
MELVILLE	239	179	60	6,433					1.1	25% OF TOTAL CUT ASSUMED TO BE ROCK CUT		
NORTON	3,986	2,990	997	5,441					4.3	25% OF TOTAL CUT ASSUMED TO BE ROCK CUT		
I-44 (INCLUDES WALLS)	12,749	9,562	3,187	837						25% OF TOTAL CUT ASSUMED TO BE ROCK CUT		
MSE WALL A9434										25% OF TOTAL CUT ASSUMED TO BE ROCK CUT		
MSE WALL A9435										25% OF TOTAL CUT ASSUMED TO BE ROCK CUT		
SUBTOTALS	16,974	12,731	4,244	12,711	14,852							
					TOTALS	16,974	0	12,711	5.4			

NOTE: 25% SHRINK AND SWELL ASSUMED DURING COMPACTION

					ROCK FILL		
	FROM	ТО			FURNISHING	PLACING ROCK	
SHEET	STATION	STATION	BASELINE	LOCATION		FILL	REMARKS
					(CUYD)	(CUYD)	
5	878+88.58	880+93.53	I -44	LT	1,141	1,141	MSE WALL A9434
5	879+57.82	881+85.33	I -44	RT	1,386	1,386	MSE WALL A9435
				TOTALS	2,527	2,527	

					[DRAINAGE	PIPES			
PLAN SHEET	CULVERT SECTION SHEET	FROM STRUCT.	TO STRUCT.	PIPE COLLAR, TYPE A (EA)	12 IN. PIPE GROUP B (LF)	18 IN. PIPE GROUP B (LF)	36 IN. PIPE GROUP B (LF)	18 IN. PIPE GROUP A (LF)	CLASS 3 EXCAVATION (CUYD)	REMARKS
5	1	T-01	FES-01	, , , ,	19		, , ,	\ \(\C\)!	0	
5	1	T-02	FES-02		22				2	
6	1	T-03	FES-03			24			2	
6	1	T-03	FES-04					45	32	
6	1	T-03	T-04		150				67	
7	1	COL-01	FES-05	1			59		5	
5	1	FES-06	COL-02	1	·			96	46	
5	1	COL-02	FES-07					73	36	
									190	SEE STRUCTURES FOR TOTAL
			TOTALS:	2	191	24	59	214	_	

THIS SHEET HAS BEEN SIGNED. SEALED AND DATED ELECTRONICALLY.

DATE PREPARED

1 2 / 2 0 / 2 0 2 4 ROUTE STATE
CRD 127 MO
DISTRICT SHEET NO.
SW 3 COUNTY GREENE JOB NO.
J8S3156
CONTRACT ID. PROJECT NO. BRIDGE NO.

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CRAWFORD, MURPHY & TILLY, INC.
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ISN WEST ELFINDALE STREET
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SUMMARY OF QUANTITIES 2 OF 10

					FLARE	D END SEC	TIONS			
PLAN SHEET	CULVERT SECTION SHEET	FROM STRUCT.	TO STRUCT.	12 IN. OR ALLOWED SUBSTITUTE GROUP B FLARED END SECTION (EA)	18 IN. OR ALLOWED SUBSTITUTE GROUP B FLARED END SECTION (EA)	36 IN. OR ALLOWED SUBSTITUTE GROUP B FLARED END SECTION (EA)	18 IN. OR ALLOWED SUBSTITUTE GROUP A FLARED END SECTION (EA)	18 IN. OR ALLOWED SUBSTITUTE SAFETY SLOPE END SECTION (EA)	ROCK LINING (CUYD)	REMARKS
5	1	T-01	FES-01	1					2	
5	1	T-02	FES-02	1					2	
6	1	T-03	FES-03		1				2	
6	1	T-03	FES-04				1		2	
7	1	COL-01	FES-05			1			9	
5	1	FES-06	COL-02					1		
5	1	COL-02	FES-07					1		
			TOTALS:	2	1	1	1	2	17	

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									RAINAGE STRUC	TURES				
STRUCT. NO.	. PLAN SHEET	CULVERT SECTION SHEET	STATION	ROADWAY	LOCATION	STD.	TYPE	ACTUAL DEPTH	PRECAST CONCRETE DROP INLET 5 FT X 3 FT (FT)	CLASS 3 EXCAVATION (CUYD)	CLASS 3 EXCAVATION IN ROCK (CUYD)	CURB INLET (EA)	PIPE OPENINGS (NUMBER - SIZE)	REMARKS
T-01	5	1	5+85.08	MELVILLE	16.50′ LT.	731.10S	T 5 x 3	3.01	3	0		1	1 - 12"	
T-02	5	1	8+76.66	MELVILLE	16.50′ LT.	731.105	T 5 x 3	3.19	3	4		1	1 - 12"	
T-03	6	1	10+00.60	MELVILLE	16.50' LT.	731.10S	T 5 x 3	5.67	6	11		1	1 - 12"; 2 - 18"	
T-04	6	1	11+55.00	MELVILLE	16.50' LT.	731.105	T 5 x 3	3.00	3	6		1	1 - 12"	
										190				TOTAL FROM PIPES
											21			10% OF CLASS 3 EXC.
								TOTALS	15	211	21	4		

NOTE: EXCAVATION QUANTITIES ASSUME PRECAST CONCRETE STRUCTURES

			DRAII	NAGE STRU	JCTURE LIDS	5	
STRUCT, NO.	PL AN SHEET	CULVERT SECTION SHEET	STATION	ROADWAY	LOCATION	MANHOLE FRAME AND COVER, TYPE 3 (EA)	REMARKS
T-01	5	1	5+85.08	MELVILLE	16.50′ LT.	1	
T-02	5	1	8+76.66	MELVILLE	16.50' LT.	1	
T-03	6	1	10+00.60	MELVILLE	16.50' LT.	1	
T-04	6	1	11+55.00	MELVILLE	16.50' LT.	1	

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IGSI WEST ELFINDALE STREET

SPRINGFIELD, MO 65807 (417) 869-6009

SUMMARY OF QUANTITIES 3 OF 10

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				OPTIONAL	PAVEMENT		
	FROM	ТО			OPTIONAL	TYPE A2	
SHEET	STATION	STATION	BASELINE	LOCATION	PAVEMENT	SHOULDER	REMARKS
					(SQYD)	(SQYD)	
4 - 5	3+65.73	6+13.78	MELVILLE	LT & RT	1,391.6		
5 - 6	8+37.20	13+51.24	MELVILLE	LT & RT	441.7	297.8	
7 - 8	874+76.64	883+58.98	NORTON	LT & RT	2,402.1		
				TOTALS	4235.4	297.8	

					NC)N-OPTIONAL	PAVEMENT					
SHEET	FROM STATION	TO STATION	BASEL INE	LOCATION	COLDMILLING BITUMINOUS PAVEMENT FOR REMOVAL OF SURFACING (3 IN. THICK OR LESS) (SQYD)	BITUMINOUS PAVEMENT MIXTURE PG64-22, (BP-1) (TONS)	TACK COAT (GAL)	TYPE 5 AGGREGATE FOR BASE (4 IN. THICK) (SQYD)	5 ³ 4 INCHES, BITUMINOUS PAVEMENT (SQYD)	CURB AND GUTTER TYPE B (LF)	MODIFIED CONCRETE GUTTER TYPE B (LF)	REMARKS
4	1+66.06	3+65.73	MELVILLE	LT & RT	583	65.6	59					
5	4+98.94	6+21.98	MELVILLE	LT						124		
5 - 6	8+43.76	14+18.20	MELVILLE	LT						610		
6	10+02.84	14+18.20	MELVILLE	LT & RT	1251	140.8	126					
8	881+52.29	881+93.03	NORTON	LT				62	61.9			
5	878+88.58	880+93.53	I -44	LT							205	MSE WALL A9434
5	879+57.82	881+85.33	I -44	RT							228	MSE WALL A9435
				TOTALS	1834	206.4	185	*	61.9	734	433	

* SEE SIDEWALK TABLE FOR TOTAL

RYAN STEHN NUMBER PE-202000008 THIS SHEET HAS BEEN SIGNED. SEALED AND DATED ELECTRONICALLY.

DATE PREPARED

12/20/2024 ROUTE STATE
CRD 127 MO
DISTRICT SHEET NO.
SW 3 COUNTY GREENE JOB NO.
J8S3156
CONTRACT ID. PROJECT NO. BRIDGE NO.

				BASE		
SHEET	FROM STATION	TO STATION	BASELINE	LOCATION	TYPE 5 AGGREGATE FOR BASE (6 IN. THICK) (SQYD)	REMARKS
4 - 5	3+65.73	6+13.78	MELVILLE	LT & RT	1,433	
5 - 6	8+37.20	13+51.24	MELVILLE	LT & RT	893	
7 - 8	874+76.64	883+58.98	NORTON	LT & RT	2,403	
				TOTALS	4729	

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12/20	/2024	RO
ROUTE	STATE	CI
CRD 127	MO	E.
DISTRICT	SHEET NO.	
SW	3	BEEN
COU	NTY	BE
GRE	ENE	HAS
JOB	NO.	Ŧ
	3156	
CONTRA	CT ID.	—
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PROJE	CT NO.	픘

GREENE
JOB NO.
J8S3156
CONTRACT ID.
PROJECT NO.

BRIDGE	NO.

DESCRIPTION				
DATE				

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DATE				
ANSPORTATION			5 WEST CAPITOL	



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78	ပိ ⊚	OR 64	SPR	EN

				TEMF	PORARY PAVE	MENT		
PHASE	FROM STATION	TO STATION	BASELINE	LOCATION	7 INCHES, BITUMINOUS PAVEMENT (SQYD)	TYPE 5 AGGREGATE FOR BASE (4 IN. THICK) (SQYD)	LINEAR GRADING CLASS 1 (STA)	REMARKS
2B	0+58.74	3+33.72	TEMP	LT & RT	637.5	638.0	2.8	
	·			TOTALS	637.5	*	2.8	
						* SEE SIDEWALK TAE	BLE FOR TOTAL	

			I E MP	URARY PAV 	EMENT MARKIN 		
PHASE	FROM STATION	TO STATION	BASELINE	LOCATION	TEMPORARY REMOVABLE MARKING TAPE 4 IN., WHITE (LF)	TEMPORARY REMOVABLE MARKING TAPE 4 IN., YELLOW (LF)	REMARKS
PHASE 2B	0+00.00	3+91.00	TEMP	CL		391.0	
PHASE 2B	0+00.00	3+91.00	TEMP	LT & RT	828.7		
				TOTALS	829.0	391.0	

	T			I		SIDEWALK			
PLAN SHEET	FROM STATION	TO STATION	BASELINE	LOCATION	TYPE 1 AGGREGATE FOR BASE (4 IN. THICK) (SQYD)	CONCRETE SIDEWALK, 4 IN. (SQYD)	CONCRETE CURB RAMP (SQYD)	TRUNCATED DOMES	REMARKS
5	4+76.15	6+28.55	MELVILLE	LT	101	101.2		16	
5 – 6	8+45.40	13+43.01	MELVILLE	LT	304	294.6	8.9	10	
					62				TOTAL FROM PAVEMENT NON-OPTIONAL
					638				TOTAL FROM TEMPORARY PAVEMENT
				TOTALS	1105	395.8	8.9	26	

				NON-CONTRAC	TUAL ITEMS		DISCLAIMER HE PROFESSIONAL WHOSE SIGNATURE AND PERSONAL EAL APPEAR HEREON ASSUMES RESPONSIBILITY MEY-FOR WHAT APPEARS ON THIS PAGE, AND ISCLAIMS (PURSUANT TO SECTION 327.411 RSMO) PECIFICATION. ESTIMATES. REPORTS. OR OTHER OCUMENTS OR INSTRUMENTS NOT SEALED BY THE MODERSIGNED PROFESSIONAL RELATING TO OR
PLAN SHEET	ROADWAY	LOCATION	UTILITY CONNECTION CHARGE (LS)	STEEL RIGHT OF WAY MARKERS (EA)	2 IN. ALUMINUM CAP WITH MODOT R/W IMPRINT (EA)		NTENDED TO BE USED FOR ANY PART OR PARTS OF HE PHOJECT TO WHICH THIS PAGE REFERS.
6	MELVILLE	LT		3	3	3	
7	NORTON	LT		2	2	2	
			1				
		TOTALS	1	5	5	5	

						SUARDRAIL	& GUARDCABLE			
PLAN SHEET	FROM STATION	TO STATION	BASELINE	LOCATION	MGS GUARDRAIL (LF)	MGS END ANCHOR (EA)	TYPE A CRASHWORTHY END TERMINAL (MASH) (EA)	MGS BRIDGE APPROACH TRANSITION SECTION (REGULAR/NO CURB) (EA)	MEDIAN GUARD CABLE 3 - STRAND (LF)	REMARKS
4-5	3+41.47	4+15.89	MELVILLE	LT	75.0					
4	3+65.73	4+04.39	MELVILLE	RT	25.0	1				
5	4+87.42	6+10.36	MELVILLE	RT	25.0		1	1		
5	8+20.92	9+54.56	MELVILLE	RT	37.5		1	1		
5	877+49.05	880+50.79	I -44	RT	237.5	1	1			
5	879+12.66	881+12.66	I -44	LT		·			200	
5	879+99.82	883+51.54	I -44	LT	287.5	1	1			
5, 8	879+33.57	880+90.27	NORTON	LT	125.0		1			
				TOTALS	813	3	5	2	200	

				PAV	VEMENT MARK	ING	
PLAN SHEET	FROM STATION	TO STATION	ROADWAY	LOCATION	4 IN. YELLOW STANDARD WATERBORNE PAVEMENT MARKING PAINT, TYPE P BEADS (LF)	4 IN. WHITE STANDARD WATERBORNE PAVEMENT MARKING PAINT, TYPE P BEADS (LF)	REMARKS
38	1+66.07	13+71.49	MELVILLE	CL	2,410		
38	1+66.07	4+24.03	MELVILLE	LT		283	
38	1+66.07	4+24.03	MELVILLE	RT		281	
38	4+50.76	14+18.20	MELVILLE	LT		1,032	
38	4+50.76	13+51.49	MELVILLE	RT		931	
38	874+76.67	878+77.77	NORTON	CL	800		
38	874+76.67	878+45.68	NORTON	LT		361	
38	874+76.67	878+45.68	NORTON	RT		376	
38	879+36.54	883+58.98	NORTON	CL	844		
38		883+58.98	NORTON	LT		410	
38	879+93.24	883+58.98	NORTON	RT		369	
			TOTA	LS	4,054	4,043	

RYAN STEHN NUMBER PE-2020000088 THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY.

DATE PREPARED 12/20/2024 ROUTE STATE CRD 127 MO
DISTRICT SHEET NO.

SW 3 COUNTY GREENE JOB NO.
J8S3156
CONTRACT ID. PROJECT NO. BRIDGE NO. HIGHWAYS AND TRANSPORTATION COMMISSION

SUMMARY OF QUANTITIES 6 OF 10

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	CONCRETE TRAFFIC BARRIER												
SHEET	FROM STATION	TO STATION	BASELINE	LOCATION	CONCRETE TRAFFIC BARRIER, TYPE D (MOMENT SLAB)	REMARKS							
5	6+03.55	6+28.55	MELVILLE	LT	25								
5	8+52.69	8+77.69	MELVILLE	LT	25								
				TOTALS	50								

	EROSION CONTROL - PRE- CONSTRUCTION												
SHEET	FROM STATION	TO STATION	BASELINE	LOCATION	SED I MENT REMOVAL	SILT FENCE	ROCK DITCH CHECK	REMARKS					
					(CUYD)	(LF)	(LF)						
32	2+0	0.01	MELVILLE	RT	1.0		8						
32	3+60.66	881+55.83 N	MELVILLE/ NORTO	N LT	3.0	291							
32	7+66.50	12+40.45	MELVILLE	RT	5.0	475							
32	8+62.82	14+33.43	MELVILLE	LT	6.0	622							
32	9+99	9.98	MELVILLE	RT	1.0		8						
32	874+	76.06	NORTON	RT	1.0		8						
32	874+	76.32	NORTON	LT	1.0		8						
32	875+40.94	877+54.13	NORTON	LT	2.0	211							
32	875+68.01	878+28.09	NORTON	RT	1.0	97							
32	876+6	61.83	NORTON	RT	1.0		8						
32	876+66.63	878+28.09	NORTON	RT	3.0	254							
32	879+0	09.84	I -44	LT	1.0		8						
32	880+0	04.47	I -44	RT	1.0		8						
32	880+	09.81	I -44	LT	1.0		8						
32	880+77.36	881+58.72	NORTON	RT	1.0	144							
32	881+0	04.47	I -44	RT	1.0		8						
32	881+0	09.70	I -44	LT	1.0		8						
32	881+	74.84	NORTON	RT	1.0		8						
32	881+93.66	882+44.07	NORTON	LT	1.0	58							
32	882+0	04.45	I -44	RT	1.0		8						
32	882+5	51.87	NORTON	LT	1.0		6						
32	883+5	56.60	NORTON	RT	1.0		8						
32	883+6	53.10	NORTON	LT	1.0		6						
				SUBTOTALS	37	2,152	116						
				TOTALS	*	2,152	*						
						2,152 CONSTRUCTION FOR TO							

RYAN STEHN NUMBER PE-202000008 THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY.

DATE PREPARED
1 2 / 2 0 / 2 0 2 4 ROUTE STATE
CRD 127 MO
DISTRICT SHEET NO.
SW 3 COUNTY GREENE JOB NO.
J8S3156
CONTRACT ID. PROJECT NO. BRIDGE NO.

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				EROSION (CONTROL - DUF	RING CONSTR	UCTION		
SHEET	FROM TO STATION STATION	BASEL INE	LOCATION	FURNISHING TYPE 2 ROCK BLANKET (CUYD)	PLACING TYPE 2 ROCK BLANKET (CUYD)	SEDIMENT REMOVAL (CUYD)	CURB INLET CHECK (EACH)	ROCK DITCH CHECK (LF)	REMARKS
33	5+85.06	MELVILLE	LT			1.0	1		
33	8+77.36	MELVILLE	LT			1.0	1		
33	10+00.88	MELVILLE	LT			1.0	1		
33	11+55.02	MELVILLE	LT			1.0	1		
33	12+98.99	MELVILLE	RT			1.0		8	
33	877+62.32	NORTON	LT			1.0		8	
33	878+39.66	NORTON	LT			1.0		8	
5	878+88.58 880+93.53	I -44	LT	48	48				BEHIND MSE WALL A9434
5	879+57.82 881+85.33	I -44	RT	53	53				BEHIND MSE WALL A9435
						37		116	TOTAL FROM PRE-CONSTRUCTION
			TOTALS	101	101	44	4	140	

				EROSION	CONTROL - POST	-CONSTRUCT	ION	
SHEET	FROM STATION	TO STATION	BASELINE	LOCATION	SEEDING - COOL SEASON MIXTURES (ACRE)	MULCHING (ACRE)	TURF TYPE TALL FESCUE SODDING (SQYD)	REMARKS
34	4+81.00	6+06.00	MELVILLE	LT	(7101127	(710112)	61	
34	7+69.00	13+71.00	MELVILLE	RT	0.23		01	
34	8+39.00	14+05.00	MELVILLE	LT	0.22			
34	8+73.00	13+37.00	MELVILLE	LT	7 - 2		231	
34	874+76.00	878+95.00	NORTON	LT	0.23			
34	874+76.00	878+90.00	NORTON	RT	0.52			
34	879+20.00	883+59.00	NORTON	LT	0.16			
34	879+20.00	883+59.00	NORTON	RT	0.33			
34	881+83.00	883+59.00	NORTON	LT	0.03			
				SUBTOTALS	1.72	1.72	292	
				TOTALS	1.7	1.7	292	

THIS SHEET HAS BEEN
SIGNED. SEALED AND DATED
ELECTRONICALLY.

DATE PREPARED
1/28/2025
ROUTE
CRD 127 MO
DISTRICT SHEET NO.
SW 3

COUNTY
GREENE
JOB NO.
J8S3156
CONTRACT ID.

PROJECT NO.

BRIDGE NO.

HILL

BRIDGE NO.

BRIDGE NO.

HI

MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION

MADOT

105 WEST CAPITOL
LARRALASK-ANDROT (1-888-275-6876)

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1631 WEST ELFINDALE STREET 869
1875 RINGFIELD, MO 68807 (477) 869-6009
NACHIETERING CAPATION - DADRES 1

SUMMARY OF QUANTITIES 8 OF 10

								POLI	ES,	ARMS,	AND L	UMIN	ARES	(MoDOT)								
LIGHTING	LUMIN.				30′		45	' POLE		45' POLE	15' POLE			LUMINAI	RE					CABLE 10 AWG	POLE FOL	INDATION
SHEET	POLE				POLE		Τì	/PE AT		TYPE B	TYPE MB				150W HP SODIUM		BRACKE	T ARM		1 CONDUCTOR	30′	45′
NO.	NO.	STATION	OFFSET	LOCATION	AT B	D1	D2	D3 D4	D5	D3	D3	POLE	LED-A	LED-B LED-C	UNDERPASS	4 ′	6' 8'	10' 12	151	POLE AND BRACKET	POLE	POLE
1	A101	3+91.83	25.00' LT	MELVILLE	1									1				1		80	1	
1	A201	5+01.69	22.80′ RT	MELVILLE	1									1				1		42	1	
				TOTALS	2									2				1 1		122	2	

l _									
				CABLE-CO	S TIUDNC	WIR	ES (MoDOT)		
									CABLE- CONDUIT,
									1 IN., 2
									CONDUCTORS AND
L	. I GHT I NG				CENTER TO		CABLE, 8 AWG 1	WIRE, 8 AWG, BARE	1 BARE NEUTRAL,
	SHEET	FROM	TO	LOCATION	CENTER	CIR.	CONDUCTOR	NEUTRAL	8 AWG
	1	PS A	CS A	NORTON		Α			45
	1	CS A	A101	NORTON	188.2	Α			198
	1	A101	A 1	MELVILLE	114.9	Α	318	159	
	1	Α1	A201	MELVILLE	49.0	Α	178	89	
					TOT	ALS:	496	248	243

			CABLE-C	ONDUIT 4	& WIF	RES (CITY)		
								CABLE- CONDUIT,
								1 IN., 2
								CONDUCTORS AND
LIGHTING				CENTER TO		CABLE, 8 AWG 1	WIRE, 8 AWG, BARE	1 BARE NEUTRAL,
SHEET	FROM	TO	LOCATION	CENTER	CIR.	CONDUCTOR	NEUTRAL	8 AWG
1	PS B	CS B	NORTON		В			5
1	CS B	B1	NORTON	46.0	В			43
1	B1	A 1	MELVILLE	92.1	В	192	96	
1	Α1	B2	MELVILLE	147.1	В			151
1	B2	B101	MELVILLE	14.6	В	40	20	
1	B101	B102	MELVILLE	32.0	В	76	38	
1	B102	B103	MELVILLE	32.0	В	76	38	
1	B103	B104	MELVILLE	32.0	В	76	38	
1	B104	B105	MELVILLE	32.0	В	76	38	
1	B105	B106	MELVILLE	32.0	В	76	38	
1	B106	B107	MELVILLE	32.0	В	76	38	
				TOT	ΓALS:	688	344	199

ı																		
				F	RIGID CO	NDU I	Τ (MoD	OT)								
ı	LIGHTING				CENTER TO		TRE	ENCHE	ED (")	PUSI	HED	(")	MED	IAN	EXTER	RNAL ON S	STRUC.
ı	SHEET	FROM	TO	LOCATION	CENTER	CIR.	2 "	3 "	4 "	5 "	3 "	4 "	5 "	4 "	5"	3 "	4 "	5 "
ı	1	PS A	CS A	NORTON		А	5											
ı	1	CS A	A101	NORTON	188.2	А	184											
ı	1	A101	A1	MELVILLE	114.9	А	113											
ı	1	Α1	A201	MELVILLE	49.0	А	48											
					TOT	ΓAI S:	350											

				RIGID CC	I UDNC	Τ (CIT	Υ)									
LIGHTING	3			CENTER TO		TRE	ENCHE	D ('	")	PUSI	HED	(")	MED	NAI	EXTER	RNAL ON S	STRUC.
SHEET	FROM	TO	LOCATION	CENTER	CIR.	2"	3"	4 "	5 "	3 "	4 "	5"	4 "	5"	3 "	4 "	5 "
1	PS B	CS B	NORTON		В	5											
1	CS B	B1	NORTON	46.0	В	43											
1	B1	A 1	MELVILLE	92.1	В	89											
1	Δ1	B2	MELVILLE	147.1	В	145											
1	B2	B101	MELVILLE	14.6	В										15		
1	B101	B102	MELVILLE	32.0	В										32		
1	B102	B103	MELVILLE	32.0	В										32		
1	B103	B104	MELVILLE	32.0	В										32		
1	B104	B105	MELVILLE	32.0	В										32		
1	B105	B106	MELVILLE	32.0	В										32		
1	B106	B107	MELVILLE	32.0	В										32		
	·			TOT	ΓALS:	282									207		

			POW	ΕR	SUF	PL.	Υ (MoD)OT)					
LIGHTING	IGHTING TYPE 1 (POLE) TYPE 2 (PED) POWER SUPPLY														
SHEET															
NO.	STATION	OFFSET	LOCATION	240V	240 120V	480V	480 120V	240V	240 120V	480V	480 120V	METER	LITE	SIGS	CIK
1	880+69.09	65.14′ LT	NORTON	1								W	15		Α
		Т	OTALS:	1								EAC	Н:	1	

			POW	/ER	SU	PPL	Υ (CI	TY)						
LIGHTIN	IG			TYF	PE 1	(POI	_E)	ΤY	PE 2	(PE	D)	1	POWER	SUPPLY	
SHEET				LT ONLY	LT+ SIG	LT ONLY	LT+ SIG	LT ONLY	LT+ SIG	LT ONLY	LT+ SIG 480	DIRECTN TO FACE	BRKR	AMPS	CIR
NO.	STATION	OFFSET	LOCATION	240V	240 120V	480V	480 120V	240V	240 120V	480V	480 120V	METER	LITE	SIGS	CIR
1	880+77.36	66.99′ LT	NORTON	1								S	15		В
		T	OTALS:	1								EACI	H:	1	

		CC	NTROL	STAT	IONS	(Mo[OOT)				
LIGHTING						BREAK	KERS (AMPS)		BASE	MOUNT
SHEET				CONTROL			CIR	CUIT		4 CIR.	4 CIR.
NO.	STATION	OFFSET	LOCATION	LETTER	MAIN	1	2	3	4	240V	480V
1	880+71.39	58.40′ LT	NORTON	А		15				1	
						T	OTAL	S, E	ACH:	1	

		C	ONTROL	STAT	IONS	(CI	TY)				
LIGHTING						BREAK	KERS (AMPS)		BASE	MOUNT
SHEET				CONTROL			CIR	CUIT		4 CIR.	4 CIR.
NO.	STATION	OFFSET	LOCATION	LETTER	MAIN	1	2	3	4	240V	480V
1	880+79.49	60.21' LT	NORTON	В		15				1	
						T	OTAL	.S, E	ACH:	1	

			PULL	ВОХ	KES (MoDO	Γ)				
LIGHTING				Р.В.	CONCRETE DOUBLE	CONCRETE DOUBLE	CONCRETE	PREFORM	PREFORM	PREFORM	
SHEET	STATION	OFFSET	LOCATION	NO.	TYPE A	TYPE B	STANDARD	CL 1	CL 2	CL 3	TYPE
1	5+04.05	26.15′ LT	MELVILLE	Α1			1				I
			TOTA	LS:			1				

			PULI	_B0	XES	CITY)				
LIGHTING				Р.В.	CONCRETE DOUBLE	CONCRETE DOUBLE	CONCRETE	PREFORM	PREFORM	PREFORM	
SHEET	STATION	OFFSET	LOCATION	NO.	TYPE A	TYPE B	STANDARD	CL 1	CL 2	CL 3	TYPE
1	880+57.59	19.33′ LT	NORTON	В1			1				I
1	6+50.98	33.13′ LT	MELVILLE	В2			1				ΙI
			TOTA	LS:			2				

SUMMARY OF QUANTITIES 9 OF 10

THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY.

DATE PREPARED

12/20/2024

ROUTE

CRD 127

DISTRICT SHEET NO.

SW 3

COUNTY

GREENE

JOB NO.

J8S3156

CONTRACT ID.

PROJECT NO.

BRIDGE NO.

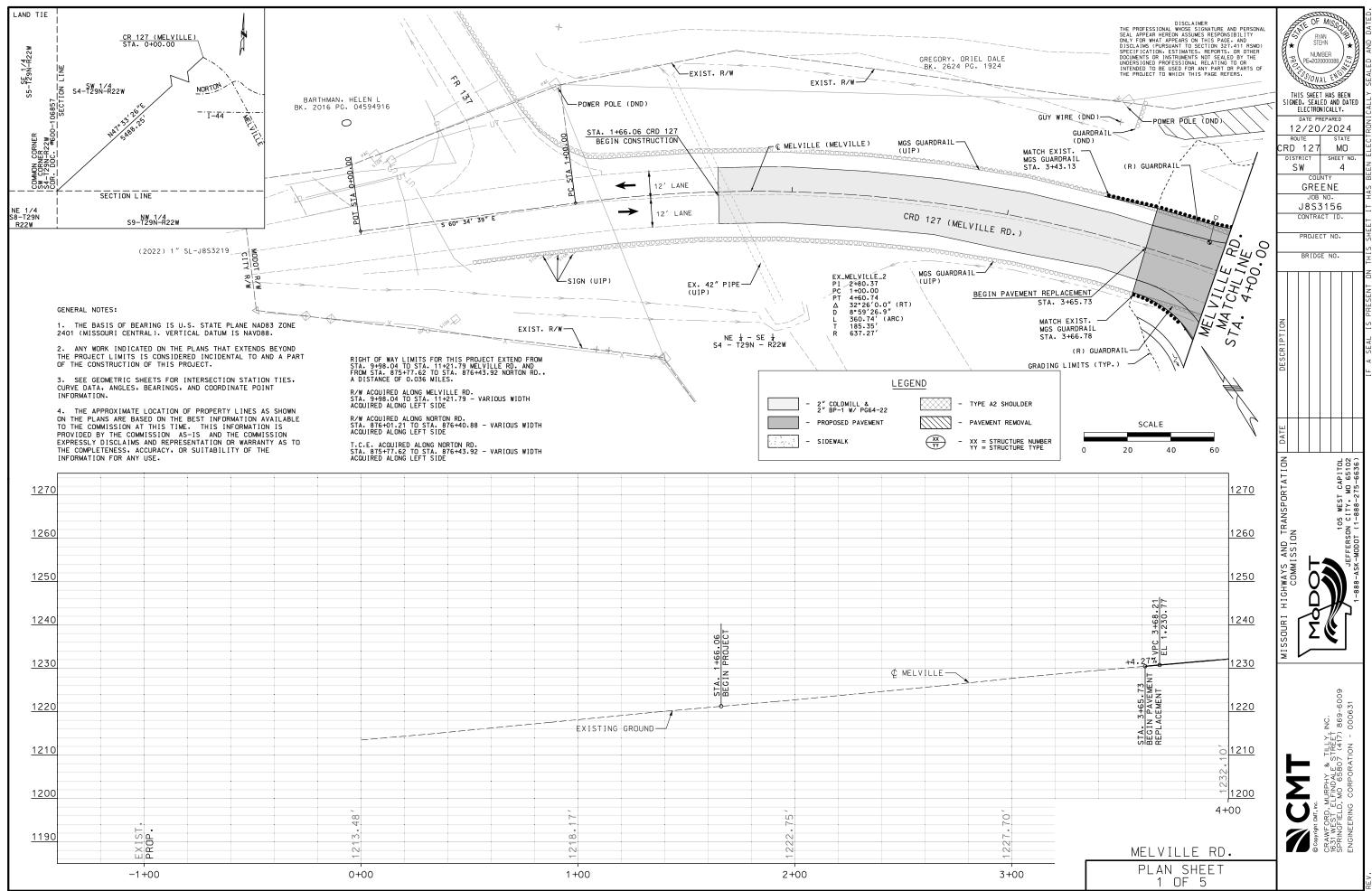
RI HIGHWAYS AND TRANSPORTATION
COMMISSION
105 WEST CAPITOL
HEFERSON CITY, MA 65172

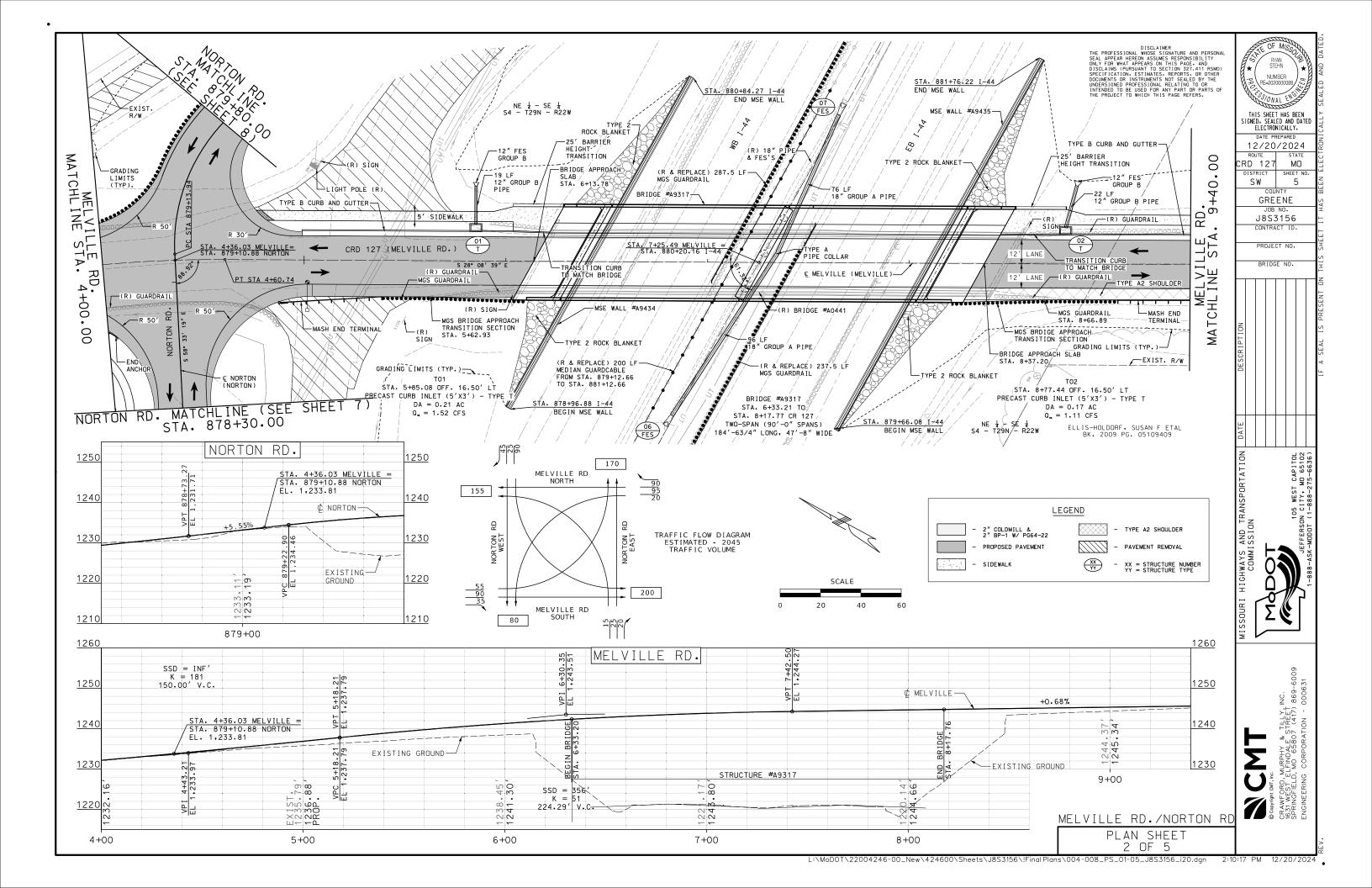
SCOPING OF THE STREET INC.
SPRINGS ELFINDALE STREET
SPRINGSFELD, MO 65807 (417) 869-6009

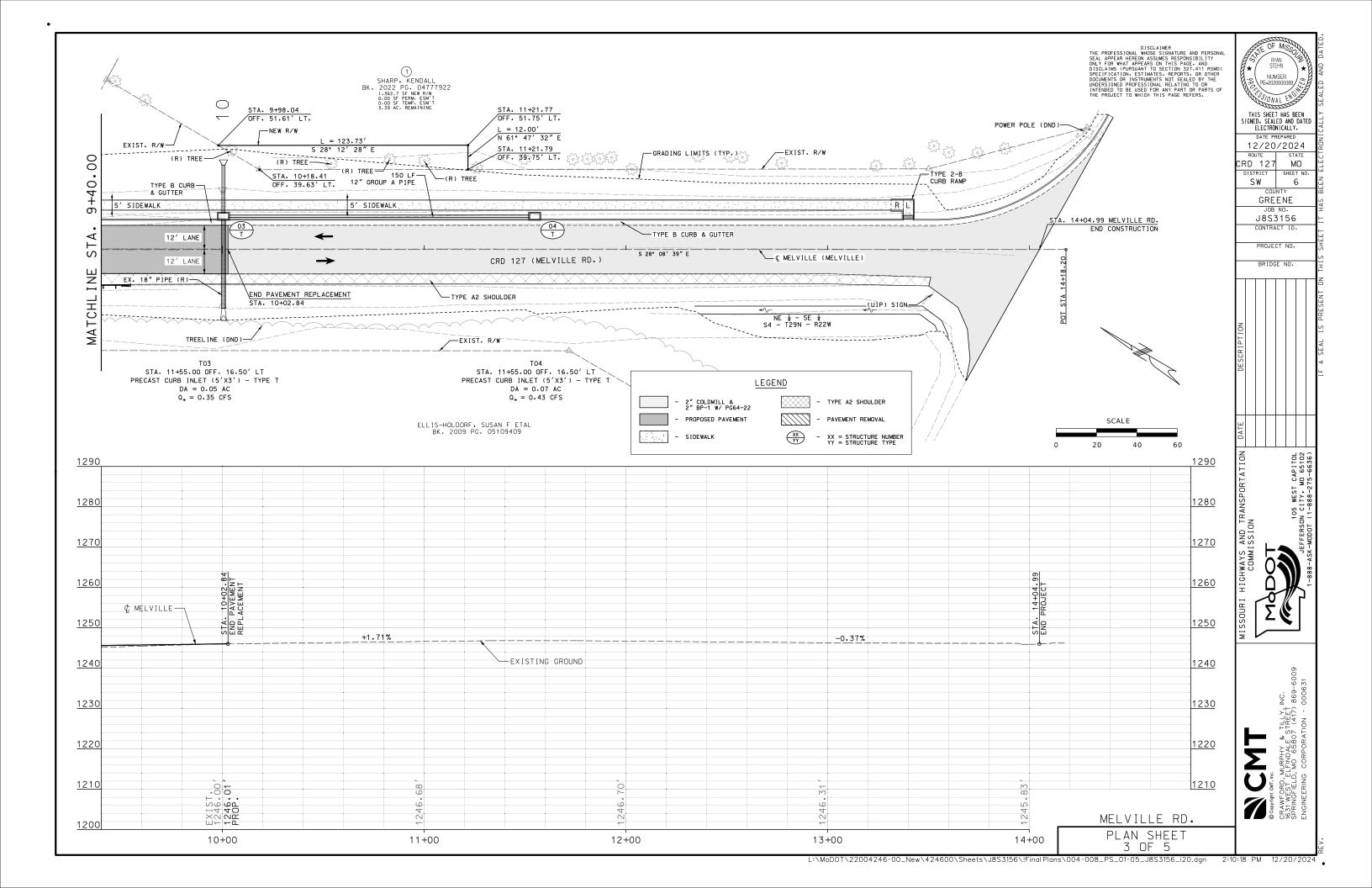
								,									
					QTY	TOTAL	SIGN	1						QTY TOTAL	SIGN		
	SIZE	AREA	QTY	TOTAL	RELOC	RELOC	NUM.	,		SIZE	AREA	QTY	TOTAL	RELOC RELOC	NUM.		
SIGN	IN.	SQ.FT.	EACH	SQ.FT.	EACH	SQ.FT.			SIGN	IN. S	SQ.FT.	EACH	SQ.FT.	EACH SQ.FT.		ITEM	TOTAL
		,	VARNIN	IG SIGNS		•	•	DESCRIPTION			WARI	NING S	IGNS, C	ONT.	DESCRIPTION	NUMBER	R QTY DESCRIPTION
WO1-1L	48X48	16.00		0.00		0.00		TURN (SYMBOL LEFT ARROW)	W022-3	42X36	10.50		0.00	0.00	END BLASTING ZONE	6122008	IMPACT ATTENUATOR 40 MPH (SAND BARRELS)
WO1-1R	48X48	16.00		0.00		0.00		TURN (SYMBOL RIGHT ARROW)	GD22-1	21X15	2.19		0.00	0.00	WET PAINT (ARROW PIVOTS)	6122009	
WO1-2L	48X48	16.00		0.00		0.00		CURVE (SYMBOL LEFT ARROW)						GUIDE SIGNS		6122010	
WO1-2R	48X48	16.00		0.00		0.00		CURVE (SYMBOL RIGHT ARROW)	E05-1	36X48			0.00	0.00	GORE EXIT	6122012	
WO1-3L WO1-3R	48X48 48X48	16.00		0.00		0.00		REVERSE TURN (SYMBOL LEFT ARROW) REVERSE TURN (SYMBOL RIGHT ARROW)	E05-2 E05-2a	48X36 48X36	12.00		0.00	0.00	EXIT OPEN EXIT CLOSED	6122014	
WO1-4L	48X48	16.00	2	32.00		0.00	15L	REVERSE CURVE (SYMBOL LEFT ARROW)	G020-1	60X24			0.00	0.00	ROAD WORK NEXT XX MILES	6122019	
WO1-4R	48X48	16.00	2	32.00		0.00	_	REVERSE CURVE (SYMBOL RIGHT ARROW)	G020-2		8.00		0.00	0.00	END ROAD WORK	6122020	
WO1-4bL	48X48	16.00		0.00		0.00		DOUBLE ARROW REVERSE CURVE (SYMBOL LT ARROWS)	G020-4	36X18	4.50		0.00	0.00	PILOT CAR FOLLOW ME	6122030	IMPACT ATTENUATOR (RELOCATION)
W01-4bR	48X48	16.00		0.00		0.00		DOUBLE ARROW REVERSE CURVE (SYMBOL RT ARROWS)	G020-4a	42X30	8.75		0.00	0.00	PILOT CAR IN USE WAIT & FOLLOW	6123000A	TRUCK OR TRAILER MOUNTED ATTENUATOR (TMA)
W01-4cL	48X48	16.00		0.00		0.00		TRIPLE ARROW REVERSE CURVE (SYMBOL LT ARROWS)	G020-4a		1.50		0.00	0.00	PILOT CAR IN USE WAIT & FOLLOW	6161008	
W01-4cR	48X48	16.00		0.00		0.00	1.0	TRIPLE ARROW REVERSE CURVE (SYMBOL RT ARROWS)	G020-5aP		6.00		0.00	0.00	WORK ZONE (PLAQUE)	6161012	
W01-6 W01-6a	60X30 72X36	12.50	6	75.00		0.00	16	HORIZONTAL ARROW (SYMBOL) HORIZ. ARROW (SYMBOL ON PERMANENT BARRICADE)	M04-8a M04-9L		3.00 12.00	11	12.00	0.00	52 END DETOUR 50D-E DETOUR (LEFT ARROW)	6161013	
W01 -7	60X30	12.50		0.00		0.00		DOUBLE HEAD HORIZONTAL ARROW (SYMBOL)	MO4-9R		12.00	8	96.00	0.00	50B-C DETOUR (RIGHT ARROW)	6161025	
W01-7a	72X36	3.00		0.00		0.00		DOUBLE HEAD HORIZ. ARROW (SYMBOL ON PERM. BARR.)	M04-9P		4.00	25	100.00	0.00	50 STREET NAME (PLAQUE)	6161030	
WO1-8	18X24	7.50		0.00		0.00		CHEVRON (SYMBOL)	MO4-10L	48X18	6.00	1	6.00	0.00	17L DETOUR (ARROW LEFT)	6161033	
WO1-8a	30X36	16.00		0.00		0.00		CHEVRON (SYMBOL FOR DIVIDED HIGHWAYS)	MO4-10R	48X18	6.00	1	6.00	0.00	17R DETOUR (ARROW RIGHT)	6161040	FLASING ARROW PANEL
W03-1	48X48	16.00		0.00		0.00		STOP AHEAD (SYMBOL)						GULATORY SIGN		6161047	
W03-2	48X48	16.00		0.00		0.00		YIELD AHEAD (SYMBOL)	R1-1	48X48			0.00	0.00	STOP	6161051	
W03-3	48X48 48X48	16.00	2	0.00		0.00	1.1	SIGNAL AHEAD (SYMBOL)	R1-2		6.93		0.00	0.00	TO ONCOMING TRAFFIC (PLAQUE)	6161052 6161053	
W03-4	48X48	16.00		32.00		0.00	111	BE PREPARED TO STOP SPEED LIMIT AHEAD	R1-2a R1-3P		9.00 2.50		0.00	0.00	ALL WAY (PLAQUE)	6161055	
W04-1L	48X48	16.00		0.00		0.00		MERGE (SYMBOL FROM LEFT)	R2-1		12.00		0.00	0.00	SPEED LIMIT XX	6161070	
WO4-1R	48X48	16.00		0.00		0.00		MERGE (SYMBOL FROM RIGHT)	R3-1		16.00		0.00	0.00	NO RIGHT TURN (SYMBOL)	6161095	
W04-1aL	48X48	16.00		0.00		0.00		MERGE (ARROW SYMBOL)	R3-2	48X48	16.00		0.00	0.00	NO LEFT TURN (SYMBOL)		CHANGEABLE MESSAGE SIGN.
W04-1aR	48X48	16.00		0.00		0.00		MERGE (ARROW SYMBOL)	R3-3	36X36	9.00		0.00	0.00	NO TURNS	6161096	COMMISSION FURNISHED/RETAINED
W05-1	48X48	16.00	1	16.00		0.00	43	ROAD/BRIDGE/RAMP NARROWS	R3-4		16.00		0.00	0.00	NO U-TURN (SYMBOL)		CHANGEABLE MESSAGE SIGN W/O COMM.
W05-3	48X48	16.00		0.00		0.00		ONE LANE BRIDGE	R3-7L		6.25		0.00	0.00	LEFT LANE MUST TURN LEFT	6161098A	
W05-5 W06-1	48X48 48X48	16.00		0.00		0.00		NNARROW LANES DIVIDED HIGHWAY (SYMBOL)	R3-7R R4-1		12.00		0.00	0.00	RIGHT LANE MUST TURN RIGHT DO NOT PASS	6161099	CHANGEABLE MESSAGE SIGN WITH COMM. 10 INTERFACE, CONTRACTOR FURNISHED/RETAINED
W06-2	48X48	16.00		0.00		0.00		DIVIDED HIGHWAY END (SYMBOL)	R4-2		12.00		0.00	0.00	PASS WITH CARE	6162000A	
W06-3	48X48	16.00		0.00		0.00	1	TWO WAY TRAFFIC (SYMBOL)	R4-8a		12.00		0.00	0.00	KEEP LEFT (HORIZONTAL ARROW)	6162002	
W07-3a	30X24	5.00		0.00		0.00		NEXT XX MILES (PLAQUE)	R4-7a		12.00		0.00	0.00	KEEP RIGHT (HORIZONTAL ARROW)	6162004	
WO8-1	48X48	16.00		0.00		0.00		BUMP	R5-1	30X30	6.25		0.00	0.00	DO NOT ENTER		TEMPORARY TRAFFIC BARRIER
WO8-2	48X48	16.00		0.00		0.00		DIP	R5-1a		6.00		0.00	0.00	WRONG WAY	6173600D	
WO8-3	48X48	16.00		0.00		0.00		PAVEMENT ENDS	R6-1L		6.75		0.00	0.00	ONE WAY ARROW (LEFT)		TEMPORARY TRAFFIC BARRIER ANCHORED
W08-4	48X48 48X48	16.00		0.00		0.00	-	SOFT SHOULDER	R6-1R		6.75		0.00	0.00	ONE WAY (LEET)	6173602B	
WD8-5 WD8-6	48X48	16.00		0.00		0.00	+	SLIPPERY WHEN WET (SYMBOL) TRUCK CROSSING (WITH FLAGS)	R6-2L R6-2R		5.00		0.00	0.00	ONE WAY (LEFT) ONE WAY (RIGHT)	6174000A 6175010A	
W08-6c	48X48	16.00		0.00		0.00		TRUCK ENTRANCE	R9-9	_	2.00		0.00	0.00	SIDEWALK CLOSED	01130104	TEMPORARY TRAFFIC BARRIER
WD8-7	36X36	9.00		0.00		0.00		LOOSE GRAVEL							SIDEWALK CLOSED AHEAD.	6176000B	
WO8-7a	36X36	9.00		0.00		0.00		FRESH OIL/LOOSE GRAVEL	R9-11L	24X18	3.00		0.00	0.00	(ARROW LEFT) CROSS HERE		TEMP. TRAFFIC BARRIER HEIGHT TRANSITION
WO8-9	48X48	16.00		0.00		0.00		LOW SHOULDER							SIDEWALK CLOSED AHEAD.	6177000B	COMMISSION FURNISHED/RETAINED
WO8-11	48X48	16.00		0.00		0.00		UNEVEN LANES	R9-11-R		3.00		0.00	0.00	(ARROW RIGHT) CROSS HERE	6208064A	
WO8-12	48X48	16.00		0.00		0.00		NO CENTER LINE	R10-6		6.00		0.00	0.00	STOP HERE ON RED (45° ARROW)	9029400	
WO8-15 WO8-15P	48X48 30X24	16.00		0.00		0.00		GROOVED PAVEMENT MOTORCYCLE (PLAQUE)	R11-2	48X30	10.00	4	40.00	0.00	29 ROAD CLOSED ROAD CLOSED XX MILES AHEAD	9029401	TEMPORARY TRAFFIC SIGNALS AND LIGHTING
W08-13F	48X48	16.00		0.00		0.00		SHOULDER DROP-OFF (SYMBOL)	R11-3a	60X30	12.50		0.00	0.00	LOCAL TRAFFIC ONLY		
W08-17P	30X24	5.00		0.00		0.00		SHOULDER DROP-OFF (PLAQUE)	R11-4	60X30		1	12.50	0.00	55 ROAD CLOSED TO THRU TRAFFIC		
WO10-1	42RND.	9.62		0.00		0.00		RAILROAD CROSSING	CONST-3A	40X48	20.00		0.00	0.00	FINE SIGN		
WO12-1	24X24	4.00		0.00		0.00		DOUBLE DOWN ARROW (SYMBOL)	COSNT-3X	56X12	4.67		0.00	0.00	SPEEDING/PASSING (PLATE)		
WO12-2	48X48	16.00		0.00		0.00		LOW CLEARANCE (SYMBOL)						ELLANEOUS SIG			
W012-2X	24X18	3.00		0.00		0.00		LOW CLEARANCE (PLAQUE)	CONST-5	48X36			0.00	0.00	POINT OF PRESENCE		
W012-2a	84X24	14.00		0.00		0.00		OVERHEAD LOW CLEARANCE (FEET AND INCHES)	CONST-5	96X48			0.00	0.00	POINT OF PRESENCE		
W012-4		50.00		0.00		0.00		LOW CLEARANCE XX FT XX IN XX MILES AHEAD WIDTH RESTRICTION XX FEET XX IN XX MILES AHEAD	CONST-7	72X36		2	0.00 36.00	0.00	RATE OUR WORK ZONE 56 RATE OUR WORK ZONE	THE PROFI	DISCLAIMER ESSIONAL WHOSE SIGNATURE AND PERSONAL
WO12-5 WO13-1	30X30		2	12.50		0.00	44	ADVISORY SPEED (PLAQUE)	CONST-8	48X36		4	48.00	0.00	59 WORK ZONE NO PHONE ZONE	ONLY FOR	EAR HEREON ASSUMES RESPONSIBILITY WHAT APPEARS ON THIS PAGE, AND
WO16-2	30X24			0.00		0.00		XXX FEET (PLAQUE)	MO4-9R		12.00	6	72.00	0.00	50A DETOUR (STRAIGHT ARROW)	SPECIFIC	IS (PURSUANT TO SECTION 327.411 RSMO) SATION, ESTIMATES, REPORTS, OR OTHER S OR INSTRUMENTS NOT SEALED BY THE
WD16-3	30X24			0.00		0.00		X MILE (PLAQUE)	SPECIAL	36X72		23	414.00	0.00	50F-R I-44 DETOUR	UNDERSIG	INED PROFESSIONAL RELATING TO OR OTO BE USED FOR ANY PART OR PARTS OF
W020-1	48X48	16.00	2	32.00		0.00	2	ROAD/BRIDGE/RAMP WORK AHEAD								THE PROJ	ECT TO WHICH THIS PAGE REFERS.
W020-2	48X48	16.00	9	144.00		0.00	18	DETOUR AHEAD									
W020-3	48X48	16.00	6	96.00		0.00	20	ROAD CLOSED AHEAD		1							
WD20-4	48X48	16.00		0.00		0.00		ONE LANE ROAD AHEAD									
W020-5	48X48 48X48	16.00		0.00		0.00	-	RIGHT/CENTER/LEFT LANE CLOSED AHEAD 2 RIGHT/CENTER/LEFT LANE CLOSED AHEAD	-	+							
W020-5a W020-6a	48X48 48X48	16.00		0.00		0.00	+		_ 616-10.(15			TOTAL				
WD20-7a	48X48	16.00	2	32.00		0.00	8		CONSTRUC		IGNS		1510.00				
WD21-2	36X36	9.00		0.00		0.00	1		616-10.					TOTAL	7		
WO21-5	48X48	16.00	2	32.00		0.00	21		RELOCATE		IS *			0.00			
WO21-5a	48X48	16.00		0.00		0.00		RIGHT SHOULDER CLOSED	* NO DIRE	ECT PAYME	NT WILL	L BE MADE	FOR REL	OCATION OF SIGNS			
WO21-5b	48X48	16.00		0.00		0.00		RIGHT SHOULDER CLOSED AHEAD									
W022-1	48X48	16.00		0.00		0.00	_	BLASTING ZONE AHEAD									SUMMARY OF QUANTI
W022-2	48X36	10.50		0.00		0.00		TURN OFF 2-WAY RADIO AND PHONE									10 OF 10

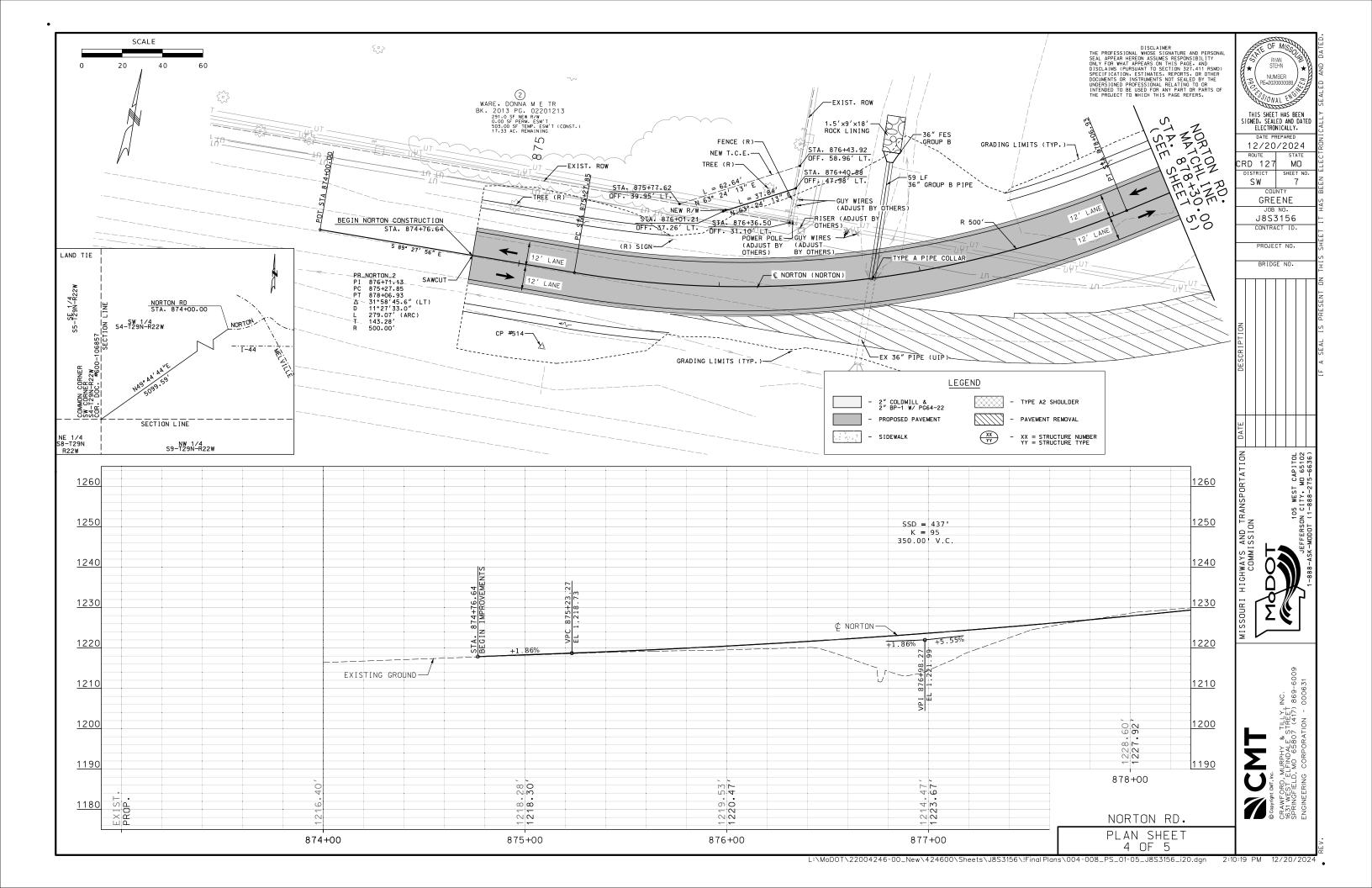
RYAN STEHN NUMBER PE-202000008 THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY. 12/20/2024 ROUTE STATE CRD 127 MO DISTRICT SHEET NO. SW 3 GREENE JOB NO. J8S3156 CONTRACT ID. PROJECT NO. BRIDGE NO. WAYS AND TRANSP COMMISSION

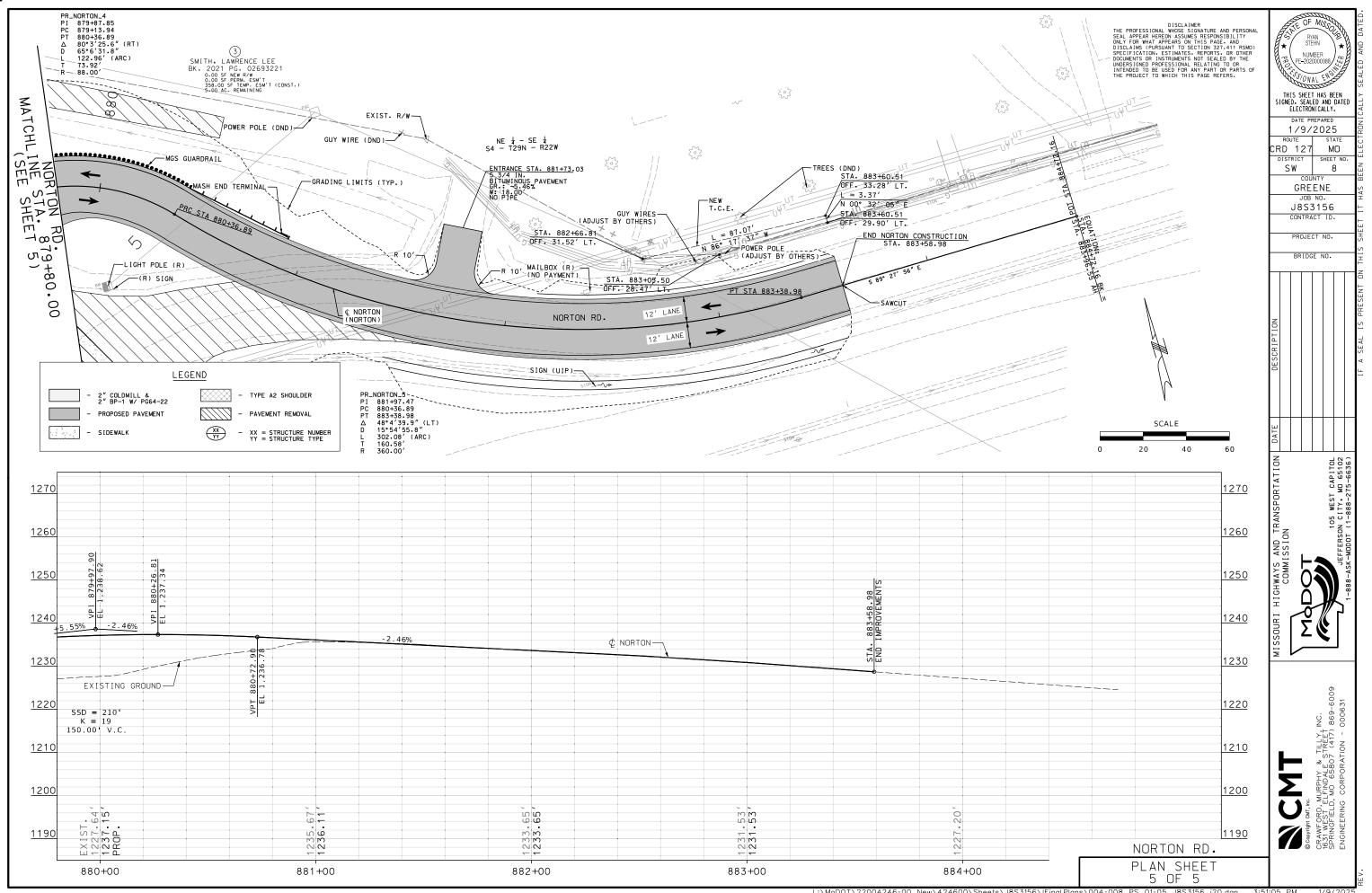
SUMMARY OF QUANTITIES 10 OF 10



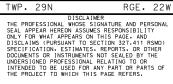


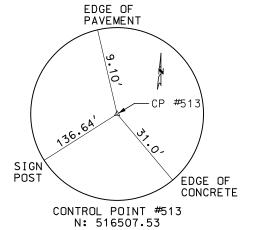






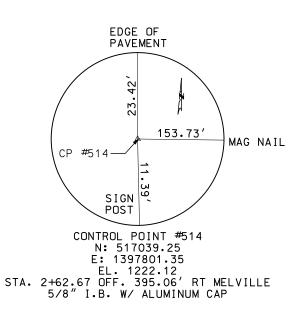
SEC./SUR. 4 TWP. 29N

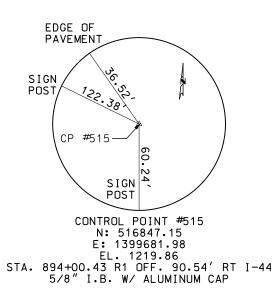


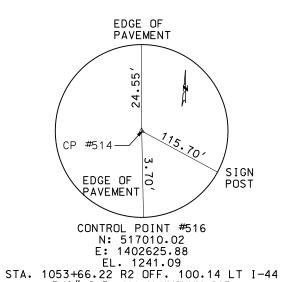


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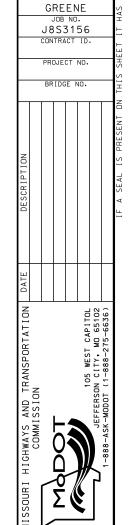
EL. 1253.87 STA. 842+05.38 R1 OFF. 93.86' LT I-44 5/8" I.B. W/ ALUMINUM CAP







5/8" I.B. W/ ALUMINUM CAP



NUMBER PE-202000008

THIS SHEET HAS BEEN SIGNED, SEALED AND DATE ELECTRONICALLY.

12/20/2024 CRD 127 MO DISTRICT

SW

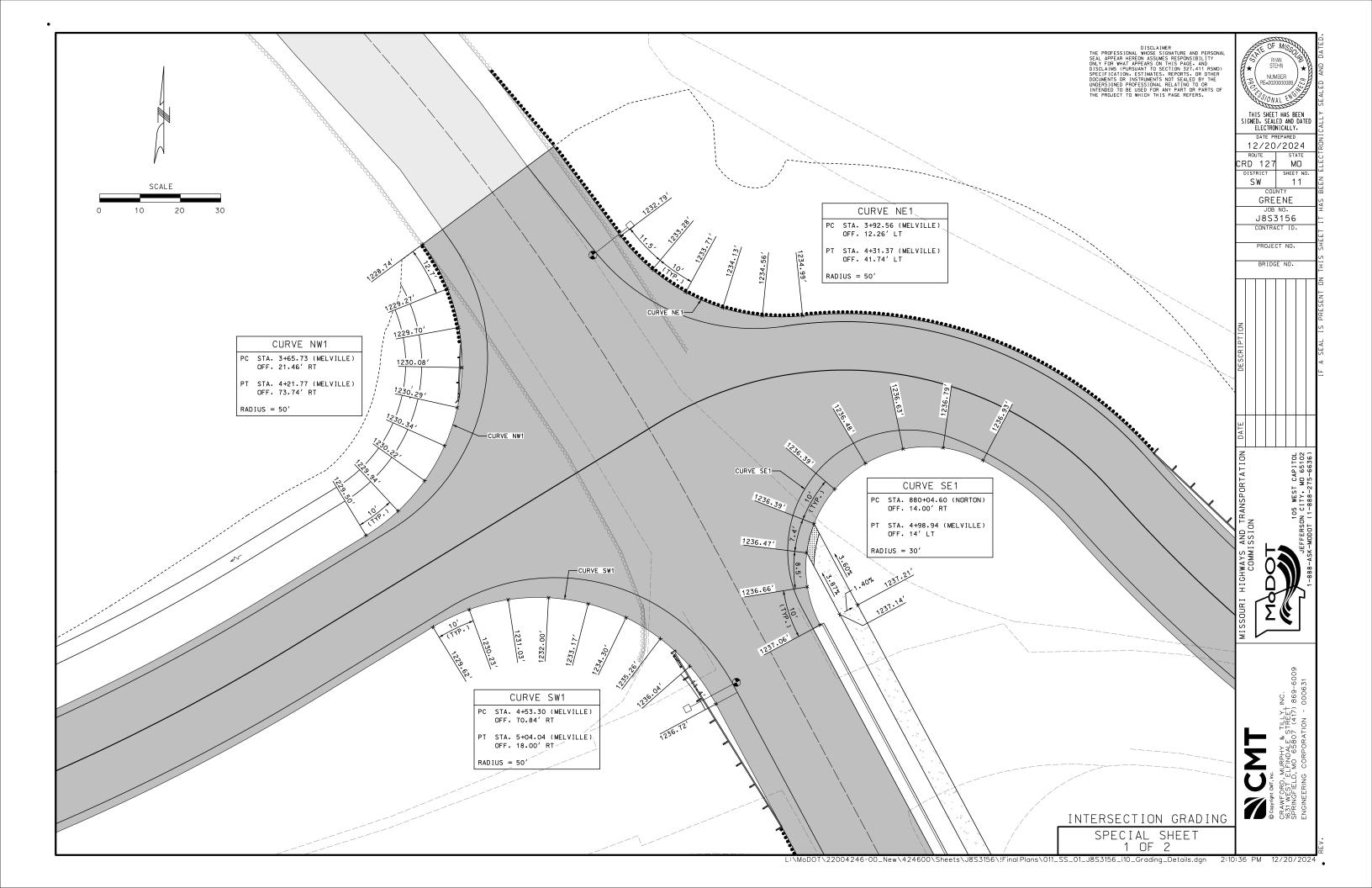
SHEET NO.

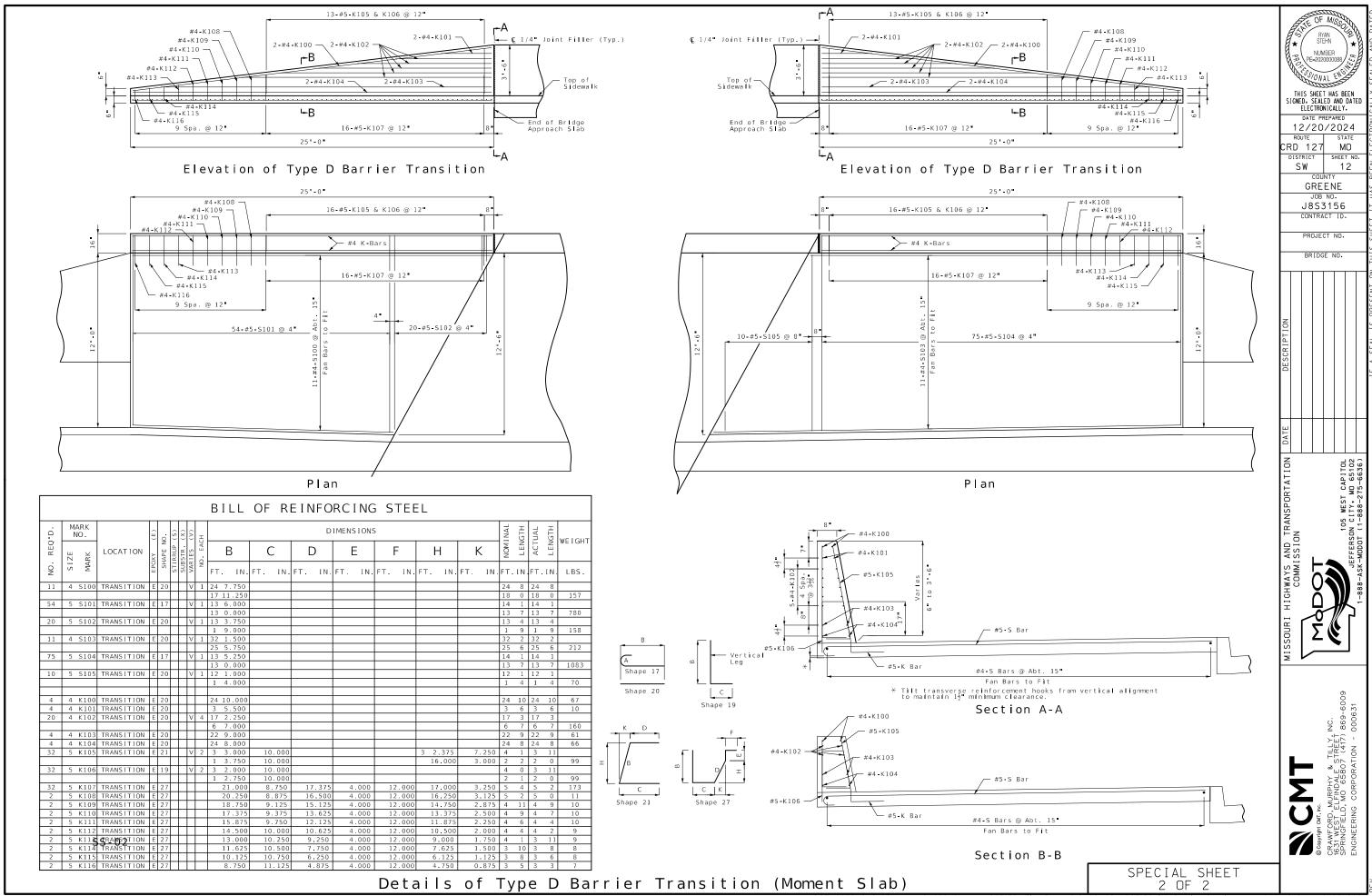
9

MELVILLE RD.

REFERENCE POINTS 1 OF

COORDINATE POINTS 1 OF L:\MoDOT\22004246-00_New\424600\Sheets\J8S3156\!Final Plans\010_CP_J8S3156_i100.dgn 2:10:30 PM 12/20/2024





GENERAL NOTES:

- 1. ANY EXISTING SIGNS THAT CONFLICT WITH TRAFFIC CONTROL PLAN SHALL BE COVERED.
- 2. ALL DISTANCES ARE APPROXIMATE & MAY BE REVISED BY THE ENGINEER TO FIT FIELD CONDITIONS.
- 3. SIGNING SHOWN SHALL REMAIN IN PLACE UNTIL CONSTRUCTION IS COMPLETE.
- 4. ALTERNATE TRAFFIC CONTROL MAY BE USED AS NEEDED AT THE APPROVAL OF THE ENGINEER.
- 5. SEE STANDARD DRAWING 616.10, 619.10, AND 620.10 FOR ADDITIONAL DETAILS AND 903.03 FOR SIGN AND SIGN MOUNTING REQUIREMENTS.
- 6. ANY WORK INDICATED ON THE PLANS THAT EXTENDS BEYOND THE PROJECT LIMITS IS CONSIDERED INCIDENTAL TO AND A PART OF THE CONSTRUCTION OF THIS PROJECT.
- 7. SPEED LIMIT SIGNS INDICATING THE EXISTING POSTED SPEED LIMIT SHALL BE INSTALLED AT THE END OF THE WORK ZONE, PROVIDED NO FURTHER WORK ZONES WILL BE ENCOUNTERED WITHIN THE NEXT ½ MILE.
- 8. TEMPORARY SPEED LIMIT SIGNS SHALL BE COVERED OR REMOVED WHEN THE CONDITIONS REQUIRING REDUCED SPEEDS DO NOT EXIST.
- 9. NO DIRECT PAYMENT WILL BE MADE FOR THE RELOCATION OF CHANNELIZERS.

PHASE 1A NOTES

- 1. PHASE 1A SHALL CONSIST OF REMOVAL OF EXISTING STRUCTURE OVER I-44.
- 2. CONSTRUCT NEW BRIDGE GIRDERS AND SLABS.
- 3. CR 127 (MELVILLE RD.) WILL BE CLOSED BETWEEN NORTON RD. AND PFEIFFER RD. (FR 112). I-44 WILL BE CLOSED BETWEEN ROUTE 13 (KANSAS EXPWY.) AND US 160 (WEST BYPASS). SEE TRAFFIC CONTROL SHEETS 16-17 FOR DETOUR INFORMATION.
- 4. TEMPORARY TRAFFIC CONTROL SIGNAGE ON I-44 FOR THIS PROJECT SHALL BE PLACED IN CONJUNTION WITH SURROUNDING PROJECTS. SEE JOB #'S J8I3O44C, JSU0076, AND JST0088B FOR FURTHER TRAFFIC CONTROL AND DETOUR INFORMATION FOR I-44.
- 5. BRIDGE REMOVAL SHALL LAST NO LONGER THAN 24 HOURS AND CLOSURE OF I-44 SHALL BE LIMITED TO WEEKEND ONLY.
- 6. BUSINESS AND RESIDENTIAL ACCESS SHALL BE MAINTAINED AT ALL TIMES.
- 7. CONFLICTING SIGNS SHALL BE COVERED.

PHASE 1B NOTES

- 1. PHASE 1B SHALL CONSIST OF REMOVAL OF EXISTING PAVEMENT ON CR 127 (MELVILLE RD.) SOUTH OF THE BRIDGE.
- 2. CONSTRUCT NEW PAVEMENT, NEW SIDEWALK, NEW CURB & GUTTER, NEW BRIDGE APPROACH SLAB SOUTH OF I-44.
- 3. CR 127 (MELVILLE RD.) WILL BE CLOSED BETWEEN NORTON RD. AND PFEIFFER RD. (FR 112). SEE TRAFFIC CONTROL SHEET 16 FOR DETOUR
- 4. BUSINESS AND RESIDENTIAL ACCESS SHALL BE MAINTAINED AT ALL TIMES.
- 5. CONFLICTING SIGNS SHALL BE COVERED.

PHASE 2A NOTES

- 1. PHASE 2A SHALL CONSIST OF THE CONSTRUCTION OF NEW PAVEMENT, NEW SHOULDER, AND GRADING ON NORTON RD. WEST OF MELVILLE RD. OUTSIDE OF THE TRAVELED WAY; AS WELL AS GRADING AND THE CONSTRUCTION OF TEMPORARY PAVEMENT ON THE NORTHEAST QUADRANT OF THE INTERSECTION CONNECTING CR 127 (MELVILLE RD.) TO NORTON RD.
- 2. CR 127 (MELVILLE RD.) WILL BE CLOSED BETWEEN NORTON RD. AND PFEIFFER RD. (FR 112). SEE TRAFFIC CONTROL SHEET 16 FOR DETOUR PLAN.
- 3. BUSINESS AND RESIDENTIAL ACCESS SHALL BE MAINTAINED AT ALL TIMES.
- 4. CONFLICTING SIGNS SHALL BE COVERED.

PHASE 2B NOTES

- 1. PHASE 2B SHALL CONSIST OF REMOVAL OF EXISTING INTERSECTION MELVILLE RD. AND NORTON RD.
- 2. CONSTRUCT NEW PAVEMENT, NEW SHOULDER, GRADING ON NORTON RD. AND CR 127 (MELVILLE RD.) AND TIE-IN WEST NORTON RD.
- 3. NORTON RD. WEST OF CR 127 (MELVILLE RD.) WILL BE CLOSED TO THRU TRAFFIC. CR 127 (MELVILLE RD.) WILL BE CLOSED BETWEEN NORTON RD. AND PFEIFFER RD. (FR 112). SEE TRAFFIC CONTROL SHEETS 16 AND 18 FOR DETOUR AND CLOSURE INFORMATION.
- 4. BUSINESS AND RESIDENTIAL ACCESS SHALL BE MAINTAINED AT ALL TIMES.
- 5. CONFLICTING SIGNS SHALL BE COVERED.

PHASE 2C NOTES

- 1. PHASE 2C SHALL CONSIST OF REMOVAL OF EXISTING NORTON RD. EAST OF MELVILLE RD.
- 2. CONSTRUCT NEW PAVEMENT, NEW SHOULDER, AND GRADING ON NORTON RD. AND CONSTRUCT TIE-IN TO NEW MELVILLE RD.
- 3. NORTON RD. WILL BE CLOSED EAST OF MELVILLE RD. SEE TRAFFIC CONTROL SHEET 19 FOR CLOSURE INFORMATION.
- 4. BUSINESS AND RESIDENTIAL ACCESS SHALL BE MAINTAINED AT ALL TIMES.
- 5. CONFLICTING SIGNS SHALL BE COVERED.

PHASE 3 NOTES

- 1. PHASE 3 SHALL CONSIST OF COLDMILLING AND RESURFACING OF MELVILLE RD. BETWEEN CR 137 AND NORTON RD.
- 2. THRU ACCESS SHALL BE MAINTAINED.
- 3. ALL LANES OPEN TO TRAFFIC. BUSINESS AND RESIDENTIAL ACCESS SHALL BE MAINTAINED AT ALL TIMES.
- 4. CONFLICTING SIGNS SHALL BE COVERED.

PHASE 4 NOTES

- 1. PHASE 4 SHALL CONSIST OF PAVEMENT MARKING AND INSTALLATION OF PERMANENT SIGNAGE.
- 2. THRU ACCESS SHALL BE MAINTAINED.
- 3. ALL LANES OPEN TO TRAFFIC. BUSINESS AND RESIDENTIAL ACCESS SHALL BE MAINTAINED AT ALL TIMES.
- 4. CONFLICTING SIGNS SHALL BE COVERED.

NUMBER THIS SHEET HAS BEEN

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ELECTRO	ELECTRONICALLY.								
DATE PREPARED									
12/20	12/20/2024								
ROUTE	STATE								
CRD 127	МО								
DISTRICT	SHEET NO.								
C 14/	1 7 7								

GREENE J8S3156 CONTRACT ID.

PROJECT NO. BRIDGE NO.

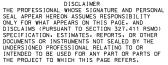
MELVILLE RD.

GENERAL TRAFFIC CONTROL

1 OF 19

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NOTES:

SIGN GO20-1 IS REQUIRED PER EPG 616.6.56.

SIGN GO20-2 IS USED ON ALL PROJECTS WHERE SIGN GO20-1 IS USED.

OTHER SIGNS SUCH AS DETOUR OR ALTERNATE ROUTE SIGNING MAY BE USED OUTSIDE THE PROJECT LIMITS.

ANY EXISTING SIGNING THAT CONFLICTS WITH THE TRAFFIC CONTROL SIGNING SHALL BE COMPLETELY COVERED OR REMOVED.

- (1) DISTANCE MAY BE ADJUSTED ACCORDING TO FIELD CONDITIONS. WHERE TRAFFIC BACKUPS ARE EXPECTED BEYOND THE ADVANCE WARNING AREA, ADDITIONAL SIGNING MAY BE NEEDED.
- (2) SIGN CONST-7-72 OR CONST-7-48 IS PLACED 500 FEET BEFORE THE BEGINNING OF PROJECT LIMITS OR THE ROAD WORK AHEAD SIGN OR ROAD WORK NEXT XX MILES SIGN. IF USED, WHEN THESE SIGNS ARE LOCATED OUTSIDE THE PROJECT LIMITS.
- (3) CONST-5-96 OR CONST-5-48 SIGN IS PLACED IN A VISIBLE AREA WITHIN THE PROJECT LIMITS PROVIDED ITS PLACEMENT DOES NOT DISRUPT A SEQUENCE OF SIGNS. IF A VISIBLE LOCATION WITHIN THE PROJECT IS NOT AVAILABLE, THE SIGN MAY BE PLACED 500 FEET BEFORE SIGN CONST-7-72 OR CONST-7-48.
- (4) THE "RATE DUR WORK ZONE" SIGN IS AVAILABLE IN TWO SIZES DEPENDING ON ROADWAY CONDITIONS. THE 48 IN. X 24 IN. SIGN IS USED ON LOW VOLUME ROUTES THAT CARRY 400 AADT OR LESS, ON PROJECTS THAT HAVE A DURATION OF 4 WEEKS OR LESS OR IN URBAN AREAS WHERE THERE IS INSUFFICIENT RIGHT OF WAY FOR A SIX FOOT WIDE SIGN. THE 72 IN. X 36 IN. VERSION OF THE SIGN IS USED IN ALL OTHER APPLICATIONS, BUT CAN ALSO BE USED IN PLACE OF THE 48 IN. imes 24 IN. SIGN IF ADDED EMPHASIS IS DEEMED NECESSARY.
- (5) THE "POINT OF PRESENCE" SIGN IS AVAILABLE IN TWO SIZES DEPENDING ON ROADWAY CONDITIONS. THE 48 IN. X 36 IN. SIGN IS USED ON LOW VOLLIME ROUTES THAT CARRY 400 AADT OR LESS. ON PROJECTS THAT HAVE A DURATION OF 4 WEEKS OR LESS OR IN URBAN AREAS WHERE THERE IS INSUFFICIENT RIGHTS OF WAY FOR AN EIGHT FOOT WIDE SIGN. THE 96 IN. X 48 IN. VERSION OF THE SIGN IS USED IN ALL OTHER APPLICATIONS, BUT CAN ALSO BE USED IN PLACE OF THE 48 IN. X 36 IN. SIGN IF ADDED EMPHASIS IS DEEMED NECESSARY.
- (6) THE "WORK ZONE NO PHONE ZONE" SIGN IS PLACED A MINIMUM OF 500 FEET BEFORE THE ROAD WORK AHEAD SIGN.





(59)



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12/20/2024 RD 127 MO DISTRICT SHEET NO SW 14

GREENE J8S3156 CONTRACT ID.

PROJECT NO.

BRIDGE NO.

BEGINNING AND END OF PROJECT SIGNING TO REMAIN IN PLACE THROUGH DURATION OF ALL PROJECTS

(59)

(56)

(1)

(59)

• (1)

1

MULTI-LANE DIVIDED

BEGINNING OF PROJECT LIMITS; OR INITIAL

WORK ZONE SIGN, IF LOCATED OUTSIDE

(59)

TWO-LANE UNDIVIDED

TYPICAL DETAILS

TRAFFIC CONTROL 2 OF 19

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DATE PREPARED

12/20/2024

ROUTE STATE

CRD 127 MO

DISTRICT SHEET NO.

SW 15

COUNTY

GREENE
JOB NO.
J8S3156
CONTRACT ID.

PROJECT NO.

BRIDGE NO.											
DESCRIPTION											
ATE											

COMMISSION
COMMISSION

OT

105 WEST CAPITOL
JEFFERSON CITY, MO 65102

MISSOURI HIGHWAYS AND TR
COMMISSION

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SPRINGFIELD, MO 65807 (417) 869-60

\$ 21) •



NOTES:

- 1. ONLY APPLICABLE WHEN WORK ZONE IS WITHIN THE CLEAR ZONE
- 2. ON MULTI-LANE, DIVIDED HIGHWAYS, SIGNS ADVISING
 OF SHOULDER WORK OR THE CONDITION OF THE SHOULDER
 SHOULD BE PLACED ONLY ON THE SIDE OF THE AFFECTED
 SHOULDER.
- 3. VEHICLE HAZARD WARNING SIGNALS SHALL NOT BE USED INSTEAD OF THE VEHICLE'S HIGH-INTENSITY ROTATING, FLASHING, OSCILLATING OR STROBE LIGHTS.

SPEED	SIGN SPAC	ING (ft.)	TAPER LEN	GTH (ft.)	OPTIONAL	CHANNELIZER SPACING (ft.)		
PERMANENT POSTED (MPH)	UNDIVIDED (S)	DIVIDED (S)	SHOULDER (1) (T1)	LANE (2) (T2)	BUFFER LENGTH (ft.) (B)	TAPERS	BUFFER/ WORK AREAS	
0-35	200	200	-	-	-	-	-	
40-45	350	500	-	-	-	-	-	
50-55	500	1000	-	-	-	-	-	
60-70	1000	SA - 1000. SB - 1500. SC - 2640	-	-	_	-	-	



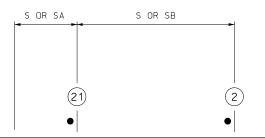
WORK BEYOND THE SHOULDER
ON DIVIDED AND UNDIVIDED HIGHWAYS
DAY TIME OR NIGHT TIME ALLOWED

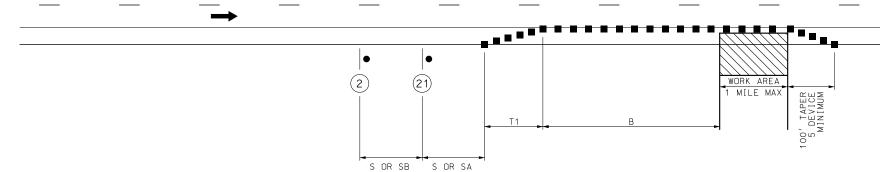
TYPICAL DETAILS
TRAFFIC CONTROL
3 OF 19

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SPEED	SIGN SPAC	ING (ft.)	TAPER LEN	GTH (ft.)	OPTIONAL	SPACING (ft.)				
PERMANENT POSTED (MPH)	UNDIVIDED (S)	DIVIDED (S)	SHOULDER (1) (T1)	LANE (2) (T2)	BUFFER LENGTH (ft.) (B)	TAPERS	BUFFER/ WORK AREAS			
0-35	200	200	70	245	280	35	40			
40-45	350	500	150	540	400	40	80			
50-55	500	1000	185	660	560	50	80			
60-70 1000		SA - 1000. SB - 1500. SC - 2640	235	840	840	60	120			
1 SHOULDER TAPER LENGTH BASED ON 10 ft. (STANDARD SHOULDER WIDTH) OFFSET. 2 LANE TAPER LENGTH BASED ON 12 ft. (STANDARD LANE WIDTH) OFFSET.										

WORK ON THE SHOULDER ON DIVIDED AND UNDIVIDED HIGHWAYS

DAY TIME OR NIGHT TIME ALLOWED

- 1. PROVIDE SIGNS ON LEFT AND RIGHT SIDES OF DIVIDED HIGHWAYS.
- 2. ROAD WORK AHEAD SIGN NOT NEEDED IF SHOULDER WORK IS LOCATED WITHIN THE LIMITS OF AN ACTIVITY AREA WHERE ANOTHER ROAD WORK AHEAD SIGN IS ALREADY USED.
- 3. SEE EPG 616.12 WORK ZONE SPEED LIMITS FOR SPEED LIMIT GUIDELINES.
- 4. VEHICLE HAZARD WARNING SIGNALS SHALL NOT BE USED INSTEAD OF THE VEHICLE'S ROTATING LIGHTS OR STROBE LIGHTS.
- 5. WHEN PAVED SHOULDERS HAVING A WIDTH OF 8 FEET OR MORE ARE CLOSED, AT LEAST ONE ADVANCE WARNING SIGN SHALL BE USED. IN ADDITION, CHANNELIZING DEVICES SHALL BE USED TO CLOSE THE SHOULDER IN ADVANCE TO DELINEATE THE BEGINNING OF THE WORK SPACE AND DIRECT VEHICULAR TRAFFIC TO REMAIN WITHIN THE TRAVELED WAY.



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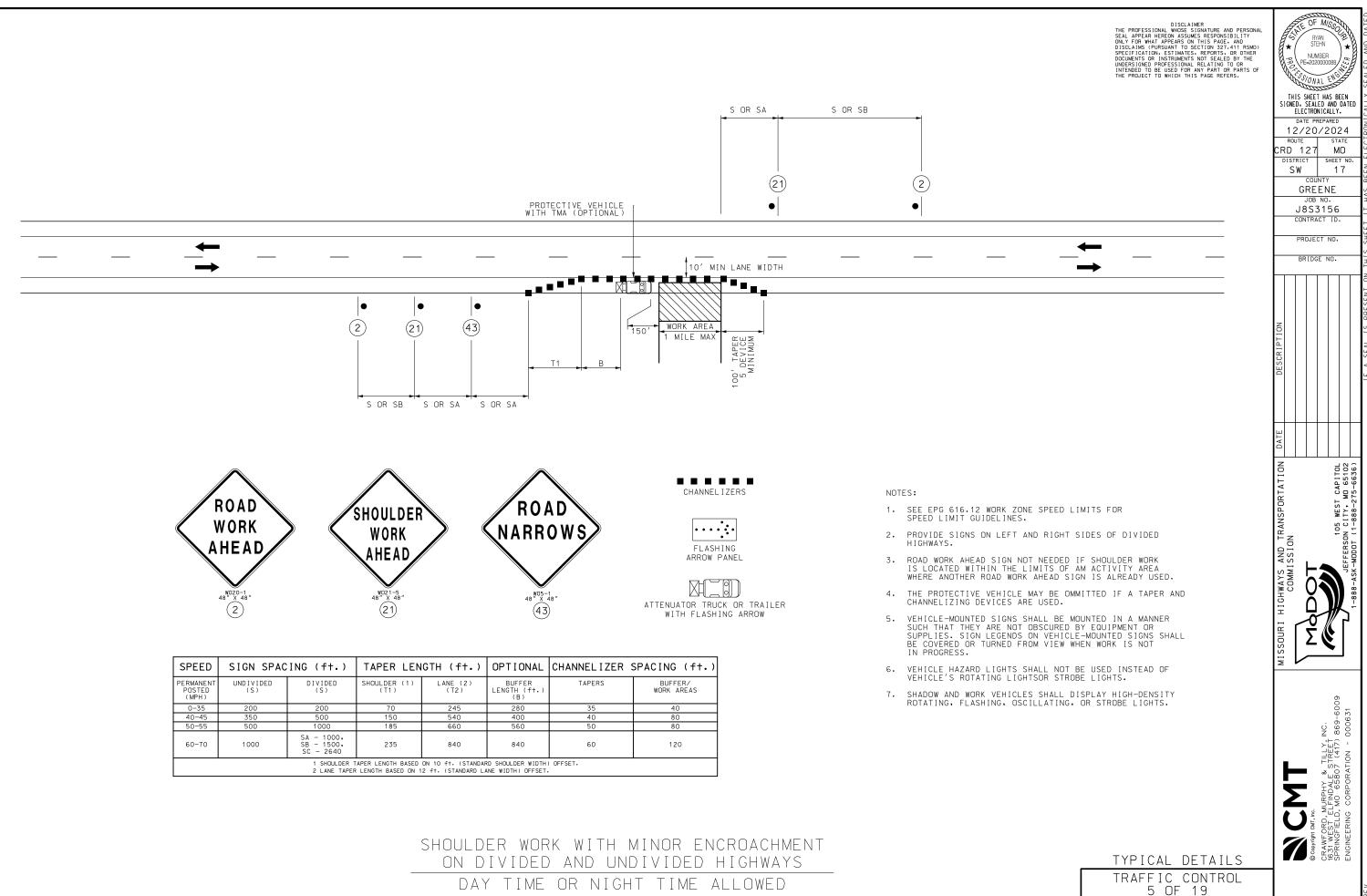
GREENE J8S3156 CONTRACT ID.

PROJECT NO.

BRIDGE NO.

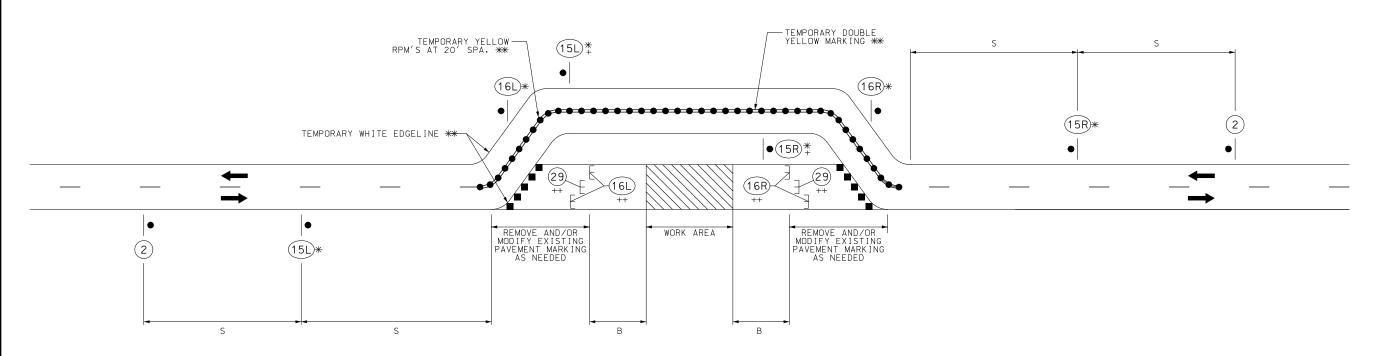
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1631 WEST ELFINDALE STREET
16531 WEST ELFINDALE STREET
16531 WEST CAPPORATION - 0009

TYPICAL DETAILS TRAFFIC CONTROL 4 OF 19



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SIGN SPACING (ft.)

DIVIDED (S)

UNDIVIDED (S)

SPEED

PERMANEN

POSTED (MPH) 40-45



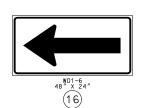
TAPER LENGTH (ft.)

LANE (2) (T2)

540 660

840 1 SHOULDER TAPER LENGTH BASED ON 10 ft. (STANDARD SHOULDER WIDTH) OFFSET. 2 LANE TAPER LENGTH BASED ON 12 ft. (STANDARD LANE WIDTH) OFFSET.

SHOULDER (1)





OPTIONAL	CHANNEL I ZER	SPACING (ft.)
BUFFER LENGTH (ft.) (B)	TAPERS	BUFFER/ WORK AREAS
280	35	40
400	40	80
560	50	80

CHANNEL IZERS

• • • • •

TEMPORARY RAISED PAVEMENT MARKER

TYPE III MOVEABLE BARRICADE

- 1. SEE EPG 616.12 WORK ZONE SPEED LIMITS FOR SPEED LIMIT GUIDELINES.
- 2. ROAD WORK AHEAD SIGN NOT NEEDED IF DIVERSION IS LOCATED WITHIN THE LIMITS OF AN ACTIVITY AREA WHERE ANOTHER ROAD WORK AHEAD SIGN IS ALREADY USED.
 - ARROW SIGNS AND REVERSE CURVE OR TURN SIGNS AS DETERMINED BY DIVERSION GEOMETRICS. ADDITIONAL WARNING SIGNS MAY ALSO BE NEEDED.
 - ** PAVEMENT MARKING IS NOT USED ON NON-PAVED SURFACES
 - + SIGNS LOCATED IN ORDER TO PROVIDE SUFFICIENT ADVANCE WARNING.
 - ++ SEE STANDARD PLAN 616.10 FOR SIGNING AND BARRICADE REQUIREMENTS.
- 3. IF RUMBLE STRIPS ARE USED, REVIEW EPG 616.6.87 RUMBLE STRIPS.
- 4. FOR FLAGS AND ADVANCE WARNING RAIL SYSTEM, REFER TO EPG 616.6.22 FLAGS AND ADVANCE WARNING RAIL SYSTEM.
- 5. PAVEMENT MARKINGS NO LONGER APPLICABLE TO THE TRAFFIC PATTERN OF THE ROADWAY SHALL BE REMOVED OR OBLITERATED BEFORE ANY NEW TRAFFIC PATTERNS ARE OPEN TO TRAFFIC.

PROJECT NO. BRIDGE NO.

RYAN STEHN NUMBER PE-20200000

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12/20/2024 RD 127 MO DISTRICT

GREENE

J8S3156

CONTRACT ID.

SW

SHEET NO.

18

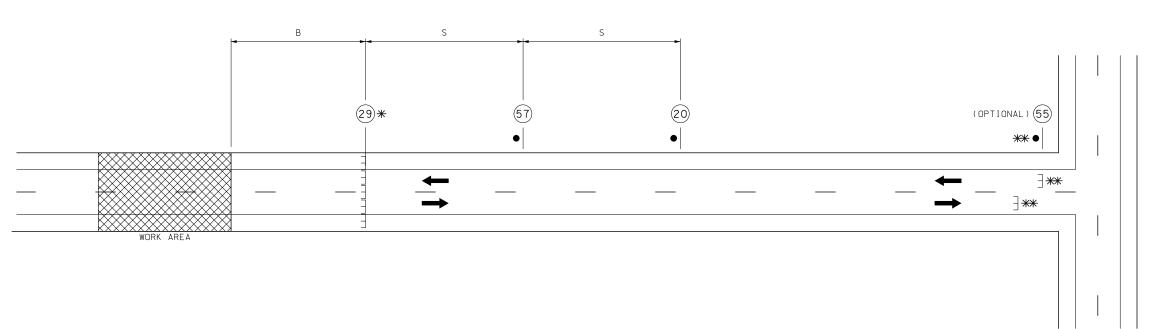
, MURPHY & TILLY, INC. ELFINDALE STREET D, MO 65807 (417) 869 IG CORPORATION - 0006

ROAD CLOSURE WITH A DIVERSION DAY TIME OR NIGHT TIME ALLOWED

TYPICAL DETAILS TRAFFIC CONTROL 6 OF 19

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ROAD

TYPE III MOVEABLE BARRICADE

DAY TIME OR NIGHT TIME ALLOWED

SPEED	SIGN SPAC	ING (ft.)	TAPER LEN	GTH (ft.)	OPTIONAL	CHANNEL I ZER	SPACING (ft.)				
PERMANENT POSTED (MPH)	UNDIVIDED (S)	DIVIDED (S)	SHOULDER (1) (T1)	LANE (2) (T2)	BUFFER LENGTH (ft.) (B)	TAPERS	BUFFER/ WORK AREAS				
0-35	200	200	-	-	250	-	-				
40-45	350	500	-	-	360	_	_				
50-55	500	1000	ı	ı	495	ı	-				
60-70	1000	SA - 1000. SB - 1500. SC - 2640	-	-	730	-	-				
	1 SHOULDER TAPER LENGTH BASED ON 10 ft. (STANDARD SHOULDER WIDTH) OFFSET.										

NOTES:

- ROAD CLOSED SIGN MAY BE PLACED 7-10 FEET BEHIND THE BARRICADES AND AT A SIGN HEIGHT APPROPRIATE TO THE TYPE OF ROADWAY. ONE BARRICADE SHOULD BE REQUIRED TO COMPLETELY CLOSE EACH 8-FEET OF PAVEMENT. PAVED SHOULDERS SHALL BE INCLUDED IN THE AREA TO BE CLOSED.
- *** IF USED, THE ROAD CLOSED XX MILES AHEAD LOCAL TRAFFIC ONLY OR ROAD CLOSED TO THRU TRAFFIC SIGN SHOULD BE LOCATED AT THE FIRST STATE ROUTE OR, AT THE DISCRETION OF THE ENGINEER, ANY OTHER INTERSECTION IN ADVANCE OF THE CLOSURE. ADDITIONAL BARRICADES MAY BE USED AND OFFSET TO FACILITATE ACCESS FOR WORK VEHICLES. LOCAL TRAFFIC, ETC.
- 1. TRAFFIC CONTROL SHOULD BE REMOVED AS SOON AS PRACTICAL AFTER CONDITION FOR THE CLOSURE NO LONGER EXISTS.
- 2. FOR LONG-TERM OPERATIONS, REFER TO EPG 616.6.2.2 FLAGS AND ADVANCE WARNING RAIL SYSTEM (AWRS).
- 3. ADDITIONAL WARNING SIGNS SHOULD BE ERECTED AT EACH INTERSECTION WITHIN THE WORK ZONE.
- 4. SUPPLEMENTAL WARNING METHODS MAY BE USED TO CALL ATTENTION TO THE WORK ZONE.
- 5. SEE STANDARD PLAN 616.10 FOR USE OF BARRICADES AND SIGNS. 6. FOR LONG-TERM CLOSURE, DETOUR SIGNING SHOULD BE CONSIDERED.
- 7. FOR DETOUR SIGNING, REVIEW EPG 616.8.9 ROAD CLOSED BEYOND JUNCTION DETOUR.

TYPICAL DETAILS TRAFFIC CONTROL 7 OF 19

RYAN STEHN NUMBER PE-20200000

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PROJECT NO.

BRIDGE NO.

HIGHWAY CLOSURE

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RD 127 MO

GREENE

J8S3156

CONTRACT ID.

PROJECT NO.

BRIDGE NO.

SHEET NO

20

DISTRICT

SW

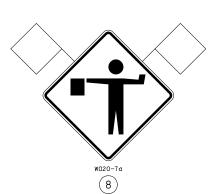
NOTES:

- 1. SEE EPG 616.12 WORK ZONE SPEED LIMITS FOR SPEED LIMIT GUIDELINES.
- 2. ADDITIONAL SIGNING MAY BE REQUIRED BASED ON WORK ZONE ACTIVITIES.
 - ROAD WORK AHEAD SIGN REQUIRED FOR ALL PUBLIC ROAD APPROACHES TO THE WORK ZONE. IF NO FLAGGERS ARE REQUIRED, THEN ROAD WORK AHEAD SIGNS ARE LOCATED "S OR "SA" FEET FROM THE INTERSECTION.
 - ** FLAGGER AND SIGNING ONLY REQUIRED WHERE EQUIPMENT IS CROSSING THE ROAD OR OTHER CONDITIONS REQUIRING FLAGGER CONTROL EXIST.
- 4. IF RUMBLE STRIPS ARE USED, REVIEW 616.6.87 RUMBLE
- 5. IF USED AT NIGHT, THE FLAGGER SHALL BE ILLUMINATED WITH AN AVERAGE MAINTAINED INTENSITY OF 0.6 FOOTCANDLES (6.5 LUX)

SPEED	SIGN SPAC	ING (ft.)	TAPER LEN	GTH (ft.)	OPTIONAL	CHANNEL I ZER	SPACING (ft.)
PERMANENT POSTED (MPH)	UNDIVIDED (S)	DIVIDED (S)	SHOULDER (1) (T1)	LANE (2) (T2)	BUFFER LENGTH (ft.) (B)	TAPERS	BUFFER/ WORK AREAS
0-35	200	-	70	245	280	35	40
40-45	350	-	150	540	400	40	80
50-55	500	-	185	660	560	50	80
60-70	1000	-	235	840	840	60	120

1 SHOULDER TAPER LENGTH BASED ON 10 ft. (STANDARD SHOULDER WIDTH) OFFSET. 2 LANE TAPER LENGTH BASED ON 12 ft. (STANDARD LANE WIDTH) OFFSET.

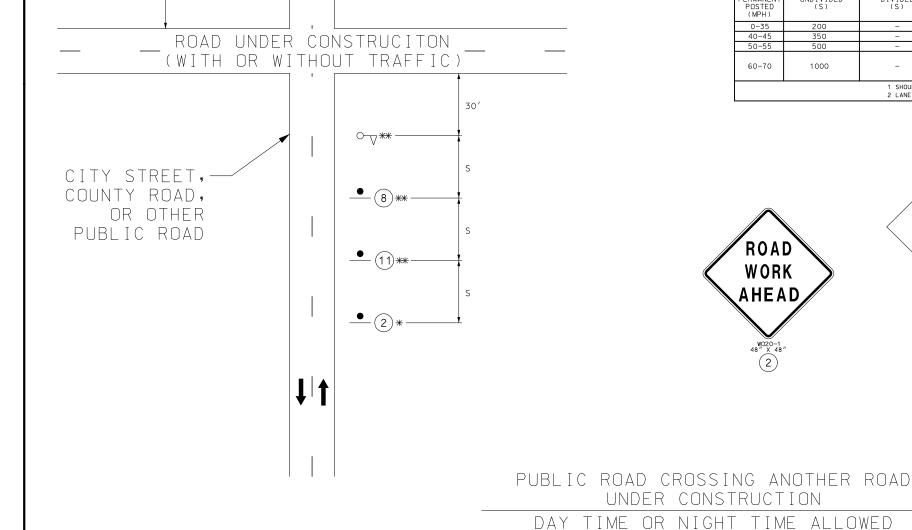








FLAGGER

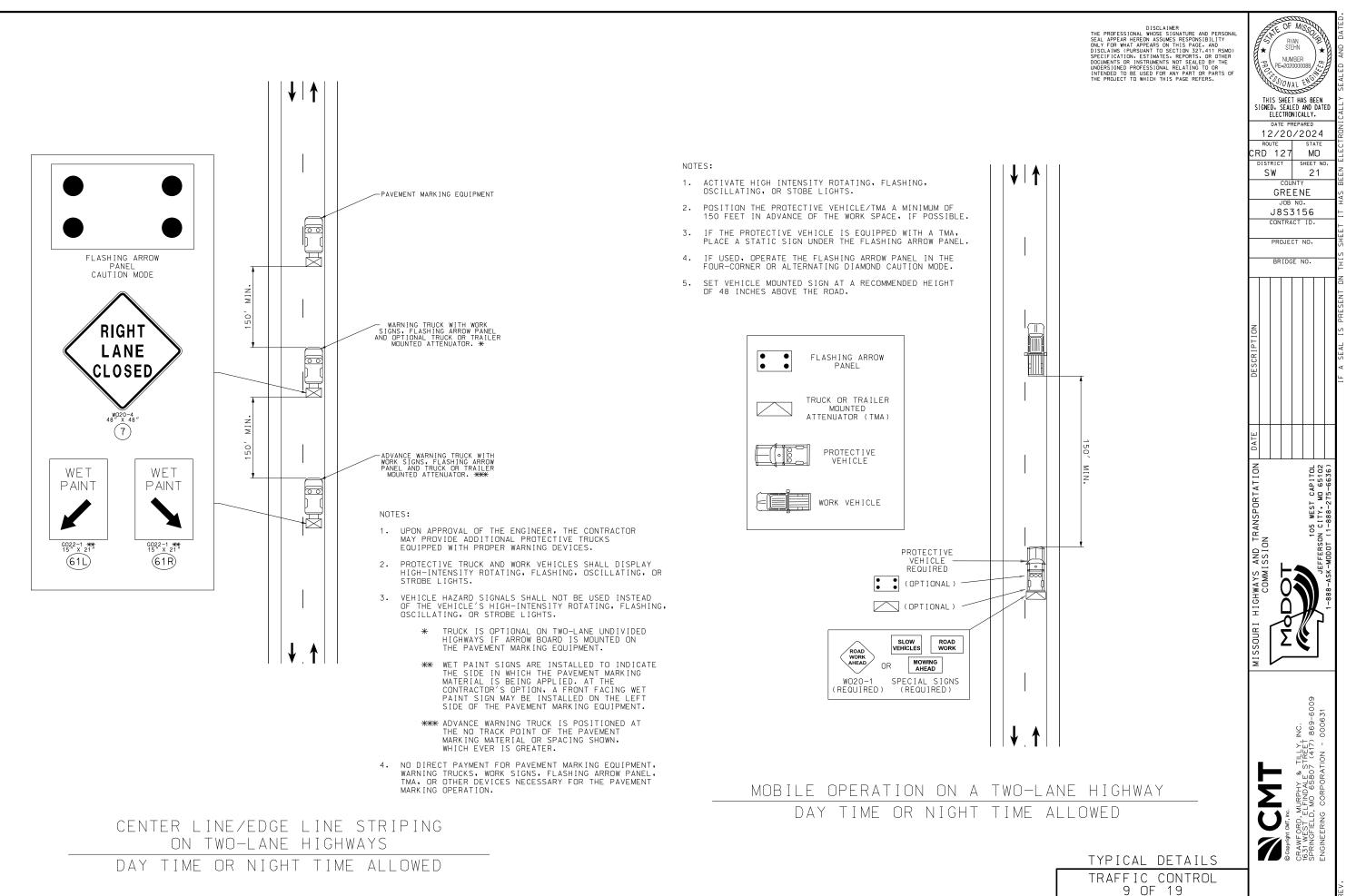


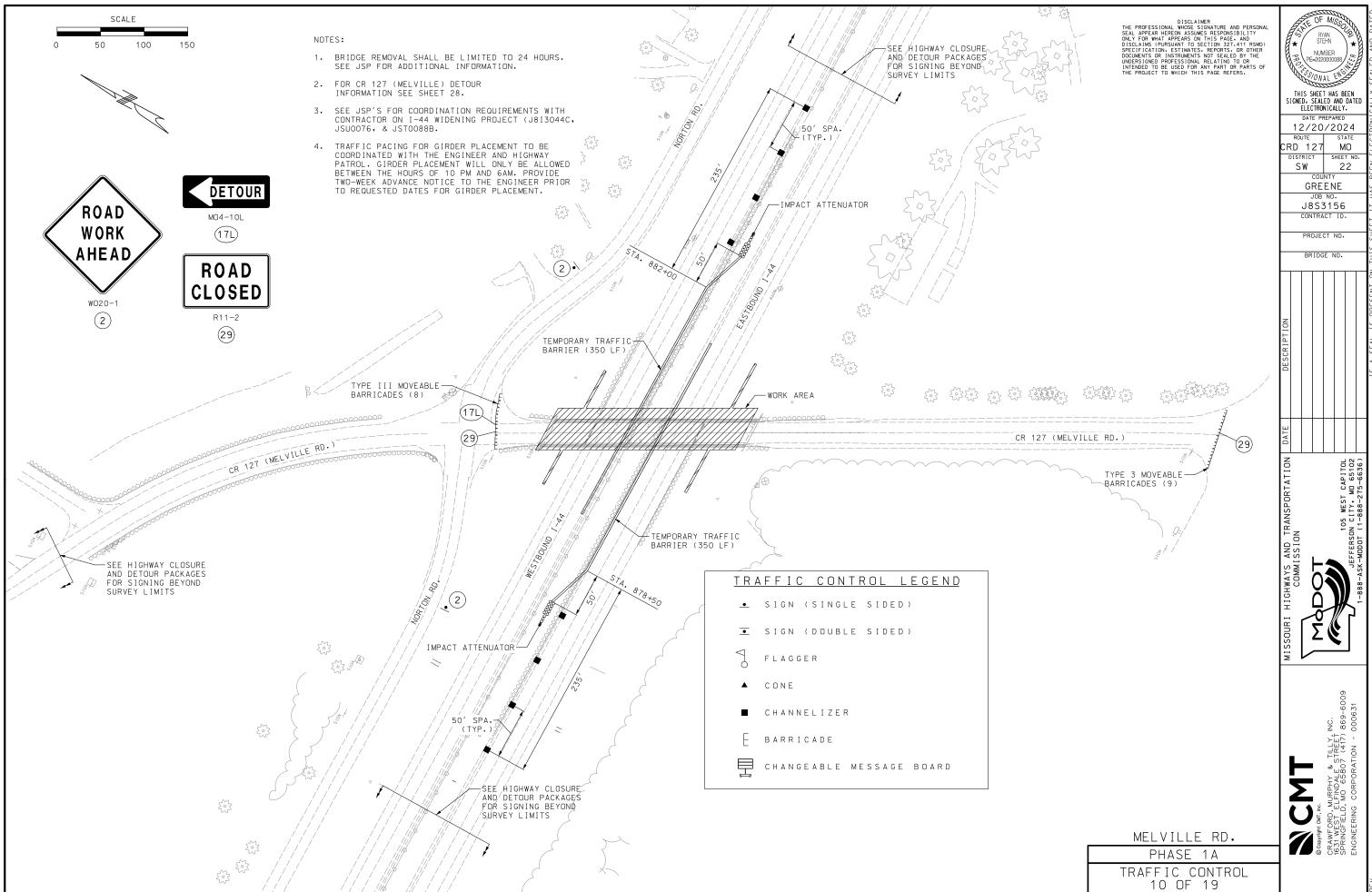
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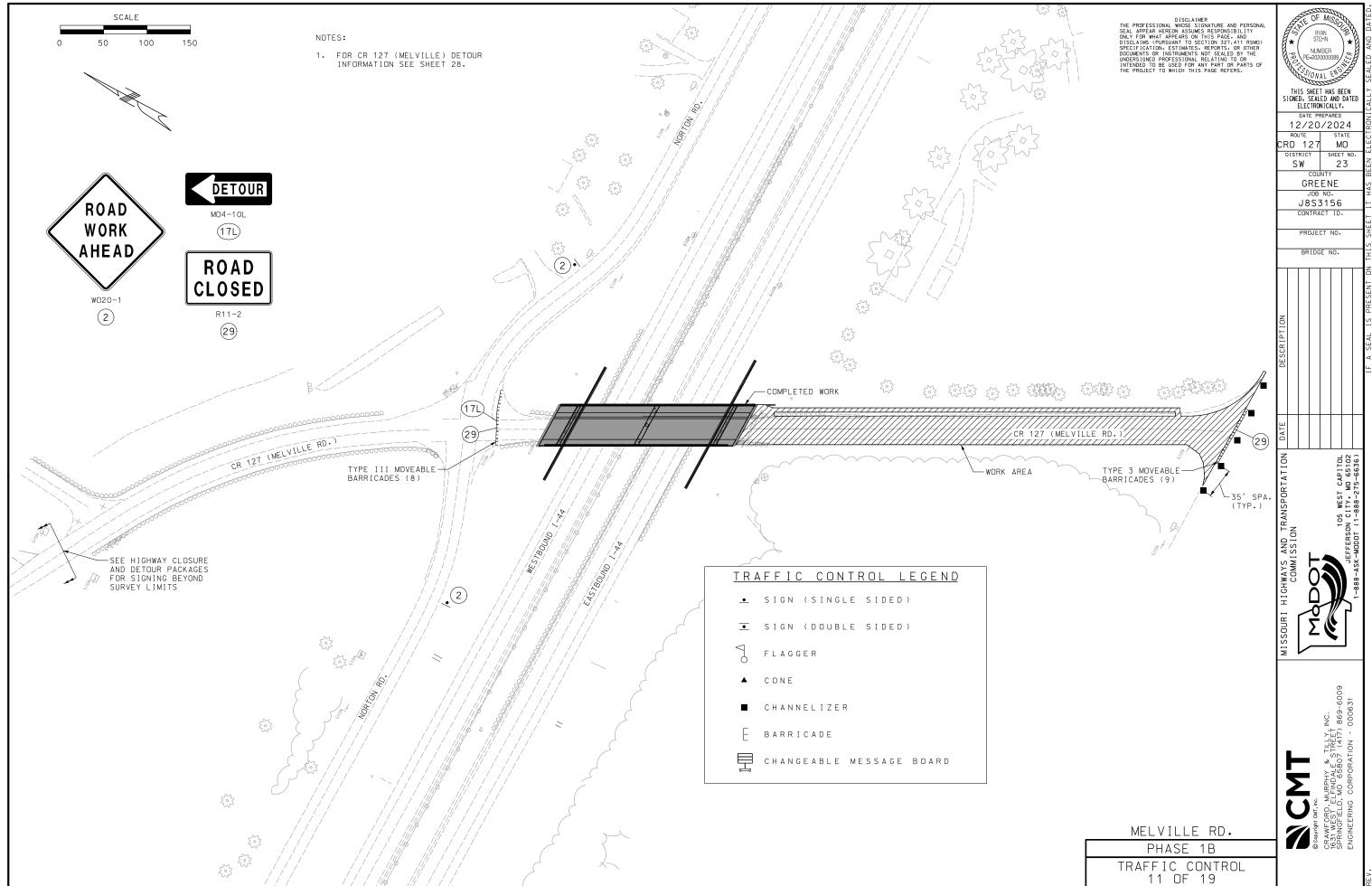
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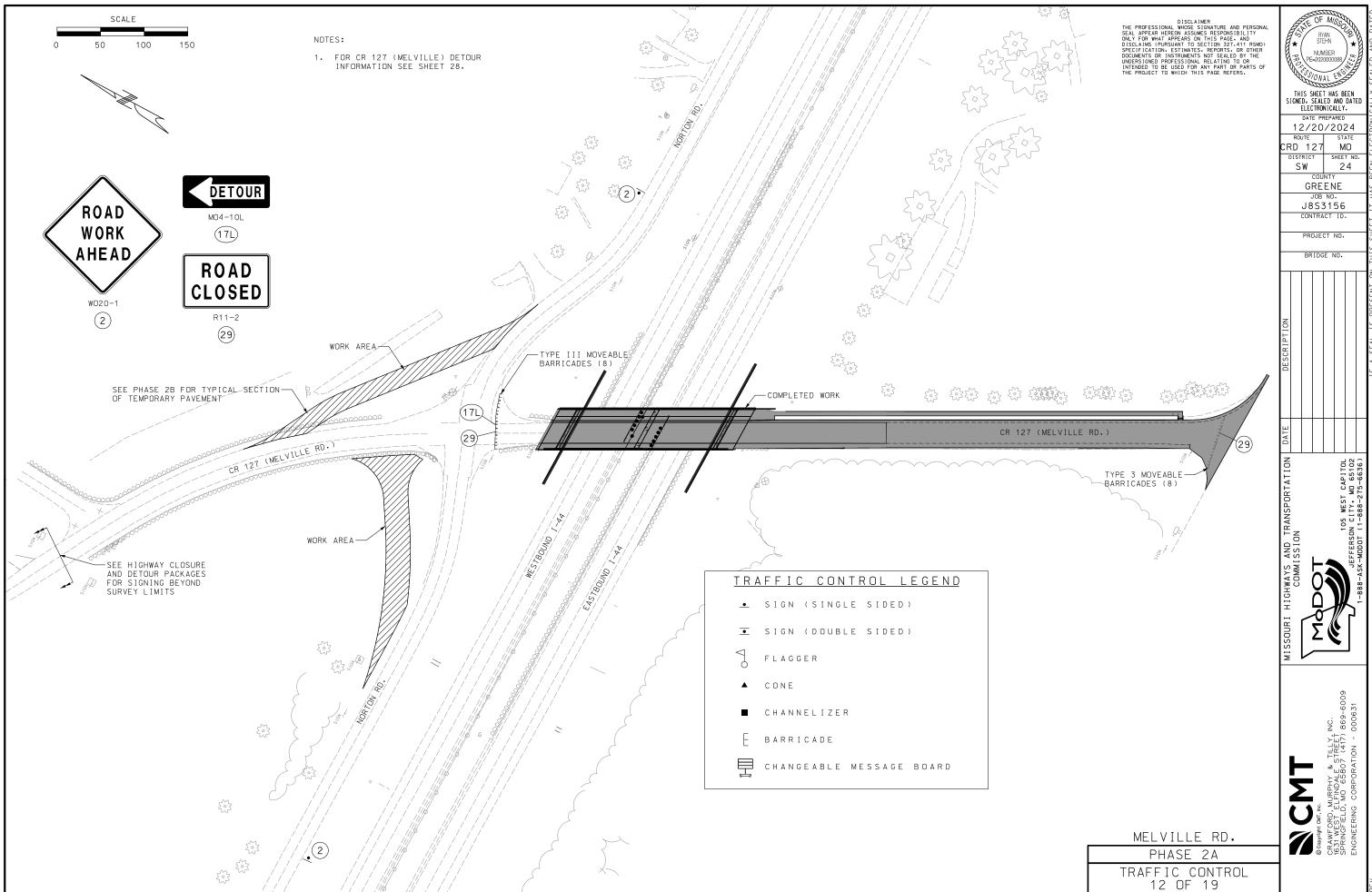
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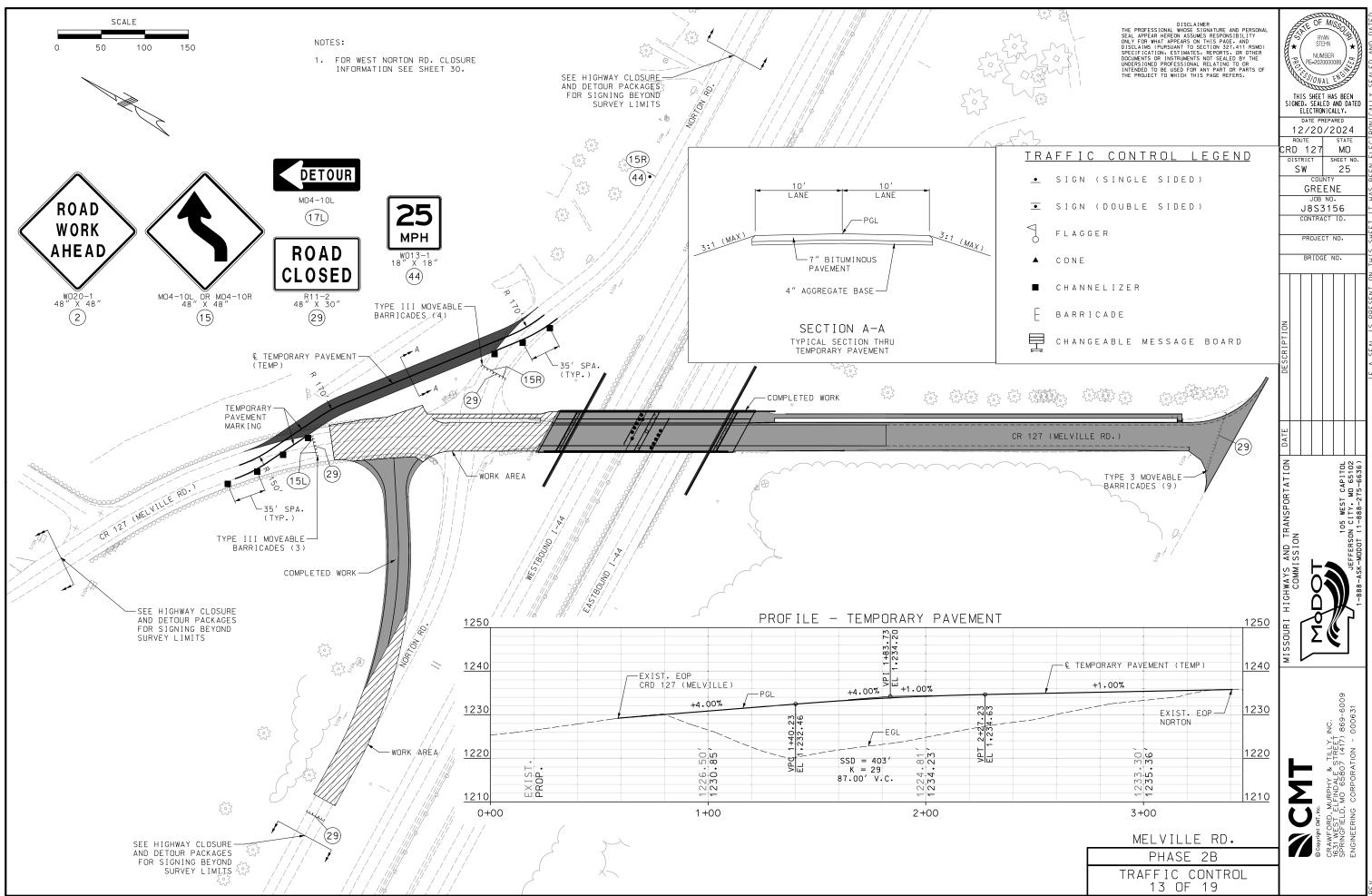
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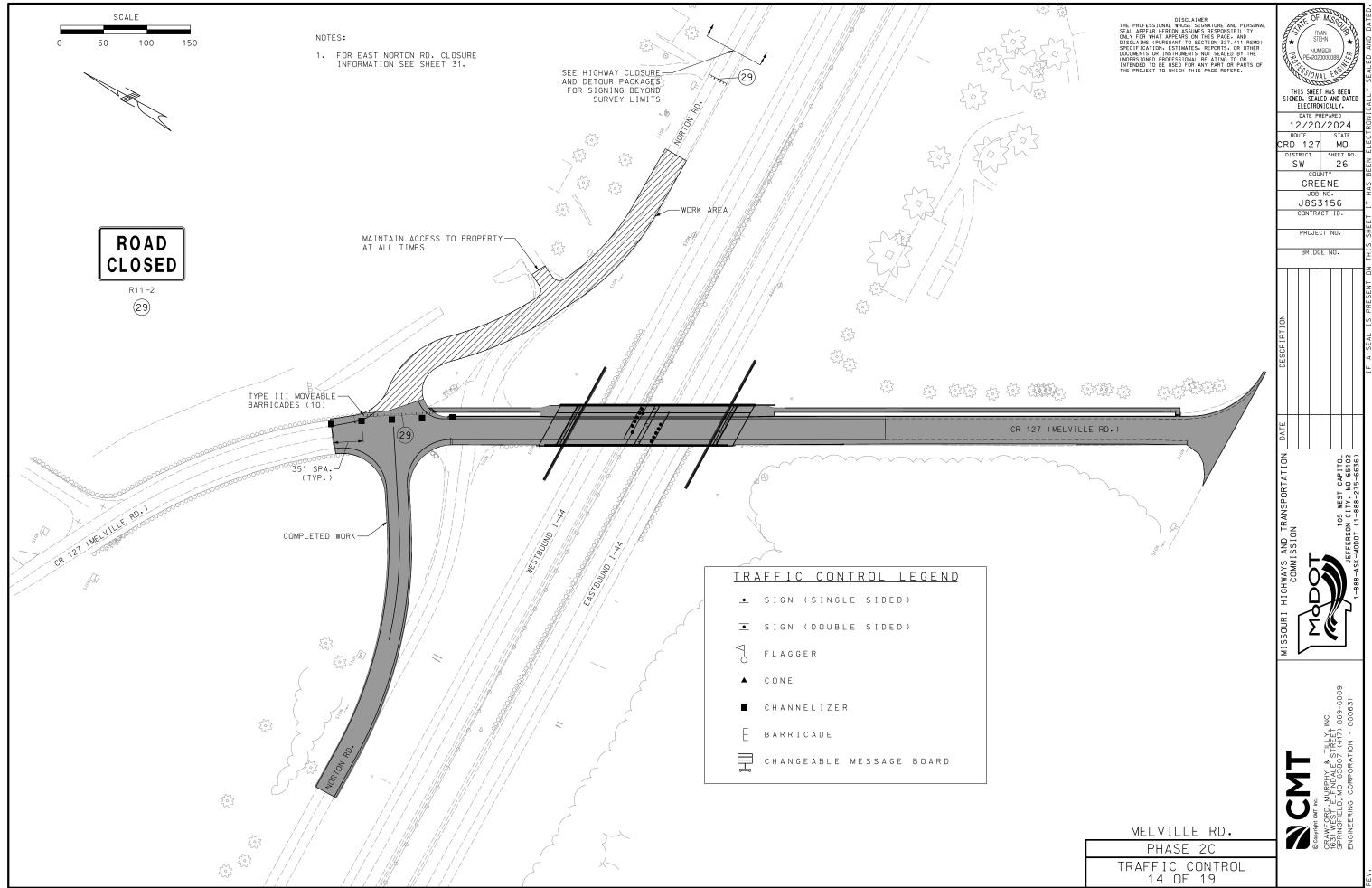


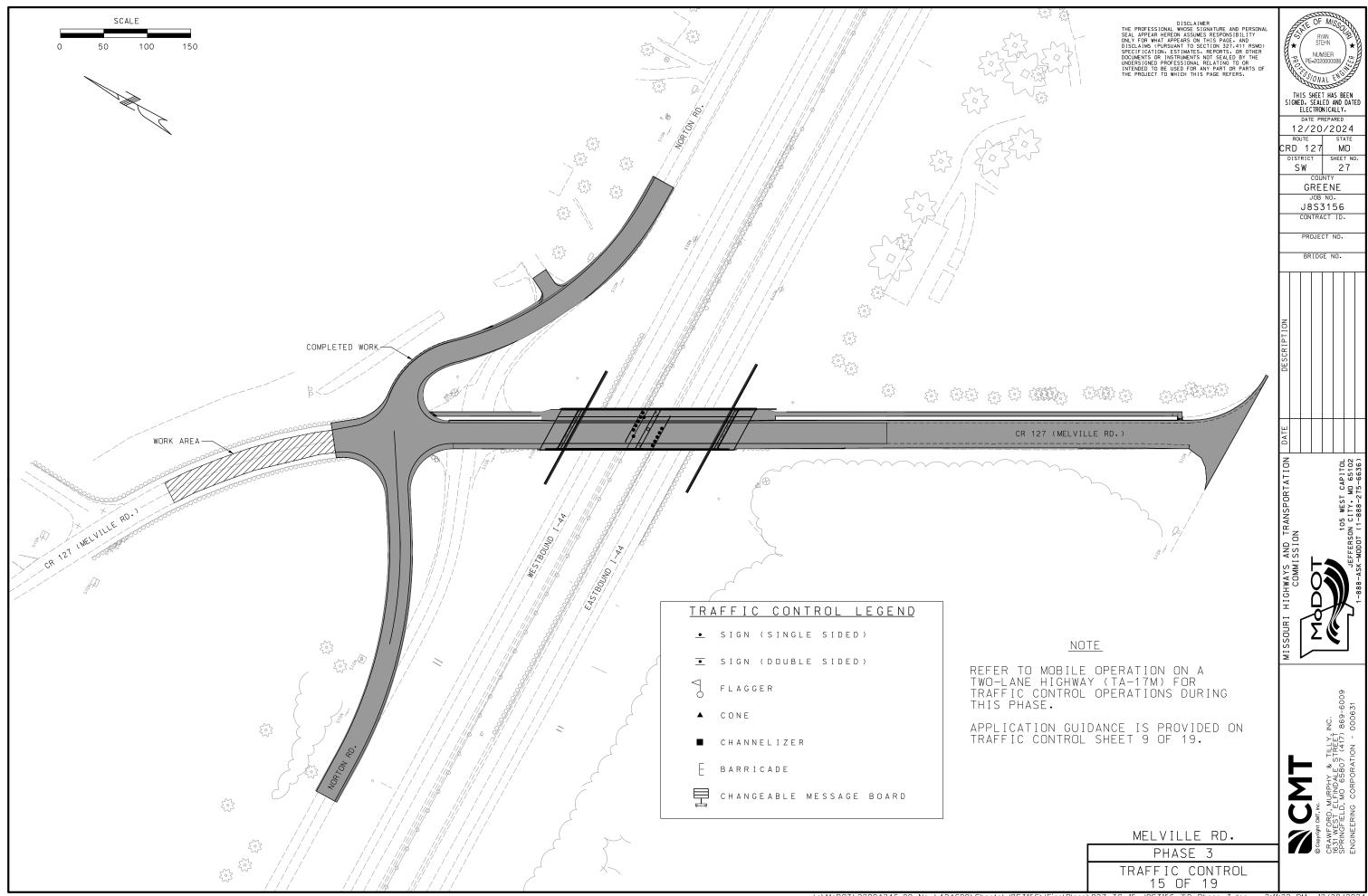


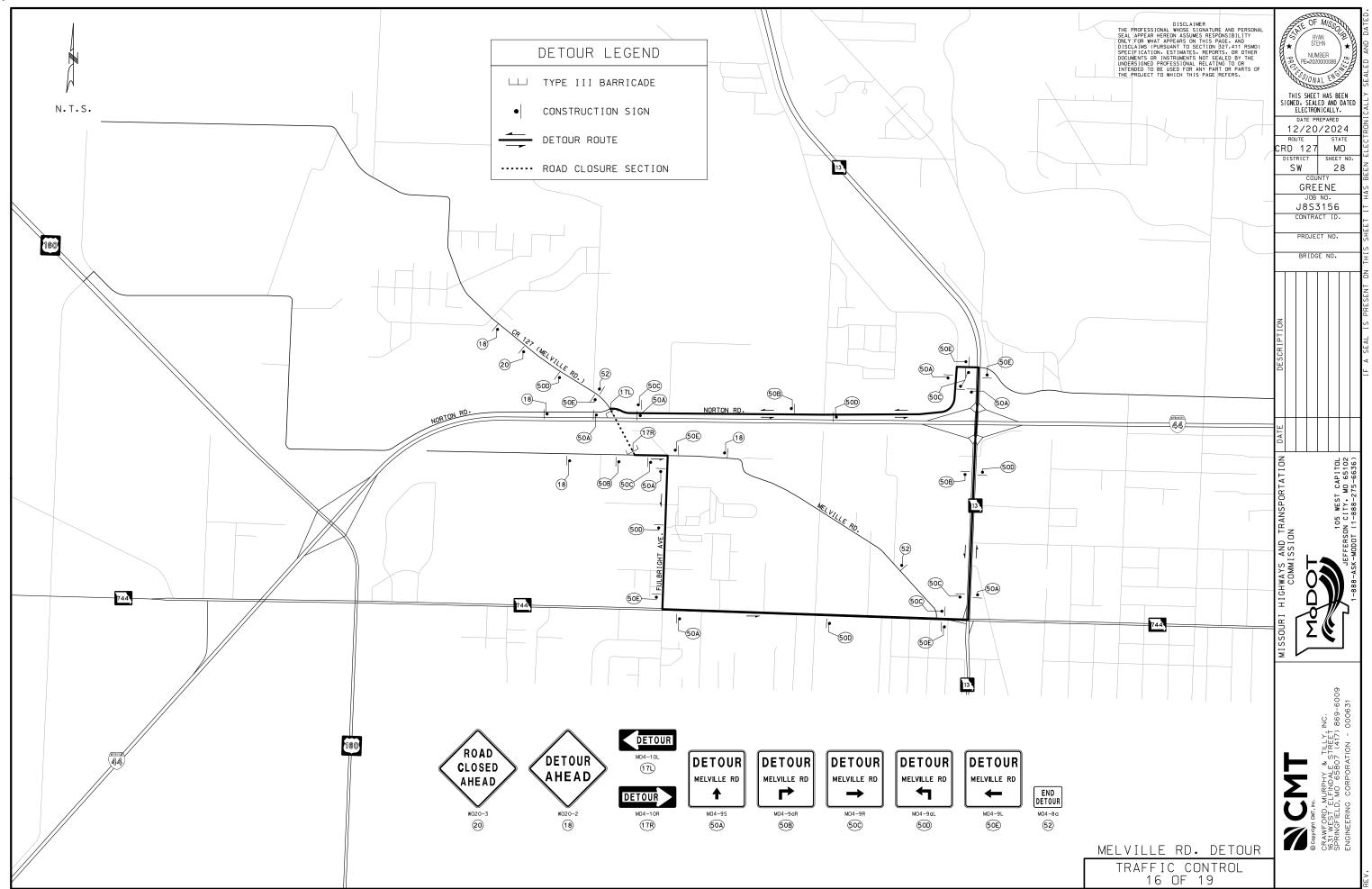


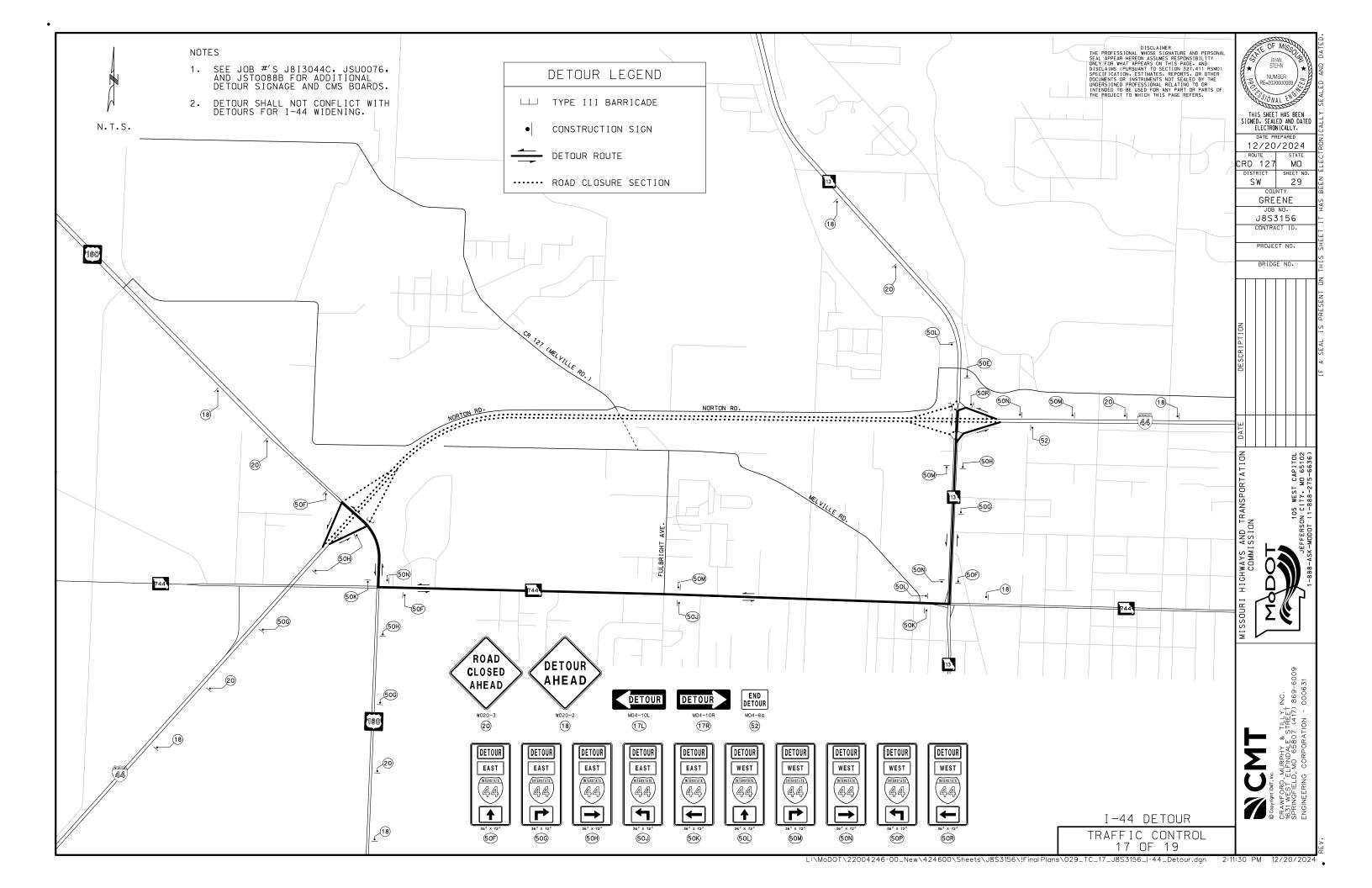


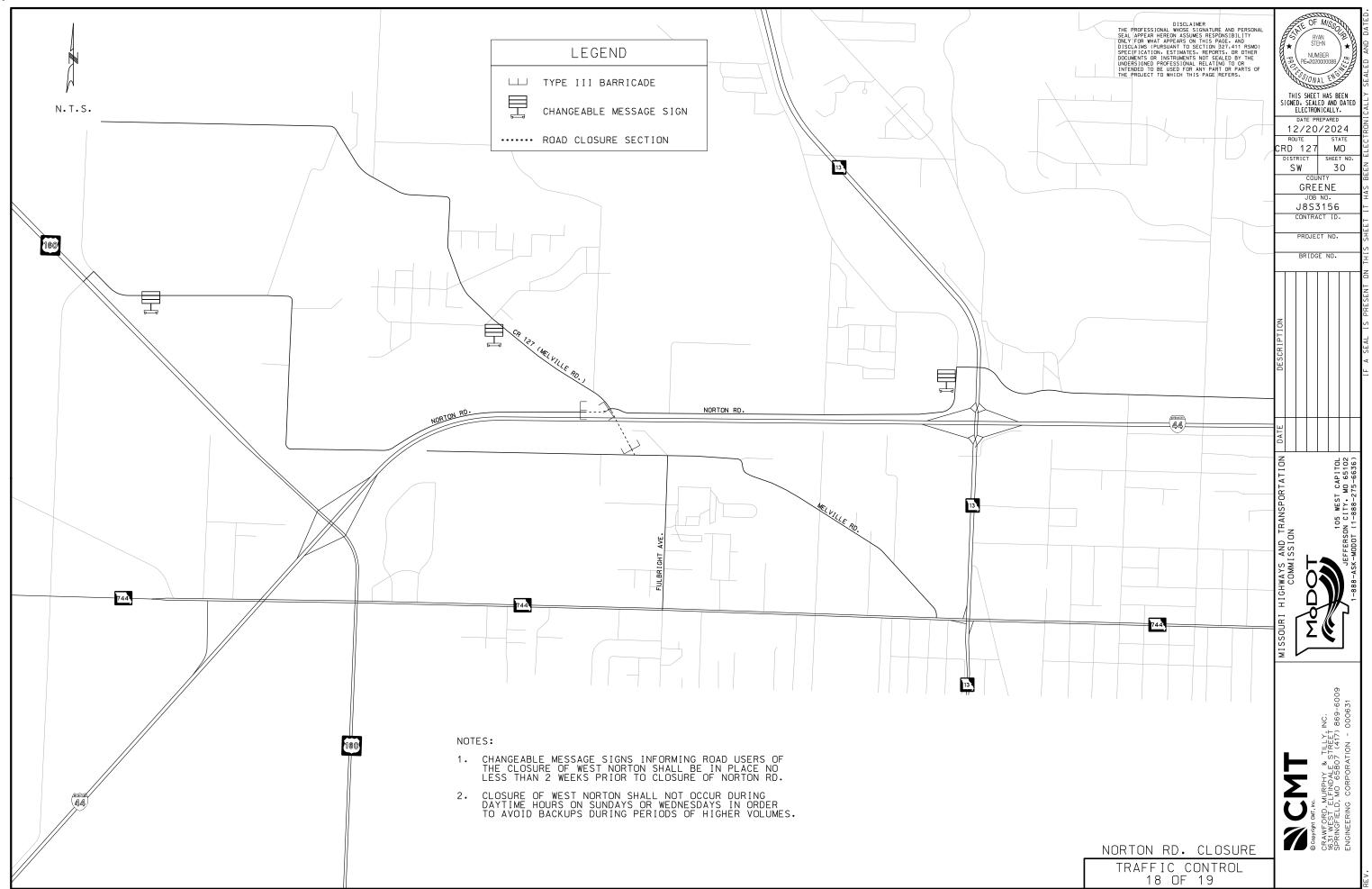


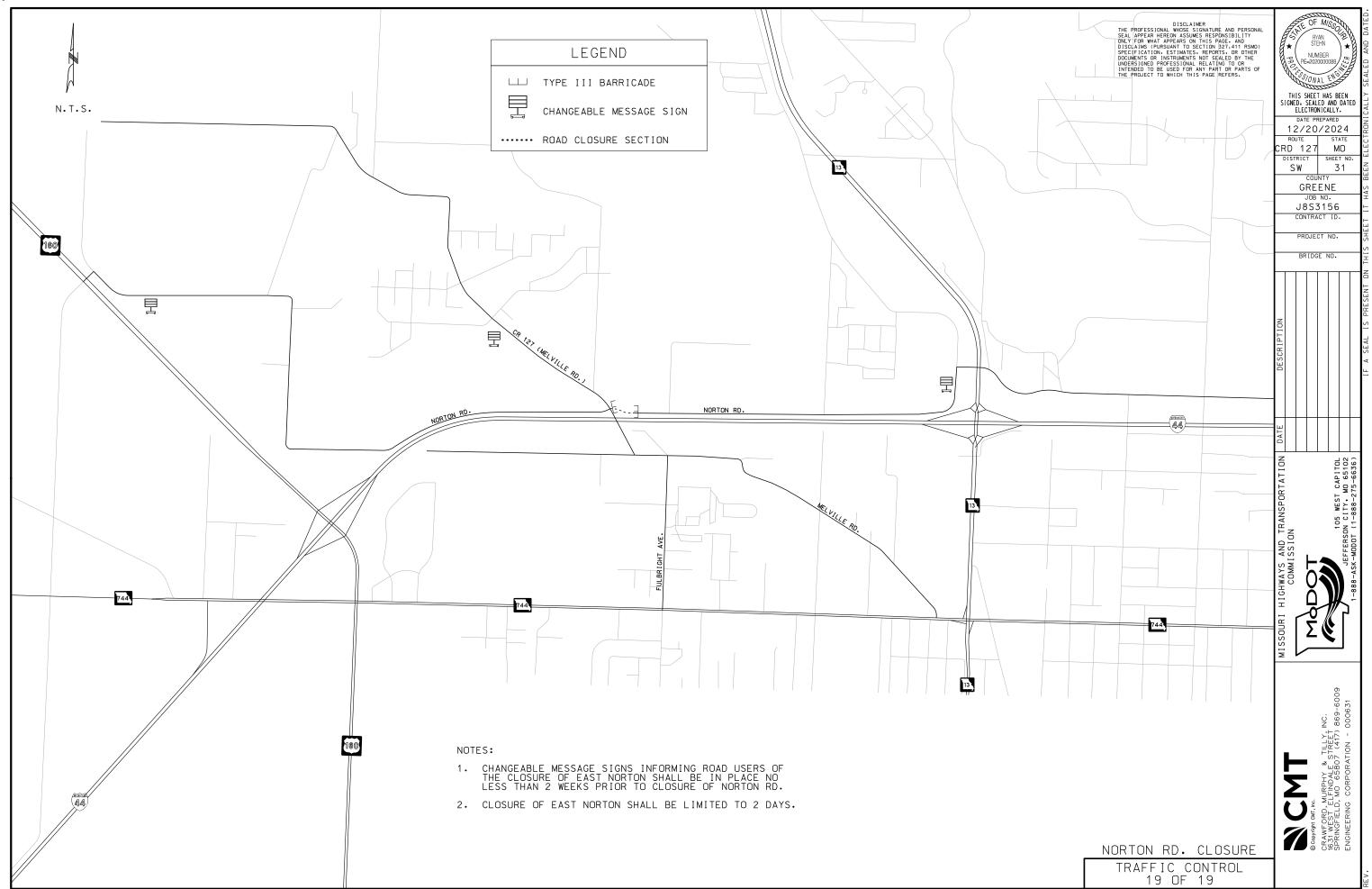


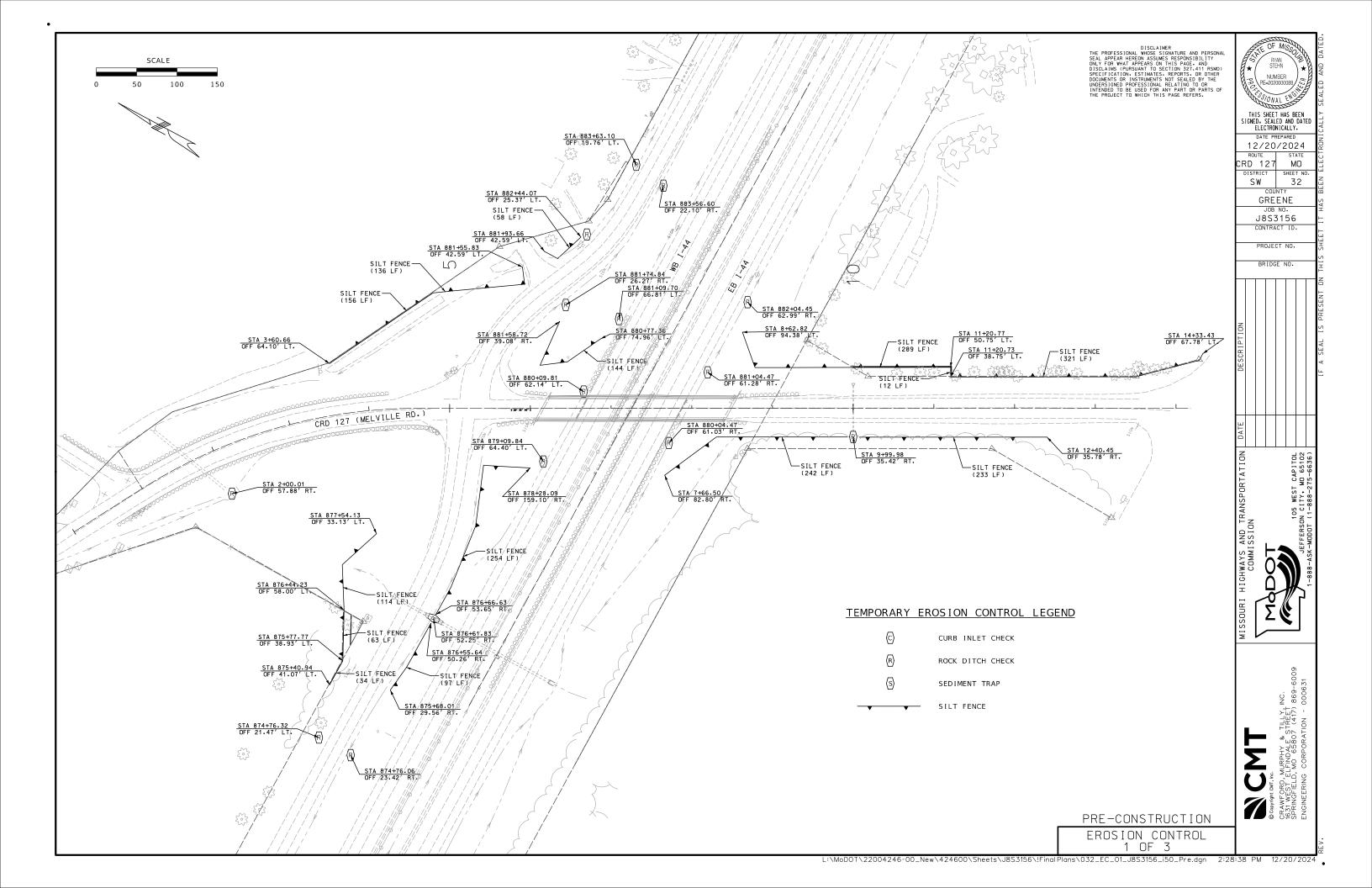


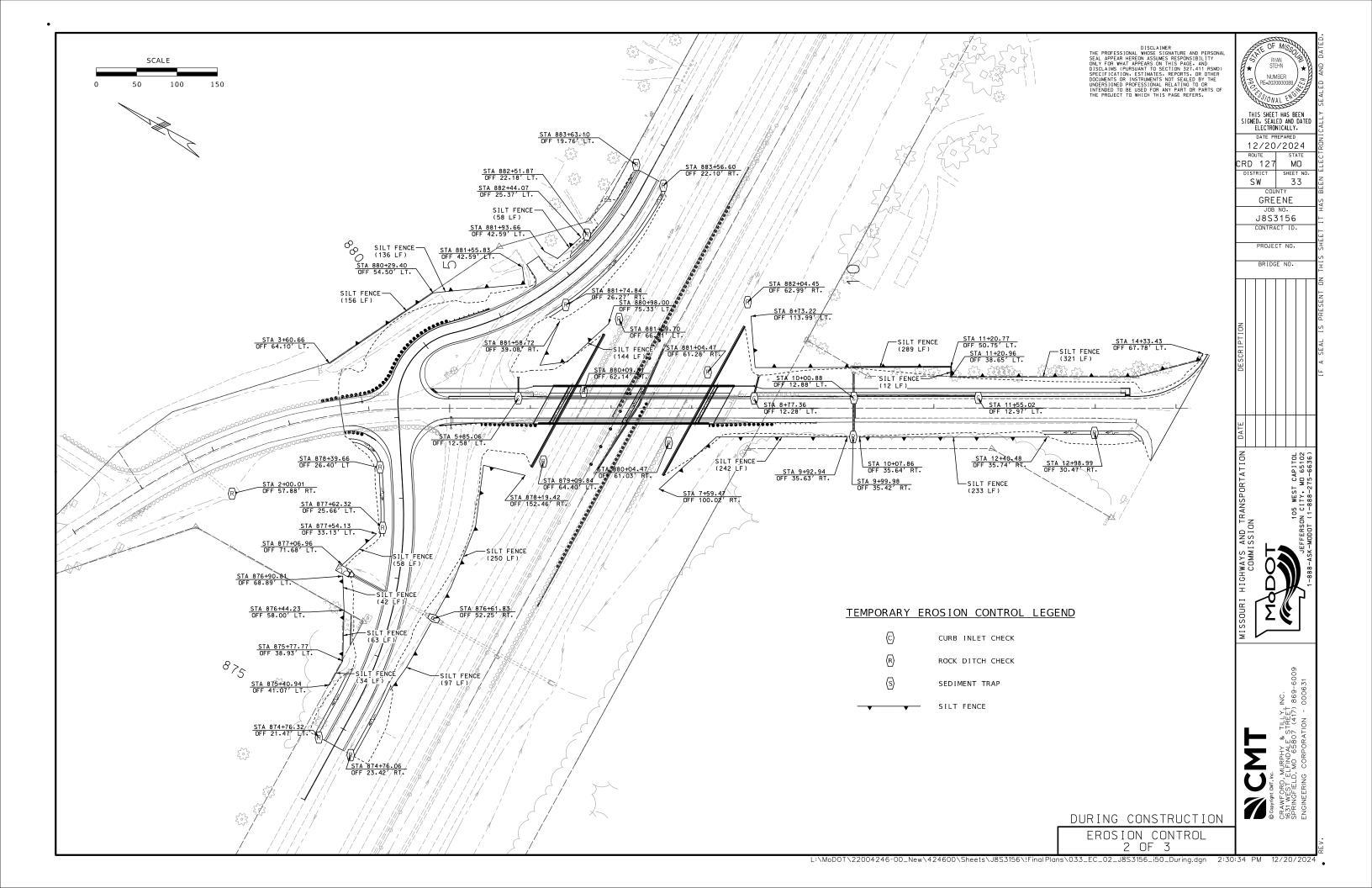


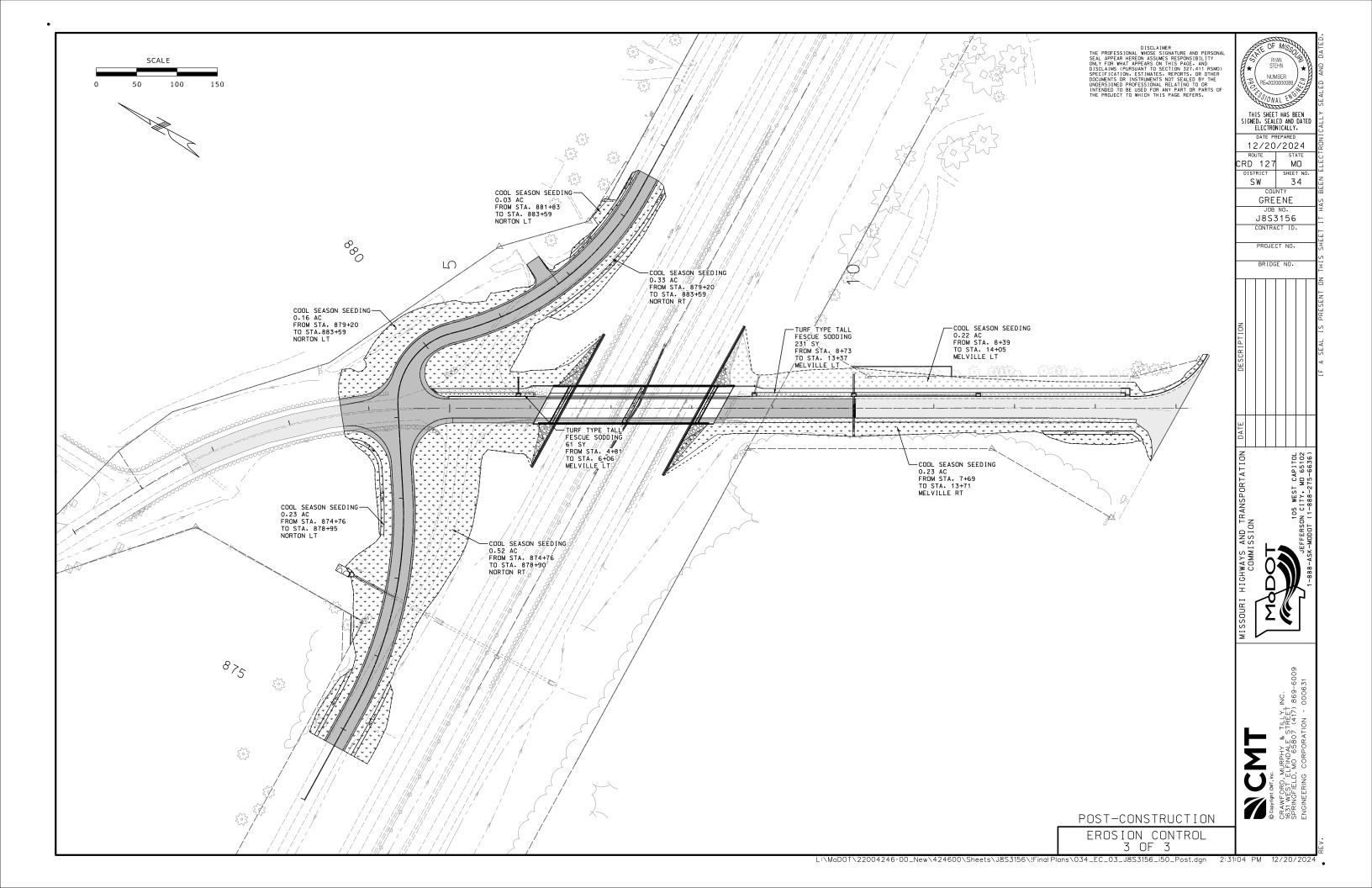


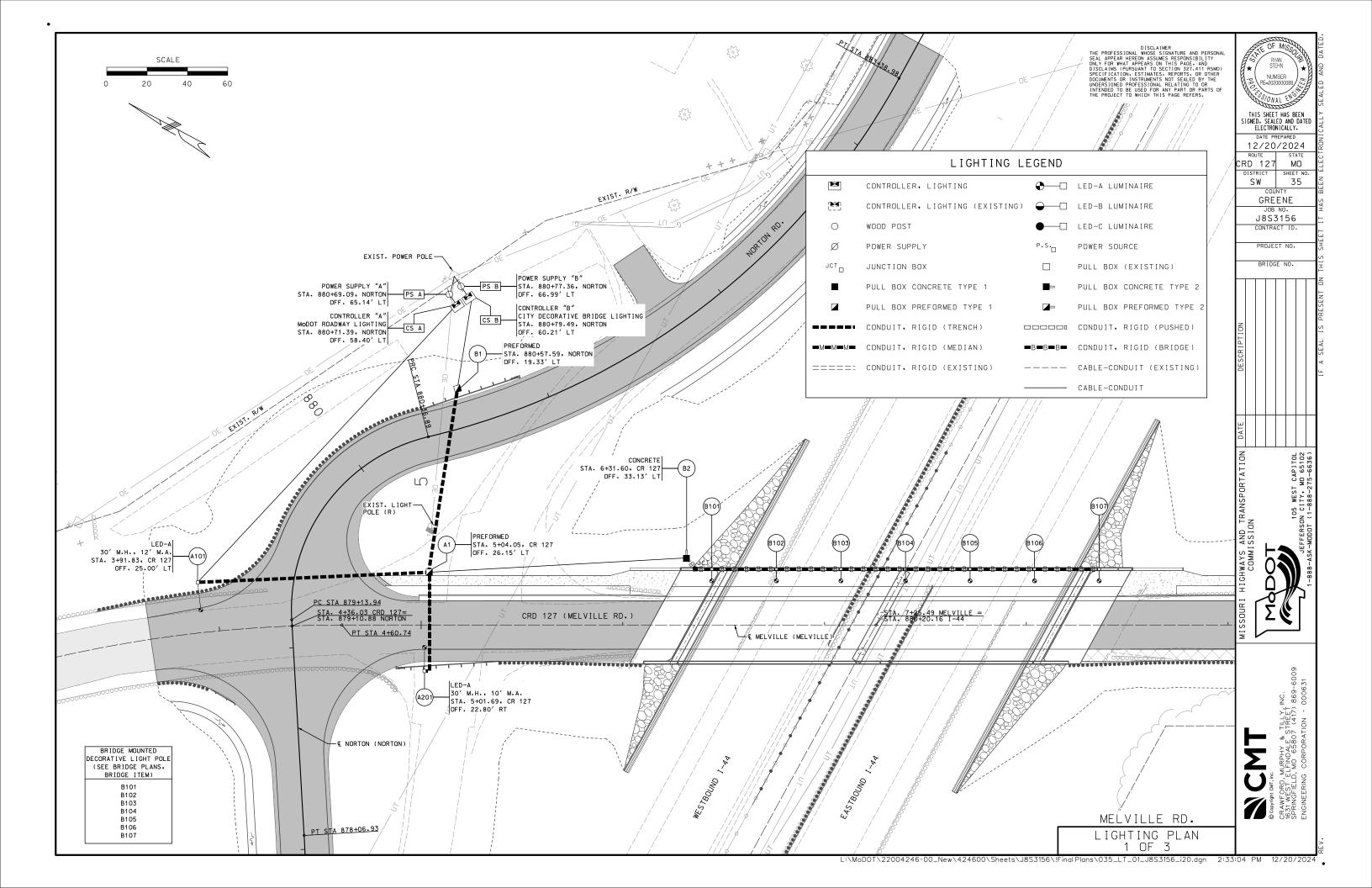


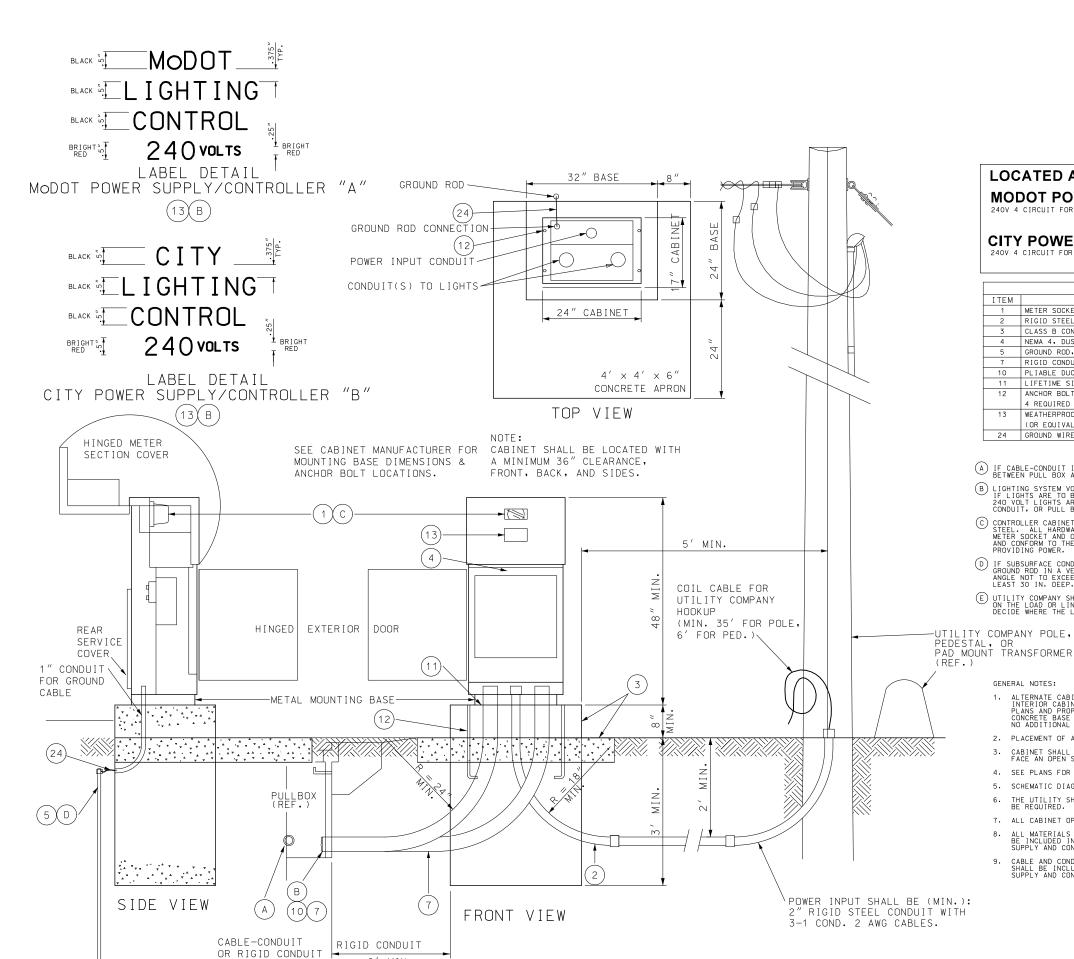












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THE PROJECT TO WHICH THIS PAGE REFERS.

LOCATED AT:

MODOT POWER SUPPLY/ CONTROLLER "A"

CITY POWER SUPPLY/ CONTROLLER "B"

	LIST OF MATERIALS
ITEM	DESCRIPTION
1	METER SOCKET, 200 AMP
2	RIGID STEEL CONDUIT, 2" MIN. TO UTILITY CO. FACILITIES
3	CLASS B CONCRETE, 0.9 C.Y.±
4	NEMA 4, DUST-TIGHT, WATERTIGHT, CABINET
5	GROUND ROD, 3/4" DIA, X 8' MIN, (MIN, 8' IN SOIL)
7	RIGID CONDUIT TO FIRST PULL BOX (SEE PLANS FOR SIZE)
10	PLIABLE DUCT SEALANT
11	LIFETIME SILICONE CAULK
12	ANCHOR BOLTS, § - 11 × 18" LONG BOLTS, HOT DIP GALVANIZED,
	4 REQUIRED
13	WEATHERPROOF ADHESIVE LABEL, VINYL RAISED LETTERING
	(OR EQUIVALENT, SEE DETAIL)
24	GROUND WIRE, #2 AWG, MIN.

- (A) IF CABLE-CONDUIT IS SPECIFIED. THE CONDUIT SHALL BE CUT AWAY FROM CABLES BETWEEN PULL BOX AND CONTROL STATION.
- B LIGHTING SYSTEM VOLTAGE AS SPECIFIED ON PLANS (120 OR 240 VOLTS). IF LIGHTS ARE TO BE MOUNTED ON SIGNAL POSTS, VOLTAGE MUST BE 120 VOLTS, 240 VOLT LIGHTS ARE NOT TO BE MOUNTED ON OR WIRED THROUGH SIGNAL POSTS, CONDUIT, OR PULL BOXES.
- CONTROLLER CABINET WITH POWER SUPPLY SHALL BE ALUMINUM OR STAINLESS STEEL. ALL HARDWARE, HINGES, CATCHES, ETC. SHALL BE STAINLESS STEEL. METER SOCKET AND OTHER EQUIPMENT AND MATERIALS SHALL BE U.L. APPROVED, AND CONFORM TO THE REQUIREMENTS OF THE UTILITY COMPANY OR MUNICIPALITY PROVIDING POWER.
- D IF SUBSURFACE CONDITIONS EXIST WHICH PROHIBIT THE PLACEMENT OF THE GROUND ROD IN A VERTICAL POSITION, THE ROD MAY BE DRIVEN AT AN OBLIQUE ANGLE NOT TO EXCEED 45 DEGREES FROM VERTICAL OR BURIED IN A TRENCH AT LEAST 30 IN. DEEP. CONNECTION TO GROUND ROD SHALL BE CADWELDED.
- (E) UTILITY COMPANY SHALL DECIDE IF LIGHTNING ARRESTERS ARE TO BE CONNECTED ON THE LOAD OR LINE SIDE OF THE METER. THE UTILITY COMPANY SHALL ALSO DECIDE WHERE THE LIGHTNING ARRESTER SHALL BE MOUNTED.

- ALTERNATE CABINET DIMENSIONS WILL BE ALLOWED AS APPROVED BY THE ENGINEER. INTERIOR CABINET VOLUME SHALL BE EQUAL TO OR GREATER THAN THAT SHOWN ON PLANS AND PROPER CLEARANCES SHALL BE PROVIDED FOR ALL EQUIPMENT. CONCRETE BASE DIMENSIONS SHALL BE MODIFIED TO FIT THE CABINET SUPPLIER AT NO ADDITIONAL COST.
- 2. PLACEMENT OF ALL ITEMS SHALL BE APPROVED BY THE ENGINEER.
- 4. SEE PLANS FOR CIRCUIT WIRING; MAXIMUM LOADING PER CIRCUIT IS 7,400 WATTS.
- 5. SCHEMATIC DIAGRAM SHALL BE MOUNTED ON INSIDE OF CABINET DOOR.
- 6. THE UTILITY SHALL BE NOTIFIED IN WRITING 30 DAYS PRIOR TO DATE SERVICE WILL BE REQUIRED.
- 7. ALL CABINET OPENINGS SHALL BE COVERED AND SEALED WITH LIFETIME SILICONE CAULK.
- MATERIALS REQUIRED EXCLUDING REFERENCE ITEMS AS SHOWN ON DRAWING SHALL INCLUDED IN PRICE BID FOR '120/240V COMBINATION BASE MOUNTED POWER PLY AND CONTROL STATION'.
- 9. CABLE AND CONDUIT FROM POWER SUPPLY ASSEMBLY TO UTILITY COMPANY FACILITIES SHALL BE INCLUDED IN PRICE BID FOR '120/240V COMBINATION BASE MOUNTED POWER SUPPLY AND CONTROL STATION'.

120/240v COMBINATION PAD MOUNTED POWER SUPPLY & LIGHTING CONTROL STATION DUAL METER

> LIGHTING PLAN 2 OF 3



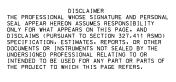
RD 127 MO DISTRICT SHEET NO SW 36 GREENE

J8S3156

CONTRACT ID.

PROJECT NO.

D, MURPHY & TILLY, IN ELFINDALE STREET LD, MO 65807 (417) E



LOCATED AT:

(18) G)MANUAL / AUTO SWITCH

(17) G)PHOTO CONTROL BREAKER

(15) B) MERCURY CONTACTORS

E 14 - 19

(10) SEAL

CONDUIT(S) OUT TO LIGHTS

6 C PHOTOELECTRIC SWITCH

TERMINAL BLOCK (PHOTOELECTRIC SWITCH TO CONTACTORS)

MODOT POWER SUPPLY/ CONTROLLER "A"

240V 4 CIRCUITS FOR MODOT LIGHTING

CITY POWER SUPPLY/ CONTROLLER "B"

	LIST OF MATERIALS
ITEM	DESCRIPTION
1	METER SOCKET, 200 AMP, WITH LEVER TEST BYPASS
2	RIGID STEEL CONDUIT, 2" MIN. TO UTILITY CO. FACILITIES
4	NEMA 3R, DUST-TIGHT, WATERTIGHT CABINET
5	GROUND ROD, 3/4" DIA, X 8' MIN, (MIN, 8' IN SOIL)
6	PHOTOELECTRIC SWITCH AND SOCKET, 105/285 V., 1000 WATT
10	PLIABLE SEALANT
11	LIFETIME SILICONE CAULK
14	INSULATED TERMINAL BLOCK
15	2-POLE, 35 AMP, 120V COIL LIGHTING CONTACTOR (MERCURY)
16	2-POLE, 650 VOLT LIGHTNING ARRESTER
17	1-POLE, 15 AMP, TYPE B BREAKER TO PHOTOELECTRIC SWITCH
18	MANUAL-AUTO SWITCH
19	INSULATED GROUNDABLE NEUTRAL, 100 AMP
20	2-POLE, 100 AMP, TYPE A MAIN BREAKER
21	2-POLE, 15 AMP (MIN), TYPE A BREAKERS *
22	#12 AWG MIN., 600 V., CONTROL CABLE
23	#2 AWG MIN., 600 V., POWER CABLE *
24	#2 AWG MIN., 600 V., GROUND CABLE
*	SEE PLANS FOR SIZE

NOTES

- B)LIGHTING SYSTEM VOLTAGE AS SPECIFIED ON PLANS (120 OR 240 VOLTS).
 IF LIGHTS ARE TO BE MOUNTED ON SIGNAL POSTS, VOLTAGE MUST BE 120 VOLTS. 240 VOLT LIGHTS ARE NOT TO BE MOUNTED ON OR WIRED THROUGH SIGNAL POSTS, CONDUIT, OR PULL BOXES.
- C)LOCATE PHOTOELECTRIC SWITCH AT CENTER OF WINDOW.
- $\stackrel{\textstyle (E)}{\textstyle \text{E}}$ if for reasons of voltage drop a wire size is specified larger than #2 awg, a larger terminal block shall be installed for that circuit.
- (F)BREAKER SIZING:

	240V TOTAL	120V TOTAL
SIZE (AMPS)	CIRCUIT LOAD (WATTS)	CIRCUIT LOAD (WATTS)
15	0-2800	0-1390
20	2850-3700	1400-1850
25	3750-4600	1860-2320
30	4650-5500	2330-2780
35	5550-6500	_
40	6550-7400	-

CIRCUIT LOAD INCLUDES LOAD DUE TO LINE LOSS, LAMP, AND BALLAST LOAD.

GALL CIRCUIT BREAKERS SHALL CONFORM TO SECTION 901.4 OF THE STANDARD SPECIFICATIONS.

> "MoDOT" & "CITY" 120/240v COMBINATION PAD MOUNTED POWER SUPPLY & LIGHTING CONTROL STATION DUAL METER

> > LIGHTING PLAN 3 OF 3

RIGID STEEL CONDUIT. NEMA 3R, DUST-TIGHT. GROUND ROD. 3/4" DI PHOTGELECTRIC SWITCH PLIABLE SEALANT LIFETIME SILICONE CA INSULATED TERMINAL B 2-POLE. 35 AMP. 120V 16 2-POLE. 650 VOLT LIG THE MANUAL-AUTO SWITCH INSULATED GROUNDABLE 2-POLE. 15 AMP. TYPE 19 INSULATED GROUNDABLE 2-POLE. 15 AMP. (MIN)	A. X 8' MIN. (MIN. 8' IN SOIL) I AND SOCKET, 105/285 V., 1000 WATT ULK
2 RIGID STEEL CONDUIT. 4 NEMA 3R, DUST-TIGHT. 5 GROUND ROD. 3/4" DI 6 PHOTOELECTRIC SWITCH 10 PLIABLE SEALANT 11 LIFETIME SILICONE CA 14 INSULATED TERMINAL E 15 2-POLE. 35 AMP, 120V 16 2-POLE. 650 VOLT LIG 17 1-POLE. 15 AMP, TYPE 18 MANUAL-AUTO SWITCH 19 INSULATED GROUNDABLE 20 2-POLE. 15 AMP, (MIN) 21 2-POLE. 15 AMP (MIN)	2" MIN. TO UTILITY CO. FACILITIES WATERTIGHT CABINET A. X 8' MIN. (MIN. 8' IN SOIL) I AND SOCKET, 105/285 V., 1000 WATT
4 NEMA 3R, DUST-TIGHT, 5 GROUND ROD, 3/4" DI 6 PHOTOELECTRIC SWITCH 10 PLIABLE SEALANT 11 LIFETIME SILICONE CA 14 INSULATED TERMINAL B 15 2-POLE, 35 AMP, 120V 16 2-POLE, 650 VOLT LIG 17 1-POLE, 15 AMP, TYPE 18 MANUAL-AUTO SWITCH 19 INSULATED GROUNDABLE 20 2-POLE, 15 AMP, (MIN) 21 2-POLE, 15 AMP (MIN)	WATERTIGHT CABINET A. X 8' MIN. (MIN. 8' IN SOIL) H AND SOCKET, 105/285 V., 1000 WATT BULK BLOCK
5 GROUND ROD. 3/4" DI 6 PHOTOELECTRIC SWITCH 10 PLIABLE SEALANT 11 LIFETIME SILICONE CA 14 INSULATED TERMINAL B 15 2-POLE. 35 AMP. 120V 16 2-POLE. 650 VOLT LIG 17 1-POLE. 15 AMP. TYPE 18 MANUAL-AUTO SWITCH 19 INSULATED GROUNDABLE 20 2-POLE. 15 AMP. TWP 21 2-POLE. 15 AMP (MIN)	A. X 8' MIN. (MIN. 8' IN SOIL) H AND SOCKET, 105/285 V., 1000 WATT MULK BLOCK
6 PHOTOELECTRIC SWITCH 10 PLIABLE SEALANT 11 LIFETIME SILICONE CA 14 INSULATED TERMINAL E 15 2-POLE, 35 AMP, 120V 16 2-POLE, 650 VOLT LIG 17 1-POLE, 15 AMP, TYPE 18 MANUAL-AUTO SWITCH 19 INSULATED GROUNDABLE 20 2-POLE, 100 AMP, TYP 21 2-POLE, 15 AMP (MIN)	H AND SOCKET, 105/285 V., 1000 WATT NULK BLOCK
11 LIFETIME SILICONE CA 14 INSULATED TERMINAL B 15 2-POLE, 35 AMP, 120V 16 2-POLE, 65 OVIT LIG 17 1-POLE, 15 AMP, TYPE 18 MANUAL-AUTO SWITCH 19 INSULATED GROUNDABLE 20 2-POLE, 100 AMP, TYP 21 2-POLE, 15 AMP (MIN)	BLOCK
14 INSULATED TERMINAL B 15 2-POLE. 35 AMP. 120V 16 2-POLE. 650 VOLT LIG 17 1-POLE. 15 AMP. TYPE 18 MANUAL-AUTO SWITCH 19 INSULATED GROUNDABLE 20 2-POLE. 100 AMP. TYP 21 2-POLE. 15 AMP (MIN)	BLOCK
15 2-POLE. 35 AMP. 120V 16 2-POLE. 650 VOLT LIG 17 1-POLE. 15 AMP. TYPE 18 MANUAL-AUTO SWITCH 19 INSULATED GROUNDABLE 20 2-POLE. 100 AMP. TYP 21 2-POLE. 15 AMP (MIN)	====
16 2-POLE. 650 VOLT LIG 17 1-POLE. 15 AMP. TYPE 18 MANUAL-AUTO SWITCH 19 INSULATED GROUNDABLE 20 2-POLE. 100 AMP. TYP 21 2-POLE. 15 AMP (MIN)	COIL LIGHTING CONTACTOR (MERCURY)
17 1-POLE. 15 AMP. TYPE 18 MANUAL-AUTO SWITCH 19 INSULATED GROUNDABLE 20 2-POLE. 100 AMP. TYP 21 2-POLE. 15 AMP (MIN)	
18 MANUAL - AUTO SWITCH 19 INSULATED GROUNDABLE 20 2-POLE, 100 AMP, TYP 21 2-POLE, 15 AMP (MIN)	SHTNING ARRESTER
19 INSULATED GROUNDABLE 20 2-POLE, 100 AMP, TYP 21 2-POLE, 15 AMP (MIN)	B BREAKER TO PHOTOELECTRIC SWITCH
20 2-POLE, 100 AMP, TYP 21 2-POLE, 15 AMP (MIN)	
21 2-POLE, 15 AMP (MIN)	NEUTRAL, 100 AMP
	PE A MAIN BREAKER
00 #40 AWO MIN COO V	, TYPE A BREAKERS *
22 #12 AWG MIN., 600 V.	, CONTROL CABLE
23 #2 AWG MIN., 600 V.,	DOWED CARLE W
24 #2 AWG MIN. 600 V.	PUWER CABLE *
* SEE PLANS FOR SIZE	·

REAR VIEW (SERVICE ENTRANCE)	′ U SIDE VIEW EQUIPMENT LAYOUT	(FOR MODOT) -CONDUIT OUT TO FUTURE IRRIGATION FRONT V (FOR CITY) (CONTROL	'IEW SIDE)
METER SOCK!			
CABINET (4)	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	NO 0 C1 C2 C2 C3	> (15) B MERCURY CONTACTOR COILS
PHOTO. CONTROL BREAKER 17 G (14)		LO C4	—— (18) G MANUAL/AUTO SWITCH
MAIN BREAKER (20) G		OL1 N	- 6 C PHOTOELECTRIC SWITCH
LIGHTNING ARRESTER (16)	N N N N N N N N N N N N N N N N N N N	27	—(14)
BREAKERS F G 21		€ €	
	C12 C22	C3- C4-	(15) B) MERCURY CONTACTORS (LIGHTING)
DUPLEX 120 VOLT GFI OUTLET (FOR SERVICE USE) OUTPUT TERMINAL (14) E	OTIO OTIO LIGHTING LIGHTING		——(14)E)OUTPUT TERMINALS
(23)	2 Elostro-trocosco Fostocosco NEUTRAL SERVICE	12 20000000 GROUND	
FROM UTILITY CO. (REF.)	19 (5)	24	AWG SIZE
	120/240V 60 HZ. 3 WIRE SINGLE PHASE POWER INPUT WIRING DIAG	RAM	WIRE

(1)(C)

WIRE WAY FROM SERVICE— ENTRANCE SIDE TO CONTROL SIDE

GFI OUTLET BREAKER (G)(17)

BREAKERS: 1 - LIGHTING CIR. 1

FRONT DOOR

CABINET (4)

MAIN G 20

F G 21-

SEAL (10)

SEAL(11)-

-CONDUIT OUT TO ITS (FOR MODOT)

120 VOLT OUTLET

(14)-

(1)(C)

(16)LIGHTNING ARRESTER

-(10)SEAL

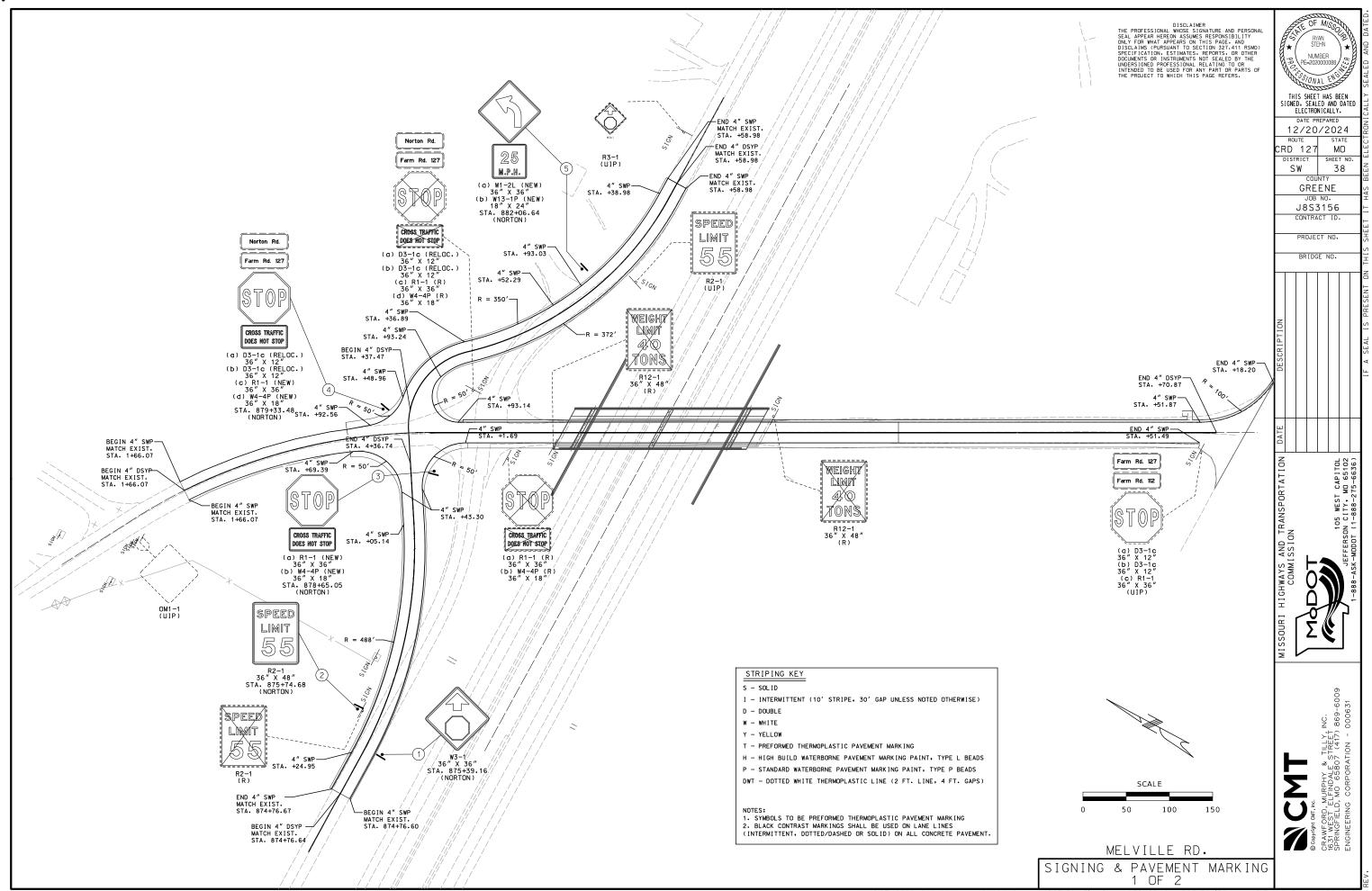
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NUMBER PE-202000008

THIS SHEET HAS BEEN SIGNED, SEALED AND DATE ELECTRONICALLY.

12/20/2024



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		SIC	SNS				PERFORATED SQUARE STEEL TUBE												
902 SIGNAL SIGNS TABULATED ON D37-A SHEET							2 IN. POST ANCHORS					2.5 IN. POST				ANCHORS BREAK-			
	902 3131	AL SIGNS TAB	HORIZ	IST-A SHEET	SIGN	POST NO.1	POST NO.2	TOTAL	DRIVEN 12-GA.	DRIVEN 7-GA.	CONCRETE 7-GA.	POST NO.1	POST NO.2	TOTAL	2.25" INSERT (6 FT)	DRIVEN 7-GA.	CONCRETE	AWAY	REMARKS AND OTHER REQUIRED ITEMS
SIGN NO.	SIGN S i ze	STATION	CLEAR IF NOT STD	LOCATION	DTL. SHT. NO.			ITEM NO. 9031270A	ITEM NO. 9031271A	ITEM NO. 9031273A	ITEM NO. 9031274			ITEM NO. 9031280	ITEM NO. 9031272A	ITEM NO. 9031281A	ITEM NO. 9031285	ITEM NO. 9031241	
						LF	LF	LF	EA	EA	EA	LF	LF	LF	EA	EA	EA	EA	
1	36"X36"	874+62.49	-	NORTON	-	-	-	•				16	-	16.00		-	1	1	
2	36"X48"	874+98.21		NORTON	-	-	-	•	•			16	-	16.00	1		1	1	
3	ASSEMBLY-2	877+88.68		NORTON	-		-	•	•	-	•	16	-	16.00	1		1	1	
4	ASSEMBLY-4	878+57.11	-	NORTON	-	-	-	•	-		-	16	-	16.00	1	-	1	1	
5	ASSEMBLY-2	882+06.64	-	NORTON	-		-	-				16	-	16.00	1	-	1	1	
	SUBTOTAL			AL			-	•	-				80	4	-	5	5		
				TOTAL				•	•	-	•			80.00	4.00	-	5.00	5.00	

						STA	NDAR	D SIGN			ES						T
				ТҮРЕ													
ŒR.	STATION LOCATION		Street Name	'0P	LI	eed mit 55					DOES R	TRAFFIC HOT STOP		35 •.H			
Ĭ				SIGN DESCRIPTION, SIZES & NUMBER OF EACH									DE114 DI/O				
SIGN NUMBER			D3-1c	(36" X 12")	R1-1	(36" X 36")	R2-1	(36" X 48")	W1-2L	(36" X 36")	W3-1	(36" X 36")	W4-4P	(36" X 18")	W13-1P	(18" X 24")	REMARKS
1	874+62.49	NORTON		-		-		-		•		1		-		-	
2	874+98.21	NORTON		-		-		1		•		•		-		-	
3	877+88.68	NORTON		-		1		-		-		-		1		-	
4	878+57.11	NORTON		2		1		-		-		-		1		-	D3-1c TO BE RELOCATED
5	878+57.11	NORTON		-		-				1		-		-		1	

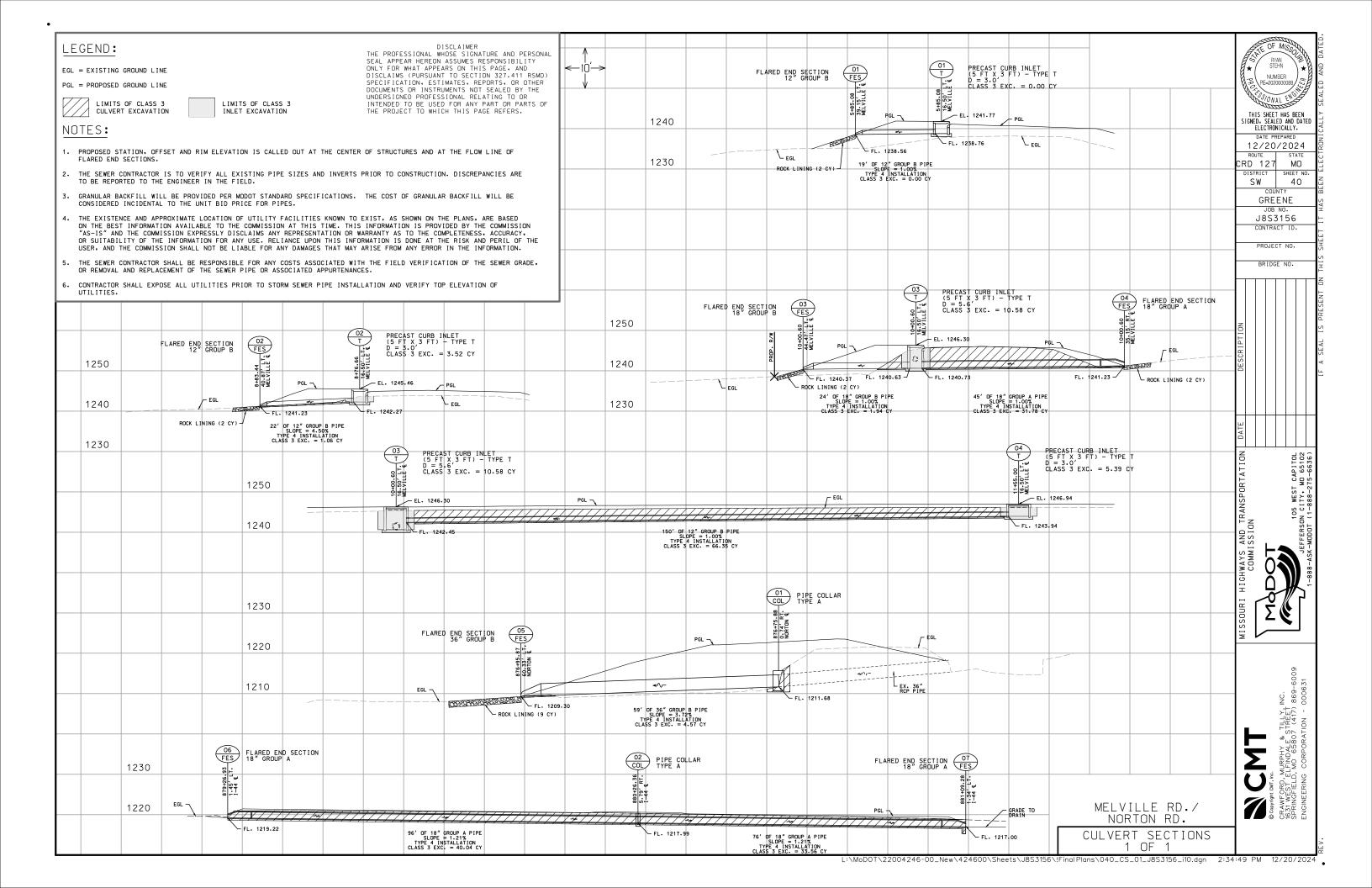
				SIGN SUMMAF	RY							
				SIZE, TYPE & SQUARE FEET								
STANDARD SIGN OR SPECIAL SIGN NUMBER	SIGN DETAIL SHEET NO.	NO. EACH	SIZE (SQFT)	FLAT SHEET SH	FLAT SHEET FLOURESCENT SHF *	STRUCTURAL STRUCTURAL	STRUCTURAL FLOURESCENT STF*	RELOCATED SIGNS (SQFT)				
				ITEM NO.	ITEM NO.	ITEM NO.	ITEM NO.	ITEM NO.				
				9035004A	9035069A	9035011A	9035071A	9039904				
R1-1 - 3, 4	-	2	9.00	18.00	-	-	-	-				
R2-1 - 2	-	1	12,00	12,00	-	-	-	-				
W1-2L - 5	-	1	9.00	9.00	-	-	·					
W3-1 - 1	-	1	9,00	9.00	•	-	-	-				
W4-4P - 3, 4	-	2	4.50	9.00	•	-	•	•				
W13-1P - 5	·	1	3,00	3.00	•	-	•	-				
D3-1c - 4	-	2	3.00	•	-	-	•	6.00				
	TOTAL	10		60.00	-	-	-	6.00				

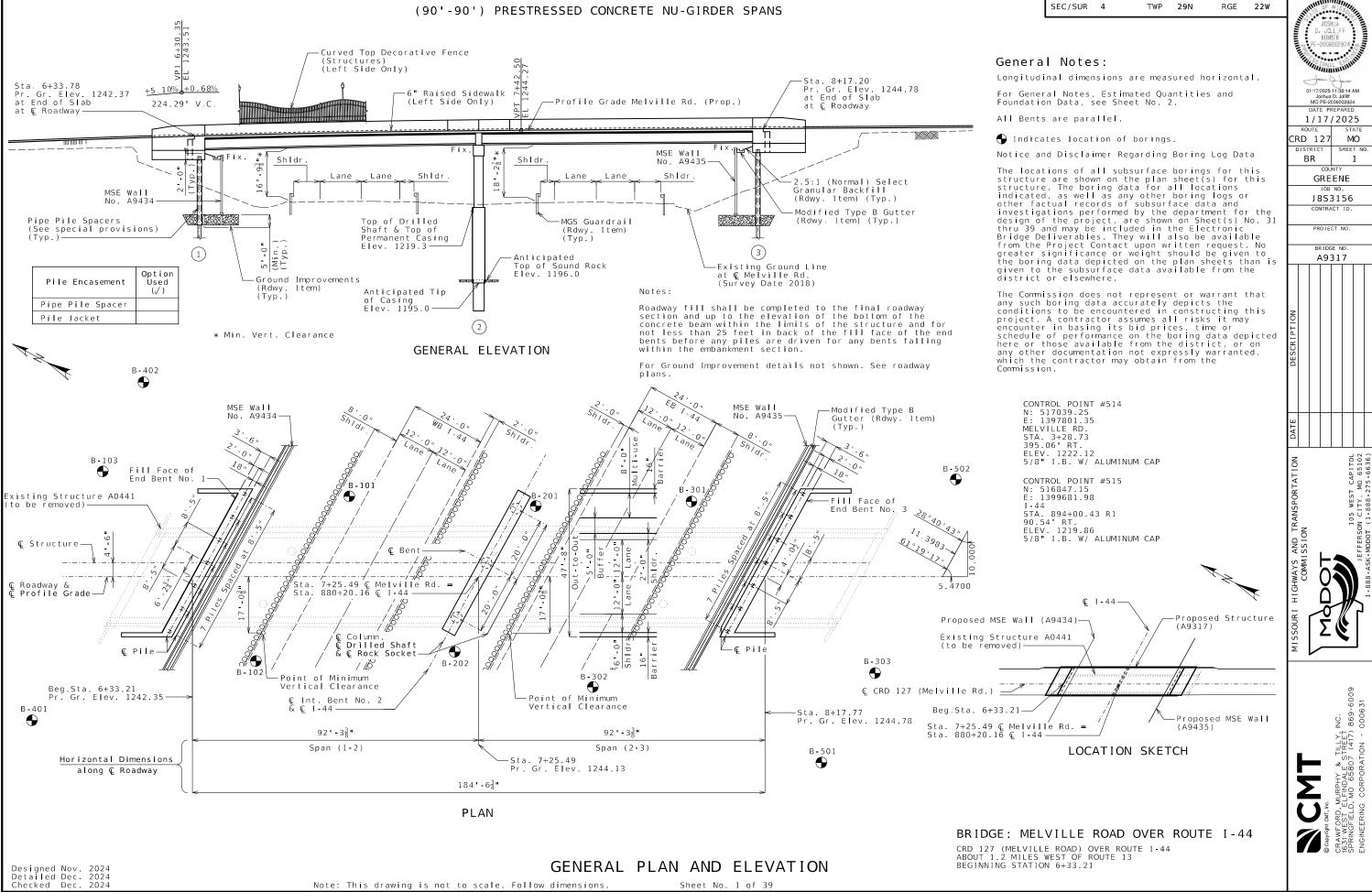
THIS SHEET HAS BEEN SIGNED. SEALED AND DATED ELECTRONICALLY.

DATE PREPARED 12/20/2024 ROUTE STATE
CRD 127 MO
DISTRICT SHEET NO.
SW 39 COUNTY GREENE JOB NO.
J8S3156
CONTRACT ID. PROJECT NO. BRIDGE NO. HIGHWAYS AND TRANSPORTATION COMMISSION

RYAN STEHN NUMBER PE-2020000088

D-29 & D-30





General Notes:

Design Specifications: 2020 AASHTO LRFD Bridge Design Specifications (9th Ed.) Seismic Design Category = A Design earthquake response spectral acceleration coefficient at 1.0 second period, $S_{\rm D1}=0.103$

coefficient at 1.0 second period, $S_{D1} = 0.103$ Acceleration Coefficient (effective peak ground acceleration coefficient), $A_{S} = 0.088$

Design Loading:

Vehicular = HL-93
Future Wearing Surface = 35 lb/sf
Earth = 120 lb/cf
Equivalent Fluid Pressure = 45 lb/cf (Min.)
Superstructure: Simply-Supported, Non-composite for dead load.
Continuous Composite for live load.

Design Unit Stresses:

Class B Concrete (Substructure)	f'c = 3,000 psi
Class B-2 Concrete (Drilled Shaft and Rock Sockets)	f c = 4,000 psi
Class B-2 Concrete (Superstructure, except Prestressed	f'c = 4,000 psi
NU-Girders and Barrier)	
Class B-1 Concrete (Barrier)	f'c = 4,000 psi
Reinforcing Steel (ASTM A615 Grade 60)	fy = 60,000 psi
Structural Steel HP Pile (ASTM A709 Grade 50)	fv = 50,000 nsi

For prestressed panel stresses, see Sheet No. 15.

For prestressed girder stresses, see Sheets No. 12 & 13.

Neoprene Pads:

Neoprene bearing pads shall be 60 durometer and shall be in accordance with Sec 716.

Joint Filler:

All joint filler shall be in accordance with Sec 1057 for preformed sponge rubber expansion and partition joint filler, except as noted.

Reinforcing Steel:

Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise shown.

Minimum clearance between galvanized piles and uncoated (plain) reinforcing steel including bar supports shall be 1 1/2". Nylon, PVC, or polyethylene spacers shall be used to maintain clearance. Nylon cable ties shall be used to bind the spacers to the reinforcement.

Traffic Handling:

Structure to be closed during construction. Traffic to be maintained on other routes during construction. See roadway plans for traffic control.

Vertical clearance for Interstate 44 traffic during construction shall be 15'-6" minimum over a 24'-0" wide horizontal opening of the roadway in each direction.

Miscellaneous

MoDOT Construction personnel will indicate the type of joint filler option used under the precast panels for this structure:

- Constant Joint Filler
- ☐ Variable Joint Filler

Estimated Quantities for Slab on Concrete NU-Girder

I t em		Total
Class B-2 Concrete	cu. yard	295.1
Reinforcing Steel (Epoxy Coated)	pound	75,820

The table of Estimated Quantities for Slab on Concrete NU-Girder represents the quantities used by the State in preparing the cost estimate for concrete slabs. The area of the concrete slab will be measured to the nearest square yard longitudinally from end of slab to end of slab and transversely from out to out of bridge slab (or with the horizontal dimensions as shown on the plan of slab). Payment for prestressed panels, conventional forms, all concrete and epoxy coated reinforcing steel will be considered completely covered by the contract unit price for the slab. Variations may be encountered in the estimated quantities but the variations cannot be used for an adjustment in the contract unit price.

Method of forming the slabs shall be as shown on the plans and in accordance with Sec 703. All hardware for forming the slab to be left in place as a permanent part of the structure shall be coated in accordance with ASTM A123 or ASTM B633 with a thickness class SC 4 and a finish type I, II or III.

The Estimated Quantities for Slab on Concrete NU-Girder are based on skewed precast prestressed end panels.

Class B-2 Concrete quantity is based on minimum top flange thickness and minimum joint material thickness.

The prestressed panel quantities are not included in the table of Estimated Quantities for Slab on Concrete NU-Girder.

Estimated Quantities					
I t em		Substr.	Superstr.	Total	
Removal of Miscellaneous ACM (Non-Friable)	sq. foot	=	-	21	
Removal of Bridges	lump sum	-	-	1	
Bridge Approach Slab (Minor)	sq. yard	-	204	204	
Curved Top Decorative Fence (Structures)	linear foot	-	208	208	
Drilled Shafts (4 ft. 0 in. Dia.)	linear foot	69.9	-	69.9	
Rock Sockets (3 ft. 6 in. Dia.)	linear foot	33.0	-	33.0	
Video Camera Inspection	each	3	-	3	
Foundation Inspection Holes	linear foot	63.0	-	63.0	
Sonic Logging Testing	each	3	-	3	
Galvanized Structural Steel Piles (12 inch)	linear foot	525	-	525	
Pre-Bore for Piling	linear foot	252	-	252	
Pile Point Reinforcement	each	14	-	14	
Class B Concrete (Substructure)	cu. yard	110.6	-	110.6	
Aesthetic Concrete Stain	lump sum	-	1	1	
Type D Barrier	linear foot	-	435	435	
Slab on Concrete NU-Girder	sq. yard	-	972	972	
Sidewalk (Bridges)	sq. foot	-	2,905	2,905	
Stamped Concrete Pattern	sq. foot	-	1,006	1,006	
NU 43, Prestressed Concrete NU-Girder	linear foot	-	903	903	
Reinforcing Steel (Bridges)	pound	5,170	-	5,170	
Conduit System on Structure	lump sum	-	1	1	
Reinforcing Steel (Epoxy Coated)	pound	27,420	-	27,420	
Vertical Drain at End Bents	each	2	-	2	
Plain Neoprene Bearing Pad	each	-	5	5	
Laminated Neoprene Bearing Pad (Tapered)	each	-	15	15	
Pipe Pile Spacers	each	14	-	14	
			i I		

All concrete above the construction joint in the end bents is included in the Estimated Quantities for Slab on Concrete NU-Girder.

All reinforcement in the end bents is included in the Estimated Quantities for Slab on Concrete NU-Girder.

All reinforcement in the intermediate bent concrete diaphragms except reinforcement embedded in the beam cap is included in the Estimated Quantities for Slab on Concrete NU-Girder.

All concrete above the intermediate beam cap is included in the Estimated Quantities for Slab on Concrete NU-Girder.

All concrete and reinforcing steel in sidewalk will be considered completely covered by the contract unit price for Sidewalk (Bridges).

Foundation Data Bent Number Design Data 3 Туре HP 12x53 HP 12×53 Pile Type and Size Number Approximate Length Per Each 41 34 ft Load Pile Point Reinforcement AH ALL Bearing Min. Galvanized Penetration (Elev.) ft Full length ull length Min. Tip Penetration (Elev.) f t 1207 1205 Pile Driving Verification Method Resistance Factor Minimum Nominal Axial Compressive Resistance kir 732 731 Number Foundation Material Strona rocl Elevation Range 1193-1185 Socket ov Minimum Nominal Axial Compressive Resistance (Side Resistance) 14.9 Minimum Nominal Axial Compressive Resistance (Tip Resistance) ksf 400

Notes:

Load Bearing Pile

Minimum Nominal Axial Compressive Resistance = $\frac{\text{Maximum Factored Loads}}{\text{Resistance Factor}}$ (0.35)

Rock Socket (Drilled Shaft)

Minimum Nominal Axial Compressive Resistance = <u>Maximum Factored Loads</u>
(Side Resistance + Tip Resistance)
Resistance Factors

st Prebore for piles at Bent No. 1 to bear on competent rock.

Prebore for piles at Bent No. 3 to elevation 1205.

The prebore hole shall be cleared to meet the requirements of Sec 701.4.10.2. The piles shall be inserted into the prebore holes and are to be seated on bedrock and not rubble in the bottom of the hole. Once the piles are seated, the minimum nominal compressive resistance shall be considered verified. Verification of the pile driving is not required. Backfill the prebore hole with sand or other approved material.

All piles shall be galvanized down to the minimum galvanized penetration (elevation).

Pile point reinforcement need not be galvanized. Shop drawings will not be required for pile point reinforcement.

The contractor shall make every effort to achieve minimum galvanized penetration (elevation) shown on the plans for all piles. Deviation in penetration less than 5 feet minimum will be considered acceptable provided the contractor makes the necessary corrections to ensure the minimum penetration is achieved in subsequent piles.

ESTIMATED QUANTITIES AND GENERAL NOTES

Detailed Dec 2024 Checked Dec 2024

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 2 of 39

MOST. INC.

FORD, MURPHY & TILLY, INC.

WEST ELFINDALE STREET

WEST ELFINDALE STREET

WEST CORPORATION - 000631

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GREENE

J8S3156

CONTRACT ID

PROJECT N

A9317

MO

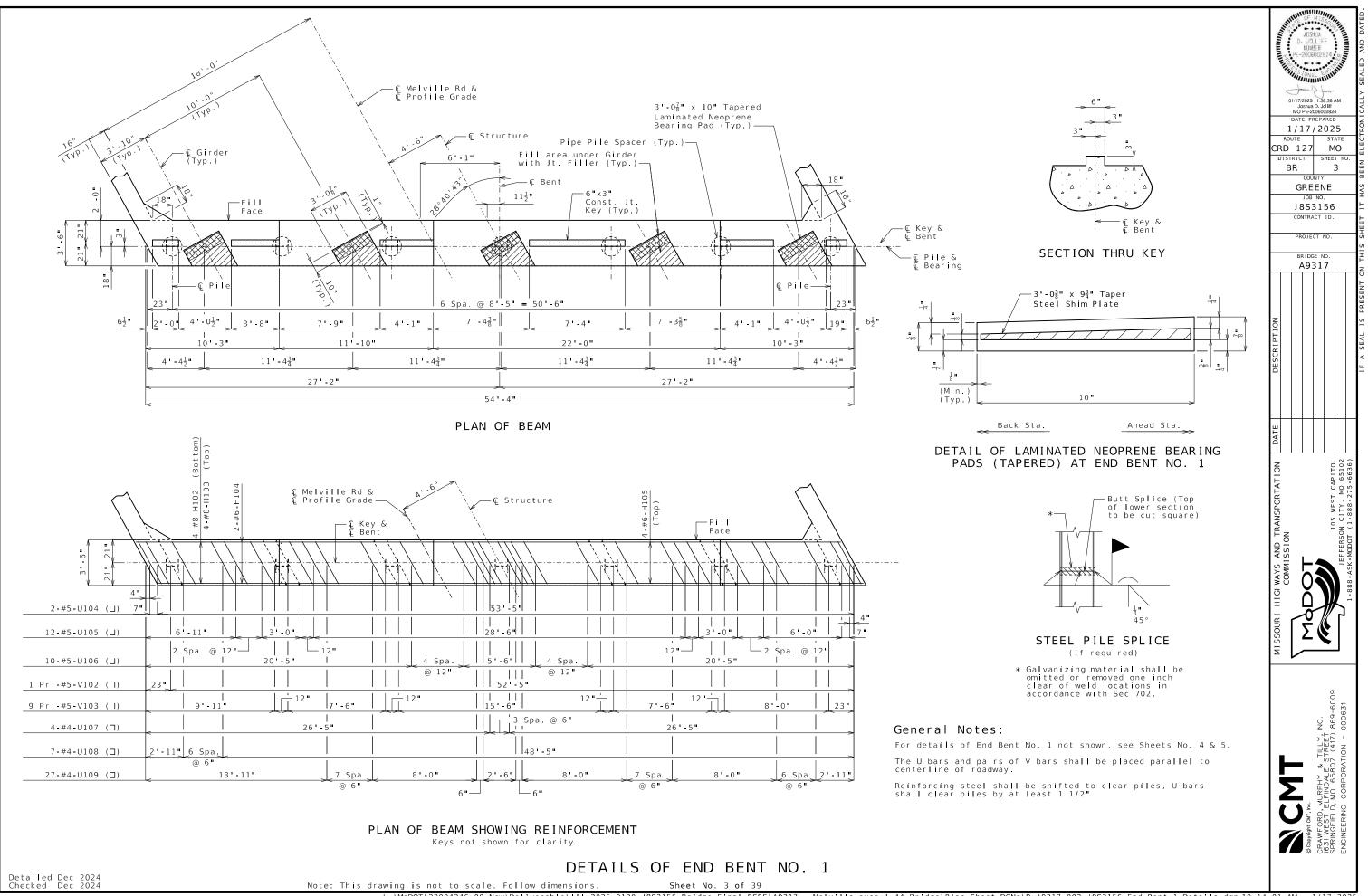
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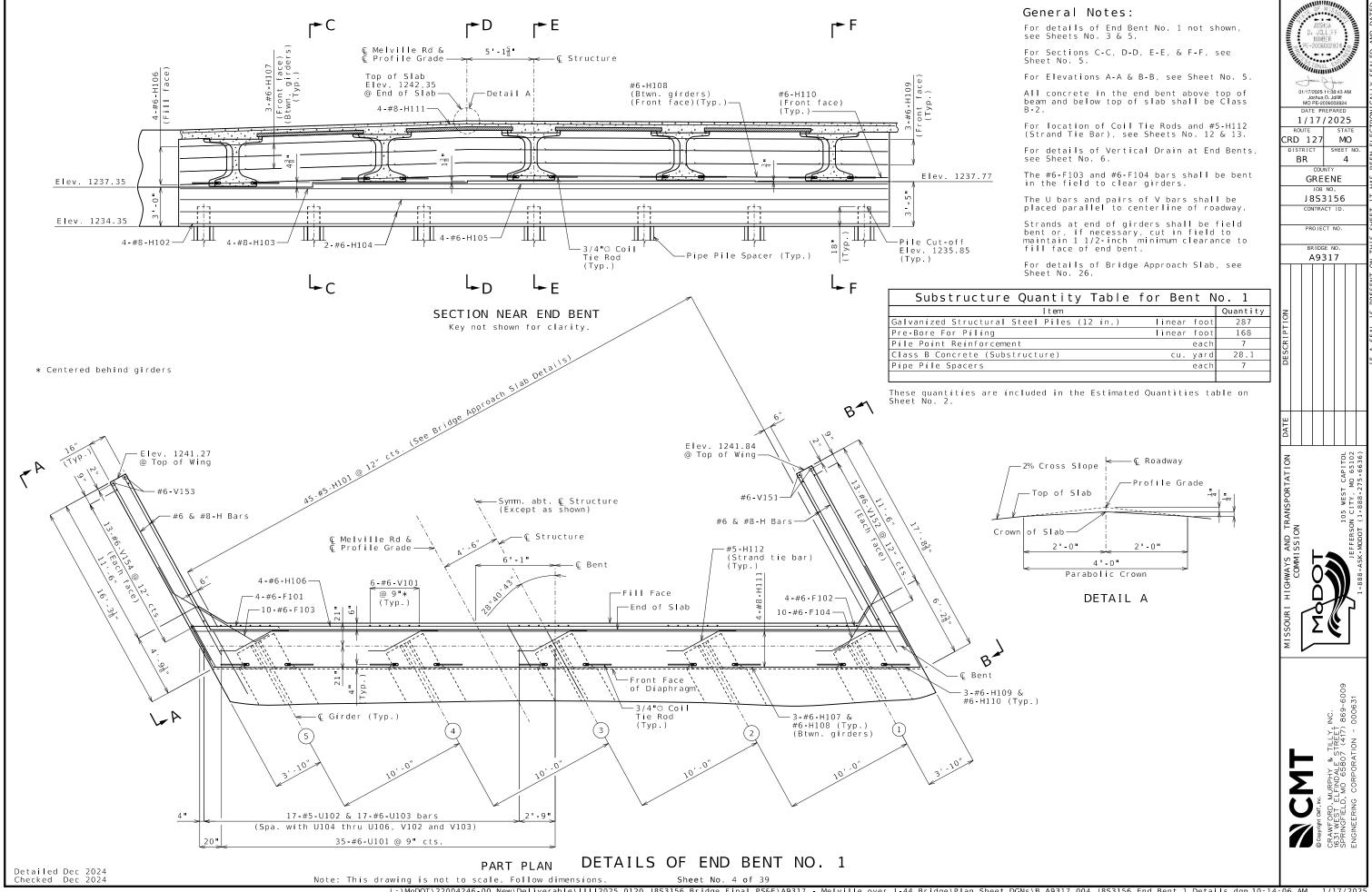
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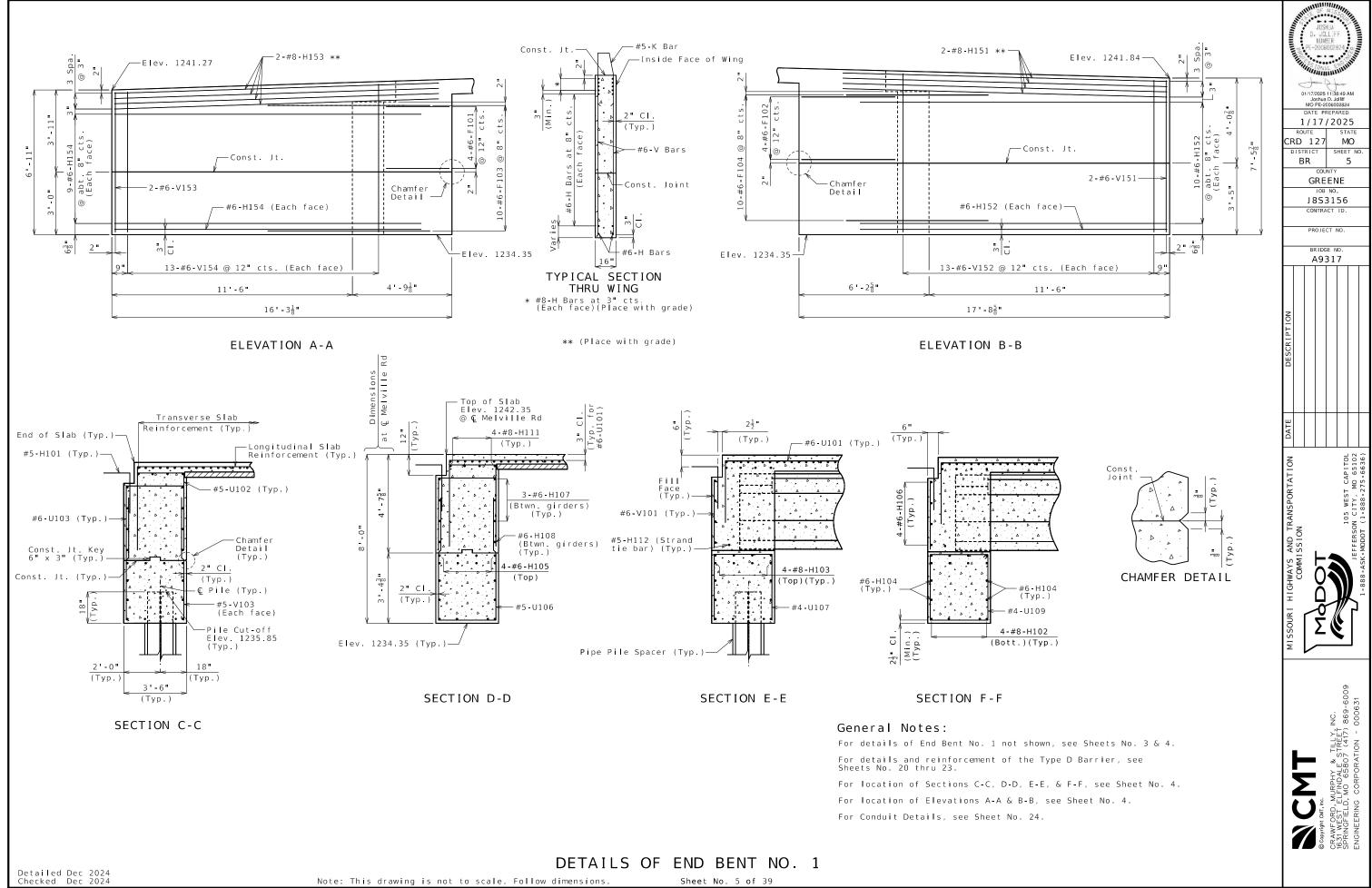
CRD 127

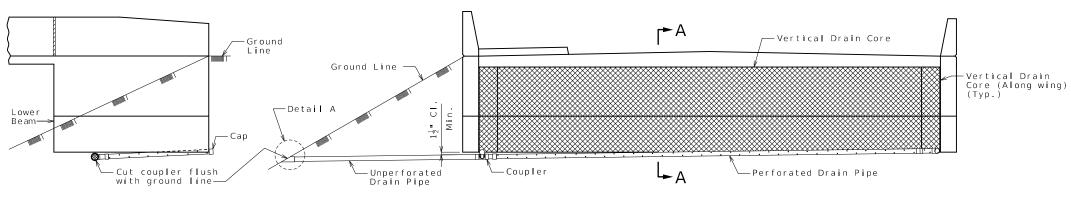
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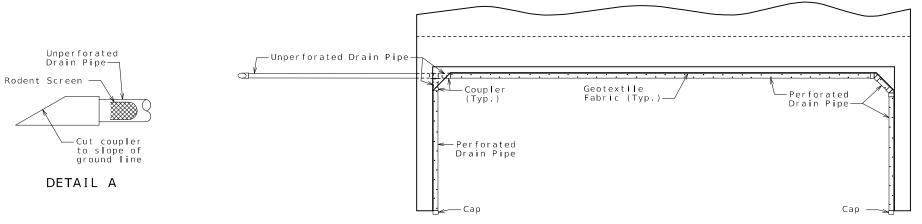




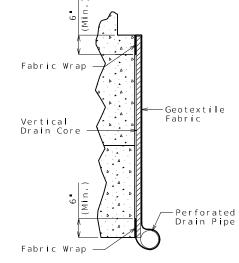
ELEVATION OF WING

Detailed Dec 2024 Checked Dec 2024

ELEVATION OF END BENT



PLAN OF END BENT



PART SECTION A-A (Section thru wing similar)

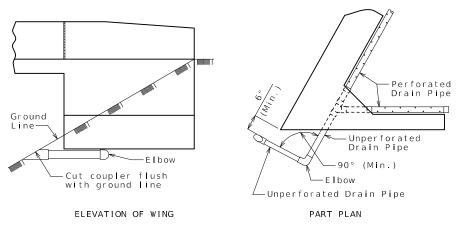
General Notes:

percent.

All drain pipe shall be sloped 1 to 2

Drain pipe may be either 6-inch diameter corrugated metallic-coated steel pipe underdrain, 4-inch diameter corrugated polyvinyl chloride (PVC) drain pipe, or 4-inch diameter corrugated polyethylene (PE)

Perforated pipe shall be placed at fill face side and inside face of wings at the bottom of end bent and plain pipe shall be used where the vertical drain ends to the exit at ground line.



OPTIONAL TURNED DRAIN

(Use only when straight drain is not practical.)

Note: Tie unperforated Drain Pipe into Type B Gutter located behind the MSE walls A9434 and A9435.

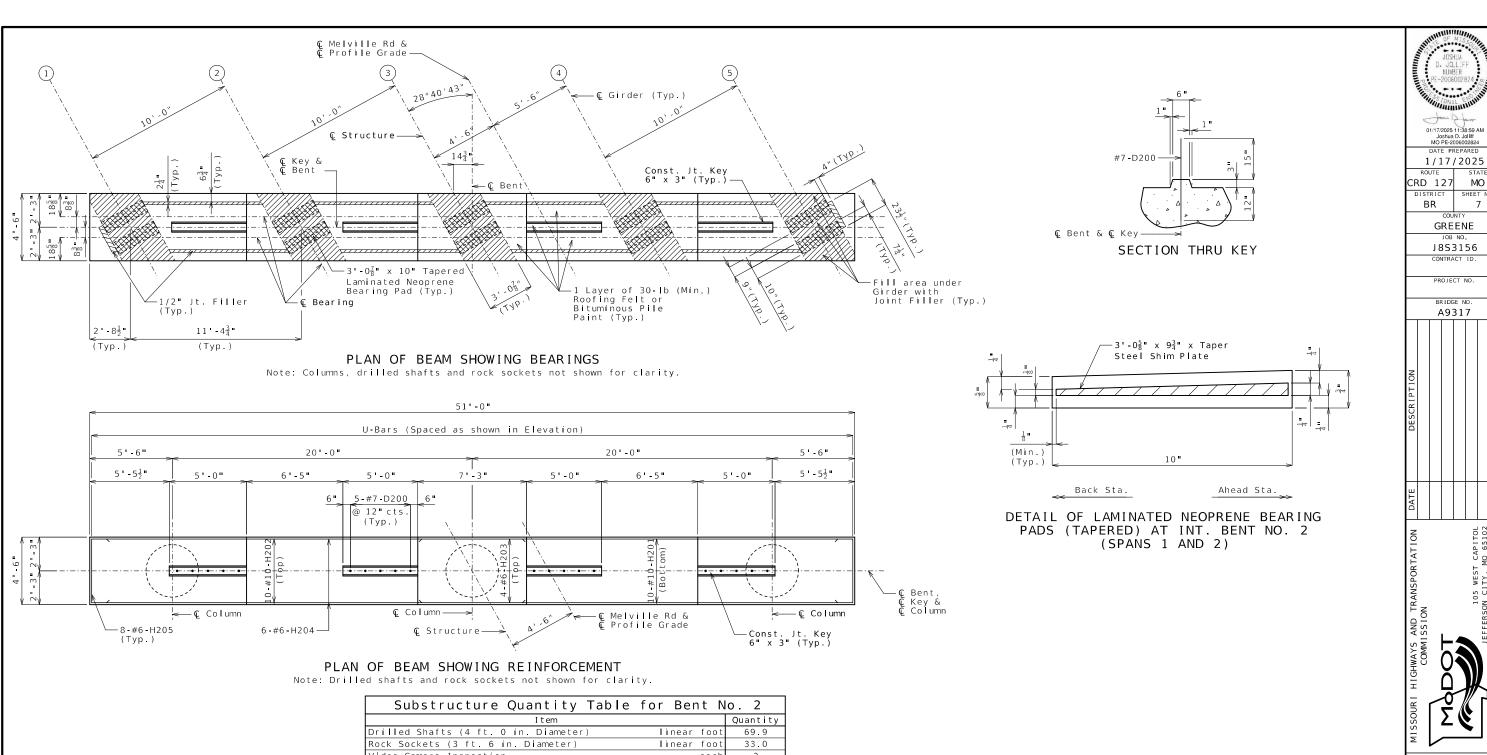
VERTICAL DRAIN AT END BENTS

(Squared end bent shown, skewed end bent similar)

1/17/2025 CRD 127 MO DISTRICT SHEET NO BR 6 GREENE J8S3156 CONTRACT ID. PROJECT NO A9317 ORD, MURPHY & TILLY, INC. EST ELFINDALE STREET FIELD, MO 65807 (417) 869 ERING CORPORATION - 0006 Drain pipe shall be placed at fill face of end bent and inside face of wings. The pipe shall slope to lowest grade of ground line, also missing the lower beam of end bent by a minimum of 1 1/2 inches.

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 6 of 39



	Substructure Quantity Table	for Bent N	o. 2
	I t em		Quantity
	Drilled Shafts (4 ft. O in. Diameter)	linear foot	69.9
	Rock Sockets (3 ft. 6 in. Diameter)	linear foot	33.0
	Video Camera Inspection	each	3
*	Foundation Inspection Holes	linear foot	63.0
	Sonic Logging Testing	each	3
	Class B Concrete (Substructure)	cu. yard	55.9
	Reinforcing Steel (Bridges)	pound	5,170
	Reinforcing Steel (Epoxy Coated)	pound	27,420

Notes:

Detailed Dec 2024 Checked Dec 2024

These quantities are included in the Estimated Quantities table on Sheet No. 2.

All reinforcement in drilled shafts and rock sockets is included in the substructure quantities $% \left(1\right) =\left\{ 1\right\} =\left\{$

* Foundation Inspection Holes shall be drilled at least 15 days prior to drilled shaft construction in accordance with Sec 701.4.11.

DETAILS OF INTERMEDIATE BENT NO. 2

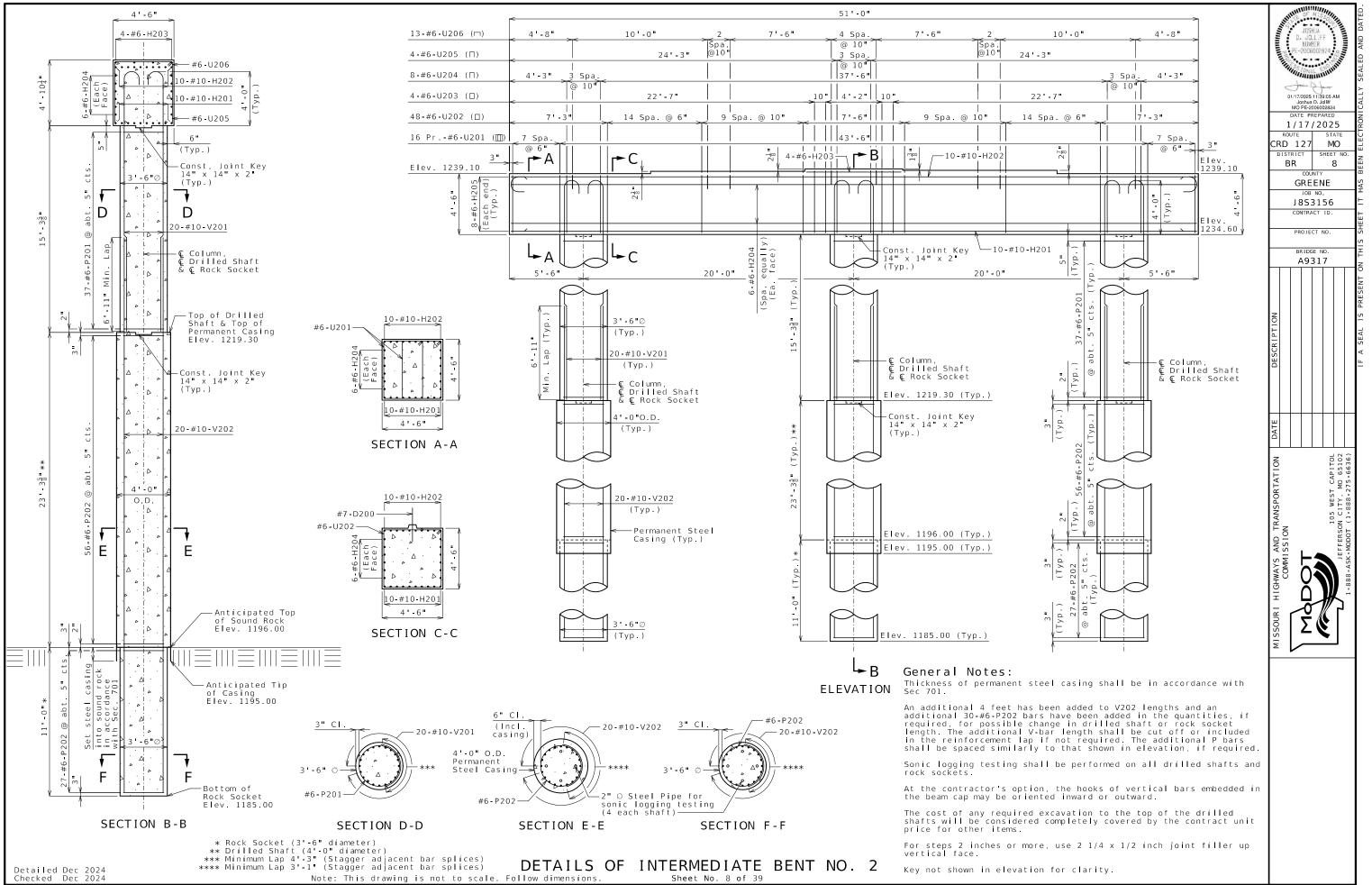
Sheet No. 7 of 39

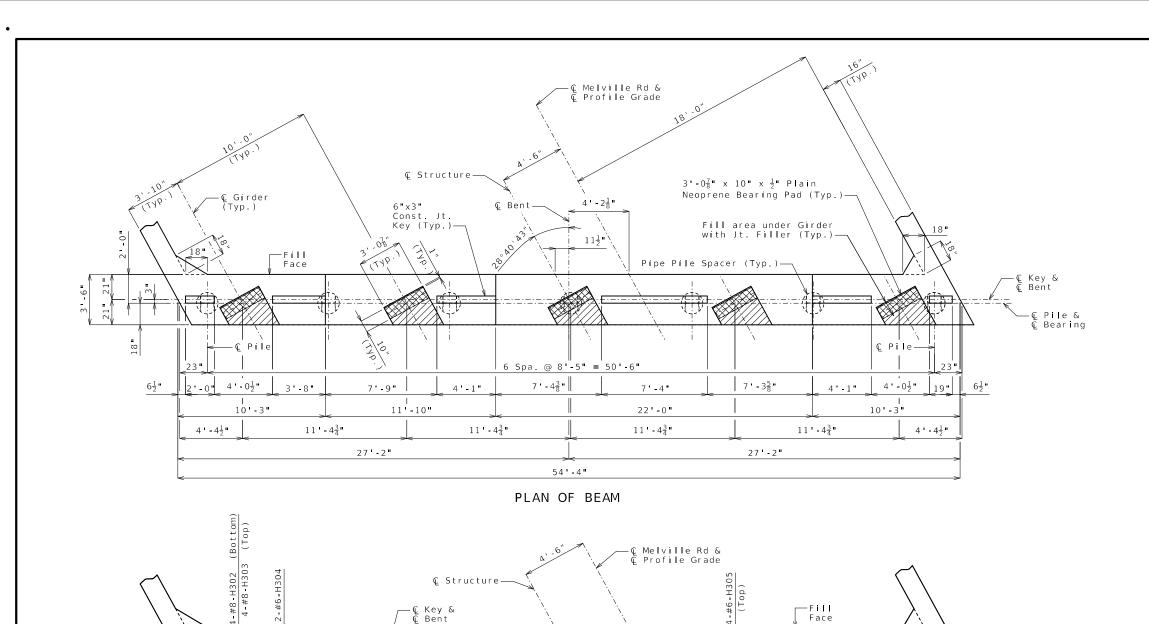
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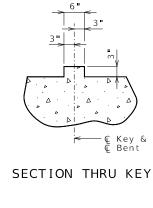
General Notes:

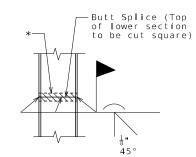
For details of Intermediate Bent No. 2 not shown, see Sheet No. 8.

For steps 2 inches or more, use 2 $1/4 \times 1/2$ inch joint filler up vertical face.









STEEL PILE SPLICE (If required)

st Galvanizing material shall be omitted or removed one inch clear of weld locations in accordance with Sec 702.

PLAN OF BEAM SHOWING REINFORCEMENT

Keys not shown for clarity.

@ 12"

L 12"

General Notes:

For details of End Bent No. 3 not shown, see Sheets No. 10 & 11.

The U bars and pairs of V bars shall be placed parallel to centerline of roadway.

Reinforcing steel shall be shifted to clear piles, U bars shall clear piles by at least 1 1/2".

DETAILS OF END BENT NO. 3

12"—

26'-5"

7 Spa.

@ 6

12"—

└ 2 Spa. @ 12

6 Spa. 2'-11"

@ 6"

8 -0"

13'-11"

Detailed Dec 2024 Checked Dec 2024

10 Pr.-#5-V302 (II)

4-#4-U305 (□)

22-#4-U306 (D)

12-#4-U307 (□)

2 Spa. @ 12"—

2'-11<u>"</u> 6 Spa

@ 6"

L 12"

26'-5"

25'-5"

7 Spa.

@ 6"

— 12**"**

@ 12"

Spa. @ 6" 12"—

31'-0"

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 9 of 39 L:\MoDOT\22004246-00_New\Deliverable\!!!!2025 0120 J8S3156 Bridge Final PS&E\A9317 - Melville over I-44_Bridge\Plan Sheet DGNs\B_A9317_009_J8S3156_End_Bent_3_Details.dgn 10:14:32 AM 1/17/2025

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1631 WEST ELFINDALE STREET
SPRINGFIELD, MO 65807 (417) 869-

1/17/2025

CRD 127 MO

GREENE

J8S3156

BRIDGE NO

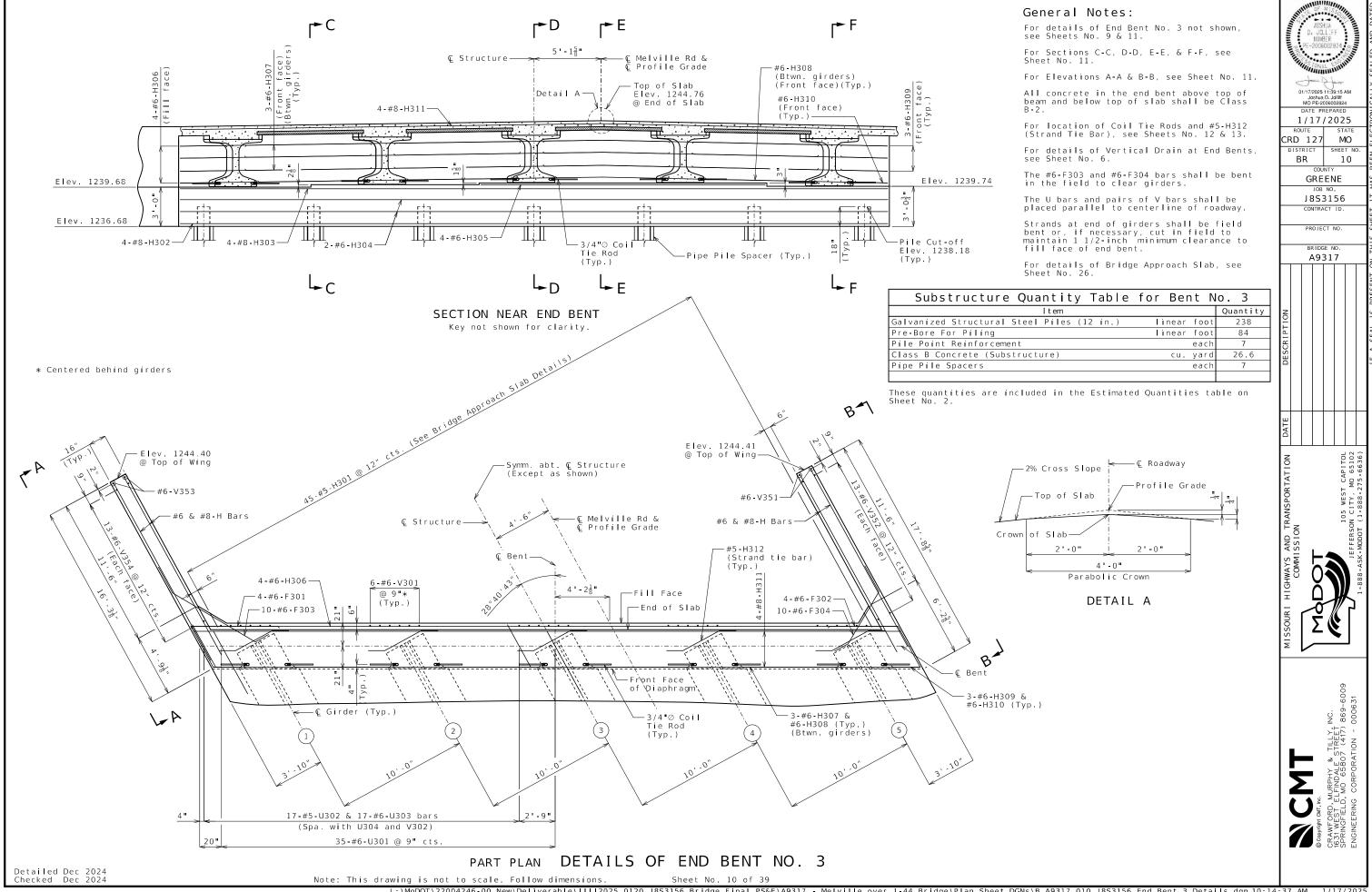
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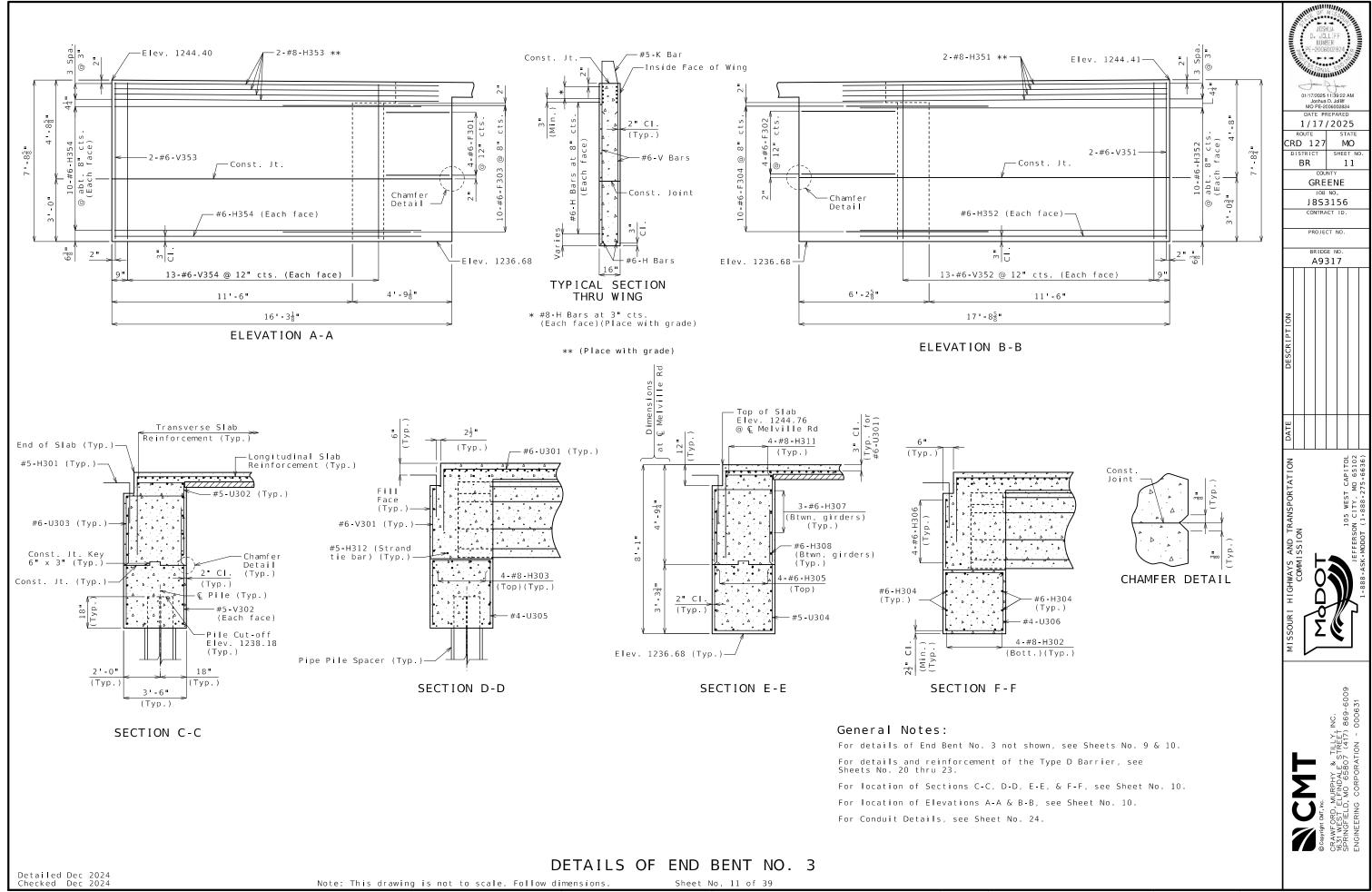
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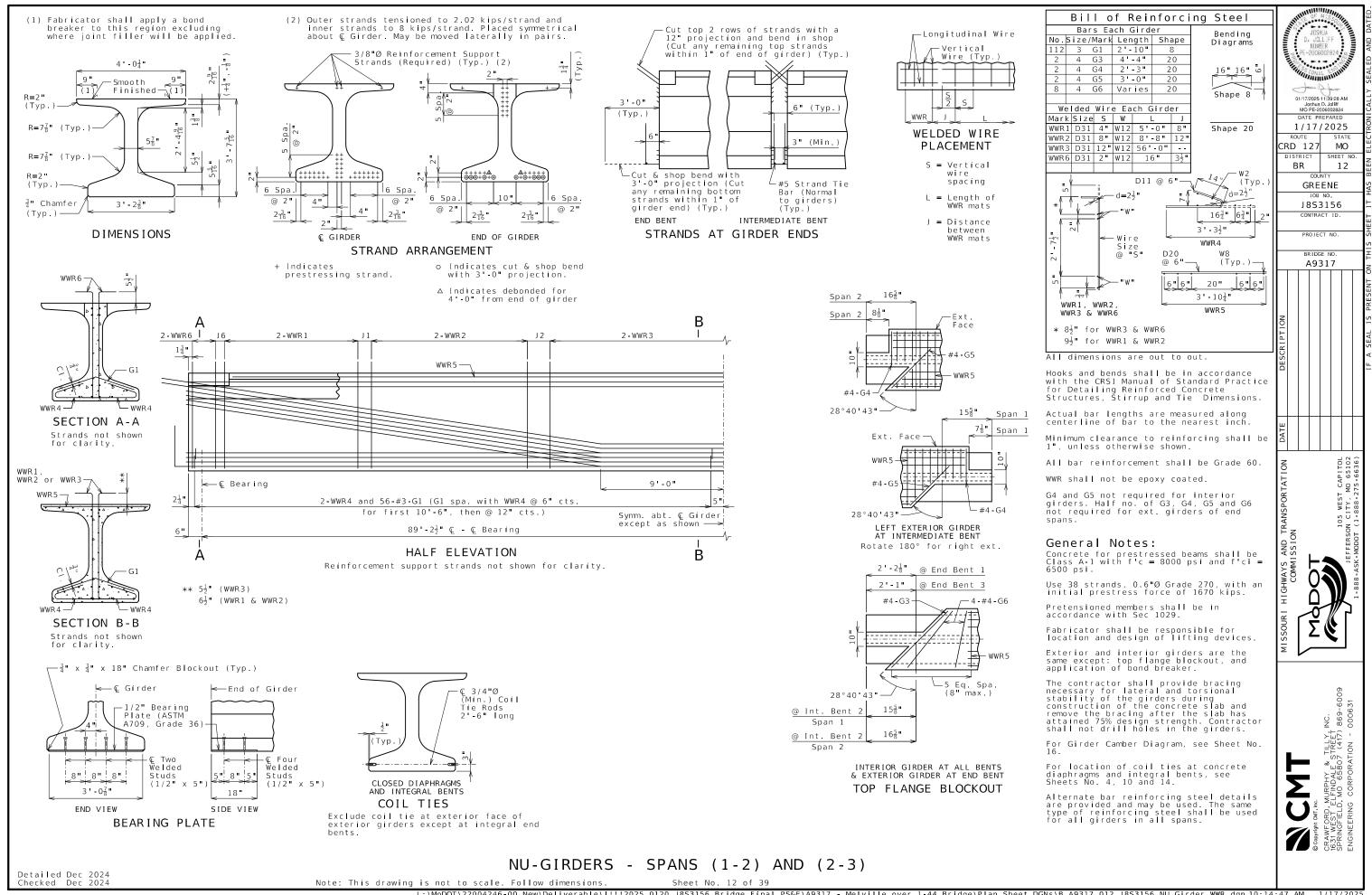
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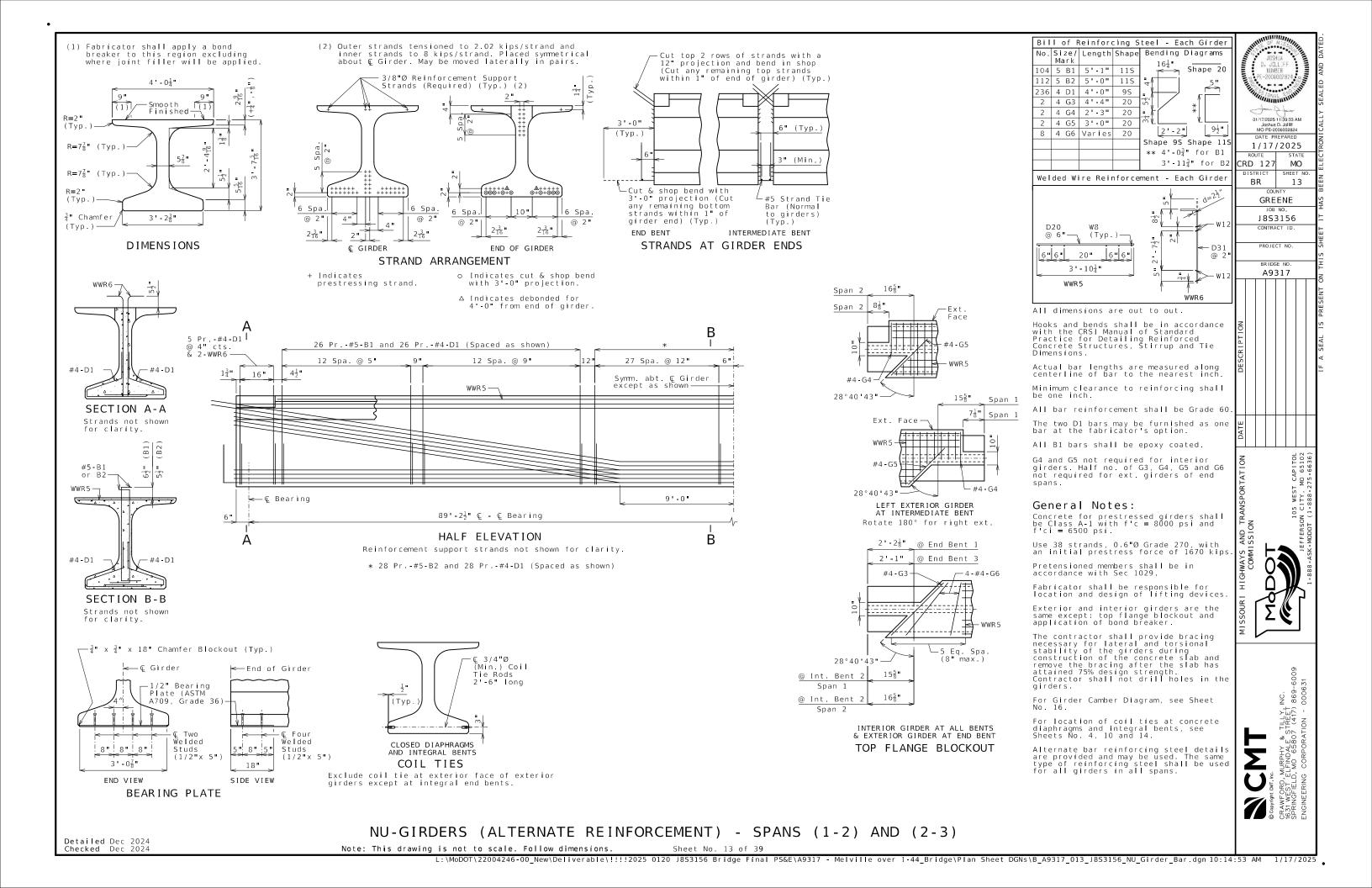
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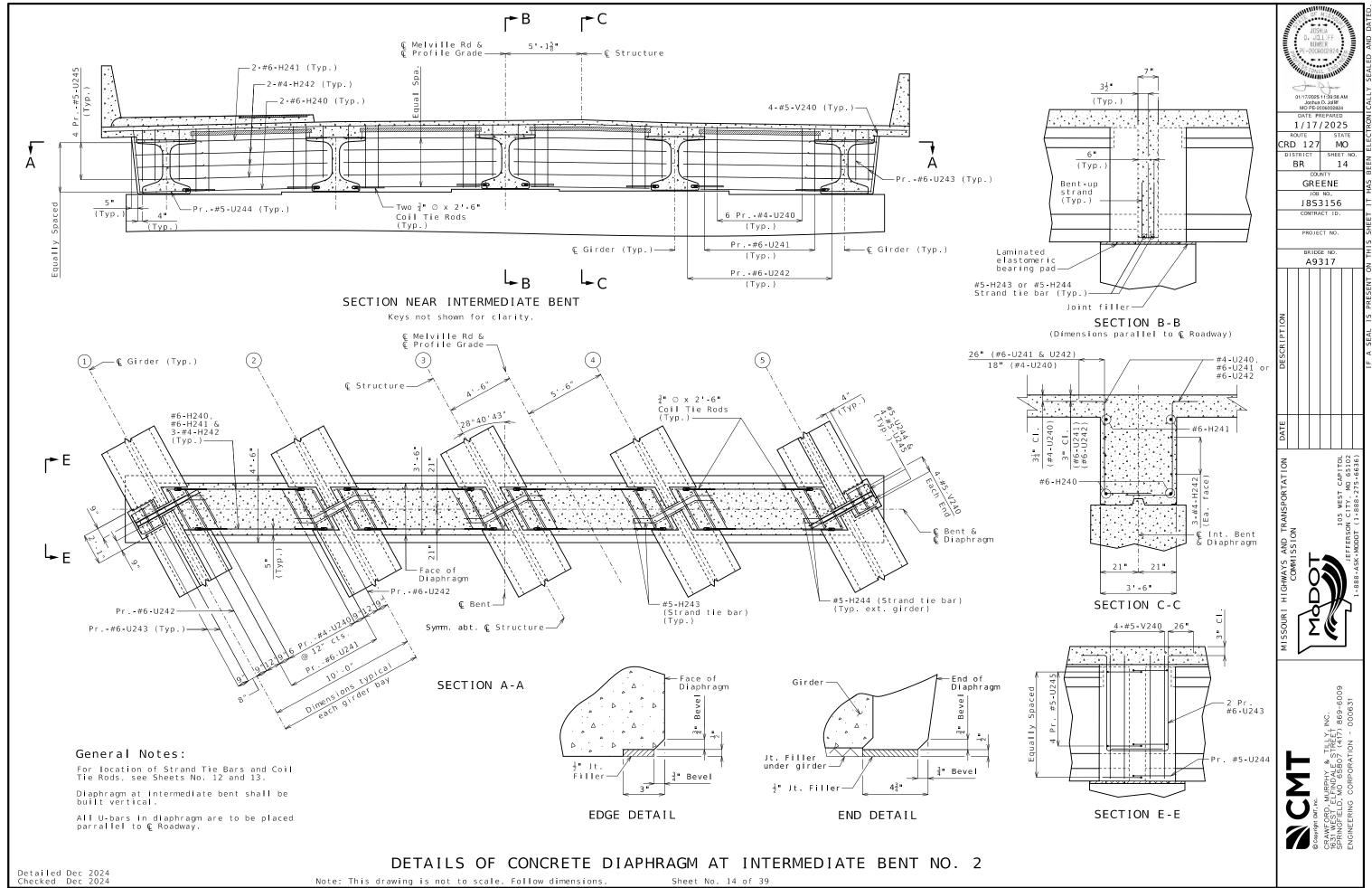
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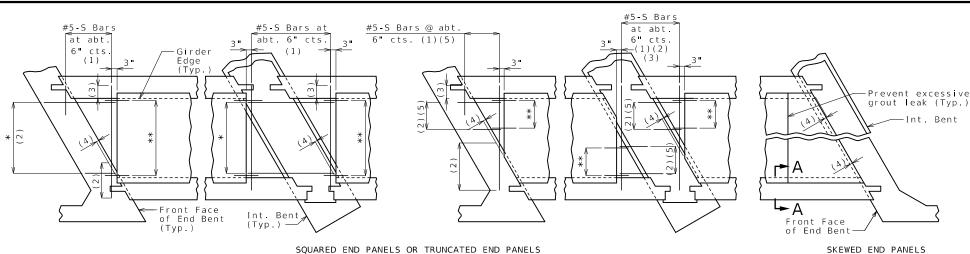








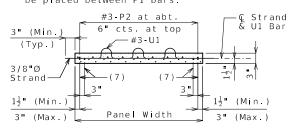




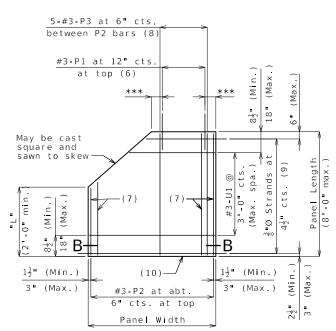
L/4 i L/4 i L/4 i L/4

U1 Bars may be oriented at right angles to location and spacing shown. U1 Bars shall be placed between P1 bars.

BENDING DIAGRAM FOR U1 BAR



SECTION B-B

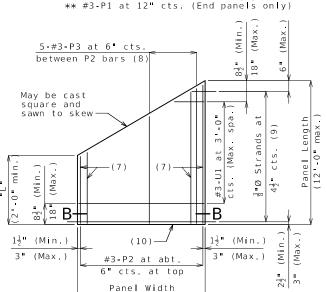


PLAN OF OPTIONAL TRUNCATED END PANEL *** 3" (Min.), 6" (Max.)

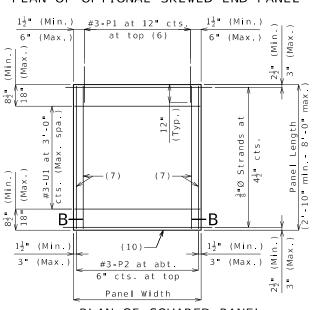
Detailed Dec 2024

PLAN SHOWING PANEL PLACEMENT

* #5-S Bars at abt. 9" cts. (1)



PLAN OF OPTIONAL SKEWED END PANEL



PLAN OF SQUARED PANEL

Joint Filler 1½" (Typ.) Panel-Dimensions (12) Height Width Min. Max. 1"

SECTION A-A

Reference Notes:

Plan of Panel Placement: (1) S-bars shown are bottom steel in slab between panels and used with squared and truncated end panels only.

(2) Extend S-bars 18 inches beyond the front face of end bents and int. bents for squared and truncated end panels only.

(3) Extend S-bars 9 inches beyond edge of girder

(4) End panels shall be dimensioned 1/2" min. to 1 1/2" max. from the inside face of diaphragm.

(5) For truncated end panels, use a min. of #5-S bars at 6" crossings in openings, or min. 4x4-

(6) For end panels only, P1 bars shall be 2'-0" in length and embedded 12". P1 bars will not be required for panels at squared integral end bents.

(7) #3-P2 bars near edge of panel at bottom (under strands).

(8) Use #3-P3 bars if panel is skewed 45° or greater.

(9) Any strand 2'-0" or shorter shall have a #4 reinforcing bar on each side of it, centered between strands. Strands 2'-0" or shorter may then be debonded at the fabricator's option.

(10) Optional 1/2" x 45° Chamfer one or both

Section A-A:

(11) Slab thickness over prestressed panels varies due to girder camber. In order to maintain minimum slab thickness, it may be necessary to raise the grade uniformly throughout the structure. No payment will be made for additional labor or materials required for necessary grade

(12) Contractor shall ensure proper consolidation under and between panels.

(13) At the contractor's option, the variation in slab thickness over prestressed panels may be eliminated or reduced by increasing and varying the girder top flange thickness. Dimensions shall be shown on the shop drawings.

PRESTRESSED PANELS

General Notes:

Prestressed Panels:

Concrete for prestressed panels shall be Class A-1 with f'c = 6,000 psi, f'ci = 4,000 psi.

The top surface of all panels shall receive a scored finish with a depth of scoring of $1/8\,\text{"}$ perpendicular to the prestressing strands in the panels.

Prestressing tendons shall be high-tensile strength, uncoated, seven-wire, low-relaxation strands for prestressed concrete in accordance with AASHTO M 203 Grade 270, with nominal diameter of strand = 3/8" and nominal area = 0.085 sq.in. and minimum ultimate strength = 22.95 kips (270 ksi). Larger strands may be used with the same spacing and initial tension.

Initial prestressing force = 17.2 kips/strand.

The method and sequence of releasing the strands shall be shown

Suitable anchorage devices for lifting panels may be cast in panels, provided the devices are shown on the shop drawings and approved by the engineer. Panel lengths shall be determined by the contractor and shown on the shop drawings.

When squared end panels are used at skewed bents, the skewed portion shall be cast full depth. No separate payment will be made for additional concrete and reinforcing required.

Support from diaphragm forms is required under the optional skewed end until cast-in-place concrete has reached 3,000 psi compressive strength

Prestressed panels shall be brought to saturated surface-dry (SSD) condition just prior to the deck pour. There shall be no free standing water on the panels or in the area to be cast.

The prestressed panel quantities are not included in the table of estimated quantities for the slab.

Reinforcing Steel:

All dimensions are out to out.

Hooks and bends shall be in accordance with the CRSI Manual of Standard Practice for Detailing Reinforced Concrete Structures, Stirrup and Tie Dimensions.

Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise shown.

If U1 bars interfere with placement of slab steel, U1 loops may be bent over, as necessary, to clear slab steel.

Deformed welded wire reinforcement (WWR) providing a minimum area of reinforcing perpendicular to strands of 0.22 sq in./ft, with spacing parallel to strands sufficient to ensure proper handling, may be used in lieu of the #3-P2 bars shown. Wire diameter shall not be larger than 0.375 inch. The above alternative reinforcement criteria may be used in lieu of the #3-P3 bars, when required, and placed over a width not less than 2 feet.

The following reinforcing steel shall be tied securely to the strands with the following maximum spacing in each direction: #3-P2 bars at 16 inches. WWR at 24 inches.

The #3-U1 bars shall be tied securely to #3-P2 bars, to WWR or to strands (when placed between P1 bars) at about 3-foot centers.

Minimum reinforcement steel length shall be 2'-0".

All reinforcement other than prestressing strands shall be epoxy

Precast panels may be in contact with stirrup reinforcing in

S-bars are not listed in the bill of reinforcing.

Cost of S-bars will be considered completely covered by the contract unit price for the slab.

Joint Filler:

Joint filler shall be preformed fiber expansion joint material in accordance with Sec 1057 or expanded or extruded polystyrene bedding material in accordance with Sec 1073

Use Slab Haunching Diagram on Sheet No. 16 for determining thickness of joint filler within the limits noted in the table of Joint Filler Dimensions.

Thicker material may be used on one or both sides of the girder to reduce cast-in-place concrete thickness to within tolerances.

The same thickness of preformed fiber expansion joint material shall be used under any one edge of any panel except at locations where top flange thickness may be stepped. The maximum change in thickness between adjacent panels shall be 1/2 inch. polystyrene bedding material may be cut with a transition to match haunch height above top of flange.

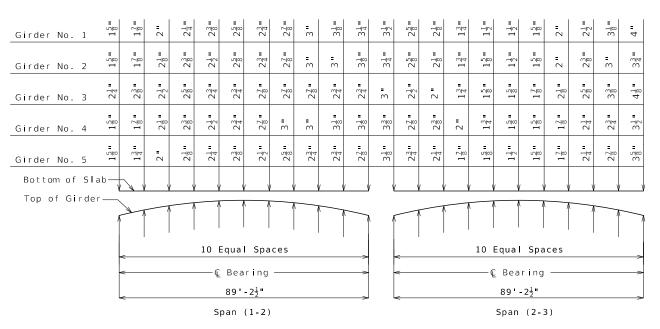
Joint filler shall be glued to the girder. When thickness exceeds 1 1/2 inches, the joint filler shall be glued top and bottom. The glue used shall be the type recommended by the joint filler

Edges of panels shall be uniformly seated on the joint filler before slab reinforcement is placed.

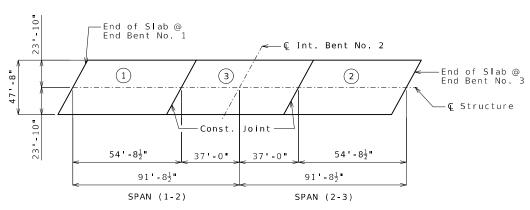


URPHY & TILLY, II FINDALE STREET MO 65807 (417)

Note: This drawing is not to scale. Follow dimensions. Sheet No. 15 of 39



THEORETICAL SLAB HAUNCHING DIAGRAM



	Sequence	of Pours	5	Min. Rate of Pour Cu. Yds./Hr.
	Dire	ection		With Retarder
Basic Sequence	1 Either	2 Direction	3	25
Alternate pour engineer in ac	s to the basic skip seque cordance with Sec 703.	nce are subje	ct to the app	roval of the
Alternate A	1	3	+ 2	36
Pours	End to 3	1 to	End	30
Alternate B	1 +	3 + 2		50
Pours	End	to End		0

SLAB POURING SEQUENCE

Notes:

Longitudinal dimensions are measured horizontal.

The contractor shall furnish an approved retarder to retard the set of the concrete to 2.5 hours, and shall pour and satisfactorily finish the slab pours at the rate given.

The concrete diaphragm at the intermediate bents and integral end bents shall be poured a minimum of 30 minutes and a maximum of 2 hours before the slab is poured.

Theoretical Bottom of Slab Elevations at Centerline of Girder (Prior to forming for slab) (Estimated at 90 days)

Girder				Span ((1-2) (89)'-2 1 '' (C	Brg Q	Brg.)			
Number	© Brg.	. 10	. 20	. 30	. 40	. 50	.60	.70	.80	.90	€ Brg.
1	1241.57	1241.84	1242.08	1242.30	1242.49	1242.65	1242.78	1242.88	1242.95	1243.01	1243.04
2	1241.63	1241.91	1242.16	1242.39	1242.60	1242.77	1242.91	1243.02	1243.10	1243.16	1243.20
3	1241.69	1241.97	1242.24	1242.48	1242.69	1242.87	1243.02	1243.14	1243.23	1243.30	1243.35
4	1241.51	1241.81	1242.08	1242.33	1242.55	1242.74	1242.90	1243.03	1243.13	1243.21	1243.27
5	1241.15	1241.45	1241.73	1241.99	1242.22	1242.42	1242.59	1242.73	1242.84	1242.93	1243.01
Girder				Span ((2-3) (89)'-2 1 '' ©	Brg Q	Brg.)			
Number	© Brg.	.10	. 20	. 30	. 40	.50	.60	. 70	.80	.90	€ Brg.
1	1243.05	1243.16	1243.26	1243.35	1243.43	1243.50	1243.55	1243.60	1243.62	1243.64	1243.66
2	1243.21	1243.32	1243.43	1243.52	1243.61	1243.68	1243.73	1243.77	1243.80	1243.81	1243.82
3	1243.36	1243.48	1243.59	1243.69	1243.77	1243.84	1243.89	1243.93	1243.96	1243.97	1243.99
4	1243.29	1243.42	1243.53	1243.63	1243.71	1243.78	1243.84	1243.88	1243.90	1243.92	1243.93
5	1243.03	1243.16	1243.28	1243.38	1243.46	1243.53	1243.59	1243.63	1243.66	1243.68	1243.69

1/17/2025

GREENE

J8S3156

CONTRACT ID PROJECT NO

BRIDGE NO A9317

ORD, MURPHY & TILLY, INC. EST ELFINDALE STREET FIELD, MO 65807 (417) 869

MO

SHEET NO

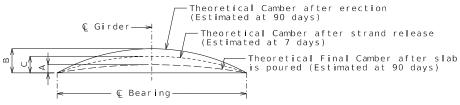
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Elevations are based on a constant slab thickness of 8 1/2" and include allowance for theoretical dead load deflections due to weight of slab (including precast panel), sidewalk, fencing and

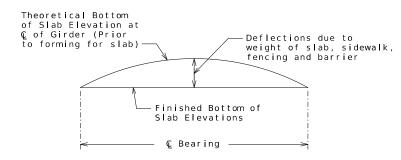


Girder	S	oan (1-2	!)	S	oan (2-3	;)
Girder	Α	В	С	Α	В	С
Exterior	2 1 "	3 7 "	2 1 "	2 1 "	3 7 "	2 ¹ / ₄ "
Interior	2 "	0	-4	2 "		-4

GIRDER CAMBER DIAGRAM

Conversion Factors for Girder Camber (Estimated at 90 days):

0.1 pt. = 0.314 x 0.5 pt. 0.2 pt. = 0.593 x 0.5 pt. 0.3 pt. = 0.813 x 0.5 pt. 0.4 pt. = 0.952 x 0.5 pt.



TYPICAL SLAB ELEVATIONS DIAGRAM

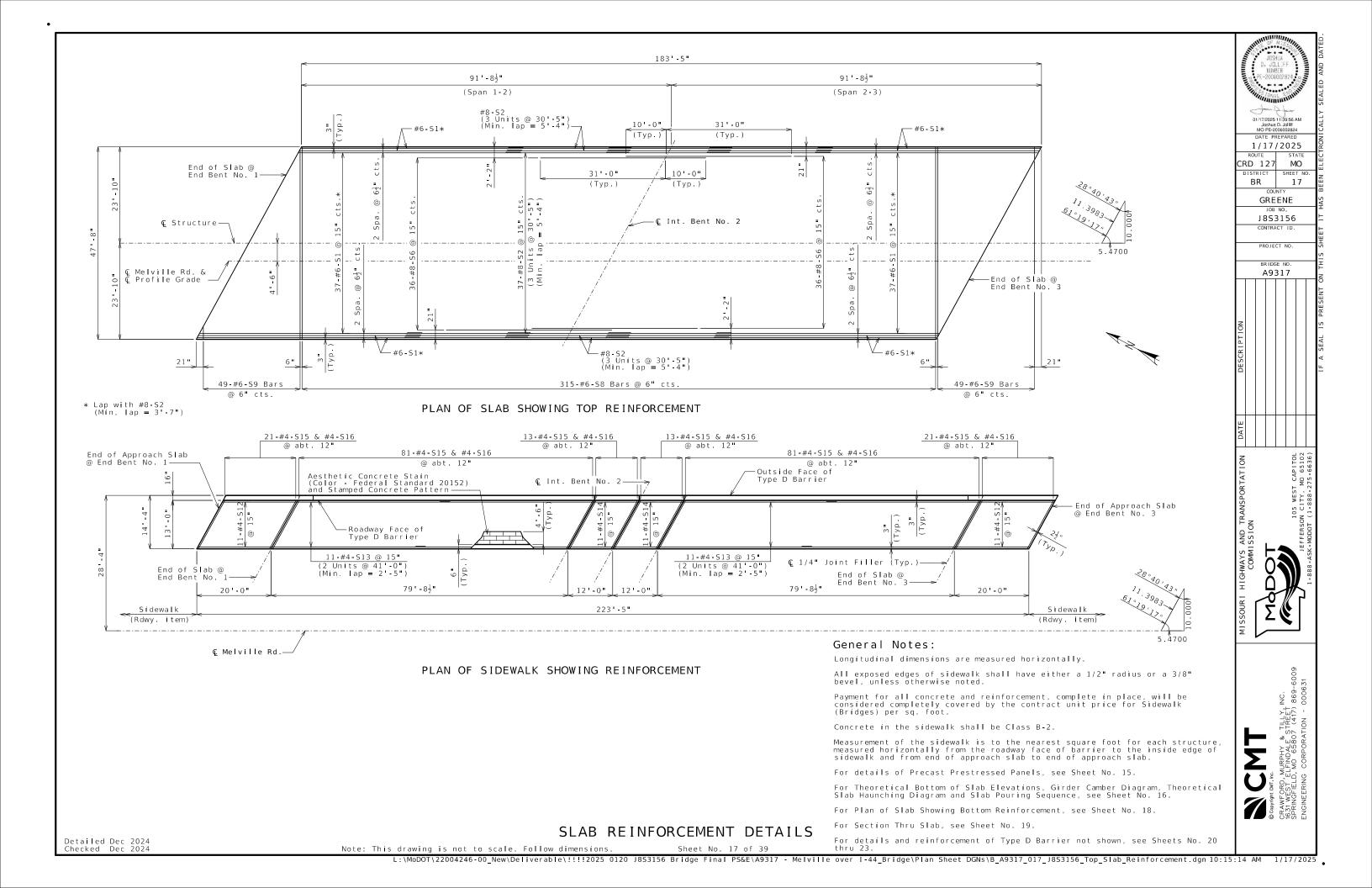
If girder camber is different from that shown in the camber diagram, in order to maintain minimum slab thickness, an adjustment of the slab haunches, an increase in slab thickness or a raise in grade uniformly throughout the structure shall be necessary. No payment will be made for additional labor or materials required for variation in haunching, slab thickness or grade adjustment.

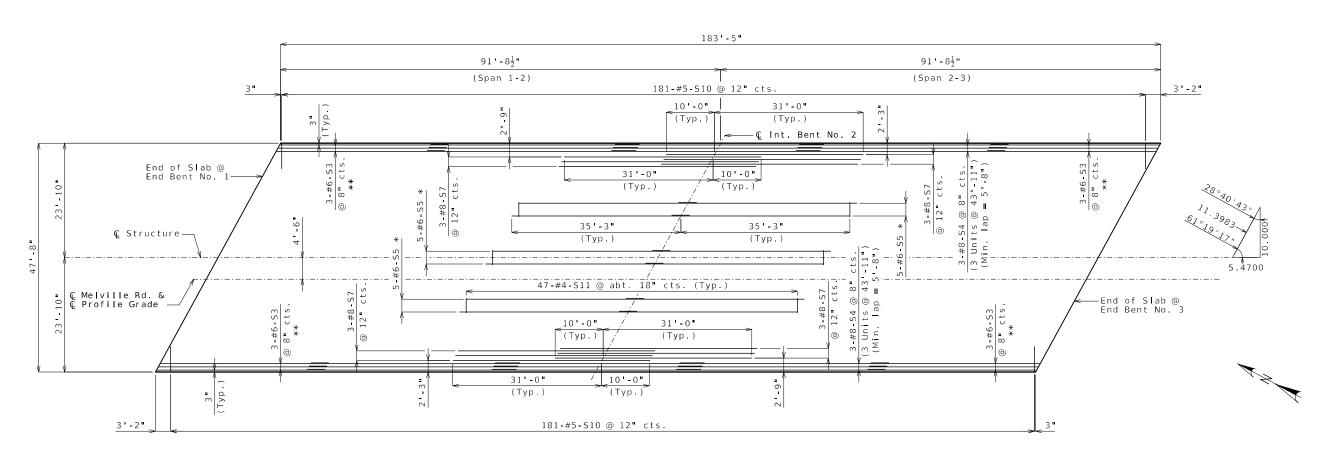
Concrete in the slab haunches is included in the Estimated Quantities for Slab on Concrete NU-Girder.

TBOS, HAUNCHING AND CAMBER

Detailed Dec 2024 Checked Dec 2024

Note: This drawing is not to scale. Follow dimensions.





PLAN OF SLAB SHOWING BOTTOM REINFORCEMENT

- * Spa. @ 8" cts. (2 Units @ 36'-10") (Min. lap = 2'-10")
- ** Lap with #8-S4 (Min. lap = 3'-7")

General Notes:

Longitudinal dimensions are measured horizontally.

For details of Precast Prestressed Panels, see Sheet No. 15.

For Theoretical Bottom of Slab Elevations, Girder Camber Diagram, Theoretical Slab Haunching Diagram and Slab Pouring Sequence, see Sheet No. 16.

For Plan of Showing Top Reinforcement, see Sheet No. 17.

For Section Thru Slab, see Sheet No. 19.

For details and reinforcement of Type D Barrier not shown, see Sheets No. 20 thru 23.



CRAWSTORD, MURPHY & TILLY, INC.
CRAWEST ELFINDALE STREET
SPRINGFIELD, MO 65807 (417) 869ENGINEERING CORPORATION - 0006

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BRIDGE NO A9317

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SHEET NO

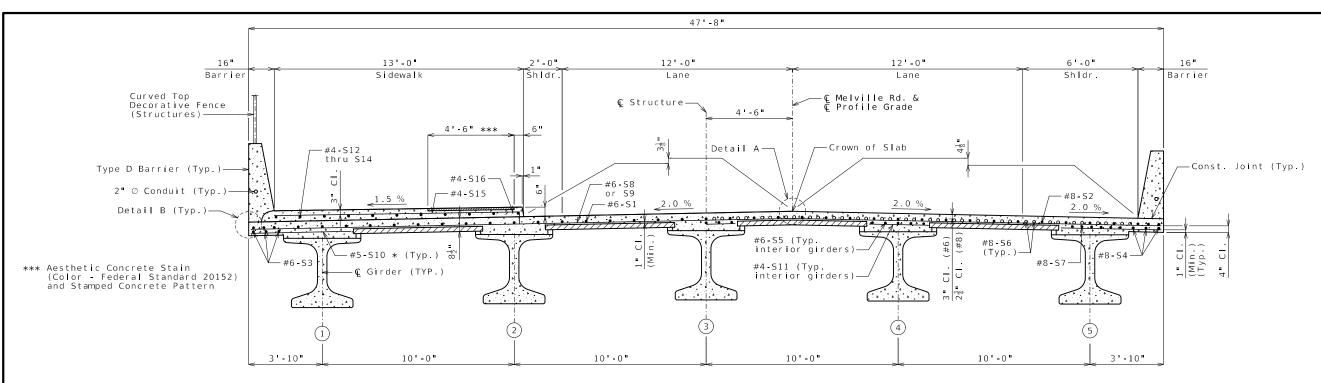
18

SLAB REINFORCEMENT DETAILS

Detailed Dec 2024 Checked Dec 2024

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 18 of 39

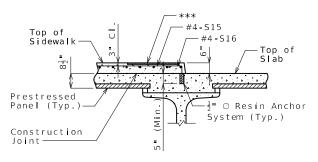


HALF SECTION NEAR C SPAN

HALF SECTION NEAR INTERMEDIATE BENT

SECTION THRU SLAB

* Alternate bar shape available, see barrier sheets.



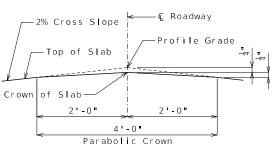
RESIN ANCHOR OPTION

Notes:

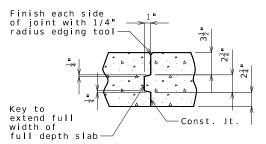
At the contractor's option, the #4-S16 bars may be replaced with a 1/2" Resin Anchor System as shown. No additional payment will be made for substitution.

The contractor shall use one of the qualified resin anchor systems in accordance with Sec 1039.

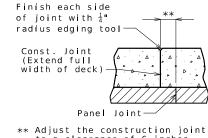
The minimum embedment depth in concrete with f'c = 4,000 psi for the resin anchor systems shall be that required to meet the minimum ultimate pullout strength in accordance with Sec 1039 but shall not be less than 5".



DETAIL A



ab —/ FULL DEPTH SLAB



to a clearance of 6 inches minimum from the panel joint.

SLAB ON PANELS

1/17/2025

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J8S3156

CONTRACT ID

A9317

D, MURPHY & TILLY, INC.
ELFINDALE STREET
LD, MO 65807 (417) 869-6

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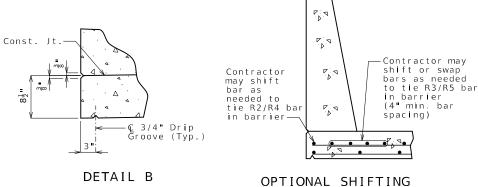
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SLAB CONSTRUCTION JOINT



General Notes:

For details of Precast Prestressed Panels, see Sheet No. 15.

For Theoretical Bottom of Slab Elevations, Girder Camber Diagram, Theoretical Slab Haunching Diagram and Slab Pouring Sequence, see Sheet No. 16.

For Plan of Slab Showing Top Reinforcement and Sidewalk Reinforcement, see Sheet No. 17.

For Plan of Slab Showing Bottom Reinforcement, see Sheet No. 18.

For details and reinforcement of Type D Barrier not shown, see Sheets No. 20 thru 23.

For Conduit details, see Sheet No. 24.

For Pedestrian Fence details, see Sheet No. 25.

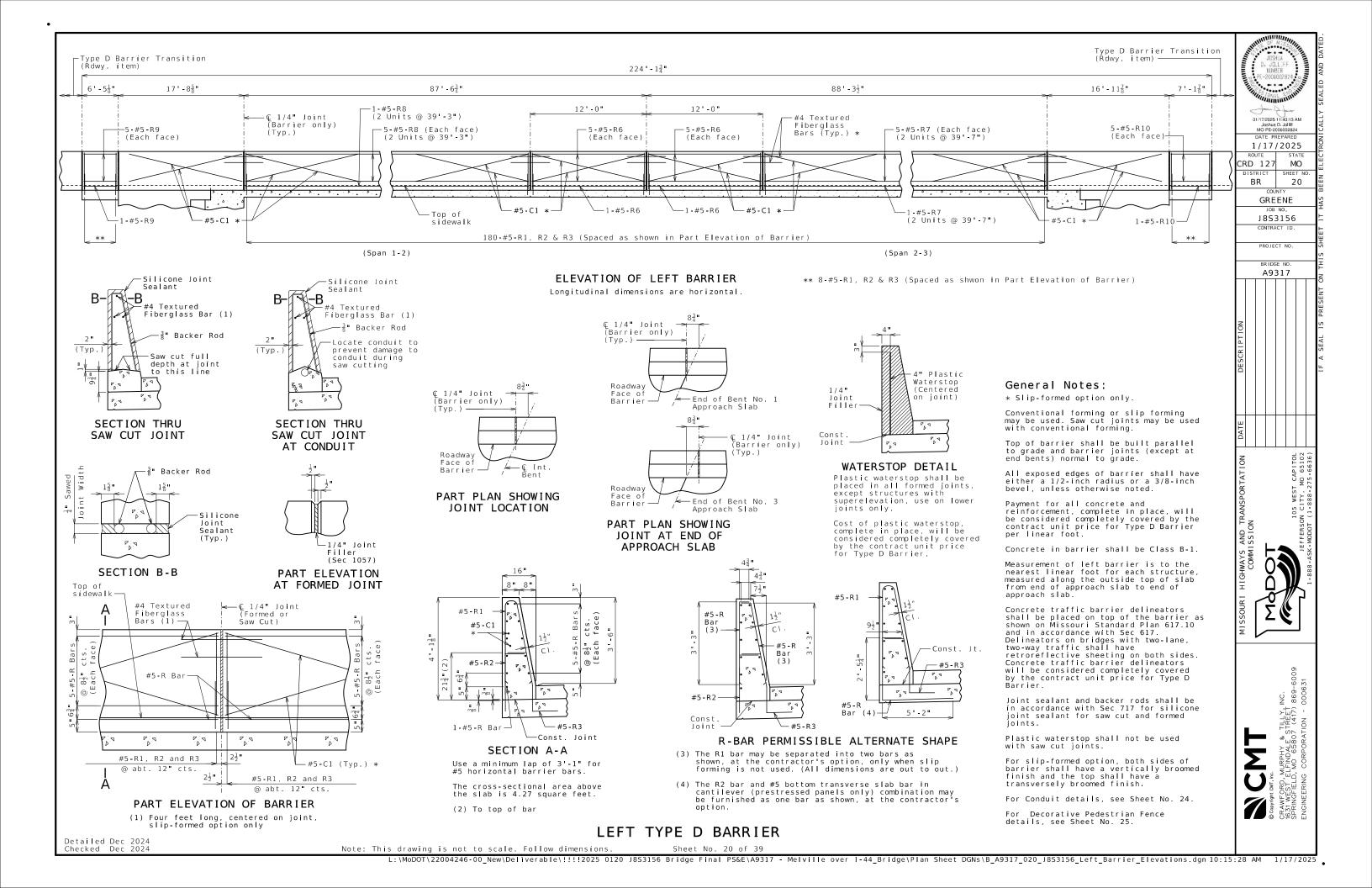
SLAB DETAILS

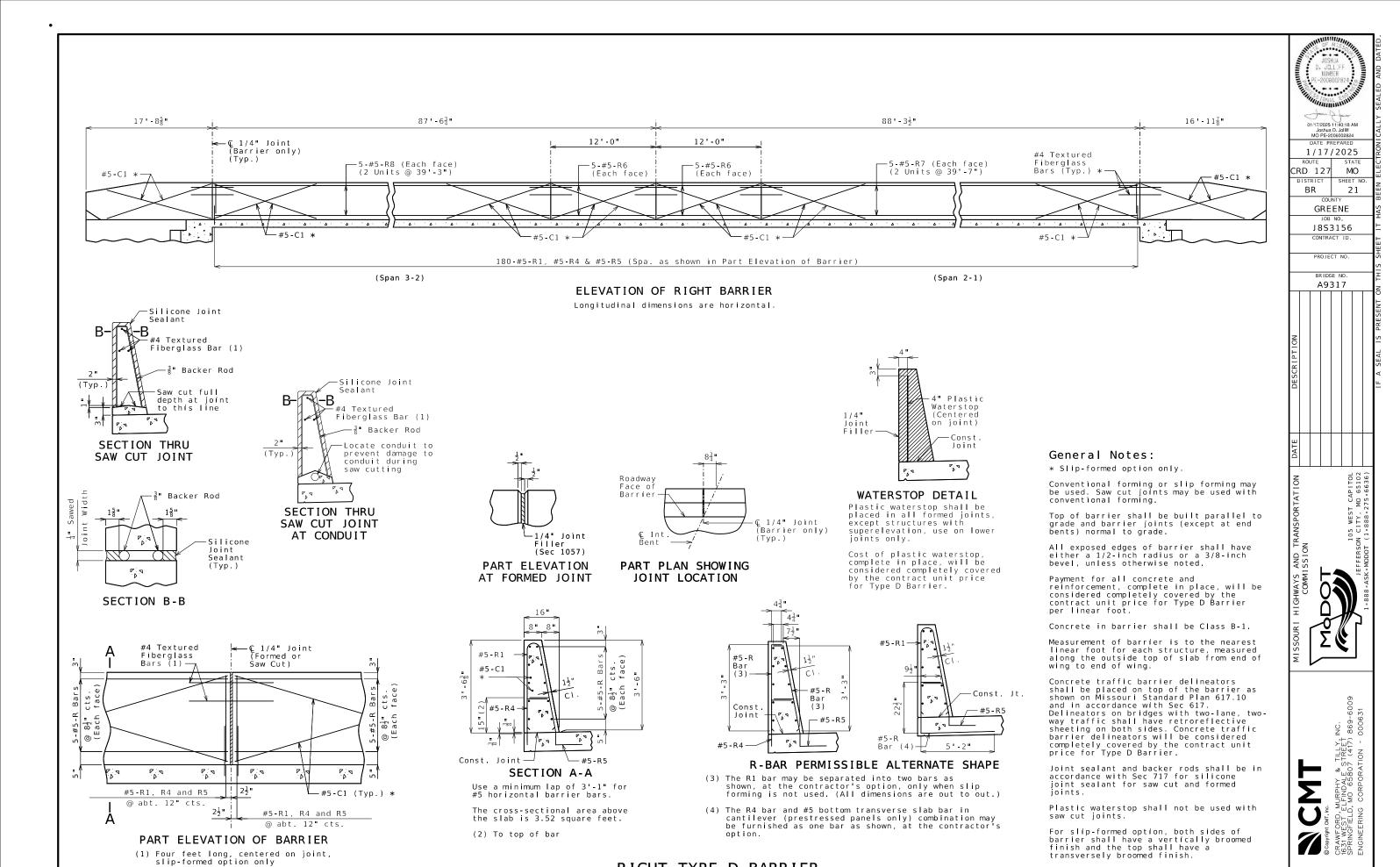
Detailed Dec 2024 Checked Dec 2024

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 19 of 39

TOP BARS AT BARRIER





RIGHT TYPE D BARRIER

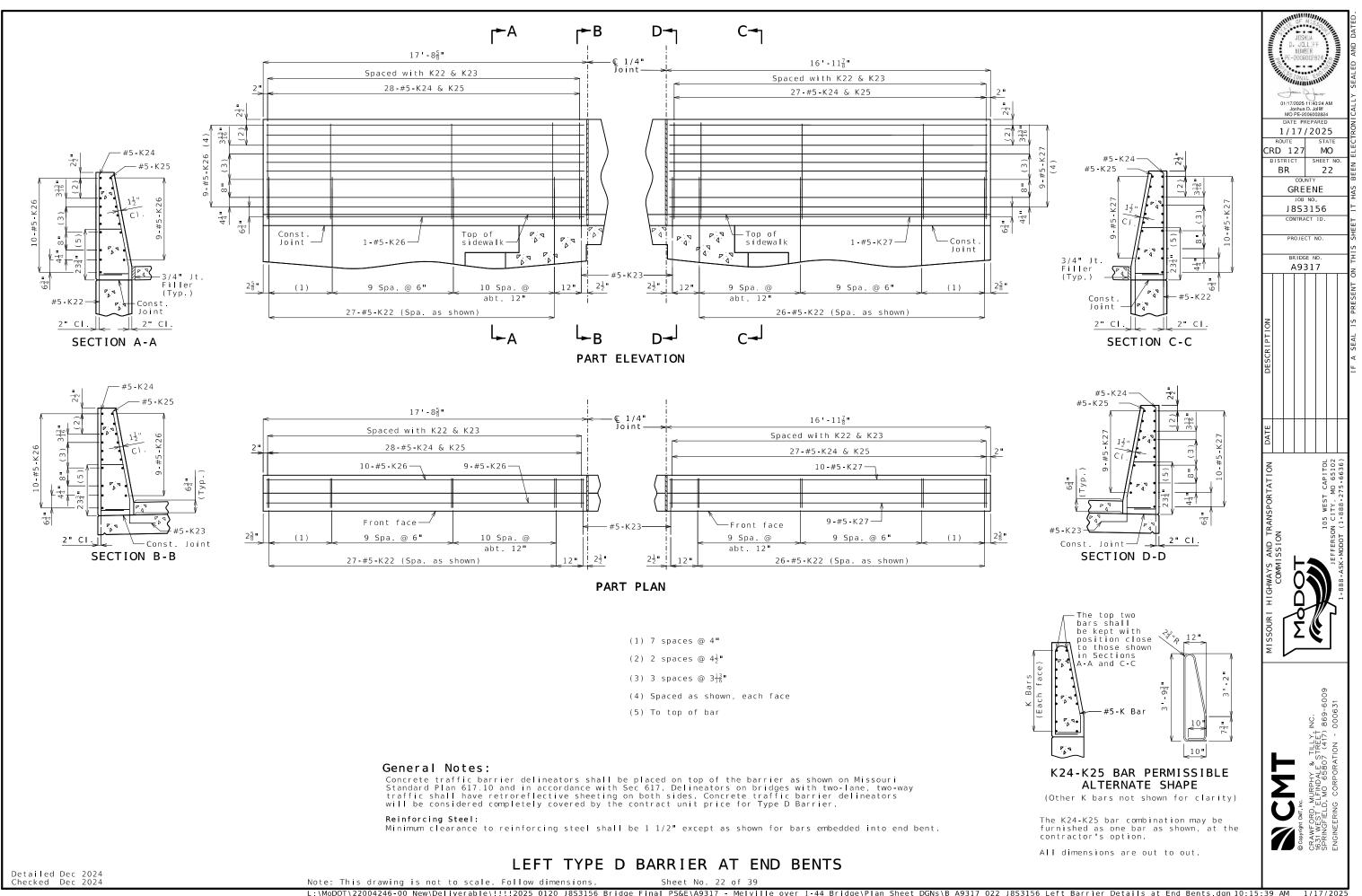
Detailed Dec 2024 Checked Dec 2024

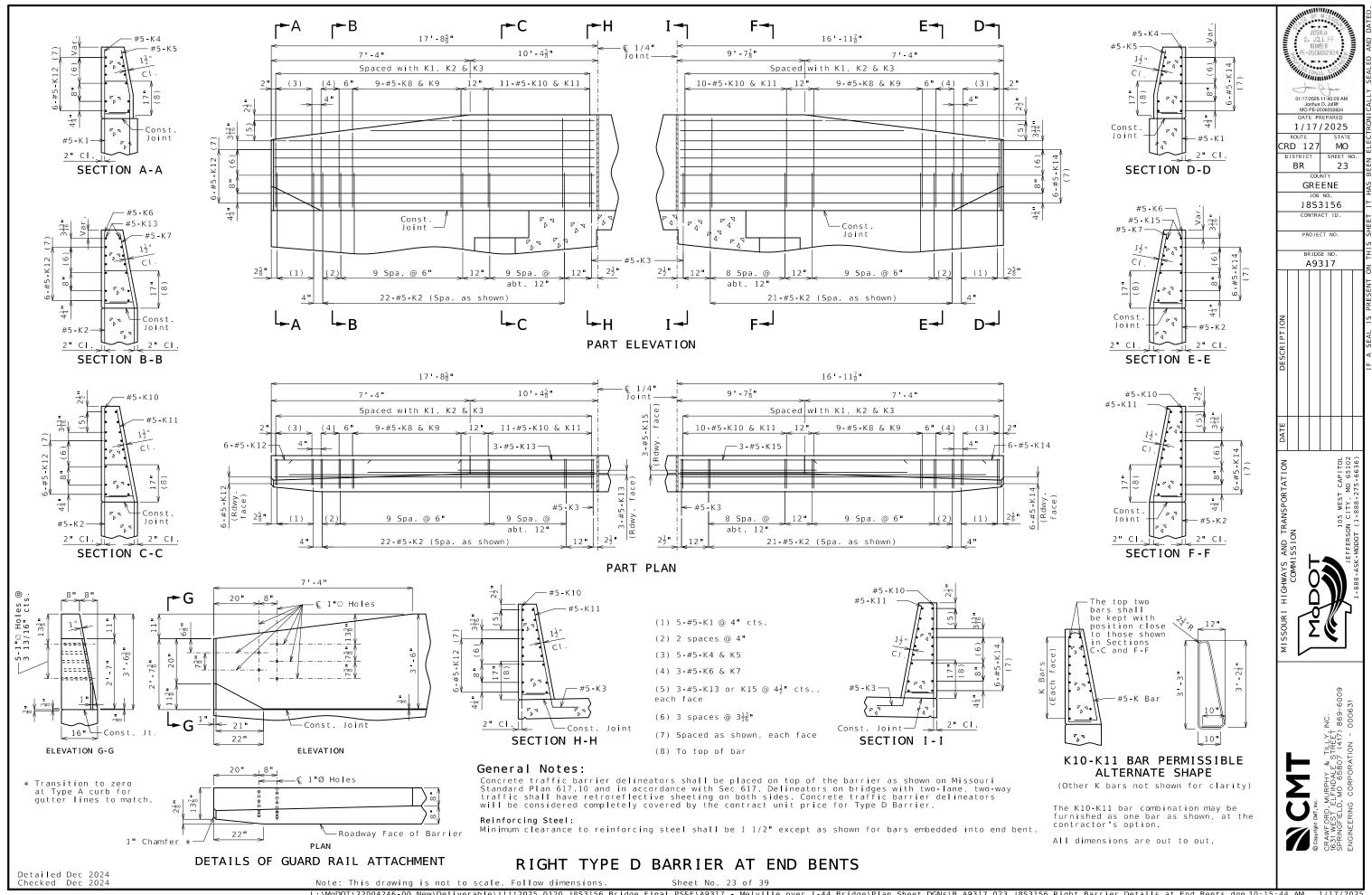
Note: This drawing is not to scale. Follow dimensions.

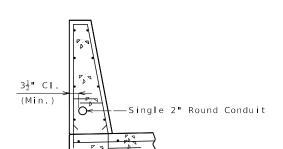
Sheet No. 21 of 39

L:\MoDOT\22004246-00_New\Deliverable\!!!2025 0120 J8S3156 Bridge Final PS&E\A9317 - Melville over 1-44_Bridge\Plan Sheet DGNs\B_A9317_021_J8S3156_Right_Barrier_Elevations.dgn 10:15:33 AM 1/17/2025

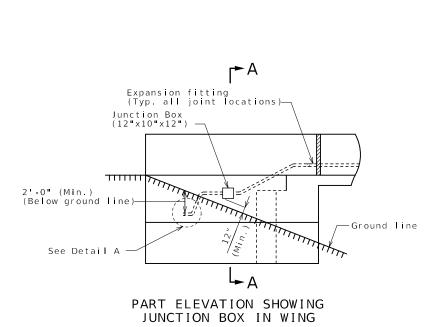
For Conduit details, see Sheet No. 24.

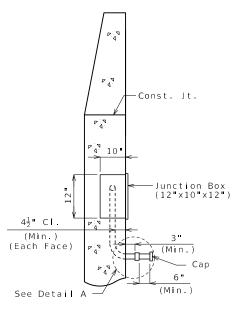






SECTION SHOWING SINGLE CONDUIT IN BARRIER







General Notes:

All conduits shall be rigid nonmetallic schedule 40 heavy wall polyvinyl chloride (PVC) with 3 1/2 inch minimum cover in barrier and 4 1/2 inch minimum cover in abutment wing. Each section of conduit shall bear the Underwriters Laboratories (UL) label.

Shift reinforcing steel in field where necessary to clear conduit and

Expansion fittings shall be placed as shown and set in accordance with the manufacturer's requirements and based on the air temperature at the time of setting given an estimated total expansion movement of 1 inch at filled joints using a maximum temperature range of 120°F and a maximum temperature of 110°F.

Drainage shall be provided at low points or other critical locations of all conduits and all junction boxes in accordance with Sec 707. All conduits shall be sloped to drain where possible.

All end bent junction boxes shall be PVC molded in accordance with Sec 1062 and designed for flush mounting. The conduit terminations shall be permanent or separable. The terminations and covers shall be of watertight construction and shall meet requirements for NEMA 4 or NEMA

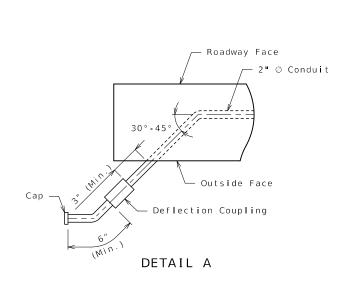
Junction box size shown on plan may require special order. Smaller junction box may be substituted if junction box meets conduit installation, clearance and project requirements.

Payment for furnishing and installing Conduit System, complete in place, will be considered completely covered by the contract lump sum price for Conduit System on Structure.

For details and reinforcement of Type D Barrier not shown, see Sheets No. 20, 21, 22 and 23.

Curved Top Decorative Fence not shown for clarity. See Sheet No. 25 for Fence details.

MoDOT Construction Personnel: Indicate in field and on bridge plans for future work the exact loaction of buried conduit at ends of bridge that are capped and not immediately used.

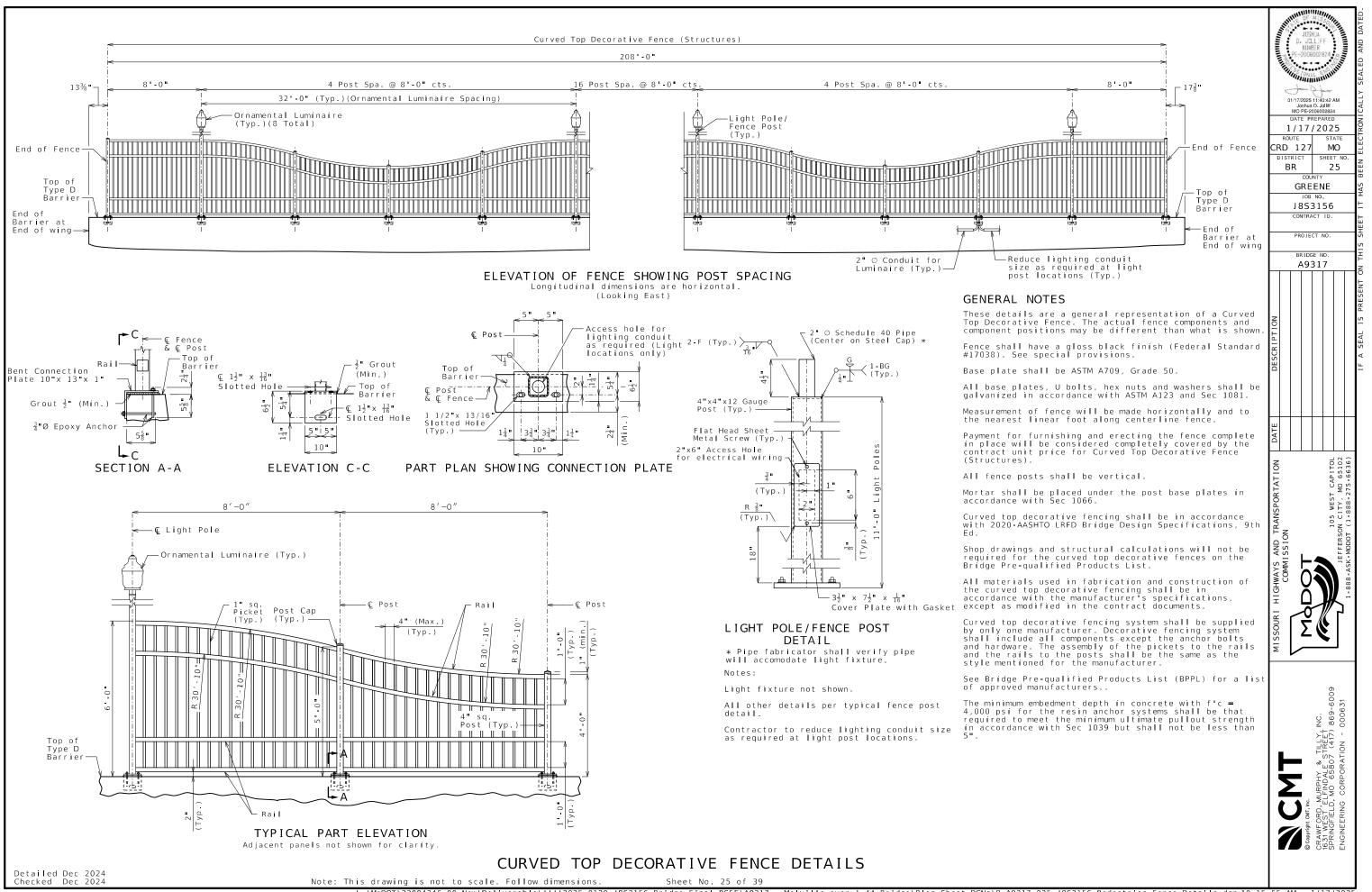


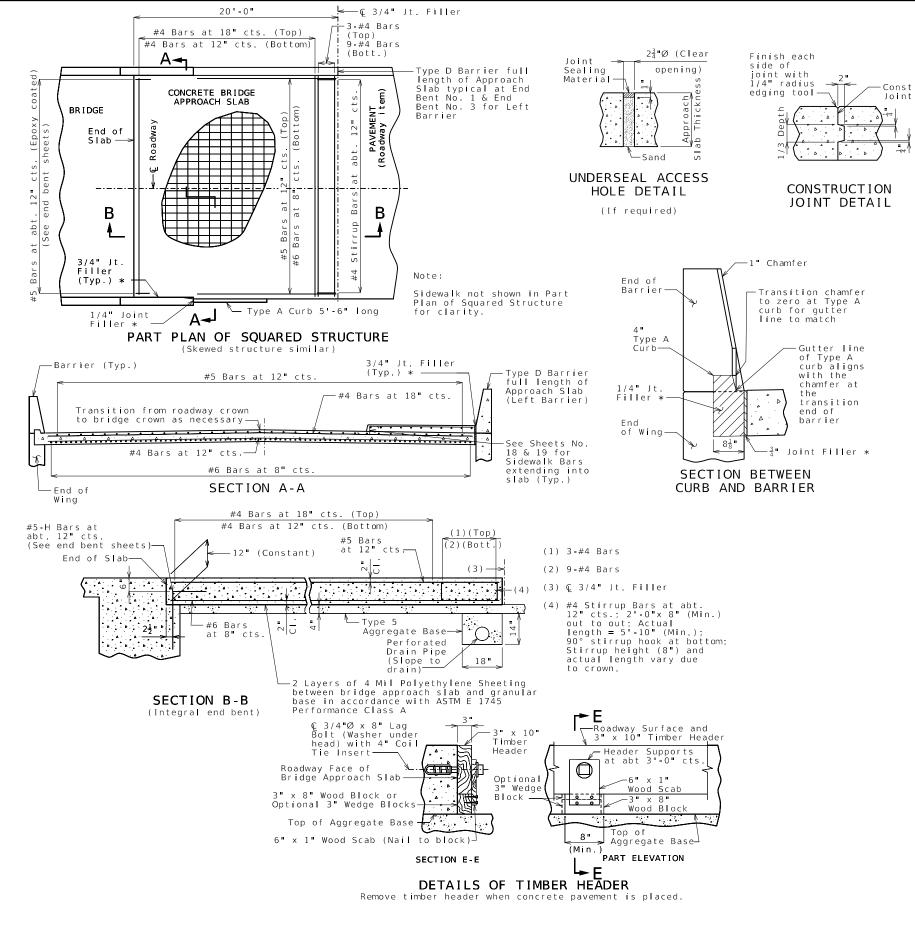
CONDUIT DETAILS

Detailed Dec 2024 Checked Dec 2024

Note: This drawing is not to scale. Follow dimensions.

1/17/2025 CRD 127 MO SHEET NO DISTRICT BR 24 GREENE J8S3156 CONTRACT ID. PROJECT NO A9317





General Notes:

Contractor shall construct the concrete slab in accordance with this sheet.

The contractor shall pour and satisfactorily finish the bridge slab before placing the bridge approach slab.

All concrete for the bridge approach slab shall be in accordance with Sec 503 (f c = 4,000 psi).

The reinforcing steel in the bridge approach slab shall be epoxy coated Grade 60 with fy = 60,000 psi.

Longitudinal construction joints in bridge approach slab shall be aligned with longitudinal construction joints in bridge slab.

Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise shown.

The reinforcing steel in the bridge approach slab shall be continuous. The transverse reinforcing steel may be made continuous by providing a minimum lap splice of 23 inches for #4 bars, or by mechanical bar splice.

All joint filler shall be in accordance with Sec 1057 for preformed fiber expansion joint filler except as noted.

Payment for furnishing all materials, labor and excavation necessary to construct the concrete bridge approach slab, including the timber header, underdrain, Type 5 aggregate base, joint filler, and all other appurtenances and incidental work as shown on this sheet, complete in place, will be considered completely covered by the contract unit price for Bridge Approach Slab (Minor) per square yard.

See Missouri Standard Plan 609.00 for details of Type A curb.

Drain pipe may be either 6" diameter corrugated metallic-coated pipe underdrain, 4" diameter corrugated polyvinyl chloride (PVC) drain pipe, or 4" diameter corrugated polyethylene (PE) drain pipe.

* Seal joint between vertical face of approach slab and wing with sealant in accordance with Sec 717 for silicone joint sealant for saw cut and formed joints.



01/17/2025 11:41:06 AM Joshua D. Jolliff MO PE-2006002824

1/17/2025
ROUTE STATE
CRD 127 MO
DISTRICT SHEET NC
BR 26

GREENE
JOB NO.
J8S3156
CONTRACT ID.

PROJECT NO

BRIDGE NO.
A9317

DATE DESCRIPTION

COMMISSION

COMMISSION

COMMISSION

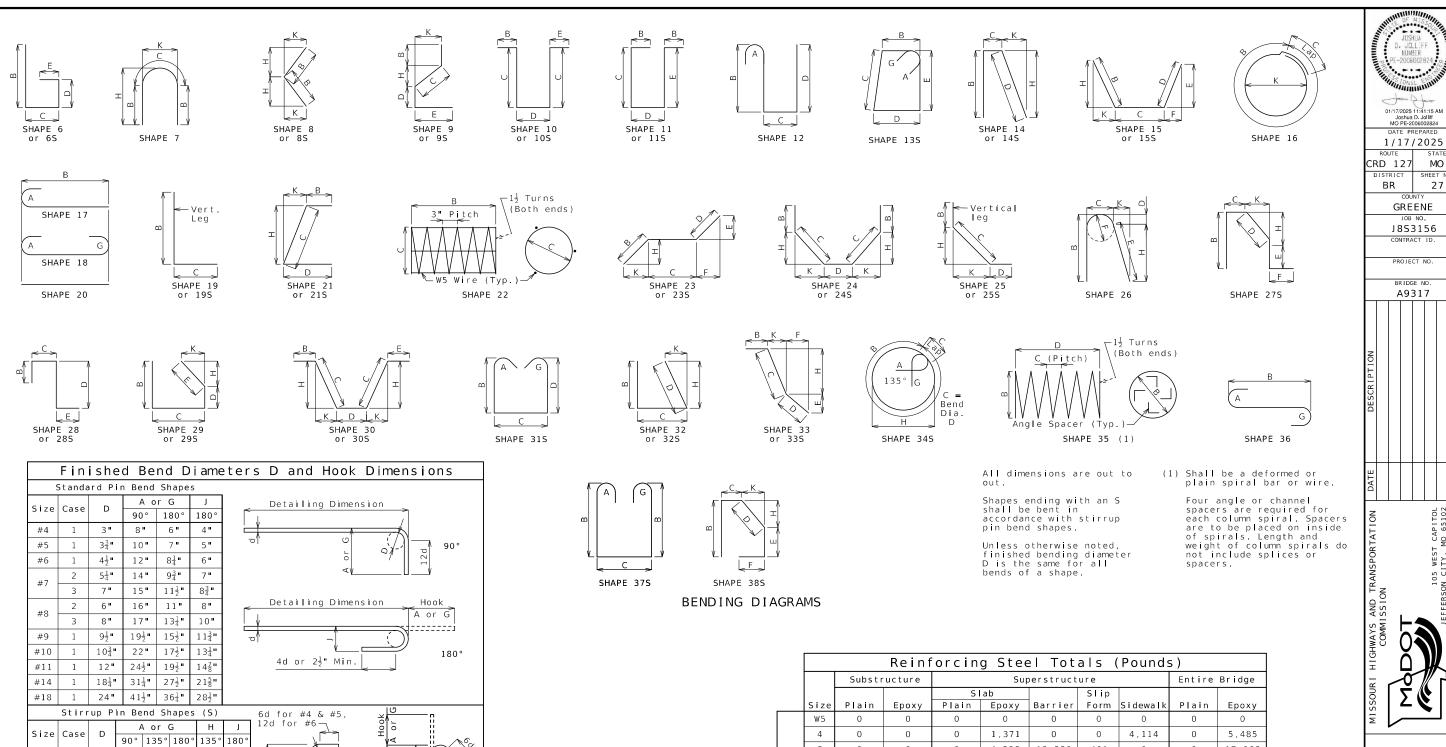
105 WEST CAPITOL

IEFERSON CITY MO 65102

WEST ELFINDALE STREET STREET AGGREEN GORPORATION - 000631

BRIDGE APPROACH SLAB (MINOR)

Sheet No. 26 of 39



			Reinf	orcin	g Stee	el Tot	als (Pounds	5)	
		Substr	ucture		Sup	erstruct	ure		Entire	Bridge
				SI	ab		Slip			
	Size	Plain	Epoxy	Plain	Epoxy	Barrier	Form	Sidewalk	Plain	Epoxy
	W5	0	0	0	0	0	0	0	0	0
	4	0	0	0	1,371	0	0	4,114	0	5,485
	5	0	0	0	4,328	12,339	401	0	0	17,068
	6	5,169	5,820	0	44,029	0	0	0	5,169	49,849
Ву	7	0	103	0	0	0	0	0	0	103
Size	8	0	0	0	26,087	0	0	0	0	26,087
	9	0	0	0	0	0	0	0	0	0
	10	0	21,492	0	0	0	0	0	0	21,492
	11	0	0	0	0	0	0	0	0	0
	14	0	0	0	0	0	0	0	0	0
	18	0	0	0	0	0	0	0	0	0
Ву	Туре	5,169	27,415	0	75,815	12,339	401	4,114	5,169	120,084

SHEET NO

27

GREENE

J8S3156

A9317

J, MURPHY & TILLY, INC.
ELFINDALE STREET
LD, MO 65807 (417) 869-1

All superstructure reinforcing steel shall be epoxy coated unless otherwise

BENDING DIAGRAMS AND REINFORCING STEEL TOTALS

Detailed Dec 2024 Checked Dec 2024

#4

4½"

6¹/₄"

Applicable for all grades of steel

Case 1 applies to all reinforcement. Case 2 applies to all reinforcement except for galvanized bars. Case 3 applies to

ğalvanized bars only.

53 "

33 •

Detailing Dimension

4d or $2\frac{1}{2}$ Min.

Hook

a or G

180°

				Bil	l I of	f Rein	nforci	ng Steel					1				Bill of Rei	nforci	na Stee	İ				THINK OF M.S.S.
							Dimension			Nom.	Actual							Dimension		•	Nom.	Actual		JOSHUA
No.			Code		С	D	E	F H	K			Weight	11	1 ' 1		Codes		E	F	H K		Length		D. JOLLIFF NUMBER
Req.	Mark	Location	C SH	V ft in ft	in. 1	ft in.	ft in.	ft in. ft in.	ft in	ft in	. ft in	. Ib	Req	. Mark	Location END BENT 3	C SH	V ft in. ft in. ft in.	ft in.	ft in ft	in. ft in.	ft in	ft in.	Ιb	PE-2006002824
		SUBSTRUCTURE											4	6 F301	DIAPHRAGM	E 21	3 7.25 6 8.00		3	2.00 20.75	10 3	9 11	60	THE TOWN THE
													4	6 F302	DIAPHRAGM	E 14	5 2.50 3 7.25		3	2.00 20.75	8 10		53	Joans D Javar
20	7 0200	INT. BENT 2	F 20	2 6 00						2 6	2 6	102	10		WING BRACE	E 15	2 3.00 7 8.00 14.00			13.75 23.25		11 0	166	01/17/2025 11:41:21 AM Joshua D. Jolliff
20	7 D200	BEAM	E 20	2 6.00						2 6	2 6	103	10	6 F304	WING BRACE	E 15	14.00 4 7.50 2 3.00	23.25	13.75	7.00 12.00	8 1	8 0	121	MO PE-2006002824 DATE PREPARED
10	10 H201	BEAM	E 20	50 9.00						50 9	50 9	2,184	45	5 H301	DIAPHRAGM	E 19	2 0.00 15.00				3 3	3 1	145	1/17/2025
	10 H202	BEAM	E 18	50 9.00							53 8		-	8 H302	BEAM	E 20	54 0.00					54 0	577	ROUTE STATE
	6 H203 6 H204	BEAM BEAM	E 20	7 0.00 50 9.00						7 0 50 9	50 9	_	4	8 H303 6 H304	BEAM BEAM	E 20	54 0.00 54 0.00					54 0 54 0	577 325	CRD 127 MO
_	6 H205	BEAM	E 10S		12.00	4 0.50				6 1		_	4	6 H305	BEAM	E 20	21 9.00					21 9	131	BR 28
													4	6 H306	DIAPHRAGM	E 20	54 0.00				54 0	54 0	325	COUNTY
	6 U201	BEAM BEAM	E 135							15 8	_	729	12		DIAPHRAGM	E 20	10 0.00					10 0	181	GREENE
48	6 U202 6 U203	BEAM BEAM	E 13S							18 4 19 0		1,286	6		DIAPHRAGM DIAPHRAGM	E 20	7 5.00					7 5 3 9	45 34	J8S3156
	6 U204	BEAM	E 10S			4 3.00	. ,				12 5	_	2		DIAPHRAGM	E 20						2 3	7	CONTRACT ID.
	6 U205	BEAM	E 10S			4 3.00				13 5			4		DIAPHRAGM	E 20	54 0.00					54 0	577	PROJECT NO.
13	6 U206	BEAM	E 10S		12.00	4 3.00				6 3	5 11	116	5	5 H312	DIAPHRAGM	E 23	15.00 3 3.00 15.00	7.25	13.25	7.25 13.25	5 9	5 9	30	THOSE THO.
60	10 V201	COLUMN	E 17	19 3.00						20 9	20 9	5,358	8	8 H351	WING WALL	E 20	15 6.00				15 6	15 6	332	BRIDGE NO.
	10 V202	COLUMN		45 1.00						45 1	_	11,640	22	6 H352	WING WALL	E 20	14 8.00				14 8	14 8	485	A9317
1,1	6 0201	COLLINAN	E 10	0 5 00 4	2 00				2 0 00	12 0	12 0	2 251	8	8 H353	WING WALL	E 20						15 6	332	1
	6 P201 6 P202	COLUMN COLUMN	E 16	9 5 00 4							13 6 12 4		1 -22	6 H354	WING WALL	E 20	14 8.00				14 8	14 8	485	1
										Ĺ				6 U301	DIAPHRAGM	E 19S	3 0.00 5 6.00					8 4	864	1
		SUPERSTRUCTURE												5 U302	DIAPHRAGM	E 10S						10 4	367	
		END BENT 1											24	6 U303 5 U304	DIAPHRAGM BEAM	E 19S						6 5 15 11	328 399	
4	6 F101	DIAPHRAGM	E 21	3	7.25	6 8.00		3 2.00	20.75	10 3	9 11	60	4	4 U305	BEAM	E 105						9 4	25	
	6 F102	DIAPHRAGM	E 14	5	2.50	3 7.25		3 2.00	20.75	8 10	8 9			4 U306	BEAM		3 7.25 2 8.00 3 7.25					13 0	192	ESC
	6 F103	WING BRACE	E 15			14.00					11 0		12	4 U307	BEAM	E 13S	3 7 25 2 11 50 3 7 25	2 11.50			13 11	13 7	109	
10	6 F104	WING BRACE	E 15	14.00 4	7.50	2 3.00	23.25	13.75 7.00	12.00	8 1	8 0	121	30	6 V301	DIAPHRAGM	E 20	3 4.00				3 4	3 4	151	
45	5 H101	DIAPHRAGM	E 19	2 0.00	15.00					3 3	3 1	145		5 V302	BEAM	E 20						6 3	131	
	8 H102	BEAM	E 20								54 0]										
	8 H103 6 H104	BEAM BEAM	E 20	54 0.00							54 0 54 0			6 V351 6 V352	WING WALL WING WALL		7 4.00 2 7 4.00					7 4	23	삗
	6 H105	BEAM		2 42 11 50							43 0	_	1 20	10 1332	INCR. = 0.08 IN.	20	7 3.00					7 3	285	
		INCR. = 20 IN.		44 8.25						44 8	44 8													
	6 H106	DIAPHRAGM		54 0.00							54 0			6 V353	WING WALL		7 4.00				7 4		23	JOZ 102
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	6 H110	DIAPHRAGM	E 20							2 3		7			INT. DIAPHRAGM	5 20	7 6 00				7 6	-		POF
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	3 11112	DIAFTIKAGIN	L 23	13.00 3	3.00	13.00	7.23	13.23 7.23	13.23	1 3	1 3	30	-	4 H242	DIAPHRAGM	E 20	10 0.00					10 0	161	N N N
	8 H151			15 6.00							15 6			5 H243	DIAPHRAGM	E 23	15.00 3 3.00 15.00			7.25 13.25		5 9	36	ND TI
	6 H152 8 H153			14 8.00							14 8		4	5 H244	DIAPHRAGM	E 14	3 10.00 15.00)		7.25 13.25	5 1	5 1	22	I SS I
	6 H154			15 6.00 14 8.00							15 6 14 8	441	48	4 U240	DIAPHRAGM	E 28S	18.00 4 2.50	2 7.00			8 4	8 1	260	NIM L
													16	6 U241	DIAPHRAGM	E 285	2 2.00 4 2.75	3 3.00			9 8	9 4	225	IGHWAYS AND TRANSPORTATION COMMISSION OOT 105 WEST CAPITOL JEFFERSON CITY, MO 65102
	6 U101	DIAPHRAGM	E 19S			2 1 00				8 6				6 U242	DIAPHRAGM	E 285						8 7		
	5 U102 6 U103	DIAPHRAGM DIAPHRAGM	E 10S			3 1.00					10 4 6 5			6 U243 5 U244	DIAPHRAGM DIAPHRAGM	E 28S	2 2.00 3 5.25 4 10.00 10.50	2 /.00				7 10 5 6		
	5 U104	BEAM	E 10S	5	7.00	3 7.25					14 7			5 U245	DIAPHRAGM	E 6S)				7 2		
	5 U105	BEAM	E 10S			3 7.25					15 9				D.L. D.L. D.L.		1 2 20						2.5	2 &
	5 U106 4 U107	BEAM BEAM	E 10S			3 7.25 3 7.25					15 11		8	5 V240	DIAPHRAGM	E 20	4 2.00				4 2	4 2	35	Ĭ _Σ
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	4 U109	BEAM	E 13S								14 0													
	C 1/101	D I A DUD A CAA	F 20	1 2 1 00							1 .	120		6 51	SLAB		55 0.00					55 0		
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	5 V103	BEAM	E 20								6 3		18	8 54	SLAB		43 11.00				43 11	43 11	2,111	9-6
														6 S5	SLAB		36 10.00					36 10		NC 886
	6 V151 6 V152			7 1.00 2 7 1.50							7 1			8 S6 8 S7	SLAB SLAB		41 0.00					41 0		17)
20	0 4107	INCR. = 0.25 IN.	20	7 5.25							7 5			6 58	SLAB		47 5.00				47 5	47 5	22,435	THE 1
														6 S9	SLAB		2 2 9.50				2 10	2 10		% 307 307 \$4T
	6 V153			6 6.00							6 6		1 365	C10	INCR. = 10.96 IN SLAB	30	46 8.00				46 8	46 8 5 2	3,644	→ × × × × × × × × × × × × × × × × × × ×
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																								. i.
									<u> </u>	L			J L											MI WAT, INC. -ORD, MURPHY & TILLY, INC. FIEST ELFINDAR (417) 869-6009 EFRING CORPORATION - 000631
Momir	al Long	oths are based on	0.1.1	e out dimons	one ch	hour in	bonding	diagrams and are									Codes C = Required coat	inac wh	oro E - Epo	vy Costod and	$C - C \sim I$	1120 TO d		■ # 2±5 H

Nominal lengths are based on out to out dimensions shown in bending diagrams and are listed to the nearest inch for fabricator's use. Actual lengths are measured along centerline bar to the nearest inch. Weights are based on actual lengths.

All bars shall be Grade 60.

BILL OF REINFORCING STEEL 1

Codes: C = Required coatings, where E = Epoxy Coated and <math>G = Galvanized.

SH = Required shape, see bending diagrams.

V = Sets of varied bars and number of bars of each length. Bar dimensions vary in equal increments between dimensions shown on this line and the following line and the actual length dimension shown on this line and the following line vary by the specified increment.

For bending diagrams and steel reinforcing totals, see Sheet No. 27.

Detailed Dec 2024 Checked Dec 2024

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 28 of 39

					Bill of	f Reinfo	orcir ensions				Nom.	Actu	all						Bill		nforcin Dimensions		eel		Nom.	Actual		THE STATE OF THE S
	Size/ Mark	L —	Codes		C ft in	D	Е		H in f	K ft in	Length		h Weig		No. Siz		Location	Codes	B C	D	E	F	H ft in	K ft in	_	Length	Weight	IIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIII
Req.	Mark		эп	VIII.	10 111.	111. 11	1111.	11. 11.	1111.	IL III.	111	1. 1. 1	11. 11		<u>ец. Ма</u>	I K	Location	C SH V	11. 11. 11	1. 10 111.	111111111111111111111111111111111111111	t 111.	10 111.	10 111.	10 111.	10 111	. 10	III
		BARRIER													\pm	\perp												1
376 196			26 195	3 3.00		2.25 3	1.25	5.50 3	0.75	6.75		_	9 2,6 l1 59		+													
196 180	R3 R4		27S 19S	20.5	9.50 0 9.50	15.25	11.75	12.00	15.00	3.00	4 0 2 6	_	10 78 5 45		-	_												F
180	R5	BARRIER E	275		9.50	15.25	5.00	12.00	15.00	3.00	3 6	3	3 61	1	#	_										<u> </u>		
	R6 R7	BARRIER E	20 20	39 7,00)						11 8 39 7		8 51 7 1,7		士	\pm												С
	R8 R9		20 20	39 3.00 6 1.00							39 3 6 1	_	3 1,7 1 70		-										-	<u> </u>		F
11			20	6 10 0									10 79															L
10			275	3 8.00		5.25 3			5.25	1.00		7 :																L
	K2 K3		27S 27S	3 8.00		14.50 2 14.50	5.75 7.75		14.25	2.75		5	11 35 2 1:		+	+												
	5 K4		195		10.00						3 2	3	1 3 34		_	_										<u> </u>		
10	K5	BARRIER E	385		10.00		9.50		18.00	4.00	3 0	2 :	l 1		#	_										<u> </u>		r
6	5 K6	INCR. = 0.5 IN. BARRIER E	195	2 6.75	10.00	20.50	9.50	8.25	20.00	4.50	3 2	3	1 32 3 2:		+	-												H
6	K7		215	2 2 8.50	2 6.75	10.00		2	6.00	6.25		_	3 2:	1												<u> </u>		
		INCR. = 0.75 IN.		3 2.50	10.00						4 1	3 :	11 69													<u> </u>		
18	6 K9	BARRIER E	215	2	2 8.50	10.00			7.75	6.75 7.75			5 l 1 60		+													
	K10 K11		19S 21S	3 3.00	3 3.00	10.00		3	2.25	7 75	4 1 4 1		0 88 11 86			_										<u> </u>		[]
12	K12	BARRIER E	20	17 5.00)	10.00		3	2.25	7,75	17 5	17	5 21													<u> </u>		1 000
6	K13	BARRIER E	= 20	2 16 8.00 10 8.00							16 8 10 8		8 8 86	5	+	+												ľ
	K14 K15			16 8 00 2 15 11 0								16 l 15	8 20	9	—	_										<u> </u>		
	, 1(15	INCR. = 36 IN.		9 11.0							9 11		11 8:		丰	_												L
53	K22	BARRIER E	275	4 2.75		14.50 3	0.50		14.25	2.75	9 3	9	1 50	3	\pm	\pm												ij
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55	K25	BARRIER E	295		10.00	6.75 3	3.00	3	2.25	7.25	4 8	4	6 25	9		_										<u> </u>		
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22		SIDEWALK E SIDEWALK E	20	41 0.00 11 8.00							41 0	41	0 1,2 8 17	06														INV SAVINIOIT
230	F S15	SIDEWALK E	20	14 6.00)						14 6	14	6 2,2	28	二二													1
230	516	SIDEWALK E	285		6.00	7.50	6.00				1 8	1	5 21	8 -	+	-												
																												
																_												
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\vdash		<u> </u>							$-\top$					$-\!$	+	\dashv		+										
,														⊒ b	土													1

Nominal lengths are based on out to out dimensions shown in bending diagrams and are listed to the nearest inch for fabricator's use. Actual lengths are measured along centerline bar to the nearest inch. Weights are based on actual lengths.

For bending diagrams and steel reinforcing totals, see Sheet No. 27.

All bars shall be Grade 60.

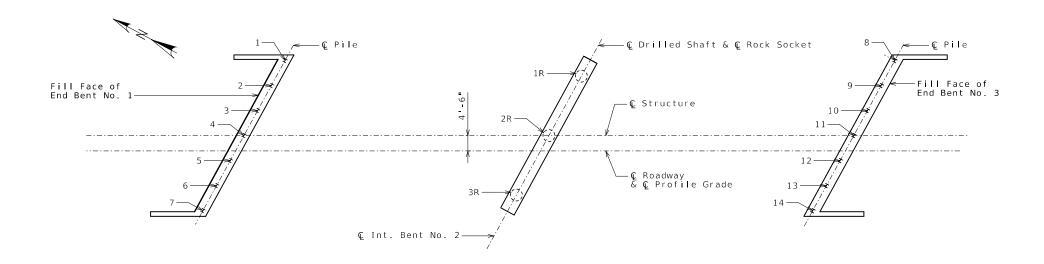
Codes: C = Required coatings, where E = Epoxy Coated and <math>G = Galvanized.

SH = Required shape, see bending diagrams.

V = Sets of varied bars and number of bars of each length. Bar dimensions vary in equal increments between dimensions shown on this line and the following line and the actual length dimension shown on this line and the following line vary by the specified increment.

BILL OF REINFORCING STEEL 2

Detailed Dec 2024 Checked Dec 2024 Note: This drawing is not to scale. Follow dimensions. Sheet No. 29 of 39



PART PLAN SHOWING NUMBERING FOR RECORDING AS-BUILT PILE AND DRILLED SHAFT DATA

			As-Built Pile Data
Pile No.	Length in Place (ft)	Computed Nominal Axial Compressive Resistance (kips)	R ema r k s
			End Bent No. 1
1			
2			
3			
4			
5			
6			
-			
7			
-			

		А	s-Bui	lt Drilled Shaft Data
Shaft No.	Top of Sound Rock (Elev.)	Tip of Casing (Elev.)	Bottom of Rock Socket (Elev.)	Remarks
				Intermediate Bent No. 2
1R				
2R				
3R				

			As-Built Pile Data
Pile No.	Length in Place (ft)	Computed Nominal Axial Compressive Resistance (kips)	R emar k s
			End Bent No. 3
8			
9			
10			
11			
12			
13			
14			

Note:
Indicate in remarks column: A. Pile type and grade
B. BatterC. Driven to practical refusal

This sheet to be completed by MoDOT construction personnel.

1111	PROMI	PE-	2006 (an)	002	324 NO.	THE STATE	Dr.	V 03 1 V 3 C
		MO	shua PE-2	D. J	1:33 a olliff 0282	AM 4		\ \ \V_1
	1	DAT / 1 UTE			02			TOOL
	RD	1 RIC	27	5		Ю	0.	7 1 1 2
		R		NTY		0		14770
			RE	EN				0 411
		J E	SS.	3 1 \CT	56			1
			OJE		NO.			
				SE N	Ю.			U 1 1 1
			19:	31	7		Н	
DESCRIPTION								A STINES VIINGETOF IN MATERIAL TO THE MODEL WITH THE THEOLOGY IN THE ACT.
DATE								
MISSOURI HIGHWAYS AND TRANSPORTATION	COMMISSION				105 WEST CAPITOL	JEFFERSON CITY, MO 65102	1-888-ASK-MODOT (1-888-275-6636)	
					o			

AS-BUILT PILE AND DRILLED SHAFT DATA

Job No.: 20	0221081.00	Con County: Gr				aterials	Rout	e: I-44		Page
	-	Skew:							ield, Missouri	
		Logged By:						ator: RK	,	-
		Northing: _5							/26/22-09/26/22	
		Easting: 13						Personal Participation of the Control of the Contro	LOILL GOILGILL	
	1220.8	Requested N								
	Station:	Requested E								
	Offset:	Equipment:					111110	onange		
	Elevation:	Location No			ager Out	ings, iva				
Drill No.:		Hammer Effi	-		2/6		Drill	ing Method:	FΑ	
		Hummer Em	Cicioy.					ing metriod		
9			L.	ype	% %	unts		ata	sts	1 2
Depth (ft) Graphic	Description		Elevation (ft)	Sample Type	REC % (RQD %)	SS		Shear Data	Field Tests	Index Tests
_ ලි	*		ä	Sam	표 교	Blow Counts (N ₆₀)		She	Field	2
0				"		ш				_
	0.0-0.5' Portland cement concrete		1220	Ь						
	0.5-0.7' Crushed limestone (1.66")		_							
	0.7-25.0' (CH) Brown, fat CLAY		ļ -	1						
-////			-							
5			-	ı						
-////			1215	1						
			+ -							
			† -							
10			[]							
			1210							
			ļ -	1						
-////			-							
-////			+ -	ı						
15			1205							
			.200							
			I -							
			[]							
20			_							
			1200	1						
-////			-							
-////			+ -							
25			† -							
20 ////	Refusal at 25.0 fee		† -							
	Bottom of borehole at 25									
N = (Em/80)	Nm N - Corrected Number for standard	I ROW SDT official	nov: Em	Mac	Lurad bar	mer efficience	in perc	ant: Nm . Obser	ned Mayaluc	I
(1) = Assume	Nm N_{ec} - Corrected N value for standard d, (2) = Actual	100% OF LETTICIEN	icy, Em	- ivieas	sured nam	mer enficiency	in perce	HIL, INTH - ODSERV	eu IN-Valuë	
Coordinate								Coordinate Pro	oj. Factor:	
Coordinate	Datum:	Coordin	nate Un	its:	U.S. Su	rvev Feet				

	chalcal, inc.	221081.00		struc	tion	and M	ransportation aterials	Route: I-44		Page 1	
		221081.00	Skew:					Location: Springfield, Missouri			
			Logged By:					Operator: RK			
		-	Northing: 5					Operator: <u>RK</u> Date of Work: 09/26/22-09/26/22			
			Easting: 13					Depth to Water:			
		1221.3	Requested N					Depth Hole Open:			
		tation:	Requested E					Time Change:			
		Offset:		-			n Sampler, NQ				
400		Elevation:	Location Not							70	
Drill	No.:		Hammer Effic	ciency	: _96	%		Drilling Method: _	FA		
Depth (ft)	Graphic	Description		Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N _{co})	Shear Data	Field Tests	Index Tests	
0	80888	0.0-0.8' Portland cement concrete	(9.6")	-	-						
- ·		0.8-1.0' Crushed limestone (2.4") 1.0-3.8' (CH) Brown, fat CLAY, gravel	Ī	1220	X	44	4-6-6 (18)		PP = 3.50 tsf	MC = 35.	
5	.00	3.8-8.0' Tan, highly weathered LIM	ESTONE		X	80	20-44-50/0.3'			MC = 13	
	000			1215		0	50/0.2'	\exists			
 10		8.0-14.0' LIMESTONE, gray, moderately to completely weath coarsely crystalline, thin bedded	soft to hard nered, finely to	-	Ш	86 (0)					
- ·		8.5' - pitted below 8.5 ft.		1210		80 (0)					
15		14.0-15.0' - possible clay washo 15.0 ft. 15.0-18.0' LIMESTONE, gray,		↓ -	H						
		moderately to completely weath coarsely crystalline, thin bedded 18.0-20.0" - possible clay washo 20.0 ft.	nered, finely to	-		60 (0)					
<u>20</u> 		20.0-30.0' LIMESTONE, gray, h weathered, finely to coarsely bedded	nard, moderately crystalline, thin	1200	H	100					
 25		20.0' - pitted from 20.0 to 25.0 ft.		-	Ш	(0)					
		25.0' - slightly to moderately weath ft.	nered below 25.0	1195		100 (85)		Qu Test Results UCS = 1410 ksf			
30		Refusal at 7.0 feet Bottom of borehole at 30		-							
	Em/60)N		.0 feet.	cy; Em	- Meas	sured han	nmer efficiency in	percent; Nm - Observ	ed N-value		
		System:						_ Coordinate Pro			
Coor	dinate [Datum:	Coordin	ate Un	its: _	U.S. Su	rvey Feet	_			



Detailed Dec 2024 Checked Dec 2024

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 31 of 39



DATE PREPARED
1/17/2025

ROUTE STATE
CRD 127 MO
DISTRICT SHEET NO.
BR 31

GREENE JOB NO.
J8S3156
CONTRACT ID.

BRIDGE NO.

Job No.: 20221031.00 Design: Bent: Station: Offset:	Count	Constructy: Greene	ction	and Ma	terials	Route: I-44		Page 1	
Design:		:				Location: Springfield, Missouri			
Bent:		e d By : JM				Operator: RK Date of Work: 09/30/22-09/30/22			
Station:		ing: <u>517058</u>							
Offset:		ng: <u>1398279</u>				Depth to Water:			
Elevation: 1236.3		ested Northir				Depth Hole Open:			
Requested Station:		ested Easting				Time Change:			
Requested Offset:		ment: CME				,			
		ion Note:			,				
Drill No.:		ner Efficienc		%		Drilling Method: _F	-A		
Depth (ff) (ff)	Description	Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests	Index Tests	
0	I) Daddiah harren fet OLA)	· +							
0.0-3.5° (CI roots, sand,	H) Reddish brown, fat CLAY and gravel	Y, trace 1235	X	56	4-9-23 (48)		PP >4.50 tsf	MC = 10	
3.5-6.0' (GP) Gravel, trace red, fat CLAY	+	X	28	7-10-11 (32)			MC = 11	
6.0-13.5' (C sand and gra	CH) Reddish brown, fat CLA avel	Y, trace	<u> </u>	83	2-3-4 (11)		PP = 3.00 tsf	MC = 48	
10		<u> </u>	X	100	1-3-4 (11)		PP = 2.00 tsf	MC = 52	
	CH) Reddish brown, fat CLA'	1225	5		2-2-3			MC = 54	
15 sand	OTI) REGUIST BIOWI, THE GEA	1220		100	(8)		PP = 2.50 tsf		
- 18.5-23.8' (20 sand and gra	CH) Reddish brown, fat CLA' avel	Ī	X	100	1-2-2 (6)		PP = 1.50 tsf	MC = 57.	
		1215	-						
	LIMESTONE, gray, hard, mo coarsely crystalline, thinly bedde		- -)_	97 (86)	50/0.3'	Qu Test Results UCS = 1100 ksf	PP = 1.50 tsf	MC = 40	
			-	92 (85)		Qu Test Results UCS = 1090 ksf			
31.5' - slight	ly to highly weathered below 31	1.5 ft.	- - -	100 (100)		Qu Test Results UCS = 1170 ksf			
N ₆₀ = (Em/60)Nm N ₆₀ - Correc (1) = Assumed (2) = Actual	eted N value for standard 60% SPT	Γ efficiency; En	ı - Mea		ner efficiency i	n percent; Nm - Observe	ed N-value		
		Coordinate Z					j. Factor:		
Coordinate Datum:			-				-		

	naical, inc. 0. : 202	221081.00	County: <u>G</u>	nstruc reene	tion	and Ma	terials	Route: _I-44		Pag	
			Skew:					Location: Springfiel	d, Missouri		
			Logged By:					Operator: RK			
			Northing: _					Date of Work: 09/30/22-09/30/22			
			Easting: 1					Depth to Water:			
		236.3	Requested					Depth Hole Open:			
		ation:	Requested	Easting				Time Change:			
Reque	ested Of	ffset:	Equipment:	CME S	550,S	plit-Spoon	Sampler, NO	Q			
Reque	ested El	evation:	Location No	ote:							
Drill N	No.:		Hammer Ef	ficiency	: 96	%		Drilling Method: FA	<u> </u>		
Depth (ft)	Graphic	Description		Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests		
35		35.0-41.5' LIMESTONE, gra	y, hard, slight	tly							
	井	weathered, coarsely crystalline, t	hinly bedded	1200	Н						
				1							
 40	\Box			+ -	۱I	100 (100)		Qu Test Results UCS = 1430 ksf			
				1195	11						
	\vdash	Refusal at 23.5 fe	eet.	1100							
Coord	ordinate Datum: Persons using this information are cautioned that the ereby and by judgement of the operator. THIS NFOR		Coordi	inate Zo inate Un	ne: _ iits: _	U.S. Surv	ey Feet	Coordinate Proj.	Factor:	ed	

CF	ROI ROI	MO DAT / 1 JUTE 1 RIC R G	2006 2005 2025 2025 2025 2025 2025 2025	111:4 100.0 100.0 100.0 111:4 111:4 100.0 111:4	11:48 // DOBBER STANDARD STAND	5 ATE	0.
		PR	OJE	CT I	NO.		
			19	3 1	о. 7		
DESCRIPTION							
DATE							
MISSOURI HIGHWAYS AND TRANSPORTATION	COMMISSION				105 WEST CAPITOL	JEFFERSON CITY, MO 65102	1.888-756-888-1/ TOOM-424-1
					0		

geomen	nical, inc.	221081.00	•	struc	tion	and Ma	iterials	Route: I-44		Page 1		
Docine		221081.00						W-0.54880000				
Bont:								Location: Springfield, Missouri Operator: RK				
								Date of Work: 09/3	28/22 00/20/22			
Offcot								NO CONTRACT PROCESSION AND ASSESSMENT OF THE PROCESSION A				
Florest		219.8						Depth to Water: Depth Hole Open:				
Poguo		tation:						Time Change:				
Pogue		ffset:										
Reque		levation:			550,0	piit-opoori	Campici, IVQ					
Drill N			Hammer Effi		: 96	%		Drilling Method: F	·A			
1			. Hammer Em		. <u> </u>			Drilling Method: FA				
Depth (ft)	Graphic	Description		Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests	Index Tests		
0		0.0-19.8' (CH) Reddish brow	n, fat CLAY, trace									
		sand, gravel, and chert 2.0' - 1.5" piece of limestone o	n top of split spoon.		X	17	3-5-17 (33)		PP = 1.50 tsf	MC = 16.		
5				1215	X	89	7-3-5 (12)		PP = 1.50 tsf	MC = 53.		
Elevat Reque Reque Reque Drill N					X	78	7-2-3 (8)		PP = 1.00 tsf	MC = 55.		
10				1210	X	39	2-2-3 (8)		PP = 1.50 tsf	MC = 51.		
15				1205	X	50	1-3-4 (11)		PP = 1.00 tsf	MC = 33.		
		18.5' - trace sand below 18.5 f					1-1-51	_	DD1.00.1.6	MC = 42.		
20	· 0.	19.8-22.5' LIMESTONE, highly	40	1200		28	(78)	\dashv	PP = 1.00 tsf			
25		22.5-35.0' LIMESTONE, gra weathered, coarsely crystalline				90 (90)		Qu Test Results UCS = 1410 ksf				
				-		97 (87)		Qu Test Results UCS = 1220 ksf				
30 35 N ₈₀ = (E (1) = As Coord Coord				1190		100 (80)		Qu Test Results UCS = 1280 ksf				
35				1185								
N ₆₀ = (E (1) = As		m N _{sc} - Corrected N value for stan , (2) = Actual (ystem:						n percent; Nm - Observe Coordinate Pro				
Coord		atum:							,			

	o.: 202	221081.00	County: Gre				aterials	Route: _I-44			
Desig	ın:		Skew:					Location: Springfie	ld, Missouri		
Bent:			Logged By:	JM				Operator: RK			
Statio	n:	ч-	Northing: 5					Date of Work: _09/28/22-09/29/22			
Offset	t:		Easting: 13					Depth to Water:			
Elevat	tion: <u>1</u>	219.8	Requested N					Depth Hole Open: _			
		tation:	Requested E	asting:	_			Time Change:			
Reque	ested O	ffset:			550,S	plit-Spoor	Sampler, NC	Í.			
		levation:	Location Not								
Drill N	No.:		Hammer Effi	ciency:	: <u>96</u>	<u>%</u> □ □		Drilling Method: _F	Α	I	
Depth (ft)	Graphic	Description		Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests	Index Tests	
35 40		35.0-48.0' LIMESTONE, gray weathered, coarsely crystalline, th	/, hard, slightly nin bedded, vuggy	1180		100 (95)		Qu Test Results UCS = 1530 ksf			
 45				1175		98 (98)		Qu Test Results UCS = 1170 ksf			
 		Refusal at 22.5 fe	8	+ - + - + -	Ш	100 (89)		Qu Test Results UCS = 1270 ksf			
		Bottom of borehole at 4	8.0 feet.								
		m N_{∞} - Corrected N value for standa (2) = Actual ystem:						in percent; Nm - Observe Coordinate Proj			
		atum:				U.S. Sur		_			

		PR	OJE	CT I	ID.		
		BF	1D0	GE N	ю. 7		
DESCRIPTION							
DATE							
MISSOURI HIGHWAYS AND TRANSPORTATION	COMMISSION		MoDOT		105 WEST CAPITOL	JEFFERSON CITY, MO 65102	1-888-ASK-MODOT (1-888-275-6636)
	<u> </u>	_		IRPHY & TILLY, INC.	FINDALE STREET NO 65807 (417) 869-6009	CORPORATION - 000631	

DATE PREPARED
1/17/2025

ROUTE STATE
CRD 127 MO
DISTRICT SHEET NO.
BR 33

GREENE JOB NO. J8S3156

BORING DATA
Note: For locations of borings, see Sheet No. 1.

	51 mical,inc. o.: 202	221081.00			struc	tion	and M	ransportati aterials	On Route: <u>I-44</u>	BORIN	G NO. B-2 Page 1		
				Skew:					Location: Springfield, Missouri				
				Logged By:					Operator: RK				
				Northing: 5					Date of Work: _09/2	and the same and t			
				Easting: 13					Depth to Water:				
		221.5		Requested N					Depth Hole Open:				
		ation:		Requested E					Time Change:				
		fset:		Equipment:									
Reque	sted El	evation:		Location No	te:								
Drill N	o.:			Hammer Effi	ciency:	96	%		Drilling Method: F	A			
Depth (ft)	Graphic	Descrip	tion		Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests	Index Tests		
0 		0.0-3.9' (CH) Reddish gravel			1220	}							
1	6 P C	3.9-4.5' LIMESTONE, hi		ered	-								
		Bottom of bore	at 4.5 feet. ehole at 4.5	feet.									
		m N _{ec} - Corrected N value for (2) = Actual system:			nate Zo	ne: _			n percent; Nm - Observe				
	coordinate System:		Coordinate Units: U.S. Survey Feet										

904101	o.: _20	221081.00	County: Gre			ana 1416	aterials	Route: 1-44			
Desig	n:		Skew:					Location: Spri	ngfield, Missouri		
Bent:			Logged By:	JM				Operator: RK			
Statio	n:		Northing: 5					Date of Work: 09/27/22-09/27/22			
			Easting: 13								
Eleva	tion: _	1220.2	Requested N	lorthin	g:						
		tation:	Requested E	-							
		offset:			550,S _l	plit-Spoor	n Sampler, She	elby Tube, NQ			
		levation:	Location Not		000	17		Daillian Mathad	- FA		
Drill N	NO.:		Hammer Effi	Crency	969	/o		Drilling Method	: <u>FA</u>	Ι	
Depth (ft)	Graphic	Description		Elevation (ft)	S	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests		
0		0.0-0.8' Portland cement concre	te (9.6")	1220	+						
.]		0.8-1.0' Crushed limestone (2.4 1.0-8.9' (CH) Reddish brown, fa and gravel			X	33	6-5-6 (17)		PP = 3.00 tsf	MC =	
5				1215		31			PP = 2.00 tsf	IVIC -	
					X	78	2-2-4 (9)		PP = 1.00 tsf	MC =	
10	• 00	8.9-11.3' LIMESTONE, tan, hig Reddish brow, fat CLAY	hly weathered and	1210	X	56	32-14-14 (42)		PP = 1.00 tsf	MC =	
	0 0				-	100	50/0.3'		PP = 0.50 tsf	MC =	
		12.5' See Boring B-301A for of information below 11.3 feet. Refusal at 11.3 Bottom of borehole at	eet.								
Coord	linate S	Im N _{to} - Corrected N value for standa , (2) = Actual System:	Coordin	nate Zo	ne: _		W. 11		served N-value Proj. Factor:		



	lo.: 20	221081.00	Con: County: Gre		tion	and Mat	erials	Route: I-44		Page 1	
			Skew:					Location: Springfie	eld, Missouri		
		-	Logged By:	JM				Operator: RK			
			Northing: 5	16901	.6852	!		Date of Work: 09/27/22-09/27/28			
			Easting: 13	98359	.7372	!		AC COMPANY THE REAL PROPERTY OF THE PERSONS			
		1220.2	Requested N	orthing	g:			Depth to Water: Depth Hole Open:			
		tation:	Requested E	asting	-						
		Offset:	Equipment:								
		levation:	Location Not	e:							
Drill I			Hammer Effic	ciency	: 96	%		Drilling Method: _F	A		
Depth (ft)	Graphic	Description		Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests	Index Tests	
0	808060	0.0-0.8' Portland cement concrete	(0 6")	1220							
		0.8-1.0' Crushed limestone (2.4")	(9.0)	-	▎₽						
		1.0-4.0' (CH) Reddish brown, fat C and gravel			1						
5	口	4.0-4.5' Tan, highly weathered LIN 4.5-19.5' LIMESTONE, gray, h		1215							
		weathered, coarsely crystalline, thi		1.	41						
				-	łl	100 (61)		Qu Test Results UCS = 1570 ksf			
				-	11						
10		9.5' - slightly to moderately weathe	red below 9.5 ft	1210	Н						
-	井	ore originally to moderately mediane			11						
		11.7' - chert inclusion.		1 -	H	98 (90)		Qu Test Results UCS = 1260 ksf			
	井	12.9' - chert inclusion.		-	11						
15	中			1205	Н						
	井			1200	11						
	H				11	100 (70)		Qu Test Results UCS = 1470 ksf			
	中	18.0' - highly weathered		ļ .	H	(70)		0CS = 1470 KS1			
-: :-	苗			-	Ш						
		Refusal at 4.5 feel Bottom of borehole at 19									
N _{so} = (Em/60)N	lm N_{cc} - Corrected N value for standard, (2) = Actual	60% SPT efficien	cy; Em	- Mea:	sured hamm	er efficiencvi	in percent; Nm - Observe	d N-value		
		System: Datum:						Coordinate Proj	i. Factor:		

TSI geefechaical, inc.		Missouri I Cor			nt of Tra		ion	BORING	G NO. B-30 Page 1 of	
Job No.: _20	221081.00	County: Gr				-	Route: 1-44			
Design:		Skew:					Location: Springf	ield, Missouri		
Bent:		Logged By:	JM				Operator: RK			
Station:		Northing: _	16901.	.8891			Date of Work: 09	/27/22-09/27/22		
Offset:		Easting: 13	398299	.7448			Depth to Water: _			
Elevation: _	1220.8	Requested I	orthin	g:			Depth Hole Open:			
Requested S	Station:	Requested I	asting	:			Time Change:			
Requested C	Offset:	Equipment:	CME	550,A	uger Cutti	ngs, NQ				
Requested E	Elevation:	Location No	te:						50	
Drill No.:		Hammer Eff	iciency	/: <u>96°</u>	%		Drilling Method: FA			
Depth (ft) Graphic	Description		Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests	Index Tests	
0	0.0-0.8' Portland cement concrete ¬ ¬ ¬ ¬ 0.1' - rough drilling	e (9.6")	1220	-						
- <i>////</i>	0.8-1.0' Crushed limestone (2.4")		+ .	┨						
	1.0-4.0' (CH) Red, fat CLAY, trac	e and and gravel	†							
5 00	4.0-5.5' LIMESTONE, highly wea	thered	Ť							
3 00	Refusal at 5.5 fee	at .	-	1	.					
	Bottom of borehole at 5									
Coordinate S	Nm N_{∞} - Corrected N value for standar \mathbf{i} , (2) = Actual System :	Coordi	nate Zo	one: _			in percent; Nm - Observ			

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geoleci	nnical,inc.	04004.00	Cor	nstruc			ransportati aterials			Page 1
		21081.00	County: G					Route: 1-44		
			Skew:						gfield, Missouri	
			Logged By:					Operator: RK		
			Northing:					X-1963-0 1-00 NA-3-1-1000	9/22/22-09/22/22	
		244.7	Easting: _1:							
		241.7	Requested I						:	
		ation:	Requested I							
		fset:				piit-Spoor	n Sampler, NQ			
Drill N		evation:	Location No			0/.		Drilling Method:	ΕΛ	
Dilli	io.:		Hammer Eff	Iciency	90	70		Drilling Method:		
	0			_	,pe	% %	Blow Counts (N ₈₀)	ata	ts:	sts
Depth (ft)	Graphic	Description		Elevation (ft)	le T	% Q Q	9 9	ا م	Ĕ	, Te
۵	5	and the same of the same of		Ele	Sample Type	REC (RQD	w e	Shear Data	Field Tests	Index Tests
0					55		Ω			_
J		0.0-29.7' (CH) Reddish brown, fa	t CLAY	1 -						
				1240	X	44	4-12-27 (59)		PP = 4.50 tsf	MC = 12.
				+ -			(00)			
				İ -	∇	17	6-11-4 (23)		PP = 4.50 tsf	MC = 6.
<u> </u>							(23)	\dashv		
-		6.0' - trace and and gravel.		1235	X	72	2-2-3 (8)		PP = 1.50 tsf	MC = 55
				Ī .			2-2-3	_		MC = 55
10				-	X	61	(8)		PP = 2.50 tsf	
				4000						LL = 1:
				1230	X	33	1-2-3 (8)		PP = 1.00 tsf	PL = 3 MC = 38
				1						
15					X	89	0-1-2 (5)		PP = 1.00 tsf	MC = 59
				-	Г					
				1225	1					
				+ -	Ĺ.,					
20					\mathbb{X}	6	1-1-1 (3)		PP = 1.50 tsf	MC = 38
				-	Γ,					
_				1220	1					
-: :-				+						
25				1	\mathbb{X}	22	0-0-1 (2)		PP = 0.50 tsf	MC = 64
				-	Γ,					
				1215	-					
				+ -	L					
		29.0-29.7' LIMESTONE, heavily	weathered		\geq	100	1-50/0.0'	_	PP = 1.00 tsf	MC = 50
		Refusal at 29.7 fe	eet.							
		Bottom of borehole at 2	.9./ TEET.							
N = /	-m/60\Nin	n N - Corrected Nivalue for stands	rd 60% SDT efficio	ncv: Em	- Mess	sured how	mer efficiency	n nercent: Nm - Obco	ned Navalue	
		n N_{gc} - Corrected N value for standar (2) = Actual				Juieu Hall	mer emolericy i			
		ystem:		nate Zo		W1 10	21	Coordinate P	roj. Factor:	
Coord	inate Da	atum:	Coordi	nate Un	its: _	U.S. Sur	rvey Feet			

	o.: 20	221081.00	Co County: _G		tion	and Ma	aterials	Route: _I-44		Pag
								Location: Spring	gfield, Missouri	
			Logged By:					Operator: AM		
Statio	n:		Northing: _	517054.	8656			Date of Work: _0	9/30/22-09/30/22	
Offset	t:	,	Easting: _1	398187.	.8349			Depth to Water:		
Elevat	tion: _1	1229.1	Requested	Northin	g:			Depth Hole Open	:	
Reque	ested S	tation:	Requested	Easting	:			Time Change: _		
Reque	ested O	ffset:	Equipment	CME :	550,S	plit-Spoor	n Sampler, Sh	elby Tube, NQ		
Reque	ested E	levation:	Location N							
Drill N	No.:		Hammer Ef	ficiency	: 969	<u>%</u>		Drilling Method:	FA	1
Depth (ft)	Graphic	Description		Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N _{co})	Shear Data	Field Tests	
0 -		0.0-7.5' (CH) Reddish brown, sand, gravel, and organics	fat CLAY, tra	ce	X	50	4-5-7 (18)		PP >4.50 tsf	MC :
5				1225	X	67	2-4-4 (12)		PP = 3.00 tsf	MC
-		and the second control of the second		+ .	\geq	86	2-50/0.1		PP = 1.50 tsf	MC:
-		6.5-7.5' LIMESTONE, highly wea Refusal at 7.5 fee	N. Secondaria	+ .		0	50/0.2'			
Coord Coord	linate S linate D	m N _∞ - Corrected N value for standar (2) = Actual iystem:	Coord	inate Zo inate Un	one: _ nits: _	U.S. Sur	vey Feet	Coordinate P	roj. Factor:	

MILLIAM	S PROMI	D. PE-		HUA LLIF BER 10021		III	WIIII.
	0	1/17/3 MO	2025 shua	11:4:	2:13 olliff 0282	AM	
CF	ROI RD	/ 1 JTE 1 RIC R	27 T COL	/ 2	ST. MHEE	5 ATE	ο.
		CON	SS:	3 1 ICT	56		
		BF	RIDO	CT I	Ю.		
		F	19:	31	7		
DESCRIPTION							
DATE							
MISSOURI HIGHWAYS AND TRANSPORTATION	COMMISSION				105 WEST CAPITOL	JEFFERSON CITY, MO 65102	1-888-ASK-MODOT (1-888-275-6636)
	-		ı	& TILLY, INC.	LE STREET 5807 (417) 869-6009	ORATION - 000631	

Job No.: 20	0221081.00	Con:		tion	and Ma	aterials	Route: _I-44		Page 1	
		Skew:					Location: Springfield, Missouri			
		Logged By:					Operator: AM			
		Northing: 5						9/22/22-09/22/22		
		Easting: 13					A SUMPLY SHOULD AND SHALLOWS			
	1235.6	Requested N						i:		
	Station:	Requested E								
	Offset:	-	_							
	Elevation:	Location Not				,				
Drill No.:		Hammer Effic			%		Drilling Method:	_FA		
Depth (ft) Graphic	Description		Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests	Index Tests	
0	0.0-8.0' (CH) Reddish brown, fat C	I AY, trace sand	1235				-			
		,	-	X	72	6-9-13 (33)		PP = 4.00 tsf	MC = 33.5	
5			1230	X	44	2-7-3 (15)		PP = 4.00 tsf	MC = 30.4	
				×	100			PP = 1.50 tsf	MC = 42.	
10	8.0-13.5' LIMESTONE, highly Reddish brown, fat CLAY (CH)	weathered and	1225	X	67	4-50		PP = 1.50 tsf	MC = 41.9	
			-	×	40	1-50/0.3'		PP = 2.00 tsf	MC = 28.0	
15 ° P	13.5-18.5' LIMESTONE, highly v Reddish brown, fat CLAY (CH)	veathered, trace	1220	X	56	7-39-26 (98)			MC = 26.2	
		,	-	X	56	20-43-25 (102)			MC = 7.8	
	Refusal at 18.5 fee Bottom of borehole at 18									
	Nm N_{ec} - Corrected N value for standard d, (2) = Actual									
Coordinate	System: Datum:						Coordinate F	Proj. Factor:		

gested lob N	halcal, inc.	221081 00		struc			aterials	Route: 1-44 Location: Springfield, Missouri			
		221081.00	County: Gre								
Posts			Skew: Logged By:					Operator: A			
Statio		*	Northing: _5					Date of Work: 09/20/22-09/20/22			
Office								Date of Work: _09/20/22-09/20/22			
		1044.0	Easting: 13								
Eleva		1241.0	Requested N						pen:		
18		tation:	Requested E	_					:		
		Offset:				piit-Spoor	i Sampier, Sn	elby Tube, NQ			
Reque		levation:	Location Not			.,					
Drill N	No.:		Hammer Effi	ciency	: <u>96°</u>	<u>%</u> □ □		Drilling Meth	od: FA		
Require	Graphic	Description		Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests	Index Tests	
0		0.0-3.5' CHERT, white and Red trace clay	dish brown gravel	1240		33	15-30-39			MC=9	
		2.5.24.51 Doddiek berrye 61.6	NAV with				(104)			MC = 2	
5		3.5-31.5' Reddish brown, fat (trace organics	LAY WITH Gravel	1235		56	35-50		PP >4.50 tsf		
-				-	X	72	6-13-7 (30)		PP = 3.00 tsf	MC = 5	
10						83			PP = 2.50 tsf	MC = 5	
				1230	X	100	3-3-4 (11)		PP = 2.50 tsf	MC = 4	
15					X	44	3-4-50 (81)		PP = 1.50 tsf	MC = 5	
				1225	X	56	3-9-14 (35)		PP = 1.50 tsf	MC = 3	
20					X	61	6-4-3 (11)		PP = 2.00 tsf	MC = 5	
- - -				1220	X	100	2-2-12 (21)		PP = 3.50 tsf	MC = 5	
25					X	100	2-2-3 (8)		PP = 1.00 tsf	MC = 6	
 - -				1215	X	100	6-2-2 (6)		PP = 1.00 tsf		
		29.0-31.5' LIMESTONE, heavily	weathered	ļ :	\times	32	4-50/0.3'		PP = 1.50 tsf	MC = 1	
			9	1210							
30 35 N _{ex} = (1		31.5-41.5' LIMESTONE, gray, weathered, finely crystalline, thin	nard, moderately bedded			75 (60)					
	Em/60)N	34.0' - 6" drop observed. Im N _⊚ - Corrected N value for standa	rd 60% SPT efficier	ncy; Em	- Meas	Name	mer efficiency	in percent; Nm - C	Observed N-value		
(1) = À	ssumed	, (2) = Actual									
Coord		System:			_			Coordina	te Proj. Factor:		
1		Datum:			entra liera	U.S. Sur		_	ne "log of materials" is limit		



/TSi geoffethical.inc. Job No.: 20221081.00	Missouri Department of Tra Construction and Mat County: Greene	
Design:	Skew:	Location: Springfield, Missouri
Bent:	Logged By: _JM	Operator: _AM
Station:	Northing: _516831.9202	Date of Work: 09/20/22-09/20/22
Offset:	Easting: 1398296.9445	Depth to Water:
Elevation: 1241.0		
Requested Station:	Requested Easting:	Time Change:
Requested Offset:	Equipment: CME 550,Split-Spoon S	Sampler, Shelby Tube, NQ
Requested Elevation:	Location Note:	
Drill No.:	Hammer Efficiency: 96%	Drilling Method: FA
Description	Elevation (ft) Sample Type REC % (RQD %)	Blow Counts (N ₆₀) Shear Data Field Tests
31.5-41.5' LIMESTONE, weathered, finely crys (continued)	gray, hard, moderately 1205 lalline, thin bedded 100 (93)	Qu Test Results UCS = 1270 ksf
Coordinate System:	tandard 60% SPT efficiency; Em - Measured hammer Coordinate Zone: Coordinate Units: U.S. Surve	Coordinate Proj. Factor:

poercon	alcal, inc.	221081.00	Con County: Gre		tion	and M	aterials	Page 1 c			
		2001.00									
							-				
								Depth to Water:			
		1241.6						Depth Hole Open:			
		tation:	Requested E					Time Change:			
Reques	sted O	ffset:						Sampler, Shelby Tub	e, NQ		
Reques	sted E	levation:	Location Not	e:						-	
Drill No	o.:		Hammer Effi	ciency	: <u>96</u>	%		Drilling Method: _F	A		
Depth (ft)	Graphic	Description		Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests	Index Tests	
		0.0-3.5' (GC) White and bro (GC)	wn clayey GRAVEL	1240		56	18-38-50			MC = 1	
· -		3.5-6.0' (SM) Red, clayey SAI	ND, trace gravel and	<u> </u>		33	(132) 3-17-20		PP = 3.00 tsf	MC = 2	
5		organics] :			(56)		5.00 101		
		6.0-27.4' (CH) Reddish brows sand	n, fat CLAY, trace	1235		100			PP = 3.00 tsf	MC = 2	
 10		10.0' - large chert fragment in	shoe.	ļ :	X	11	2-5-11 (24)			MC = 4	
				1230	X	44	2-3-6 (14)		PP = 2.50 tsf	MC = 5	
- 15					X	100	2-2-3 (8)		PP = 2.50 tsf	MC = 5	
_				1225	X	100	2-3-7 (15)		PP = 1.50 tsf	MC = 6	
20					X	11	3-4-4 (12)		PP = 1.50 tsf	MC = 5	
				1220	X	100	5-3-1 (6)		PP = 1.00 tsf	LL = PL = MC = 3	
 25					X	56	1-1-2 (5)		PP = 1.00 tsf	MC = 3	
			Y with chart	1215	X	44	6-18-32 (75)		PP = 1.00 tsf	MC = 6	
30		27.5'- highly weathered chert. 28.0-38.5' LIMESTONE, gi weathered, finely crystalline, th 30.0'-vuggy for 1/10 ft.	ay, hard, slightly	1210		100 (71)	(O)	Qu Test Results UCS = 1250 ksf			
(1) = As		N_{∞} - Corrected N value for stand N_{∞} (2) = Actual System:									
		9atum:					rvey Feet	Coordinate Pro	y. ractor:		

	MISSOURI HIGHWAYS AND TRANSPORTATION DATE	DATE	
	COMMISSION		
© Copyright CMT, Inc.			
CRAWFORD, MURPHY & TILLY, INC.			
1631 WEST ELFINDALE STREET SPRINGFIELD, MO 65807 (417) 869-6009	105 WEST CAPITOL		
FNGINFFRING CORPORATION - 0006:31	JEFFERSON CITY, MO 65102		
	1.888.475.MODOH.375.688.1		

DATE PREPARED 1/17/2025

ROUTE STATE
CRD 127 MO
DISTRICT SHEET NO.
BR 38

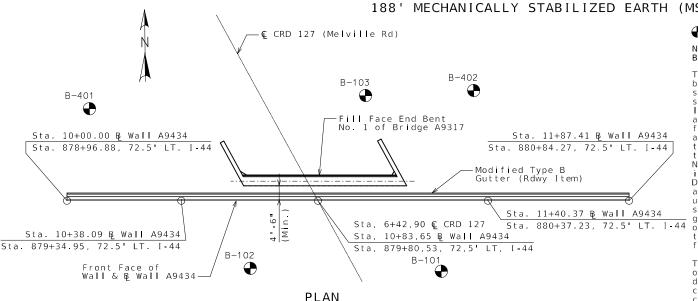
GREENE
JOB NO.
J8S3156
CONTRACT ID.

A9317

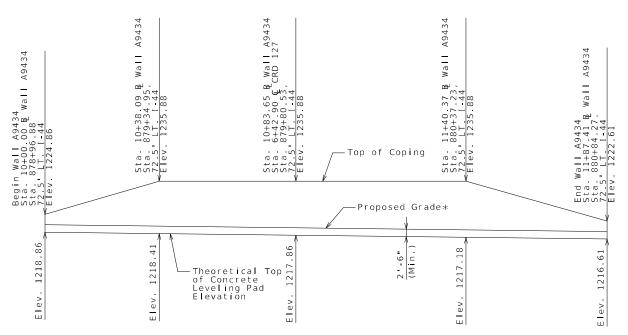
geolechnical, inc.		Coi	nstruct	ion	and Ma	ansportat aterials			Page
Job No.: _202		County: G					Route: 1-44		
	*	Skew:					Location: Springfiel	d, Missouri	
		Logged By:					Operator: AM		
		Northing: _					Date of Work: _09/1		
		Easting: 1					Depth to Water:		
	241.6	Requested					Depth Hole Open: _		
	ation:	Requested					Time Change:		
	fset:	F 180	-	5U,IV	o Recove	гу, Ѕрш-Ѕроо	n Sampler, Shelby Tube	NQ	-
Drill No.:	evation:	Location No Hammer Eff		060)/ ₋		Drilling Method: F/		
DIII No.:		nammer En	inciency.	967	/c		Drilling Method: _F/	٠	П
				e Be	_	nts	ta l	छ	
Depth (ft) Graphic	Description		Elevation (ft)	Sample Type	% 00	Blow Counts (N ₆₀)	Shear Data	Field Tests	
g g	Boompton		Ele	amb	RAD (RAD	NO Z	Shea	Plei:	1
35				S		ā	0,		'
35	28.0-38.5' LIMESTONE, gray,						+		
	weathered, finely crystalline, (continued)	thin bedde	ed 1205		97		Qu Test Results		
			+ -		(97)		UCS = 1180 ksf		
	Refusal at 28.5 fee Bottom of borehole at 38		7 [
	300000000000000000000000000000000000000								
N _∞ = (Em/60)Nn (1) = Assumed, d	n N _{sc} - Corrected N value for standard (2) = Actual /stem:		ency; Em -			mer efficiency	in percent; Nm - Observed		

MISSOURI HIGHWAYS AND TRANSPORTATION DISTRICT BR COUNTY GREENE JOB NO. J8533156 CONTRACT ID. PROJECT NO. BR 1005 NO. A9317
DATE
1010
MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION 105 WEST CAPITOL JEFFERSON CITY, MO 65102 1-888-ASK-WODOT (1-888-275-6636)

188' MECHANICALLY STABILIZED EARTH (MSE) RETAINING WALL SYSTEM



Concrete Leveling pad and coping not shown for clarity. (1) $\ensuremath{\mathbb{R}}$ wall stationing at front face of wall



DEVELOPED ELEVATION

- Concrete Leveling pad not shown for clarity. (1) * Proposed grade shown accommodates future full buildout of I-44.
- (1) Wall contractor shall show the following items on the design drawings and/or on the fabricator shop drawings.
- Leveling pad horizontal.

Designed Nov 2024 Detailed Nov 2024 Checked Nov 2024

Leveling pad length and step elevations shall be based on wall manufacturer's recommendation. Top of leveling pad elevations shall not be higher than theoretical top of leveling pad elevations shown on these plans.

I t em		Total
Aesthetic Concrete Stain	lump sum	1
Concrete and Masonry Protection System	lump sum	1
Sacrificial Graffiti Protection System	lump sum	1
Mechanically Stabilized Earth Wall Systems	sq. foot	2878

MSE Wall Systems Data Table											
Proprietary Wall Sy	s t ems	Comb i	Combination Wall Systems								
Manufacturer	System	Facing Unit Manufacturer	Facing Unit	Geogrid Manufacturer	Geogrid						

MSE Wall Systems Data Table is to be completed by MoDOT construction personnel to record the manufacturer of the proprietary wall system or the manufacturers of the combination wall system that was used for constructing the MSE wall.

🛖 Indicates location of borings.

Notice and Disclaimer Regarding Boring Log Data

The locations of all subsurface borings for this structure are shown on the plan sheet for this structure. The boring data for all locations indicated, as well as any other boring logs or other factual records of subsurface data and investigations performed by the department for the design of the project, are shown on Sheets No. 4 thru 6 and may be included the Electronic Bridge Deliverables. They will also be available from the Project Contact upon written request. No greater significance or weight should be given to the boring data depicted on the plan sheets than is given to the subsurface data available from the district or elsewhere.

The Commission does not represent or warrant that any such boring data accurately depicts the conditions to be encountered in constructing this project. A contractor assumes all risks it may encounter in basing its bid prices, time or schedule of performance on the boring data depicted here or those available from the district, or on any other documentation not expressly warranted, which the contractor may obtain from the Commission.

C CRD 127 (Melville Rd)

-Proposed MSE Wall A9434

-Proposed Bridge A9317

SEC./SUR. 4 RGE, 22W General Notes:

Design Specifications:

2002 AASHTO LFD (17th Ed.) Standard Specifications (Section 5 ASD Design) Seismic Performance Category = A Acceleration Coefficient = 0.09

Design Loading:

 $\theta_D=20^\circ$ and Unit weight, $\gamma_D=120$ pcf for retained backfill material to be retained by the mechanically stabilized earth wall system.

 Φ_f = 34° for improved foundation ground where wall is to bear. Unimproved foundation data is not applicable. Foundation Improvement is required full length of the wall.

For improved foundation ground, the allowable bearing pressure is 5.5 ksf.

The maximum applied bearing pressure for the controlling design case at the foundation level shall be shown on the shop drawings and shall be less than the allowable bearing pressure for foundation ground provided herein. For seismic design the maximum applied bearing pressure shall be less than two times the allowable bearing pressure.

Allowable bearing pressure and limits of improved foundation ground shall not be adjusted from that as shown on the plans.

Contractor shall include design $\Phi_{f'}$ (actual $\Phi_{f'} \ge 34^{\circ}$) and the total unit weight, $\gamma_{f'}$, for the select granular backfill (reinforced backfill and wedge area backfill) for structural systems on shop drawings. Contractor shall identify source of select granular backfill material, submit proctor in accordance with AASHTO T 99 (ASTM D698) and gradation with the shop drawings. When backfill material is too coarse to develop a proctor curve the contractor shall determine the maximum dry density (relative density) in accordance with ASTM D4253 and ASTM D4254 and assume percent passing the 200 sieve for optimum water content.

Total unit weight, γ_f = (95% compaction) x (maximum dry density) x (1 + optimum water content)

Design $\Phi_{\it F}=34\,^{\circ}$ for the select granular backfill (reinforced backfill) only for štructural systems.

Factor of safety shall be 2.0 for overturning and 1.5 for sliding.

For seismic design the factor of safety shall be 1.5 for overturning and 1.1 for sliding.

Use default values for the pullout friction factor, F^* , in accordance with AASHTO figure 5.8.5.2A and default value for scale effect correction factor, α , in accordance with AASHTO table 5.8.5.2A. For approved steel strips not shown in AASHTO figure 5.8.5.2A, use $F^* \leq 2.0$ at zero depth and $F^* \leq T$ an Φ_{r} at 20 feet depth and Φ_{r} design = 34°. F^* and α values shall be shown on the shop drawings.

Design Unit Stresses:

All concrete for leveling pad and coping shall be Class B or B-1 with f'c = 4000 psi.

The minimum compressive strength of concrete for precast modular panel shall be 4,000 psi in accordance with Sec 1052.

Miscellaneous:

The MSE wall system shall be built vertical.

The MSE wall system shall be built in accordance with Sec 720.

The MSE wall system shall be a precast modular panel wall system.

Precast modular panel and coping (or capstone) reinforcement shall be epoxy

A filter cloth meeting the requirements for a Separation Geotextile material shall be placed between the select granular backfill for structural systems and the backfill being retained by the mechanically stabilized earth wall

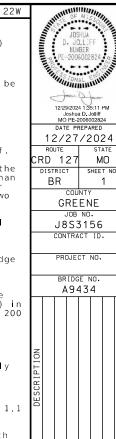
Coping shall be required on this structure. When CIP coping sections extend beyond the limits of a single panel, bond breaker (roofing felt or other approved alternate) between wall panel and coping is required. Coping joints shall use 3/4-inch chamfers and shall be sealed with 3/4-inch joint filler. Coping reinforcement shall terminate 1 1/2-inch minimum from face of coping ioint

The contractor shall be solely responsible to coordinate construction of the wall with bridge and roadway construction and ensure that the bridge and roadway construction, resulting or existing obstructions, shall not impact the construction or performance of the wall. Soil reinforcement shall be designed and placed to avoid damage by pile driving, guardrail post installation, utility and sign foundations. (See Roadway and Bridge plans.)

Control Point #514 N: 517039 25 E: 1397801.35 Elev. 1222.12 Sta. 875+18.07 83.90 Lt. (I-44) 5/8" I.B. W/ Aluminum Cap Control Point #515 N· 516847 15 E: 1399681.98 Elev. 1219.86 Sta. 894+00.43 90.54 Rt. (I-44) 5/8" I.B. W/ Aluminum Cap

RETAINING WALL ALONG WB I-44 NEAR END BENT NO. 1 OF BRIDGE A9317

CRD 127 OVER 1-44 ABOUT 1.2 MILES WEST OF ROUTE 13 BEGINNING STATION 878+96.88 C 1-44

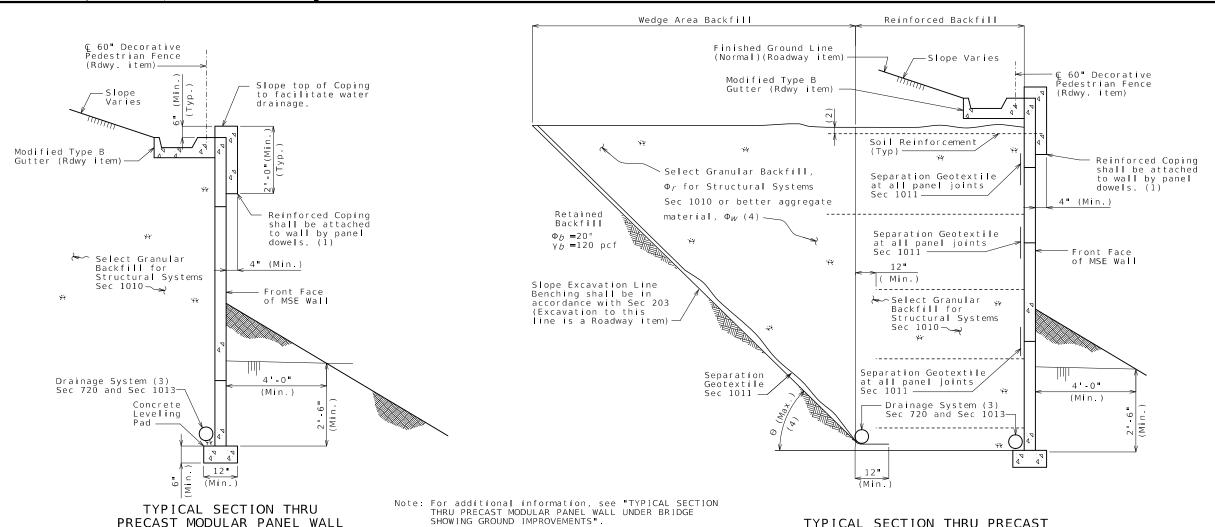


CAPITOL MO 65102 105 JN CI

J, MURPHY & TILLY, IN 'ELFINDALE STREET LD, MO 65807 (417) 8

Proposed MSE Wall A9435—

LOCATION SKETCH



General Notes Cont.:

Minimum 18" wide Geotextile strips shall be centered at vertical and horizontal joints of panel. Geotextile material shall be adhered to back face of panel using an adhesive compound supplied by the manufacturer. All edges of each fabric strip shall provide a positive seal. A minimum 18" overlap shall be provided between spliced filter fabric.

Aluminized soil reinforcement shall have edges coated with coating material

Soil reinforcement shall be spaced to avoid roadway drop inlet behind wall.

All steel soil reinforcements shall be separated from other metallic elements by at least 3 inches.

The splay angle should be less than 15° and tensile capacity of splayed reinforcement shall be reduced by the cosine of the splay angle. Soil reinforcement shall clear the obstruction by at least 3 inches.

No reinforcement shall be left unconnected to the wall face or arbitrarily cut/bent in the field to avoid the obstruction.

Where interference between the vertical obstruction and the soil reinforcement is unavoidable, the design of the wall near the obstruction may be modified using one of the alternatives in FHWA-NHI-10-24, Section 5.4.2. Show detail layout on the drawings. For wall designs with horizontal obstructions in reinforced soil mass, see FHWA-NHI-10-024, Section 5.4.3.

Excavation:

Excavation quantities and pay items are given on the roadway plans. Excavation quantities are based on a soil reinforcement length of 0.8H. The soil reinforcement length may vary based upon the wall design selected by the contractor. Plan excavation quantities will be paid regardless of any actual quantities removed based on the soil reinforcement length and design selected.

- by a minimum of 1 1/2 inches and a maximum of 2 1/2 inches.
- Topmost layer of reinforcement shall be fully covered with select granular backfill for structural systems, as approved by the wall manufacturer, before placement of the Separation Geotextile.
- (3) Minimum 6" diameter perforated PVC or PE pipe.

Manufacturer shall show drain details on design plans to be submitted as shown on MoDOT MSE wall plans and/or roadway plans.

Contractor shall modify the drain details as shown if it will improve flow as may be the case for stepped leveling pad, and for an uneven ground line (approval of the engineer required).

Mat	erial Prop	erties L	Ised in D	esign
	II/Select Backfill	Active Computa		Foundation
Ф°	γ (pcf)	Ф°	γ (pcf)	Ф°

Note: MSE Wall designer shall include table on shop drawings and provide values used in the design computations. Effects of cohesion shall be ignored unless approved by the engineer.

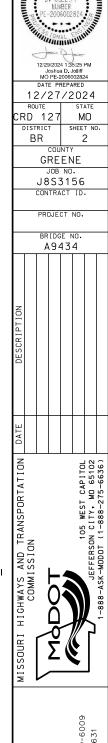
Show range of acceptable theta (Θ) angle on shop drawings which must be consistent with design computations and proposed construction of wall. Show active force computation properties on shop drawings and in design computations. Coordination between wall designer (manufacturer) and contractor is required before shop drawing submittal.

MODULAR PANEL WALL SHOWING FILTER CLOTH

- (1) Inverted U-shape reinforced capstone may be used in lieu of coping. (4) Select granular backfill shall extend a minimum of 12" beyond the end of Panel dowels for level-up concrete shall be required and provided by manufacturer. The dowels shall be field trimmed to clear the capstone all soil reinforcement. Where the angle, Θ , between the retained backfill excavation/fill line and the horizontal is less than 90°, the wedge area backfill between ⊖ and 90° shall be filled with select granular backfill for structural systems meeting the requirements of Section 1010.
 - For $(45^{\circ} + \Phi_b/3) < \Theta \le 90^{\circ}$, properties for retained backfill shall be used for active force computations.

- For $\Theta \leq (45^{\circ} + \Phi_{D}/3)$, contractor shall have the option to use select granular backfill, $\Phi_{ec{r}}$, or better aggregate material, $\Phi_{ec{W}}$ for active force computations in the wedge area backfill. For active force computations, the angle of internal friction for wedge area backfill material, Φ_{Γ} or Φ_{W} , shall be limited to 34° unless determined otherwise in accordance with Section 1010. If $\Phi_{\mathcal{T}}$ or $\Phi_{\mathcal{W}} > 34^{\circ}$ is desired for wedge area backfill then test report shall be submitted with shop drawings. Φ_r or Φ_W shall not be greater than 40° for computations. Final configuration of this option shall be sent to Geotechnical Section for a new overall global stability analysis. Design Φ_W shall be shown on the shop drawings if used.

The slope excavation line shall be benched and separation geotextile shall be placed between the retained backfill and either select granular backfill or better aggregate material, and between the select granular backfill and better aggregate material.



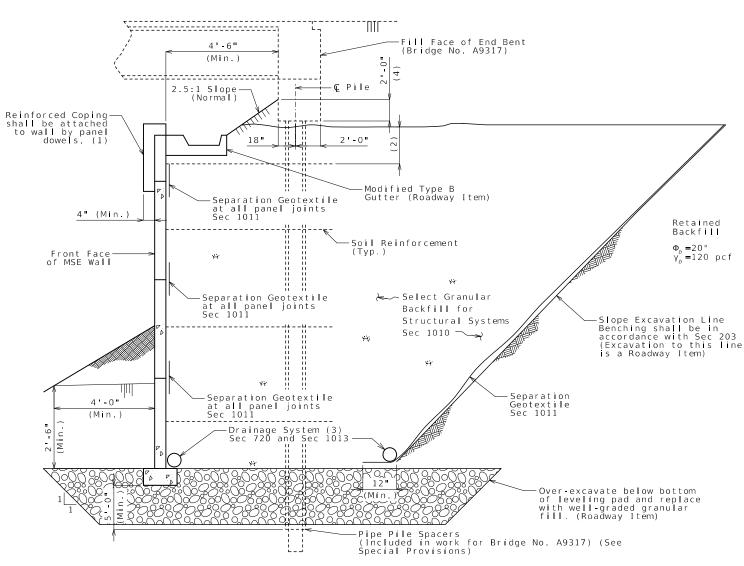
MURPHY & TILLY, ELFINDALE STREET), MO 65807 (417)

DETAILS FOR MSE WALL

Detailed Nov 2024

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 2 of 6



TYPICAL SECTION THRU PRECAST MODULAR PANEL WALL UNDER BRIDGE SHOWING GROUND IMPROVEMENTS

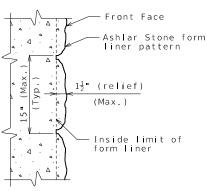
Note: For additional information, see "TYPICAL SECTION THRU PRECAST MODULAR PANEL WALL SHOWING FILTER CLOTH".

- (1) Inverted U-shape reinforced capstone may be used in lieu of coping. Panel dowels for level-up concrete shall be required and provided by manufacturer. The dowels shall be field trimmed to clear the capstone by a minimum of 1 1/2 inches and a maximum of 2 1/2 inches.
- (2) Topmost layer of reinforcement shall be fully covered with select granular backfill for structural systems, as approved by the wall manufacturer, before placement of the Separation Geotextile.
- (3) Minimum 6" diameter perforated PVC or PE pipe.

Manufacturer shall show drain details on design plans to be submitted as shown on MoDOT MSE wall plans and/or roadway plans.

Contractor shall modify the drain details as shown if it will improve flow as may be the case for stepped leveling pad, and for an uneven ground line (approval of the engineer required).

(4) See bridge plans.



FORM LINER DETAIL (PRECAST MODULAR PANEL WALL)

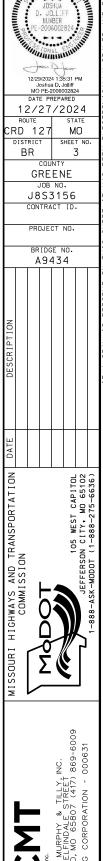
Notes:

The cost of form liners for MSE wall systems, complete in place, will be considered completely covered by the contract unit price for Mechanically Stabilized Earth Wall System.

Form liner shall be constructed in accordance with Special Provisions.

The following is a list of form liner manufacturers and types which may be used. Depth of relief for all form liner pattern's shall vary up to 1 1/2". The height of any single 'stone' shall be 15" maximum.

- Scott System, Inc.: Form liner pattern #167 "Ashlar Stone"
- Fitzgerald Formliners: Form liner pattern #16986 "Ashlar Stone"
- Greenstreak: Form liner pattern #330 "Ashlar Stone"
- Spec Formliners: Form liner pattern #1515 "Ashlar Stone"
- Customrock: Form liner pattern #12020 "Tollway Ashlar"
- An approved equal



DETAILS FOR MSE WALL

geolechnical, inc.		i Departmonstruction					Page 1
Job No.: 20221081.00		Greene		-	Route: _ -44		
Design:					Location: Springfi	eld, Missouri	
Bent:		y: <u>JM</u>			Operator: RK		
Station:		516999.568			Date of Work: _09/		
Offset:	Easting:	1398304.61			Depth to Water: _		
Elevation: 1220.8	Requested	d Northing: _			Depth Hole Open:		
Requested Station:		d Easting:			Time Change:		
Requested Offset:		t: <u>CME 550</u> ,	Auger Cuttin	gs, NQ			
Requested Elevation:		Note:					
Drill No.:	Hammer E	fficiency: 9	6% 		Drilling Method: _	FA I	
Description		Elevation (ft)	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests	Index Tests
0 0.0-0.5' Portland cement concr	ete (6")	⊨ 1220					
0.5-0.7' Crushed limestone (1.	66")		[
0.7-25.0' (CH) Brown, fat CLA	Y	+ -					
///		+ -					
<u>-5</u>		1215					
///		1213					
		1 1	4				
		I]					
10		+ -					
///		1210					
///		2+ 2					
- <i>-{///</i> }		+ -	"				
- 15							
		1205					
		+ -					
///		+ -					
///		1 -					
		1200					
- <i>-{///</i> }		1200					
///			/				
25 Refusal at 25.0 Bottom of borehole a							
N_{60} = (Em/60)Nm N_{60} - Corrected N value for stand (1) = Assumed, (2) = Actual	dard 60% SPT effic	ciency; Em - Me	asured hamm	er efficiency in	percent; Nm - Observ	ed N-value	
Coordinate System:		dinate Zone:					
Coordinate Datum:	Coor	dinate Units:	U.S. Surve	ev Feet			

		*	-				-	Route: 1-44	old Micocuri	
							*	Location: Springfi	ela, Missouri	
								Operator: RK	00/00 00/00/00	
								Date of Work: _09/		
								Depth to Water:		
								Depth Hole Open:		
		· · · · · · · · · · · · · · · · · · ·		_		ess test		Time Change:		
			-			-	n Sampler, NQ			
			tion Note							
Drill N	o.:	Hami	mer Effici	iency:	969	<u>% </u>		Drilling Method: _F	A	
O Depth (ft)	Graphic	Description		Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests	Index Tests
	4 4 b	0.0-0.8' Portland cement concrete (9.6")		1220						
		0.8-1.0' Crushed limestone (2.4") 1.0-3.8' (CH) Brown, fat CLAY, trace s gravel			X	44	4-6-6 (18)		PP = 3.50 tsf	MC = 3
 5	. 0.	3.8-8.0' Tan, highly weathered LIMESTON	+	-	X	80	20-44-50/0.3'			MC = 1
[99		+	1215	-	0	50/0.2'			
_	· 0.		+	-	П					
10		8.0-14.0' LIMESTONE, gray, soft to moderately to completely weathered, coarsely crystalline, thin bedded		-	Ц	86 (D)				
· -	井	8.5' - pítteď below 8.5 ft.	Ī	1210		80				
15	井	14.0-15.0' - possible clay washout from	14.0 to	-	H	(0)				
		15.0 ft. 15.0-18.0' LIMESTONE, gray, soft moderately to completely weathered, coarsely crystalline, thin bedded	to hard	-		60 (0)				
20		18.0-20.0' - possible clay washout from 20.0 ft.	-	-	L	(-)				
		20.0-30.0' LIMESTONE, gray, hard, m weathered, finely to coarsely crystalli bedded 20.0' - pitted from 20.0 to 25.0 ft.	ioderately ine, thin	1200		100 (0)				
25		25.0' - slightly to moderately weathered be ft.	elow 25.0	1195	$\ $	100		Qu Test Results UCS = 1410 ksf		
30				-		(85)		UCS = 1410 KSf		
		Refusal at 7.0 feet. Bottom of borehole at 30.0 feet.								
(1) = As	sumed,	im N_{60} - Corrected N value for standard 60% SF, (2) = Actual				sured han	nmer efficiency in			
			Coordina		ine S			_ Coordinate Pro	j. Factor:	
Coord	nate D	Datum:	Coordina	te Un	ıts: _	U.S. Su	vey Feet	_		

©COOMIGH CAT. INC. 10 SAWFORD, MURPHY & TILLY, INC. 10 SAWFORD, MU	MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION 105 WEST CAPITOL JEFFERSON CITY, M0 65102 1-888-ASK-MODOT (1-889-275-6636)	DATE DES	A9434	PROJECT NO. BRIDGE NO.	JOB NO. J8S3156 CONTRACT ID.	BR 4 COUNTY GREENE	12/27/2024 ROUTE STATE CRD 127 MO DISTRICT SHEET NO.	Joshua D. Jolliff MO PE-2006002824 DATE PREPARED	D. JOLLFF MUMER PE-20060(2824 2) DVAA 12/29/2024 135:36 PM	JOSHUA D. JOLL:FF NUMBER
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/TSi geotechnical.inc.				and Ma		···		Page 1
Job No.: 20221081.00		eene				Route: -44		
Design:	Skew:					Location: Springfi	eld, Missouri	
Bent:	Logged By:	JM				Operator: RK		
Station:	Northing: 5	17058.	3781			Date of Work: 09/	30/22-09/30/22	
Offset:	Easting: 13	98279.	9004			Depth to Water: _		
Elevation: 1236.3	Requested N	orthing	g:			Depth Hole Open:		
Requested Station:	Requested E	asting:				Time Change:		
Requested Offset:	Equipment:	CME 5	550,S	plit-Spoon	Sampler, NQ			
Requested Elevation:		e:						
Drill No.:	Hammer Effi	ciency:	969	%		Drilling Method: _f	-A	
Description	i	Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests	Index Tests
0 0.0-3.5' (CH) Reddish br	own fat CLAY trace							
roots, sand, and gravel	omi, idi 0511, uda	1235	X	56	4-9-23 (48)		PP >4.50 tsf	MC = 10
3.5-6.0' (GP) Gravel, trace	red, fat CLAY	-	X	28	7-10-11 (32)		-	MC = 11
6.0-13.5' (CH) Reddish b sand and gravel	rown, fat CLAY, trace	1230	X	83	2-3-4 (11)		PP = 3.00 tsf	MC = 48
		-	X	100	1-3-4 (11)		PP = 2.00 tsf	MC = 52
	areum fot CLAV trace	1225			2-2-3			MC = 54
15.5-15.5 (CH) Reduish I	Jown, fai CLAY, trace	1220		100	(8)		PP = 2.50 tsf	WIO
18.5-23.8' (CH) Reddish I sand and gravel	prown, fat CLAY, trace	-	X	100	1-2-2 (6)		PP = 1.50 tsf	MC = 57
		1215						
23.8-35.0' LIMESTONE, weathered, coarsely crystall	gray, hard, moderately ine, thinly bedded	-	П	97 (86)	50/0.3	Qu Test Results UCS = 1100 ksf	PP = 1.50 tsf	MC = 40
		1210				Ou Tool Book!		
30		1205		92 (85)		Qu Test Results UCS = 1090 ksf		
31.5' - slightly to highly wea	thered below 31.5 ft.			100		Qu Test Results		
35		<u></u>	Ц	(100)		UCS = 1170 ksf		
N_{so} = (Em/60)Nm N_{so} - Corrected N value for s (1) = Assumed, (2) = Actual Coordinate System:	tandard 60% SPT efficien				ner efficiency i	n percent; Nm - Observ		
Coordinate Datum:			_	U.S. Sun	ev Feet			

	221081.00	County: Gre	eene				Route: _I-44		Page
		Skew:					Location: Springfie	ld, Missouri	
		Logged By:					Operator: RK		
		Northing: 5					Date of Work: _09/3	0/22-09/30/22	
		Easting: 13					Depth to Water:		
Elevation: _1	1236.3	Requested N	orthing	g:			Depth Hole Open: _		
Requested St	tation:	Requested E	asting	_			Time Change:		
	ffset:	Equipment:							
	evation:	Location Not							
Drill No.:		Hammer Effi	ciency	: _96	<u>%</u> 		Drilling Method: _F	A	-
Depth (ft) (ft) Graphic	Description		Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N _{E0})	Shear Data	Field Tests	Tyop T
35	35.0-41.5 LIMESTONE, gray		1200	П					
	weathered, coarsely crystalline, the	niniy bedded	1200	H					
- =====================================			Ι.]	2,000				
40				١I	100 (100)		Qu Test Results UCS = 1430 ksf		
			1195	H					
7	Refusal at 23.5 fe	et.	1195	IJ					
N ₆₀ = (Em/60)N	im N _{so} - Corrected N value for standar , (2) = Actual system:					ner efficiency			
Coordinate S	Datum:	966 456	St. 1885	2000	U.S. Surv	(<u></u>			

MISSOURI HIGHWAYS AND TRANSPORTATION DATE COMMISSION COMMISSION CRAWFORD MURPHY & TILLY, INC. 1631 WEST ELFINDALE STREET SPRINGFIELD, MO 65807 (417) 869-6009 ENGINEERING CORPORATION - 000631 1-888-ASK-MODOT (1-888-ST5-6636)	E DESCRIPTION	PROJECT NO. BRIDGE NO. A9434	COUNTY GREENE JOB NO. J8S3156 CONTRACT ID.	ROUTE STATE CRD 127 MO DISTRICT SHEET NO. BR 5	12/29/2024 1:35:42 PM Joshua D. Jolliff MO PE-2006002824 DATE PREPARED 12/27/2024	JOSHUA D. JOLLIFF NUBER PE-2006002824
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/ T	SI hnical, inc.		Missouri D Con				ransportat aterials	IOII	20.00	G NO. B-4 Page 1
		221081.00	County: Gre	eene				Route: -44		
Desig	n:		Skew:					Location: Spring	field, Missouri	
Bent:			Logged By:	JM				Operator: AM		
Statio	n:		Northing: 5	17054.	8656			Date of Work: 09	9/30/22-09/30/22	
Offset	t:		Easting: 13	98187.	8349			Depth to Water:		
Eleva	tion: _	1229.1	Requested N	orthing	g:			Depth Hole Open:		
Reque	ested S	tation:	Requested E	asting	:			Time Change:		
		Offset:	Equipment:	CME :	550,S	plit-Spoor	n Sampler, Sh	elby Tube, NQ		
		levation:	Location Not	e:						
Drill N			Hammer Effi	ciency	: _96	%		Drilling Method:	FA	
Depth (ft)	Graphic	Description		Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests	Index Tests
0	///	0.0-7.5' (CH) Reddish brown,	fat CLAY, trace	-						
		sand, gravel, and organics		[]	X	50	4-5-7 (18)		PP >4.50 tsf	MC = 35.7
				1225	X	67	2-4-4 (12)		PP = 3.00 tsf	MC = 57.
				† :		86	2-50/0.1'		PP = 1.50 tsf	MC = 40.
		6.5-7.5' LIMESTONE, highly weat Refusal at 7.5 fee			_	0	50/0.2		1.50 (5)	
Coord	linate S linate D	lm N_{v_0} - Corrected N value for standard, (2) = Actual $system:$	Coordin	ate Zo ate Un	ne: _ its: _	U.S. Sur	rvey Feet	Coordinate Pr	oj. Factor:	ted

	20221081.00						Route: 1-44		
			1007-19			*		gfield, Missouri	
							Operator: AM		
							_	09/22/22-09/22/22	
	n: <u>1235.6</u>							n:	
	ed Station:				ess set				
	ed Offset:				-	Sampler, NC)		
	ed Elevation:								
Drill No.:	<u> </u>	Hammer Eff	riciency	: 96	<u>%</u>		Drilling Method:	FA	
O Depth (ft)	Description	on	Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests	
	0.0-8.0' (CH) Reddish brow	vn, fat CLAY, trace san	d 1235				7		
			+ :	X	72	6-9-13 (33)		PP = 4.00 tsf	MC
5			ļ	X	44	2-7-3 (15)		PP = 4.00 tsf	MC
//			1230	$\overline{}$	100			PP = 1.50 tsf	MC
] .						
0	8.0-13.5' LIMESTONE, Reddish brown, fat CLAY		ıd .	X	67	4-50		PP = 1.50 tsf	MC
10	9	,,	1225						
	<u>J.</u>		1	\times	40	1-50/0.3'		PP = 2.00 tsf	MC:
- 0	P		+ -	-					
15	13.5-18.5' LIMESTONE,		æ	X	56	7-39-26 (98)			MC:
15 0	Reddish brown, fat CLAY	CH)	1220			(30)			
) °		+ .	\overline{X}	56	20-43-25 (102)			MC
2.			+ -			(102)	-		
	Refusal at Bottom of boreho								
(1) = Assu Coordina	60)Nm N _⊚ - Corrected N value for med, (2) = Actual ate System: ate Datum:	Coordi	nate Zo	ne: _	sured ham U.S. Sur		in percent; Nm - Obst		

 CR	12 ROL D IST B	DAT 2/ JIE 1 RIC R	/ GN / GN / GN / GN / GN / GN / GN / GN	NTY EN NO 31	5:49 9:00liff 00282 RED ST M HEE	24 10 T N	WILLIAM.
			JJE		NO.		
DESCRIPTION			\9 <u>'</u>				
DATE							
MISSOURI HIGHWAYS AND TRANSPORTATION	COMMISSION		MODOT		105 WEST CAPITOL	JEFFERSON CITY, MD 65102	1-888-ASK-MODOT (1-888-275-6636)
				TILLY, INC.	1631 WEST ELFINDALE STREET SPRINGFIFI D. MO. 65807 (417) 869-6009	FINGINE FRING CORPORATION - 000631	

Concrete Leveling pad not shown for clarity. (1) * Proposed grade shown accommodates future full buildout of I-44.

- (1) Wall contractor shall show the following items on the design drawings and/or on the fabricator shop drawings.
 - Leveling pad horizontal.

Designed Nov 2024 Detailed Nov 2024 Checked Nov 2024

Leveling pad length and step elevations shall be based on wall manufacturer's recommendation. Top of leveling pad elevations shall not be higher than theoretical top of leveling pad elevations shown on these plans.

Total 1
1
1
1
3697
+

	MSE	Wall Systems D	ata 1	「ab l e				
Proprietary Wall Sy	ystems Combination Wall Systems							
Manufacturer	System	Facing Unit Manufacturer	Facing Unit	Geogrid Manufacturer	Geogrid			

MSE Wall A9435

LOCATION SKETCH

MSE Wall Systems Data Table is to be completed by MoDOT construction personnel to record the manufacturer of the proprietary wall system or the manufacturers of the combination wall system that was used for constructing the MSE wall.

General Notes: Design Specifications:

Indicates location of borings.

Notice and Disclaimer Regarding

The locations of all subsurface borings for this structure are shown on the plan sheet for this

structure. The boring data for all locations indicated, as well as

any other boring logs or other factual records of subsurface data

and investigations performed by the department for the design of

the project, are shown on Sheets No. 4 thru 7 and may be included

Deliverables. They will also be available from the Project Contact

upon written request. No greater

significance or weight should be

given to the boring data depicted on the plan sheets than is given

to the subsurface data available from the district or elsewhere.

The Commission does not represent

or warrant that any such boring

data accurately depicts the conditions to be encountered in

contractor assumes all risks it may encounter in basing its bid

documentation not expressly warranted, which the contractor may obtain from the Commission.

depicted here or those available from the district, or on any other

C CRD 127 (Melville Rd)

-Proposed MSE Wall A9434

Proposed Bridge A9317

prices, time or schedule of performance on the boring data

constructing this project. A

in the Electronic Bridge

Boring Log Data

2002 AASHTO LFD (17th Ed.) Standard Specifications (Section 5 ASD Design) Seismic Performance Category = A Acceleration Coefficient = 0.09

SEC./SUR. 4

Design Loading:

 $\theta_D=20^\circ$ and Unit weight, $\gamma_D=120$ pcf for retained backfill material to be retained by the mechanically stabilized earth wall system.

 Φ_f = 34° for improved foundation ground where wall is to bear. Unimproved foundation data is not applicable. Foundation Improvement is required full lenth of the wall.

For improved foundation ground, the allowable bearing pressure is 5.5 ksf.

The maximum applied bearing pressure for the controlling design case at the foundation level shall be shown on the shop drawings and shall be less than the allowable bearing pressure for foundation ground provided herein. For seismic design the maximum applied bearing pressure shall be less than two times the allowable bearing pressure.

Allowable bearing pressure and limits of improved foundation ground shall not be adjusted from that as shown on the plans.

Contractor shall include design $\Phi_{\mathcal{T}}$ (actual $\Phi_{\mathcal{T}} \geq 34^{\circ}$) and the total unit weight, γ_r, for the select granular backfill (reinforced backfill and wedge area backfill) for structural systems on shop drawings. Contractor shall identify source of select granular backfill material, submit proctor in accordance with AASHTO T 99 (ASTM D698) and gradation with the shop drawings. When backfill material is too coarse to develop a proctor curve the contractor shall determine the maximum dry density (relative density) in accordance with ASTM D4253 and ASTM D4254 and assume percent passing the 200 sieve for optimum water content.

Total unit weight, γ_{Γ} = (95% compaction) x (maximum dry density) x (1 + optimum water content)

Design $\Phi_{\Gamma} = 34^{\circ}$ for the select granular backfill (reinforced backfill) only for structural systems.

Factor of safety shall be 2.0 for overturning and 1.5 for sliding.

For seismic design the factor of safety shall be $1.5\,$ for overturning and $1.1\,$ for sliding.

Use default values for the pullout friction factor, F^* , in accordance with AASHTO figure 5.8.5.2A and default value for scale effect correction factor, α , in accordance with AASHTO table 5.8,5.2A. For approved steel strips not shown in AASHTO figure 5.8.5.2A, use $F^* \leq 2.0$ at zero depth and $F^* \leq \text{Tan } \Phi_{\Gamma}$ at 20 feet depth and Φ_{Γ} design = 34°. F^* and α values shall be shown on the shop drawings.

Design Unit Stresses:

All concrete for leveling pad and coping shall be Class B or B-1 with f'c = 4000 psi.

The minimum compressive strength of concrete for precast modular panel shall be 4,000 psi in accordance with Sec 1052.

Miscellaneous:

The MSE wall system shall be built vertical

The MSE wall system shall be built in accordance with Sec 720.

The MSE wall system shall be a precast modular panel wall system.

coated.

, concrete block and coping (or capstone) reinforcement shall be epoxy

A filter cloth meeting the requirements for a Separation Geotextile material shall be placed between the select granular backfill for structural systems and the backfill being retained by the mechanically stabilized earth wall

Coping shall be required on this structure. When CIP coping sections extend beyond the limits of a single panel, bond breaker (roofing felt or other approved alternate) between wall panel and coping is required. Coping joints shall use 3/4-inch chamfers and shall be sealed with 3/4-inch joint filler. Coping reinforcement shall terminate 1 1/2-inch minimum from face of coping

The contractor shall be solely responsible to coordinate construction of the wall with bridge and roadway construction and ensure that the bridge and roadway construction, resulting or existing obstructions, shall not impact the construction or performance of the wall. Soil reinforcement shall be designed and placed to avoid damage by pile driving, guardrail post installation, utility and sign foundations. (See Roadway and Bridge plans.)

Control Point #514 N: 517039.25 E: 1397801.35 Elev. 1222.12 Sta. 875+18.07 83.90 Lt (I-44) 5/8" I.B. W/ Aluminum Cap

Control Point #515 N: 516847.15 F: 1399681.98 Elev. 1219.86 Sta. 894+00.43 90.54 Rt. (I-44) 5/8" I.B. W/ Aluminum Cap

RETAINING WALL ALONG EB I-44 NEAR END BENT NO. 3 OF BRIDGE A9317

CRD 127 OVER 1-44 ABOUT 1.2 MILES WEST OF ROUTE 13 BEGINNING STATION 879+66.08 C 1-44



RGE, 22W

12/29/2024 1:39:21 PM Joshua D. Jolliff MO PE-2006002824

12/27/2024 MO

RD 127 DISTRICT SHEET NO BR 1 GREENE

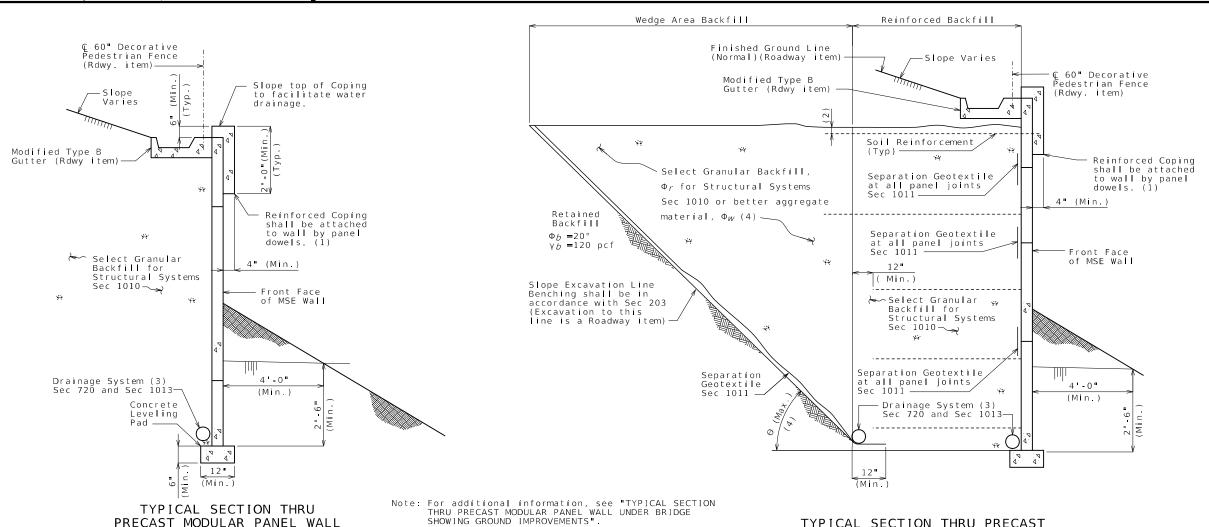
> CONTRACT ID. PROJECT NO.

J8S3156

BRIDGE NO A9435

4P I T D L 65102 CAt MO 77

J, MURPHY & TILLY, IN 'ELFINDALE STREET LD, MO 65807 (417) 8



General Notes Cont.:

Minimum 18" wide Geotextile strips shall be centered at vertical and horizontal joints of panel. Geotextile material shall be adhered to back face of panel using an adhesive compound supplied by the manufacturer. All edges of each fabric strip shall provide a positive seal. A minimum 18" overlap shall be provided between spliced filter fabric.

Aluminized soil reinforcement shall have edges coated with coating material

Soil reinforcement shall be spaced to avoid roadway drop inlet behind wall.

All steel soil reinforcements shall be separated from other metallic elements by at least 3 inches.

The splay angle should be less than 15° and tensile capacity of splayed reinforcement shall be reduced by the cosine of the splay angle. Soil reinforcement shall clear the obstruction by at least 3 inches.

No reinforcement shall be left unconnected to the wall face or arbitrarily cut/bent in the field to avoid the obstruction.

Where interference between the vertical obstruction and the soil reinforcement is unavoidable, the design of the wall near the obstruction may be modified using one of the alternatives in FHWA-NHI-10-24, Section 5.4.2. Show detail layout on the drawings. For wall designs with horizontal obstructions in reinforced soil mass, see FHWA-NHI-10-024, Section 5.4.3.

Excavation:

Excavation quantities and pay items are given on the roadway plans. Excavation quantities are based on a soil reinforcement length of 0.8H. The soil reinforcement length may vary based upon the wall design selected by the contractor. Plan excavation quantities will be paid regardless of any actual quantities removed based on the soil reinforcement length and design selected.

- by a minimum of 1 1/2 inches and a maximum of 2 1/2 inches.
- Topmost layer of reinforcement shall be fully covered with select granular backfill for structural systems, as approved by the wall manufacturer, before placement of the Separation Geotextile.
- (3) Minimum 6" diameter perforated PVC or PE pipe.

Manufacturer shall show drain details on design plans to be submitted as shown on MoDOT MSE wall plans and/or roadway plans.

Contractor shall modify the drain details as shown if it will improve flow as may be the case for stepped leveling pad, and for an uneven ground line (approval of the engineer required).

Mat	erial Prop	erties L	Ised in D	esign
	II/Select Backfill	Active Computa		Foundation
Ф°	γ (pcf)	Ф°	γ (pcf)	Ф°

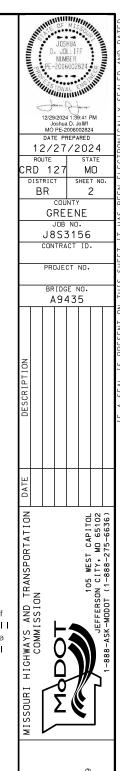
Note: MSE Wall designer shall include table on shop drawings and provide values used in the design computations. Effects of cohesion shall be ignored unless approved by the engineer.

Show range of acceptable theta (Θ) angle on shop drawings which must be consistent with design computations and proposed construction of wall. Show active force computation properties on shop drawings and in design computations. Coordination between wall designer (manufacturer) and contractor is required before shop drawing submittal.

DETAILS FOR MSE WALL

- (1) Inverted U-shape reinforced capstone may be used in lieu of coping. (4) Select granular backfill shall extend a minimum of 12" beyond the end of Panel dowels for level-up concrete shall be required and provided by manufacturer. The dowels shall be field trimmed to clear the capstone all soil reinforcement. Where the angle, Θ , between the retained backfill excavation/fill line and the horizontal is less than 90°, the wedge area backfill between ⊖ and 90° shall be filled with select granular backfill for structural systems meeting the requirements of Section 1010.
 - For $(45^{\circ} + \Phi_b/3) < \Theta \le 90^{\circ}$, properties for retained backfill shall be used for active force computations.
 - For $\Theta \leq (45^{\circ} + \Phi_{D}/3)$, contractor shall have the option to use select granular backfill, $\Phi_{ec{r}}$, or better aggregate material, $\Phi_{ec{W}}$ for active force computations in the wedge area backfill. For active force computations, the angle of internal friction for wedge area backfill material, Φ_{Γ} or Φ_{W} , shall be limited to 34° unless determined otherwise in accordance with Section 1010. If $\Phi_{\mathcal{T}}$ or $\Phi_{\mathcal{W}} > 34^{\circ}$ is desired for wedge area backfill then test report shall be submitted with shop drawings. Φ_r or Φ_W shall not be greater than 40° for computations. Final configuration of this option shall be sent to Geotechnical Section for a new overall global stability analysis. Design Φ_W shall be shown on the shop drawings if used.

The slope excavation line shall be benched and separation geotextile shall be placed between the retained backfill and either select granular backfill or better aggregate material, and between the select granular backfill and better aggregate material.



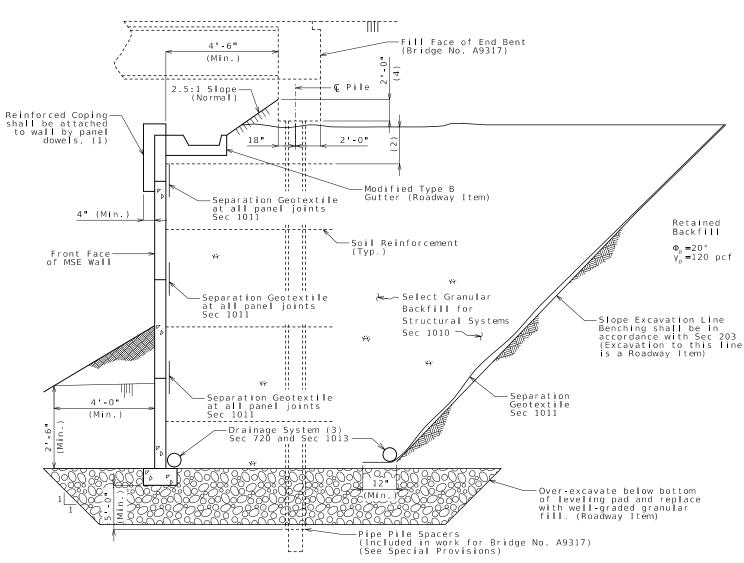
MURPHY & TILLY, ELFINDALE STREET), MO 65807 (417)

Detailed Nov 2024

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 2 of 7

MODULAR PANEL WALL SHOWING FILTER CLOTH



TYPICAL SECTION THRU PRECAST MODULAR PANEL WALL UNDER BRIDGE SHOWING GROUND IMPROVEMENTS

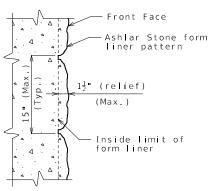
Note: For additional information, see "TYPICAL SECTION THRU PRECAST MODULAR PANEL WALL SHOWING FILTER CLOTH".

- (1) Inverted U-shape reinforced capstone may be used in lieu of coping. Panel dowels for level-up concrete shall be required and provided by manufacturer. The dowels shall be field trimmed to clear the capstone by a minimum of 1 1/2 inches and a maximum of 2 1/2 inches.
- (2) Topmost layer of reinforcement shall be fully covered with select granular backfill for structural systems, as approved by the wall manufacturer, before placement of the Separation Geotextile.
- (3) Minimum 6" diameter perforated PVC or PE pipe.

Manufacturer shall show drain details on design plans to be submitted as shown on MoDOT MSE wall plans and/or roadway plans.

Contractor shall modify the drain details as shown if it will improve flow as may be the case for stepped leveling pad, and for an uneven ground line (approval of the engineer required).

(4) See bridge plans.



FORM LINER DETAIL (PRECAST MODULAR PANEL WALL)

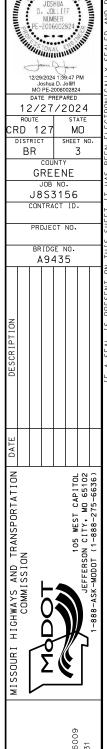
Notes:

The cost of form liners for MSE wall systems, complete in place, will be considered completely covered by the contract unit price for Mechanically Stabilized Earth Wall System.

Form liner shall be constructed in accordance with Special Provisions.

The following is a list of form liner manufacturers and types which may be used. Depth of relief for all form liner pattern's shall vary up to 1 1/2". The height of any single 'stone' shall be 15" maximum.

- Scott System, Inc.: Form liner pattern #167 "Ashlar Stone"
- Fitzgerald Formliners: Form liner pattern #16986 "Ashlar Stone"
- Greenstreak: Form liner pattern #330 "Ashlar Stone"
- Spec Formliners: Form liner pattern #1515 "Ashlar Stone"
- Customrock: Form liner pattern #12020 "Tollway Ashlar"
- An approved equal



MURPHY & TILLY, INC. ELFINDALE STREET

D, MO 65807 (417) 869

CORPORATION - 0006

DETAILS FOR MSE WALL

geolechnical	20221081.00					aterials	Route: -44		Page 1	
	20221081.00						Location: Springs	field Missouri		
								ieid, Missouri		
							Operator: RK	107/00 00/07/00		
							Date of Work: _09			
							Depth to Water: Depth Hole Open: Time Change:			
	1220.2									
	ed Station:									
	ed Offset:				plit-Spoor	n Sampler, Sh	elby Tube, NQ			
	ed Elevation:									
Drill No.:		Hammer Effi	ciency	: 96	<u>%</u>		Drilling Method:	FA		
Depth (ft)	Description		Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests	Index Tests	
0	38 0000 H H	4 (2.0E)	1220					1 2		
77	0.0-0.8' Portland cement conc 0.8-1.0' Crushed limestone (2.			_	-	6-5-6			MC = 20.1	
-//	1.0-8.9' (CH) Reddish brown,			-X	33	(17)		PP = 3.00 tsf		
5	and gravel		1215		31			PP = 2.00 tsf	MC = 16.4	
				X	78	2-2-4 (9)		PP = 1.00 tsf	MC = 66.1	
10	8.9-11.3' LIMESTONE, tan, h Reddish brow, fat CLAY	ighly weathered and	1210	X	56	32-14-14 (42)		PP = 1.00 tsf	MC = 61.4	
· -	4		ļ.	->=	100	50/0.3	\exists	PP = 0.50 tsf	MC = 36.6	
	12.5' See Boring B-301A for information below 11.3 feet. Refusal at 11.3 Bottom of borehole a	3 feet.								
Coordina	60)Nm N _{so} - Corrected N value for stan med. (2) = Actual tte System:	Coordin	ate Zo	one: _	sured ham		in percent; Nm - Obser Coordinate Pr			

		221081.00	County: Gre				-	Route: 1-44		
								Location: Springfie	ld, Missouri	-
Bent:			Logged By:			8		Operator: RK		
			Northing: 5					Date of Work: _09/2		
			Easting: 13					Depth to Water:		
Eleva	tion: _	1220.2	Requested N	lorthing]:			Depth Hole Open: _		
		tation:	Requested E					Time Change:		
Requ	ested C	Offset:	Equipment:		550,A	uger Cut	tings, NQ			
Requ	ested E	levation:	Location Not							
Drill I	No.:		Hammer Effic	ciency	96	%		Drilling Method: F	A	
Depth (ft)	Graphic	Description		Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests	Index Tests
0	8390E3	0.0-0.8' Portland cement concrete	(9.6")	1220						
-	1111	0.8-1.0' Crushed limestone (2.4")	(0.0)		1					
		1.0-4.0' (CH) Reddish brown, fat and gravel		-	1					
5	団	4.0-4.5' Tan, highly weathered LIN 4.5-19.5' LIMESTONE, gray,		1215	,					
		weathered, coarsely crystalline, th	nard, moderatiey in bedded	1.	П					
	丗				H	100 (61)		Qu Test Results UCS = 1570 ksf		
				- 1	łI					
10	Ш	9.5' - slightly to moderately weath	ared below 0 F ft	1210	Н					
		9.5 - slightly to moderately weath	ered below 9.5 it.	1210	11					
	井	11.7' - chert inclusion.		[]	П	98 (90)		Qu Test Results UCS = 1260 ksf		
		12.9' - chert inclusion.		ļ -	H	(50)		000 = 1200 K31		
 15	丗			1205	Н			,		
	井			1205	H					
	戸			Ė	11	100		Qu Test Results		
	퍽	10 0' bight worth and		Ι.	П	(70)		UCS = 1470 ksf		
-		18.0' - highly weathered	12		Ш					
		Refusal at 4.5 fee Bottom of borehole at 1								
N ₆₀ = (Em/60)N	$_{\rm M}$ $_{\rm N_{\odot}}$ - Corrected N value for standar , (2) = Actual	d 60% SPT efficien	cy; Em	- Meas	sured ham	nmer efficiency	in percent; Nm - Observe	d N-value	
		, (2) = Actual System:	Coordin					Coordinate Proj		
		Datum:					vey Feet			
		g this information are cautioned that the judgement of the operator. THIS INFOR			-					

 CR	12 ROU ED DIST B	MO DAT 2 / JTE 1 RIC R GI	PE-2 PI	LL IF BER 60002	STA M MHEE	2 4 O T NO 1	
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ORTATION DATE					105 WEST CAPITOL	, MO 65102	3-275-6636)
MISSOURI HIGHWAYS AND TRANSPORT	COMMISSION				105 WES	JEFFERSON CITY, MO 65102	1-888-ASK-MODOT (1-888-275-6636)
			© Copyright CMT, Inc.	CRAWFORD, MURPHY & TILLY, INC.	1631 WEST ELFINDALE STREET SPRINGFIELD, MO 65807 (417) 869-6009	FNGINFFRING CORPORATION - 000631	

	nnical, inc.	221081.00	Con County: Gr		tion	and Ma	terials	Route: -44		Page 1
		221001.00	Skew:					Location: Springfi	eld. Missouri	
			Logged By:					Operator: RK		
			Northing: _5					Date of Work: 09/		
			Easting: 13					Depth to Water:		
		220.8	Requested N							
								Depth Hole Open:		
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		ffset:	Equipment:			uger Cuttin	igs, NQ			
		evation:	Location No			•		Delline Made to EA		
Drill N	lo.:		Hammer Effi	ciency	: 96	<u>%</u>		Drilling Method: _	FA T	
Depth (ft)	Graphic	Description		Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts $(N_{\rm to})$	Shear Data	Field Tests	Index Tests
0	8888	0.0-0.8' Portland cement concrete	(9.6")	1220						
-		0.1' - rough drilling		1						
_		0.8-1.0' Crushed limestone (2.4") 1.0-4.0' (CH) Red, fat CLAY, trace		[]						
. [
5	· 0.	4.0-5.5' LIMESTONE, highly wea	thered	 -	- [
		Refusal at 5.5 fee Bottom of borehole at 5		1		1				
(1) = A	ssumed,	m N _∞ - Corrected N value for standar (2) = Actual ystem:						in percent; Nm - Observ		
		J				U.S. Surv	all's sa		4 uc.o.,	

Job No.: 2022	21081.00	County: Green					Route: 1-44	ingfield, Missouri	
		Logged By:					Operator: RK		
		Northing: _5						09/22/22-09/22/22	
		Easting: 13						r:	
	41.7	Requested N						en:	
· ·	tion:	Requested E		-					
	set:	Equipment:	-	-					
	vation:	Location Not			-	reampier, 140	•		
Drill No.:		Hammer Effi					Drilling Metho	d: <u>FA</u>	
Depth (ft) (ft) Graphic	Description		Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests	
	0.0-29.7' (CH) Reddish brown, fa	at CLAY							
			1240	X	44	4-12-27 (59)		PP = 4.50 tsf	МС
- ₅ -			-	X	17	6-11-4 (23)		PP = 4.50 tsf	MC
	6.0' - trace and and gravel.		1235	X	72	2-2-3 (8)		PP = 1.50 tsf	MC
10				X	61	2-2-3 (8)		PP = 2.50 tsf	МС
			1230	X	33	1-2-3 (8)		PP = 1.00 tsf	F MC
15			-	X	89	0-1-2 (5)		PP = 1.00 tsf	MC
			1225						
20			-	X	6	1-1-1 (3)		PP = 1.50 tsf	MC
			1220						
25			-	X	22	0-0-1 (2)		PP = 0.50 tsf	MC
			1215						
				\geq	100	1-50/0.0'		PP = 1.00 tsf	MC
	29.0-29.7' LIMESTONE, heavily Refusal at 29.7 fe Bottom of borehole at 2	eet.							
N _∞ = (Em/60)Nm (1) = Assumed, (N _∞ - Corrected N value for standar 2) = Actual	rd 60% SPT efficier	ncy; Em	- Meas	sured ham	mer efficiency	in percent; Nm - Ot	oserved N-value	
Coordinate Sy								Proj. Factor:	
Coordinate Da	tum:	Coordin	ate Un	its: _	U.S. Sur	vey Feet	_		

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7	MURPHY & ELFINDALE 30, MO 65807
	CRAWFORD, MURPHY 1631 WEST ELFINDAL SPRINGFIELD, MO 65
© Copyright CMT, Inc.	ORD,
pyvright	CRAWFORD 1631 WEST SPRINGFIEL
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Joshua D. Jollit
MO PE-2008002824

DATE PREPARED
12/27/2024

ROUTE STATE
CRD 127 MO
DISTRICT SHEET NO.
BR 5

GREENE JOB NO.
J8S3156
CONTRACT ID.

PROJECT NO. BRIDGE NO.

Joh N	hnical,inc.	221081.00	Cons County: Gre		tion	and Ma	aterials	Route: -44		Page 1 o
		21001.00	Skew:					Location: Springfie	eld Missouri	
			Logged By:					Operator: AM		
			Northing: 5					Date of Work: _09/	20/22-09/20/22	
			Easting: 139					Depth to Water:		
		241.0	Requested No					Depth Hole Open:		
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		fset:						elby Tube, NQ		
Reque	sted Ele	evation:	Location Not	e:						
Drill N	No.:		Hammer Effic	iency	: 969	%		Drilling Method: _F	'A	
Depth (ft)	Graphic	Description		Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests	Index Tests
0	"	0.0-3.5' CHERT, white and Redd	sh brown gravel.	1240				-		
		trace clay			X	33	15-30-39 (104)			MC = 9.9%
 5		3.5-31.5' Reddish brown, fat CL trace organics	.AY with gravel;		X	56	35-50		PP >4.50 tsf	MC = 20.69
				1235	X	72	6-13-7 (30)		PP = 3.00 tsf	MC = 54.5°
10						83			PP = 2.50 tsf	MC = 51.89
				1230			3-3-4			MC = 45.19
- 					X	100	(11)		PP = 2.50 tsf	
15			-	1225	X	44	3-4-50 (81)		PP = 1.50 tsf	MC = 53.69
					X	56	3-9-14 (35)		PP = 1.50 tsf	MC = 35.89
20				1220	X	61	6-4-3 (11)		PP = 2.00 tsf	MC = 58.6°
					X	100	2-2-12 (21)		PP = 3.50 tsf	MC = 57.49
25				4045	X	100	2-2-3 (8)		PP = 1.00 tst	MC = 64.79
				1215	X	100	6-2-2 (6)		PP = 1.00 tsf	
_ [00.0.04 511 11/50701/5			\geq	32	4-50/0.3'		PP = 1.50 tsf	MC = 13.79
30		29.0-31.5' LIMESTONE, heavily w	3	1210						
		31.5-41.5' LIMESTONE, gray, h weathered, finely crystalline, thin b				75 (60)				
35	\Box	34.0' - 6" drop observed.				0 h				
$N_{60} = (1)$	Em/60)Nn .ssumed,	m N_{\odot} - Corrected N value for standard (2) = Actual	60% SPT efficien	cy; Em	- Meas	sured ham	mer efficiency	in percent; Nm - Observe	d N-value	
		ystem:		ate Zo	ne: _			Coordinate Pro	j. Factor:	
Coord	linate Da	atum:	Coordin	ate Un	its: _	U.S. Sur	vey Feet			

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Design:		Skew:					Location: Springfie	eld, Missouri	
Bent:		Logged By:					Operator: AM		
Station:		Northing: 5					Date of Work: _09/2	20/22-09/20/22	
Offset:		Easting: 13					Depth to Water:		
	11.0	Requested N	orthing	g:			Depth Hole Open: _		
Requested Stat	ion:	Requested E	asting:				Time Change:		
	set:						nelby Tube, NQ		
	vation:	Location Not							
Drill No.:		Hammer Effi	ciency	: <u>96</u>	% 		Drilling Method: F	A	
Depth (ft) (ft) Graphic	Description		Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests	oper Toote
	31.5-41.5' LIMESTONE, gray, weathered, finely crystalline,	hard, moderately	1205	Н					
	(continued)	umi beadet		ł I	40-				
- +				1 I	100 (93)		Qu Test Results UCS = 1270 ksf		
40			† -	1	58 25				
			1200	Н	100		Qu Test Results		
	Refusal at 31.5 fe Bottom of borehole at 4		1		(100)		UCS = 1350 ksf		
(1) = Assumed, (2	N_{e_0} - Corrected N value for standar :) = Actual :tem:					ner efficiency			

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1631 WEST ELFINDALE STREET SPRINGFIFI D. MO 65807 (417) 869-6009	105 WEST CAPITOL			5 5	NO.		,	М	RED	1111		SOU
FNGINFFRING CORPORATION - 000631	JEFFERSON CITY, MO 65102						T NO	О	4	III	WEER	THE PERSON NAMED IN
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	hnical, inc.	204004.00		struc			aterials			Page 1
		221081.00	County: Gre					Route: -44		
			Skew:	200.00				Location: Springfie	eld, Missouri	
			Logged By:					Operator: AM	10.00 00110100	
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		tation:	Requested E					Time Change:		
		ffset:			550,N	o Recove	ry, Split-Spoo	n Sampler, Shelby Tub	e, NQ	
		evation:	Location Not							
Drill N	lo.:		Hammer Effic	ciency	: 96	<u>%</u> 		Drilling Method: _F	FA .	
Depth (ft)	Graphic	Description		Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests	Index Tests
0	67 <u>7</u> 7	0.0-3.5' (GC) White and brown	clayey GRAVEL						*	
 		(GC)		1240	X	56	18-38-50 (132)			MC = 16.5
 5	AVXV	3.5-6.0' (SM) Red, clayey SAND, organics	trace gravel and		X	33	3-17-20 (56)		PP = 3.00 tsf	MC = 29.5
		6.0-27.4' (CH) Reddish brown, sand	fat CLAY, trace	1235		100			PP = 3.00 tsf	MC = 25.9
10		10.0' - large chert fragment in sho		-	X	11	2-5-11 (24)			MC = 46.6
		10.0 - large chert if agment in Sho	е.	1230	X	44	2-3-6 (14)		PP = 2.50 tsf	MC = 58.5
15			9	-	X	100	2-2-3 (8)		PP = 2.50 tsf	MC = 57.7
			5	1225	X	100	2-3-7 (15)		PP = 1.50 tsf	MC = 60.4
20			9	-	X	11	3-4-4 (12)		PP = 1.50 tsf	MC = 50.0
				1220	X	100	5-3-1 (6)		PP = 1.00 tsf	LL = 144 PL = 33 MC = 38.6
25			a 2		X	56	1-1-2 (5)		PP = 1.00 tst	MC = 38.4
 		₹ 27.4-27.5' (CH) Sandy fat CLAY,	with chert	1215	X	44	6-18-32 (75)		PP = 1.00 tsf	MC = 67.1
30		27.5' - highly weathered chert. 28.0-38.5' LIMESTONE, gray, weathered, finely crystalline, thin to 30.0' -vuggy for 1/10 ft.	hard, slightly	1210		100 (71)	(0)	Qu Test Results UCS = 1250 ksf		
35			2 3							
$N_{60} = (1)$	Em/60)Ni ssumed,	m N_{60} - Corrected N value for standard (2) = Actual	d 60% SPT efficien	ıcy; Em	- Mea	sured ham	mer efficiency	in percent; Nm - Observe	ed N-value	
		ystem:						Coordinate Pro		
Coord	linate D	atum:	Coordin	ate Un	nits: _	U.S. Sur	vey Feet			

Job No.: 20221081.00 Design: Bent: Station: Offset: Elevation: 1241.6		County: Gre					Route: -44		
Bent:							Location: Springfie	eld, Missouri	
Station:		Logged By:					Operator: AM		
Offset:		Northing: 5					Date of Work: _09/*	19/22-09/19/22	
Elevation: 1241.6		Easting: 139					Depth to Water:		
		Requested No					Depth Hole Open:		
Requested Station:		Requested Ea					Time Change:		
Requested Offset:							n Sampler, Shelby Tub		
Requested Elevation:		Location Not							
Drill No .		Hammer Effic					Drilling Method: _F	Ά	
Depth (f) (f) (f) (f) (graphic	Description		Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N ₆₀)	Shear Data	Field Tests	oper Tests
		nard, slightly thin bedded			97 (97)		Qu Test Results UCS = 1180 ksf		
Bott	Refusal at 28.5 feet. tom of borehole at 38.5	feet.							
N _∞ = (Em/60)Nm N _∞ - Correct (1) = Assumed, (2) = Actual Coordinate System:							in percent; Nm - Observe		
Coordinate Datum:		Coordin	ate Un	its: _	U.S. Surv	ey Feet	_		

CF	12 ROI RD	PE-12/25/MO DAT 1 PRICE R G	NUM -2000 -2	HUA LL IF BER 6002 44 1:44 1:40 1:40 1:40 1:40 1:40 1:40 1:40	D:11 If 002822 RED ST. M	PM 4 2 4 ATE 10	
_		J 8	SS:	3 1	56 ID	•	
			DJE				
			194		√o. 5		
DESCRIPTION							
DATE							
MISSOURI HIGHWAYS AND TRANSPORTATION	COMMISSION		MADOT		105 WEST CAPITOL	JEFFERSON CITY, MO 65102	1-888-ASK-MODOT (1-888-275-6636)
			1	PHY & TILLY, INC.	1631 WEST ELFINDALE STREET SPRINGFIFI D. MO 65807 (417) 869-6009	FNGINFFRING CORPORATION - 000631	