

MOBILI	ZATION
PROJECT TOTAL	1 LUMP SUM

ADDITIONAL MOBIL	ZATION FOR SEEDING
PRO LECT TOTAL	3 FA

347.0 88.4 1533.9 409.4

567+40.00

435 1943

CONTRACTOR	FURN I SHED	SURVEYING	&	STAKING
PROJECT	TOTAL	1 LUM	P S	UM

		EN	TRANCES		
			GRAVEL A	15"	15"
			CRUSHED STONE B	GROUP C	GROUP
			4 "	PIPE	FES
LOCATION	STATION	SIDE	(SY)	(LF)	(EA)
ROUTE 46	561+22.21	LT	82.6	24	2
ROUTE 46	561+23.60	RT	44.8	32	2
		TOTALS	127	56	4

	SEED AND MULCH									
SEEDING - COOL										
			SEASON GRASSES	MULCHING						
LOCATION	STATION	STATION	(ACRE)	(ACRE)	REMARKS					
ROUTE 46	559+30.00	564+15.55	0.5	0.5	WEST OF BRIDGE					
ROUTE 46	ROUTE 46 565+98.05 567+73.80 0.2 0.2 EAST OF BRIDGE									
		TOTALS	0.7	0.7						

		.AY	L AND OVERL	MIL			
! !		MODIEJED	TACK	1.75"			
		COLDMILLING	COAT	BP-1-			
!		(DEPTH TRANSITIONS)	0.10 GAL/SY	PG 58-28H			
	REMARKS	(SY)	(GAL)	(TON)	STATION	STATION	OCATION
	DEPTH TRANSITION	152	15.2	14.4	567+ 40.00	566+90.00	ROUTE 46
1 DELE		152	15.2	14.4	TOTALS		

	PERMANENT EROSION CONTROL									
	FURNISHING PLACING PERMANENT									
		TYPE 2	EROSION CONTROL							
			ROCK BLANKET	ROCK BLANKET	GEOTEXTILE					
LOCATION	STATION	SIDE	(CY)	(CY)	(SY)					
ROUTE 46	564+38.02	LT/RT	426.7	426.7	640					
ROUTE 46	565+75.55	LT/RT	148.7	148.7	223					
		TOTALS	575	575	863					

	BASE AND PAVEMENT											
	TYPE 1 AGG.											
				OPTIONAL	TYPE A2	FOR BASE						
				PAVEMENT	SHOULDER	(4 IN. THICK)						
LOCATION	STATION	STATION	SIDE	(SY)	(SY)	(SY)	REMARKS					
ROUTE 46	559+30.00	564+15.55	LT/RT	1186.9	321.0	1508	WEST OF BRIDGE					
ROUTE 46	565+98.05	566+90.00	LT/RT	224.8	58.9	284	EAST OF BRIDGE					
·	·		TOTALS	1411.7	379.9	1792						

	TEMPORARY EROSION CONTROL										
					ROCK	TYPE C					
				SILT	DITCH	TEMPORARY	SEDIMENT				
				FENCE	CHECK	BERM	REMOVAL				
LOCATION	STATION	STATION	SIDE	(LF)	(LF)	(LF)	(CY)				
ROUTE 46	559+44	561+77	LT		150		10				
ROUTE 46	559+44	562+00	RT		165		11				
ROUTE 46	561+80	564+60	LT	281			2.8				
ROUTE 46	562+04	564+57	RT	254			2.5				
ROUTE 46	564+82		LT/RT			150	1.5				
ROUTE 46	565+54		LT/RT			126	1.3				
ROUTE 46	565+60	567+21	RT	180			1.8				
ROUTE 46	565+66	568+05	LT	244			2.4				
			TOTALS	959	315	276	33				

	EARTHWORK									
			UNCLASSIFIED	COMPACTING	EMBANKMENT					
			EXCAVATION	EMBANKMENT	IN PLACE					
LOCATION	STATION	STATION	(CY)	(CY)	(CY)					
ROUTE 46	559+30.00	564+38.02	114	91	2700					
ROUTE 46	565+75.55	566+90.00	108	86	434					
	•	TOTALS	222	177	3134					

			PAVE	MENT MARKING		
				4" WHITE STANDARD	4" YELLOW STANDARD	
				WATERBORNE PVMT	WATERBORNE PVMT	
				MARKING PAINT	MARKING PAINT	RUMBLE
				WITH TYPE P BEADS	WITH TYPE P BEADS	STRIP
LOCATION	STATION	STATION	SIDE	(LF)	(LF)	(STA)
ROUTE 46	559+30	567+40		1620	558	
ROUTE 46	559+30	564+35.55	LT			5
ROUTE 46	559+30	564+35.55	RT			5
ROUTE 46	565+78.05	567+40	LT			1.6
ROUTE 46	565+78.05	567+40	RT			1.6
		TOTALS		1620	558	13.2

		REMC	VAL OF II	MPROVE	MENTS		
SHEET	LOCATION	STATION	STATION	SIDE	DESCRIPTION	QUANTITY	UNITS
4	ROUTE 46	561+11.71	561+35.76	RT	15" CMP	24	LF
4	ROUTE 46	561+11.88	561+35.55	LT	15" RCP	24	LF
4	ROUTE 46	563+99.94		LT	OM-3	1	EA
4	ROUTE 46	564+01.72		RT	OM-3	1	EA
4	ROUTE 46	564+15.55		RT	OM-3	1	EA
4	ROUTE 46	564+20.48		LT	OM-3	1	EA
4	ROUTE 46	564+21.30		RT	OM-3	1	EA
4	ROUTE 46	564+28.05	564+41.00	RT	GUARD RAIL	13	LF
4	ROUTE 46	564+40.11		LT	OM - 3	1	EA
4	ROUTE 46	565+71.72	565+86.75	LT	GUARD RAIL	15	LF
4	ROUTE 46	565+72.19	565+86.76	RT	GUARD RAIL	15	LF
4	ROUTE 46	565+73.87		RT	OM - 3	1	EA
4	ROUTE 46	565+73.88		LT	OM - 3	1	EA
4	ROUTE 46	565+94.26		LT	OM - 3	1	EA
4	ROUTE 46	565+95.37		RT	OM-3	1	EA
4	ROUTE 46	566+14.08		LT	OM-3	1	EA
4	ROUTE 46	566+14.63		RT	OM-3	1	EA
	INCLUDE ROUT	E W REMOVALS	(SEE NEXT SH	IEET)	TOTAL	1	LS

	GUARDRA I L									
	"TYPE A" MASH SHAPING									
				BRIDGE		CRASHWORTHY	SLOPES			
				ANCHOR	MGS	END	CLASS III	REMARKS		
				SECTION	GUARDRAIL	TERMINAL				
LOCATION	STATION	STATION	SIDE	(EA)	(LF)	(EA)	(100F)			
ROUTE 46	562+39.80	564+28.05	RT	1	100	1	0.75	ADJACENT LANE		
ROUTE 46	563+02.30	564+28.05	LT	1	37.5	1	0.75	OPPOSING LANE		
ROUTE 46	565+85.55	567+11.30	RT	1	37.5	1	0.75	OPPOSING LANE		
ROUTE 46	565+85.55	567+73.80	LT	1	100	1	0.75	ADJACENT LANE		
			TOTALS	4	275	4	3			

	CLEARING AND GRUBBING									
	CLEARING AND									
				GRUBB I NG						
LOCATION S	TATION	STATION	SIDE	ACRE	REMARKS					
ROUTE 46 55	59+30.00	564+15.55	LT/RT	0.5	WEST OF BRIDGE					
ROUTE 46 56	55+98.05	567+73.80	LT/RT	0.2	EAST OF BRIDGE					
			TOTAL	1						

ROUTE 46
SUMMARY OF QUANTITIES
SHEET 1 OF 3

1 REVISED 02-10-2025

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		PRO	OJE	СТ	NO.			
		BR	IDO	1 36	νο.			
DESCRIPTION	R001-PAVEMENT REVISIONS							
DATE	02/10/25							
MISSOURI HIGHWAYS AND TRANSPORTATION	COMMISSION				105 WEST CAPITOL	JEFFERSON CITY, MO 65102	1-888-ASK-MODOT (1-888-275-6636)	
	1411 East 104th St.	Kansas City, MO 64131	Fax (816) 874-4675	www.trekkdesigngroup.com	Missouri Cert. of	Authority 2002010300		

	ENTRANCES								
			GRAVEL A						
			CRUSHED STONE B						
			4 "						
LOCATION	STATION	SIDE	(SY)						
ROUTE W	190+94.15	LT	52.1						
		TOTALS	52						

	BASE AND PAVEMENT								
						TYPE 1 AGG.			
				OPTIONAL	TYPE A3	FOR BASE			
				PAVEMENT	SHOULDER	(4 IN. THICK)			
LOCATION	STATION	STATION	SIDE	(SY)	(SY)	(SY)	REMARKS		
ROUTE W	185+19.70	185+78.73	LT/RT	132.4	12.9	145	WEST OF BRIDGE		
ROUTE W	189+01.27	195+00.00	LT/RT	1329.5	255.0	1585	EAST OF BRIDGE		
			TOTALS	1461.9	267.9	1730			

	EARTHWORK									
			UNCLASSIFIED	COMPACTING	EMBANKMENT	COMPACTING				
			EXCAVATION	EMBANKMENT	IN PLACE	IN CUT	REMARKS			
LOCATION	STATION	STATION	(CY)	(CY)	(CY)	(STA)				
ROUTE W	185+19.70	185+98.73	140	2		1	WEST OF BRIDGE			
ROUTE W	193+95.00	195+00.00	263	210	681	1.1	EAST OF BRIDGE			
	TOTALS 403 212 681 2.1									

	REMOVAL OF IMPROVEMENTS									
SHEET	LOCATION	STATION	STATION	SIDE	DESCRIPTION	QUANT I TY	UNITS			
5	ROUTE W	185+67.91		LT	OM-3	1	EA			
5	ROUTE W	185+68.32		RT	OM-3	1	EA			
5	ROUTE W	185+88.05		LT	OM-3	1	EA			
5	ROUTE W	185+88.28		RT	OM-3	1	EA			
5	ROUTE W	186+07.60		RT	OM-3	1	EA			
5	ROUTE W	186+07.68		LT	OM-3	1	EA			
5	ROUTE W	186+08.15	188+70.94	RT	GUARDRAIL	263	LF			
5	ROUTE W	186+08.51	188+70.50	LT	GUARDRAIL	262	LF			
5	ROUTE W	188+70.55		RT	OM- 3	1	EA			
5	ROUTE W	188+70.96		LT	OM-3	1	EA			
5	ROUTE W	188+90.62		LT	OM-3	1	EA			
5	ROUTE W	189+11.29		LT	OM-3	1	EA			
5	ROUTE W	189+11.34		RT	OM-3	1	EA			

	CLEARING AND GRUBBING								
		CLEARING AND							
				GRUBB I NG					
LOCATION	STATION	STATION	SIDE	ACRE	REMARKS				
ROUTE W	185+19.70	185+80.49	LT/RT	0.1	WEST OF BRIDGE				
ROUTE W	188+99.15	195+00.00	LT/RT	0.4	EAST OF BRIDGE				
			TOTAL	1					

	SEED AND MULCH									
	SEEDING - COOL									
			SEASON GRASSES	MULCHING						
LOCATION	STATION	STATION	(ACRE)	(ACRE)	REMARKS					
ROUTE W	185+19.70	185+80.49	0.1	0.1	WEST OF BRIDGE					
ROUTE W	188+99.15	195+00.00	0.4	0.4	EAST OF BRIDGE					
	TOTALS 0.5 0.5									

	PERMANENT EROSION CONTROL									
	FURNISHING PLACING PERMANENT									
			TYPE 2	TYPE 2	EROSION CONTROL					
			ROCK BLANKET	ROCK BLANKET	GEOTEXTILE					
LOCATION	STATION	SIDE	(CY)	(CY)	(SY)					
ROUTE W	186+01.27	LT/RT	176	176	264					
ROUTE W	188+78.73	LT/RT	172	172	257					
ROUTE W	196+40.11	RT	22	22	33					
		TOTALS	370	370	554					

	TEN	/IPORARY	EROS	ION CO	NTROL	
					TYPE C	
				SILT	TEMPORARY	SEDIMENT
				FENCE	BERM	REMOVAL
LOCATION	STATION	STATION	SIDE	(LF)	(LF)	(CY)
ROUTE W	185+19	186+00	LT	87		0.9
ROUTE W	185+19	186+04	RT	88		0.9
ROUTE W	186+36		LT/RT		103	1.0
ROUTE W	188+60		LT/RT		106	1.1
ROUTE W	188+76	195+10	LT	683		6.8
ROUTE W	188+82	195+10	RT	634		6.3
			TOTALS	1492	209	17

			PAVE	MENT MARKING		
				4" WHITE STANDARD	4" YELLOW STANDARD	
				WATERBORNE PVMT	WATERBORNE PVMT	
				MARKING PAINT	MARKING PAINT	RUMBLE
				WITH TYPE P BEADS	WITH TYPE P BEADS	STRIP
LOCATION	STATION	STATION	SIDE	(LF)	(LF)	(STA)
ROUTE W	185+19.70	195+00.00		1961	1961	
ROUTE W	185+19.70	185+96.81	LT			7.7
ROUTE W	185+19.70	186+00.85	RT			8 1
ROUTE W	188+79.15	195+00	LT			62.1
ROUTE W	188+83.39	195+00	RT			61.7
		TOTALS		1961	1961	139.6

JASON

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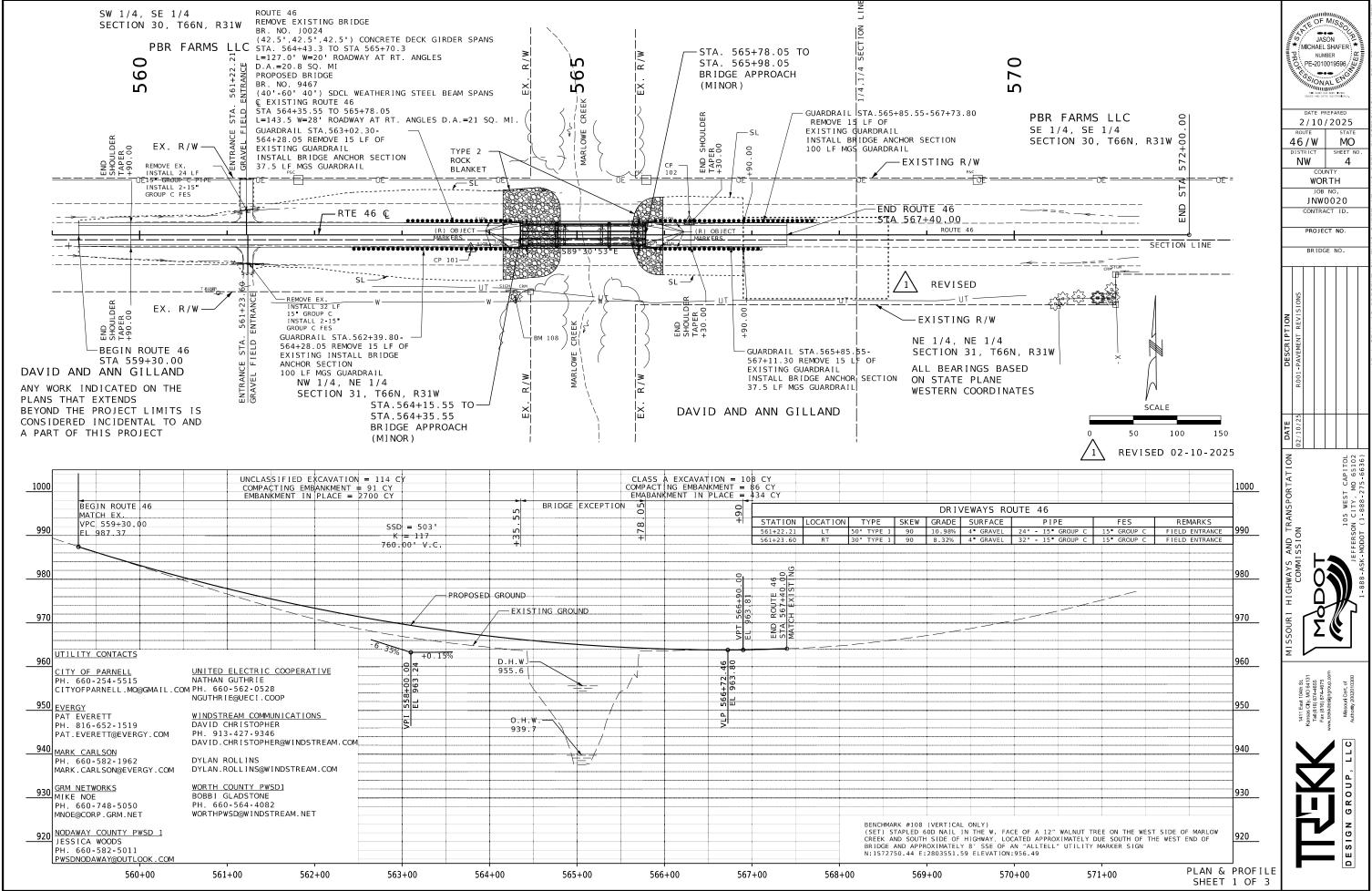


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1411 East 104th St.
Kansas Ctty, MO 64131
Tel (816) 874-4655
Fax (816) 874-4675
www.tekkdesigngroup.com



ROUTE W
SUMMARY OF QUANTITIES
SHEET 2 OF 3



Estimated	Quantiti	es		
I t em		Substr.	Superstr.	Total
Class 1 Excavation	cu. yard	95		95
Removal of Bridges (J0024)	lump sum			1
Bridge Approach Slab (Minor)	sq. yard		126	126
Galvanized Structural Steel Piles (14 in.)	linear foot	1,376		1,376
Pile Wave Analysis	each	2		2
Pile Point Reinforcement	each	16		16
Class B Concrete (Substructure)	cu. yard	97.7		97.7
Slab on Steel	sq. yard		486	486
Type D Barrier	linear foot		315	315
Reinforcing Steel (Bridges)	pound	4,620		4,620
Temporary coating- Concrete Bents and Pier (Weathering Steel)	lump sum			1
Fabricated Structural Low Alloy Steel (I-Beam) A709, Grade 50 W	pound		69,990	69,990
Slab Drain	each		22	22
Vertical Drain at End Bent	each	2		2
Laminated Neoprene Bearing Pad (Tapered)	each		24	24

All concrete between the upper and lower construction joints in the end bents is included in the Estimated Quantities for Slab on Steel.

All reinforcement in the end bents is included in the Estimated Quantitites for Slab on Steel.

All reinforcement in the intermediate bent concrete diaphragms except reinforcement embedded in the beam cap is included in the Estimated Quantities for Slab on Steel.

All concrete above the intermediate beam cap is included in the Estimated Quantities for Slab on Steel.

Sheet metal in intermediate concrete diaphragms is subsidiary to Slab on Steel.

Cost of L4x4 ASTM A709 Grade 36 HP pile anchors and 3/4-inch diameter ASTM F3125 Grade A325 Type 1 bolts, complete in place, will be considered completely covered by the contract unti price for Galvanized Structural Steel Piles (14 in.)

Estimated Quantities for Slab on Steel

1	3145 311 32221	/
	I t em	Total
	Class B-2 Concrete cu. yard	153
	Reinforcing Steel (Epoxy Coated) pound	51,410

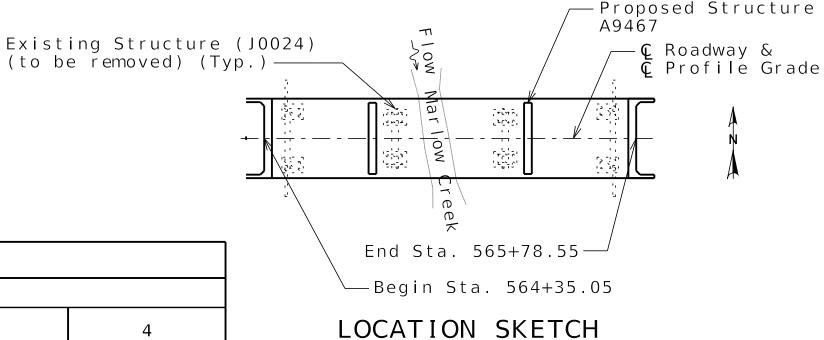
The table of Estimated Quantities for Slab on Steel represents the quantities used by the State in preparing the cost estimate for concrete slabs. The area of the concrete slab will be measured to the nearest square yard longitudinally from end of slab to end of slab and transversely from out to out of bridge slab (or with the horizontal dimensions as shown on the plan of slab). Payment for stay-in-place corrugated steel forms, conventional forms, all concrete and epoxy coated reinforcing steel will be considered completely covered by the contract unit price for the slab. Variations may be encountered in the estimated quantities but the variations cannot be used for an adjustment in the contract unit price.

Method of forming the slab shall be as shown on the plans and in Reinforcing Steel (ASTM A615 Grade 60) accordance with Sec 703. All hardware for forming the slab to be left in place as a permanent part of the structure shall be coated in accordance with ASTM A123 or ASTM B633 with a thickness class SC 4 and a finish type I, II or III.

Slab shall be cast-in-place with conventional forms or stay-in-place corrugated steel forms. Precast prestressed panels will not be permitted.

Bridge deck surface may be finished with a vibratory screed.

The contractor shall provide bracing necessary for lateral and torsional stability of the beams during construction of the concrete slab and remove the bracing after the slab has attained 75% design strength. Contractor shall not weld on or drill holes in the beams. The cost for furnishing, installing, and removing bracing will be considered completely covered by the contract unit price for Slab on Steel.



	Fou	nda	ation Dat	а		
				Bent 1	Numb e r	
Туре	Design Data		1	2	3	4
	Pile Type and Size		HP 14x73	HP 14x73	HP 14x73	HP 14x73
	Numb e r	ea	4	4	4	4
	Approximate Length Per Each	ft	88	86	85	85
Load	Pile Point Reinforcement	ea	AII	AII	All	AII
Bearing Pile	Min. Galvanized Penetration (Elev.)	ft	939.00	924.00	923.00	937.00
	Est. Max. Scour Depth 500 (Elev.)	ft	-	934.00	935.00	-
	Criteria for Min. Tip Penetration		Bear on Rock	Bear on Rock	Bear on Rock	Bear on Rock
	Pile Driving Verification Method		DF	WEAP	WEAP	DF
	Resistance Factor		0.4	0.5	0.5	0.4
	Minimum Nominal Axial Compressive Resistance	kip	448	640	640	448

Load Bearing Pile: DF = FHWA-modified Gates Dynamic Pile Formula WEAP = Wave Equation Analysis

Minimum Nominal Axial Compressive Resistance = Maximum Factored Loads (Side Resistance + Tip Resistance) Resistance Factors

Manufactured pile point reinforcement shall be used on all piles in this structure.

HP Piles are anticipated to be driven to refusal on rock. Review all borings for depth of rock and restrict driving as appropriate to comply with hard rock driving criteria in accordance with Sec 702.

Pile point reinforcement need not be galvanized. Shop drawings will not be required for pile point reinforcement.

Hydrologic Data Drainage Area = 21 sq. mi. Design Flood Frequency = 50 yr. Design Flood Discharge = 6,500 cfs Design Flood (D.F.) Elevation = 955.6 ft. Base Flood (100-Year) Base Flood Elevation = 956.5 Base Flood Discharge = 7,500 cfs. Estimated Backwater = 0.1 ft. Average Velocity thru Opening = 8.5 ft./p Freeboard (50-year) Freeboard = 5.3 ft. Roadway Overtopping Overtopping Flood Discharge = N/AOvertopping Flood Frequency = >500-yr 500-yr Flood Elevation = 958.5

51,420 / 1

General Notes:

Design Specifications: 2020 AASHTO LRFD Bridge Design Specification (9th Ed) Seismic Design Category A (Seismic Details)

Design earthquake response spectral acceleration coefficient at 1.0 second period, SD1 = 0.104g

Acceleration Coefficient (Effective peak ground acceleration coefficient), As = 0.064g

Design Loading:

Vehicular = HL-93 Future Wearing Surface = 35 lb/sf Earth = 120 lb/cf, Equivalent Fluid Pressure 45 lb/cf Superstructure: Simply-Supported, Non-Composite for dead load.

Design Unit Stresses: Class B Concrete (Substructure)

f'c = 3,000 psif'c = 4,000 psiClass B-2 Concrete (Superstructure, except Barrier) Class B-1 Concrete (Barrier) f'c = 4,000 psify = 60,000 psiStructural Steel HP Pile (ASTM A709 Grade 50) fy = 50,000 psi

Continuous Composite for live load.

Neoprene Pads:

Neoprene bearing pads shall be 60 durometer and shall be in accordance with Sec 716.

Fabricated Steel Connections:

Field connections shall be made with 3/4-inch diameter ASTM F3125 Grade A325 Type 3 bolts and 13/16-inch diameter holes, except as noted.

Joint Filler:

All joint filler shall be in accordance with Sec 1057 for preformed sponge rubber expansion and partition joint filler, except as noted.

Structural Steel Protective Coating:

Protective Coating: System G in accordance with Sec 1080.

Prime Coat: The cost of the inorganic zinc prime coat will be considered completely covered by the contract unit price for the fabricated structural steel.

Field Coats: The color of the field coats shall be Brown (Federal Standard #30045). The cost of the intermediate and finish field coats will be considered completely covered by the contract unit price for the fabricated structural steel.

At the option of the contractor, the intermediate and finish field coats may be applied in the shop. The contractor shall exercise extreme care during all phases of loading, hauling, handling, erection and pouring of the slab to minimize damage and shall be fully responsible for all repairs and cleaning of the coating systems as required by the engineer.

Concrete Protective Coatings:

Temporary coating for concrete bents and piers (weathering steel) shall be applied on all concrete surfaces above the ground line or low water elevation on all abutments and intermediate bents in accordance with Sec 711.

Reinforcing Steel:

Minimum clearance to reinforcing steel shall be 11/2", unless otherwise shown

Minimum clearance between galvanized piles and uncoated (plain) reinforcing 去steel including bar supports shall be 1 1/2". Nylon, PVC, or polyethylene spacers shall be used to maintain clearance. Nylon cable ties shall be used to bind the spacers to the reinforcement.

Traffic Handling:

Structure to be closed to traffic during construction. See roadway plans for traffic control.

Miscellaneous:

High strength bolts, nuts and washers will be sampled for quality assurance as specified in Sec 106

WORTH JOB NO. JNW0020 CONTRACT ID. PROJECT NO. BRIDGE NO. A9467

DATE PREPARED

1/22/2025

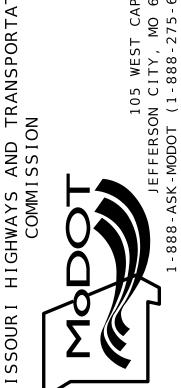
COUNTY

46

DISTRICT

MO

SHEET NO.

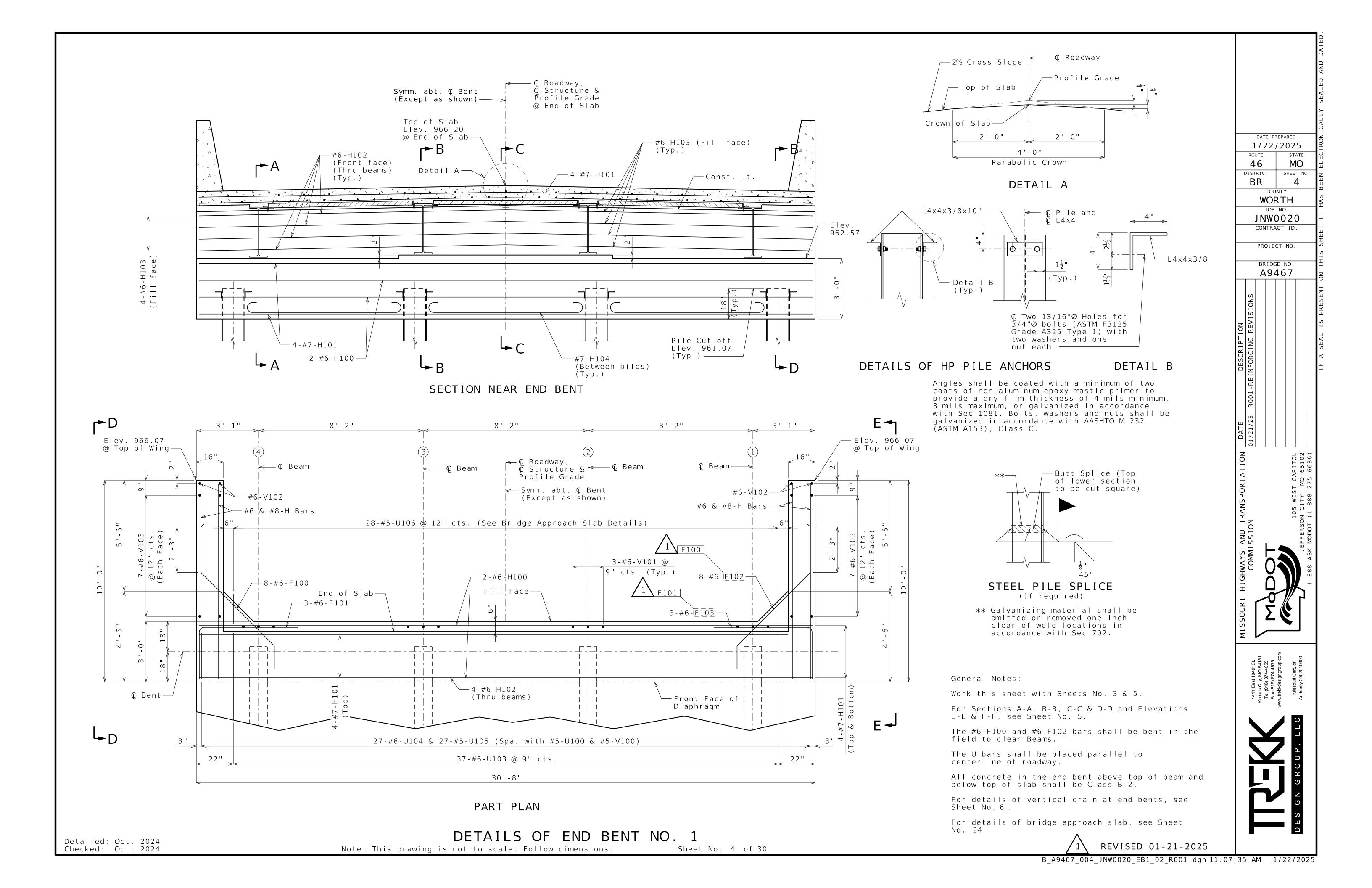


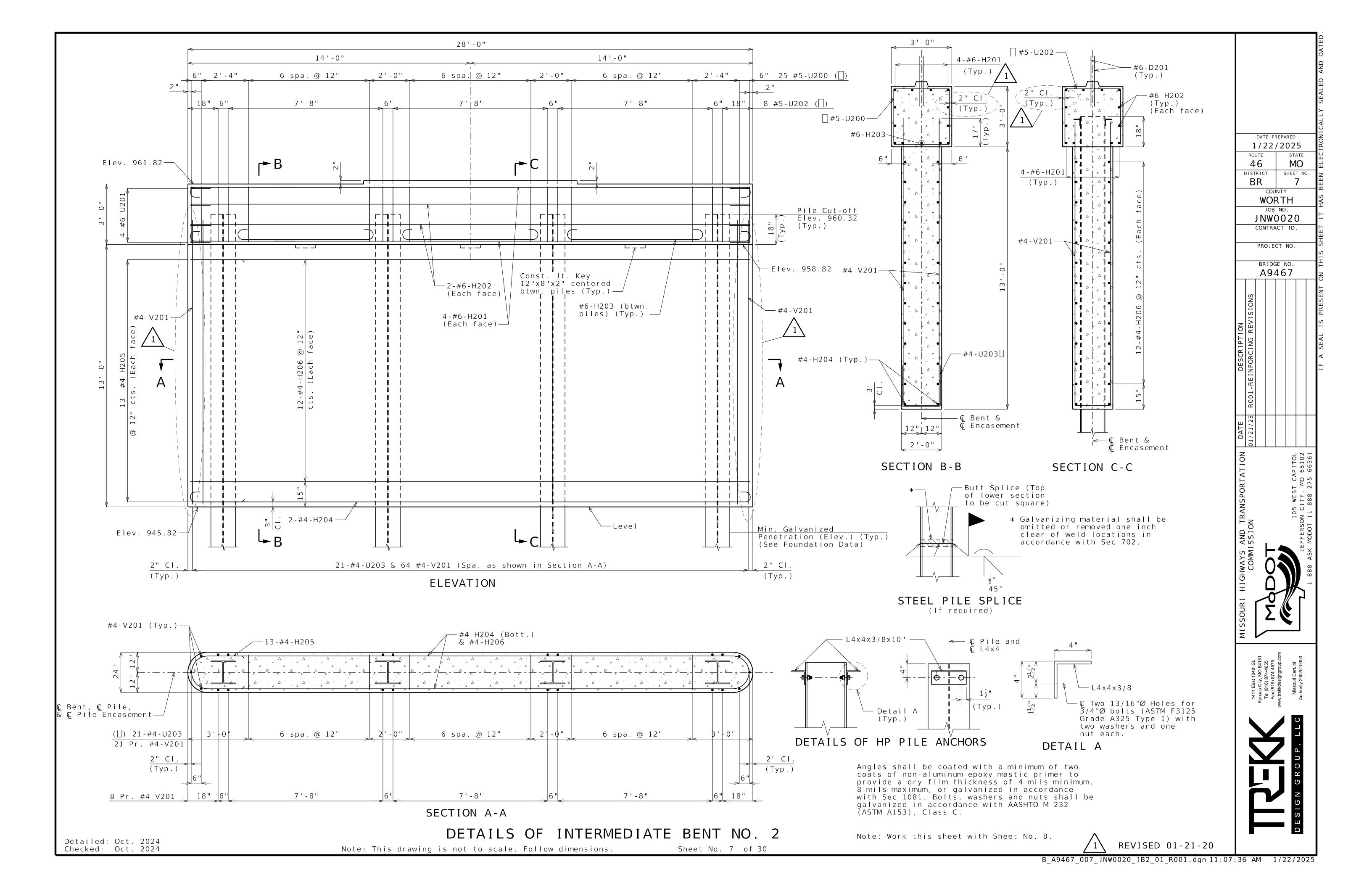


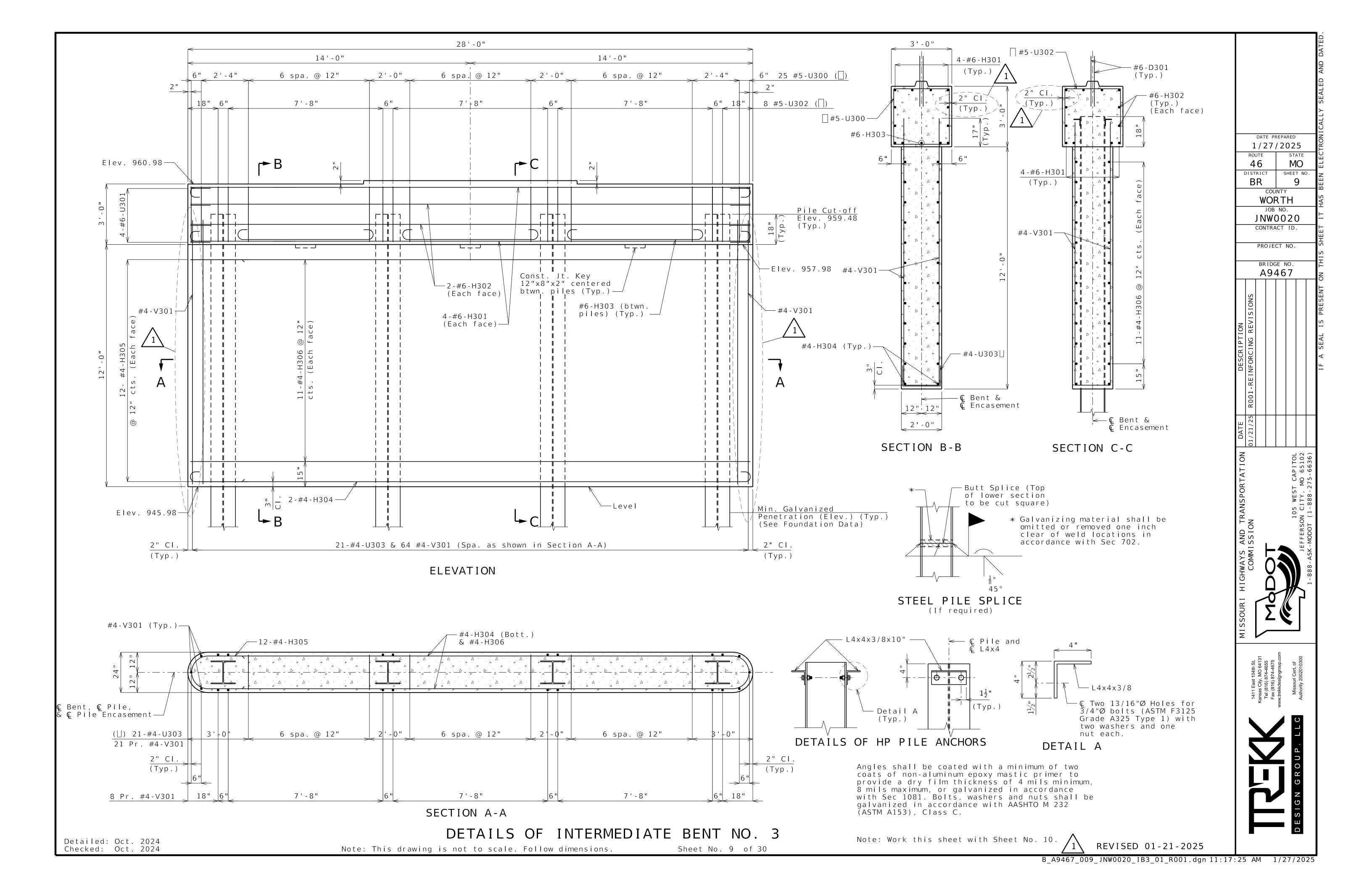


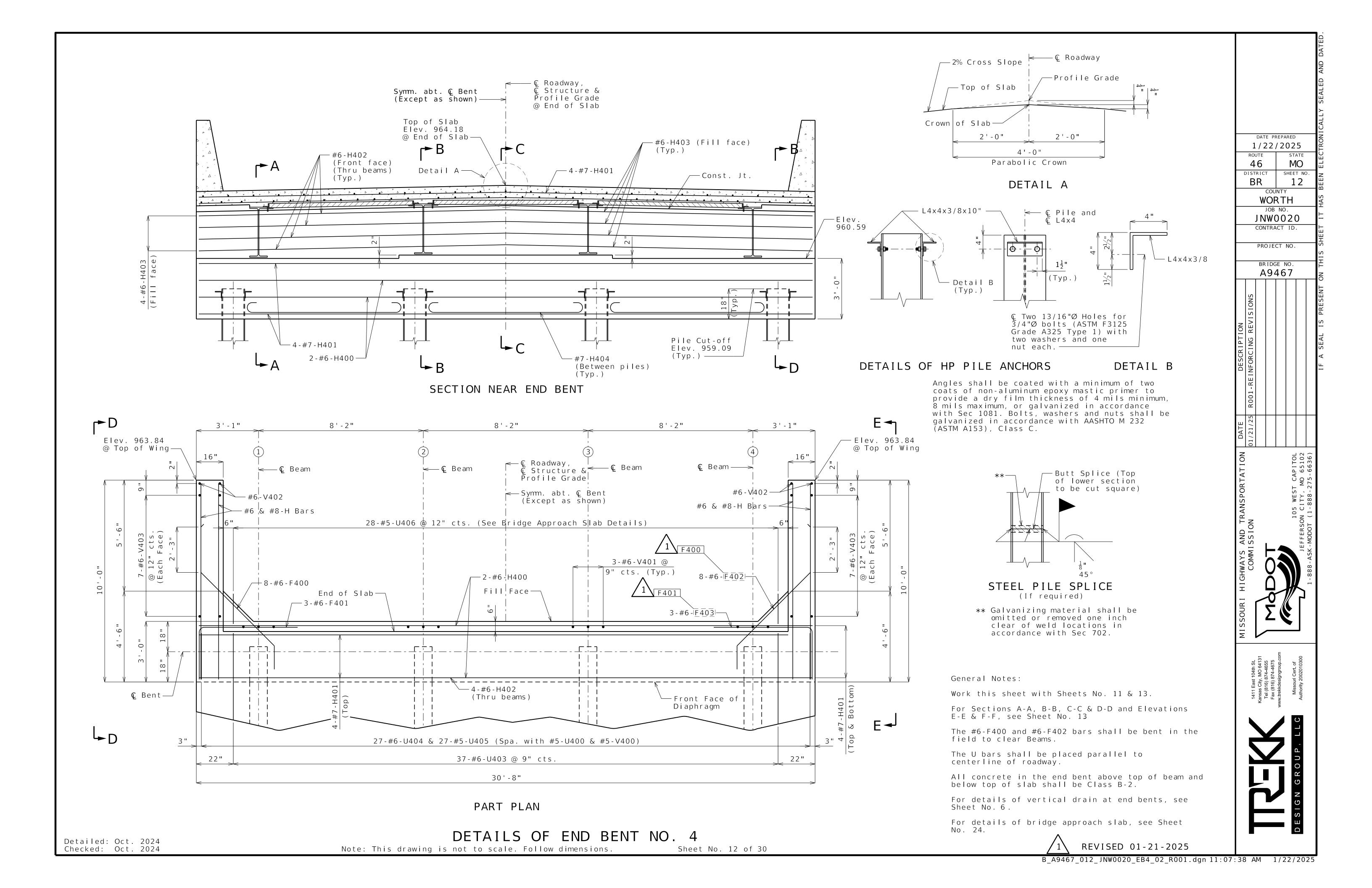
GENERAL NOTES AND QUANTITIES

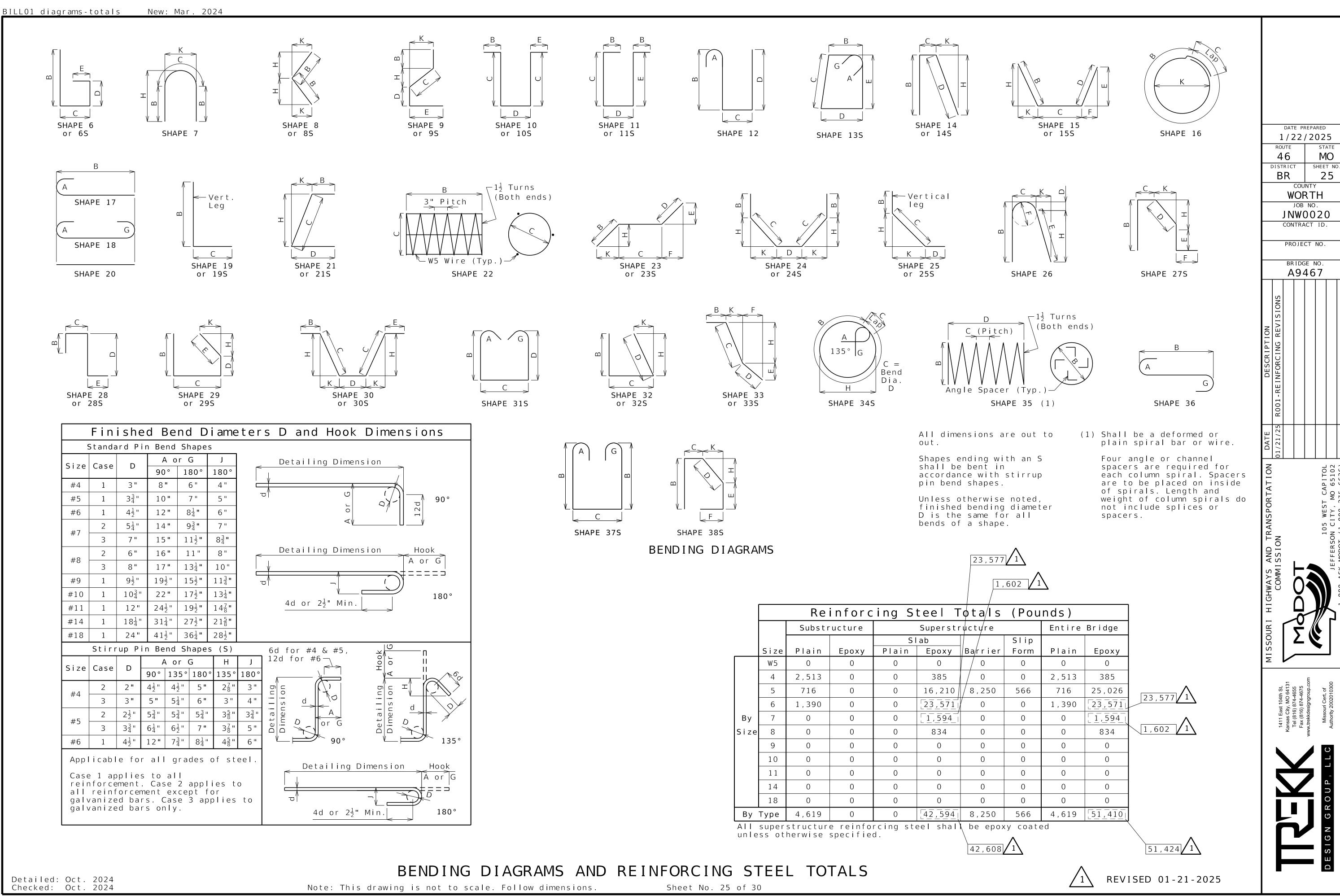












Control Cont	Company Comp							16				64	<u> </u>		8	25	24	24				25	05		04	64	21	8		25	24				8	25		Req. N			
Parameter Para	The color of the					6 U52 DIAPHRAGM		6 H50 DIAPHRAGM		O I IDEDOTO I OTI I		4 V301 COLUMN	4 USUS DEAW	5 U302 BEAM	6 U301 BEAM	5 U300 BEAM	4 H306 BEAM	4 H305 BEAM	4 H304 BEAM		6 H301 BEAM	6 D301 BEAM KEY			4 V ZU I COLUIVIN	4 V201 COLLIMN	4 U203 BEAM	5 U202 BEAM	6 U201 BEAM	5 U200 RFAM	4 H206 BEAM	4 H205 BEAM		6 H202 BEAM	6 H201 BEAM	6 D201 BEAM KEY			Size/		
The content of the	New Year 1									IDE		20	10			13	20	7	- 			20			20	20	10	- 		13	20	7		-+-+	18	20					
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For bending diagrams and steel reinforcing totals, see Sheet No. 25.

All bars shall be Grade 60.

Codes: C = Required coatings, where E = Epoxy Coated and <math>G = Galvanized.

SH = Required shape, see bending diagrams.

V = Sets of varied bars and number of bars of each length. Bar dimensions vary in equal increments between dimensions shown on this line and the following line and the actual length dimension shown on this line and the following line vary by the specified increment.

REVISED 01-21-2025

							Ī	Вi	II c	f	Rei	n f	orci	ng	g St	ee	l							
												Dim	ensions							N	lom.	Ad	ctual	
No.	Size/			Code	S		В		С		D		Е		F		Н		K	Le	ength	Le	ngth	Weight
Req.	Mark	Location	С	SH	V	ft	in.	ft	in.	ft	in.	ft	in.	ft	in.	ft	in.	ft	in.	ft	in.	ft	in.	lb
		SLAB																						
278	6 S1	SLAB	E	20		30	5.000													30	5	30	5	12701
196	5 S2	SLAB	E	20		30	5.000													30	5	30	5	6218
174	5 S3	SLAB	E	20		49	5.000													49	5	49	5	8968
96	6 S4	SLAB	E	20		32	0.000													32	0	32	0	4614
		TYPE D BARRIER	•	•	•	•	,		•			•		•		•		•	•	•				
278	5 R1	BARRIER	E	26		3	3.000		5.500	3	3.625					3	3.000		6.750	6	10	6	9	1957
278	5 R2	BARRIER	E	19			20.500		9.500											2	6	2	5	701
278	5 R3	BARRIER	E	27					9.500		15.250		5.000		12.000		15.000		3.000	3	6	3	4	967
80	5 R4	BARRIER	E	20		11	9.000											7	$1\sqrt{6}$	11	9	11	9	980
40	5 R5	BARRIER	E	20		26	5.000			$\overline{\Lambda}$									$\overline{}$	26	5	26	<u></u>	1102
20	5 R6	BARRIER	E	20		35	9.000	T	6.000/	1								1	$1\sqrt{6}$	35	9	35	9	746
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60	5 K2	BARRIER	E	27		3	8.000		9.250		14.500	2	5.750				14.250		2.750	8	2	7	11	495
20	5 K4	BARRIER	E	19	V	2	4.250		10.000											3	2	3	1	66
		INC. = 0.500 INCH				2	6.250		10.000											3		3	3	
20	5 K5	BARRIER	E	14	V		8.250		9.500		18.500						4.000		18.000	3	0	2	11	63
		INC. = 0.500 INCH					8.250		9.500		20.500						4.500		20.000	3	2	3	1	
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		INC. = 0.500 INCH				2	7.750		10.000											3	6	3	5	
12	5 K7	BARRIER	E	21	V	2	6.625		10.000							2	6.000		6.250	3	5	3	3	41
		INC. = 0.500 INCH				2	7.625		10.000							2	7.000		6.500	3	6	3	4	
36	5 K8	BARRIER	E	19	Ιv	2	8.500		10.000											3		3	5	138
		INC. = 0.750 INCH				3	2.500		10.000											4	1	3	11	
12	5 K9	BARRIER	E	21	V	2	8.500		10.000							2	7.750		6.750	3	7	3	5	46
		INC. = 0.750 INCH				3	2.500		10.000							3	1.750		7.750	+	1	3	11	
12	5 K10	BARRIER	E	19		3	3.000		10.000											4	_	4	0	50
12	5 K11	BARRIER	E	21		3	3.000		10.000							3	2.250		7.750	4	1	3	11	49
48	5 K12	BARRIER	TE	20		9	9.000												_	9		9	9	488
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8	5 C2	BARRIER	E		 	7	9.000													7		7	9	65
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For bending diagrams and steel reinforcing totals, see Sheet No. 25.

Detailed: Oct. 2024

Checked: Oct. 2024

All bars shall be Grade 60.

Codes: C = Required coatings, where E = Epoxy Coated and <math>G = Galvanized.

SH = Required shape, see bending diagrams.



REVISED 01-21-2025

V = Sets of varied bars and number of bars of each length. Bar dimensions vary in equal increments between dimensions shown on this line and the following line and the actual length dimension shown on this line and the following line vary by the specified increment.

DATE PREPARED 1/22/2025

COUNTY WORTH JOB NO. JNW0020 CONTRACT ID.

PROJECT NO.

BRIDGE NO. A9467

Estimated	Quantiti	es		
I t em		Substr.	Superstr.	Total
Class 1 Excavation	cu. yard	80	-	80
Removal of Bridges (X0142)	l ump sum	-	-	1
Bridge Approach Slab (Minor)	sq. yard	-	109	109
Drilled Shaft (4ft. Oin. Dia.)	linear foot	80.0	-	80.0
Rock Sockets (3ft. 6in. Dia.)	linear foot	98.0	-	98.0
Video Camera Inspection	each	4	-	4
Foundation Inspection Holes	linear foot	138.0	-	138.0
Sonic Logging Test	each	4	-	4
Galvanized Structural Steel Piles (12 in.)	linear foot	372	-	372
Dynamic Pile Testing	each	2	-	2
Pre-Bore for Piling	linear foot	190	-	190
Pile Point Reinforcement	each	8	-	8
Class B Concrete (Substructure)	cu. yard	102.6	-	102.6
Type D Barrier	linear foot	-	607	607
Slab on Concrete NU-Girder	sq. yard	-	837	837
NU-43 Prestressed Concrete NU-Girder	linear foot	_	840	840
Reinforcing Steel (Bridges)	pound	33,250	-	[33, 250]
Vertical Drain at End Bent	each	-	-	2
Laminated Neoprene Bearing Pad	each	-	12	12
Laminated Neoprene Bearing Pad (Tapered)	each	-	6	6

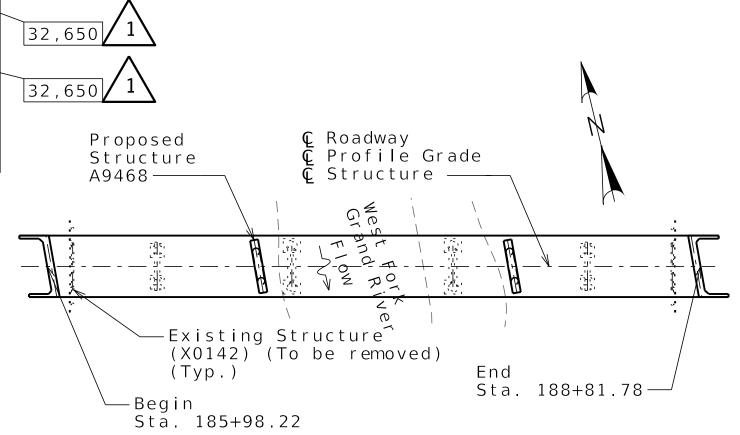
All concrete above the construction joint in the end bents is included in the Estimated Quantities for Slab on Concrete NU-Girder.

All reinforcement in the end bents is included in the Estimated Quantitites for Slab on Concrete NU-Girder.

All reinforcement in the intermediate bent concrete diaphragms except reinforcement embedded in the beam cap is included in the Estimated Quantities for Slab on Concrete NU-Girder.

All concrete above the intermediate beam cap is included in the Estimated Quantities for Slab on Concrete NU-Girder.

Hydrologic Data
Drainage Area = 249 sq. mi.
Design Flood Frequency = 50 yr.
Design Flood Discharge = 18,500 cfs
Design Flood (D.F.) Elevation = 924.3 ft.
Base Flood (100-Year)
Base Flood Elevation = 924.9
Base Flood Discharge = 21,300 cfs.
Estimated Backwater = 0.2 ft.
Average Velocity thru Opening = 4.1 ft./p
Freeboard (50-year)
Freeboard = 5.7 ft.
Roadway Overtopping
Overtopping Flood Discharge = 12,800 cfs.
Overtopping Flood Frequency = 12-yr
Overtopping Flood Elevation = 922.7 ft.



LOCATION SKETCH

General Notes:

Design Specifications:

2020 AASHTO LRFD Bridge Design Specification (9th Ed)

Seismic Design Category = A (Nonseismic) Design earthquake response spectral acceleration coefficient at 1.0 second

period, SD1 = 0.104a Acceleration Coefficient (effective peak ground acceleration coefficient), As = 0.066g

Design Loading:

Vehicular = HL-93 Future Wearing Surface = 35 lb/sf

Earth = 120 lb/cf. Equivalent Fluid Pressure 45 lb/cf

Superstructure: Simply-Supported, Non-Composite for dead load. Continuous Composite for live load.

Design Unit Stresses:

Class B Concrete (Substructure, except Drilled Shaft f'c = 3,000 psi& Rock Sockets) Class B-2 Concrete (Drilled Shafts & Rock Sockets) f'c = 4,000 psi

Class B-2 Concrete (Superstructure, except Prestressed Girders and Barrier) f'c = 4,000 psiClass B-1 Concrete (Barrier) f'c = 4,000 psiReinforcing Steel (ASTM A615 Grade 60) fy = 60,000 psify = 50,000 psi

Structural Steel HP Pile (ASTM A709 Grade 50) For NU-Girders, see Sheets No. 14 thru 19.

Neoprene Pads:

Neoprene Bearing Pads shall be 60 durometer and shall be in accordance with Sec 716.

Joint Filler:

All joint filler shall be in accordance with Sec 1057 for preformed sponge rubber expansion and partition joint filler, except as noted.

Reinforcing Steel:

Minimum clearance to reinforcing steel shall be 11/2", unless otherwise shown.

Traffic Handling:

Structure to be closed to traffic during construction. Traffic to be maintained on other routes during construction. See Roadway plans for traffic control.

				Bent N	Numb e r	
Туре	Design Data		1	2	3	4
	Pile Type and Size		HP 12x53	-	-	HP 12x53
	Numb e r	ea	4	-	-	4
	Approximate Length Per Each	ft	49	-	-	44
Load	Pile Point Reinforcement	e a	AII	-	-	AII
Bearing Pile	Min. Galvanized Penetration (Ele	ev.) ft	Full Length	-	-	Full Length
	Pile Driving Verification Method	d	DT	-	-	DT
	Resistance Factor		0.65	-	-	0.65
	Minimum Nominal Axial Compressive Resistance	kip	358	-	-	341
	Numb e r	e a	-	2	2	-
	_ Foundation Material		-	Weak Rock	Weak Rock	-
	Elevation Range	ft	-	888 - 878	886-874	-
	Minimum Nominal Axial Compressive Resistance (Side Resistance)	ksf	<u>-</u>	4.0	4.0	<u>-</u>
	Foundation Material		-	Strong Rock	Strong Rock	-
Rock Socket	Elevation Range	ft	-	878 - 855	874-861	-
JUCKET	Minimum Nominal Axial Compressive Resistance (Side Resistance)	ksf	-	12.9	12.9	-
	Minimum Nominal Axial					

34.0

39.2

Note: This drawing is not to scale. Follow dimensions.

Foundation Data

Estimated Quantities for Slab on Concrete NU-Girder	
I t em	Total
Class B-2 Concrete cu. yard	262
Reinforcing Steel (Epoxy Coated) pound	97,070

The table of Estimated Quantities for NU-Girder represents the quantities used by the State in preparing the cost estimate for concrete slabs. The area of the concrete slab will be measured to the nearest square yard longitudinally from end of slab to end of slab and transversely from out to out of bridge slab (or with the horizontal dimensions as shown on the plan of slab). Payment for stay-in-place corrugated steel forms, conventional forms, all concrete and epoxy coated reinforcing steel will be considered completely covered by the contract unit price for the slab. Variations may be encountered in the estimated quantities but the variations cannot be used for an adjustment in the contract unit price.

Method of forming the slab shall be as shown on the plans and in accordance with Sec 703. All hardware for forming the slab to be left in place as a permanent part of the structure shall be coated in accordance with ASTM A123 or ASTM B633 with a thickness class SC 4 and a finish type I, II or III.

Slab shall be cast-in-place with conventional forms or stay-in-place corrugated steel forms. Precast prestressed panels will not be permitted.

Load Bearing Pile: DT = Dynamic Testing Minimum Nominal Axial Compressive Resistance = Maximum Factored Loads Resistance Factors

Minimum Nominal Axial Compressive Resistance (Side Resistance + Top Resistance) = Maximum Factured Loads/Resistance Factors.

Prebore for piles at Bent No. 1 to elevation 889.00.

Compressive Resistance

(Tip Resistance)

Manufactured pile point reinforcement shall be used on all piles in this structure.

Detailed Oct. 2024 Checked Nov. 2024 HP Piles are anticipated to be driven to refusal on rock. Review all borings for depth of rock and restrict driving as appropriate to comply with hard rock driving criteria in accordance with Sec 702.

All piles shall be galvanized down to the minimum galvanized penetration (elevation).

Pile point reinforcement need not be galvanized. Shop drawings will not be required for pile point reinforcement.

Thickness of permanent steel casing shall be in accordance with Sec 701.

Sonic logging testing shall be performed on all drilled shafts and rock sockets.

Sheet No. 2 of 36

GENERAL NOTES AND QUANTITIES



REVISED 01-22-2025

DATE PREPARED

1/22/2025

COUNTY

WORTH

JOB NO.

JNW0020

CONTRACT ID.

PROJECT NO.

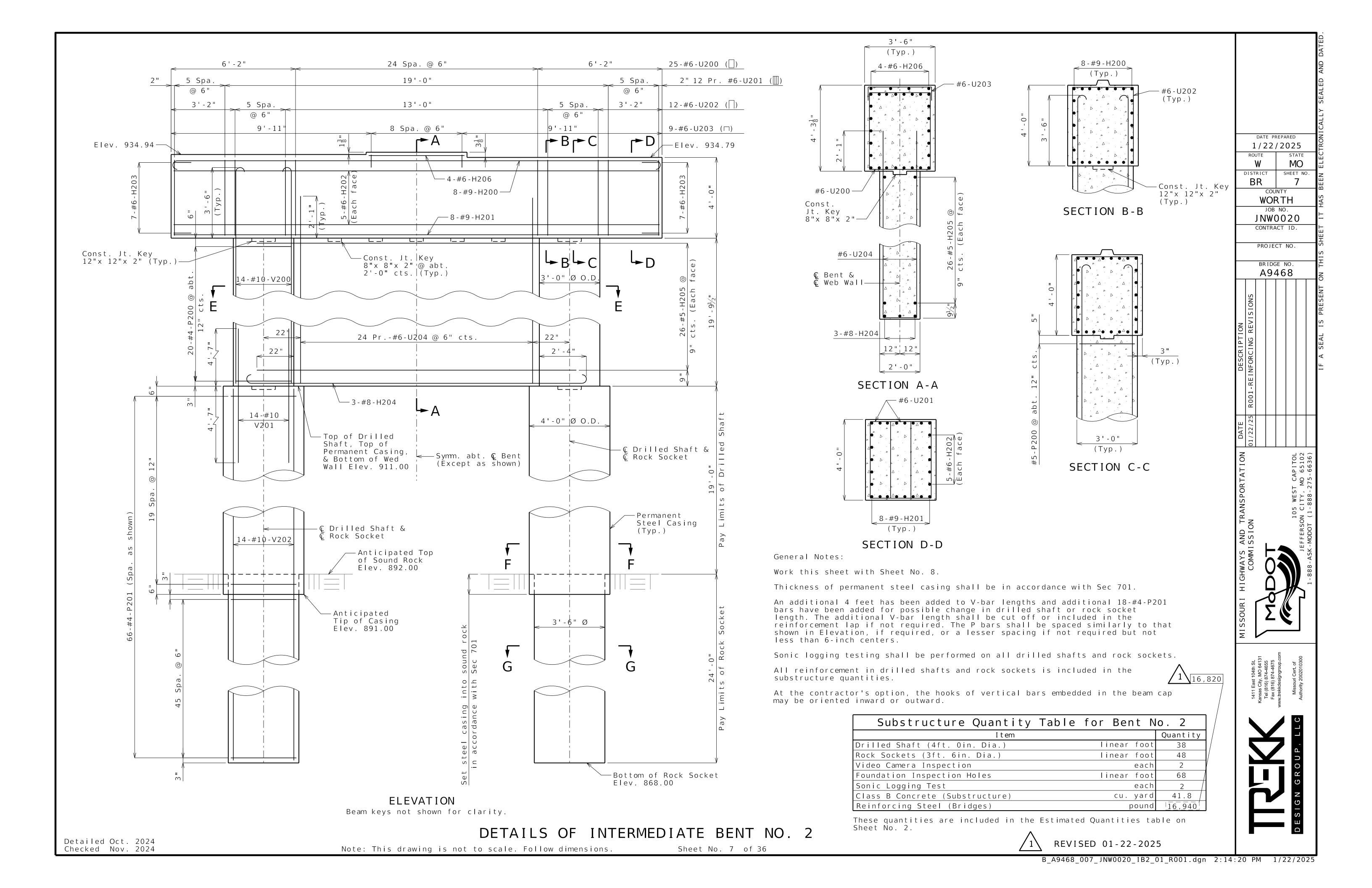
BRIDGE NO.

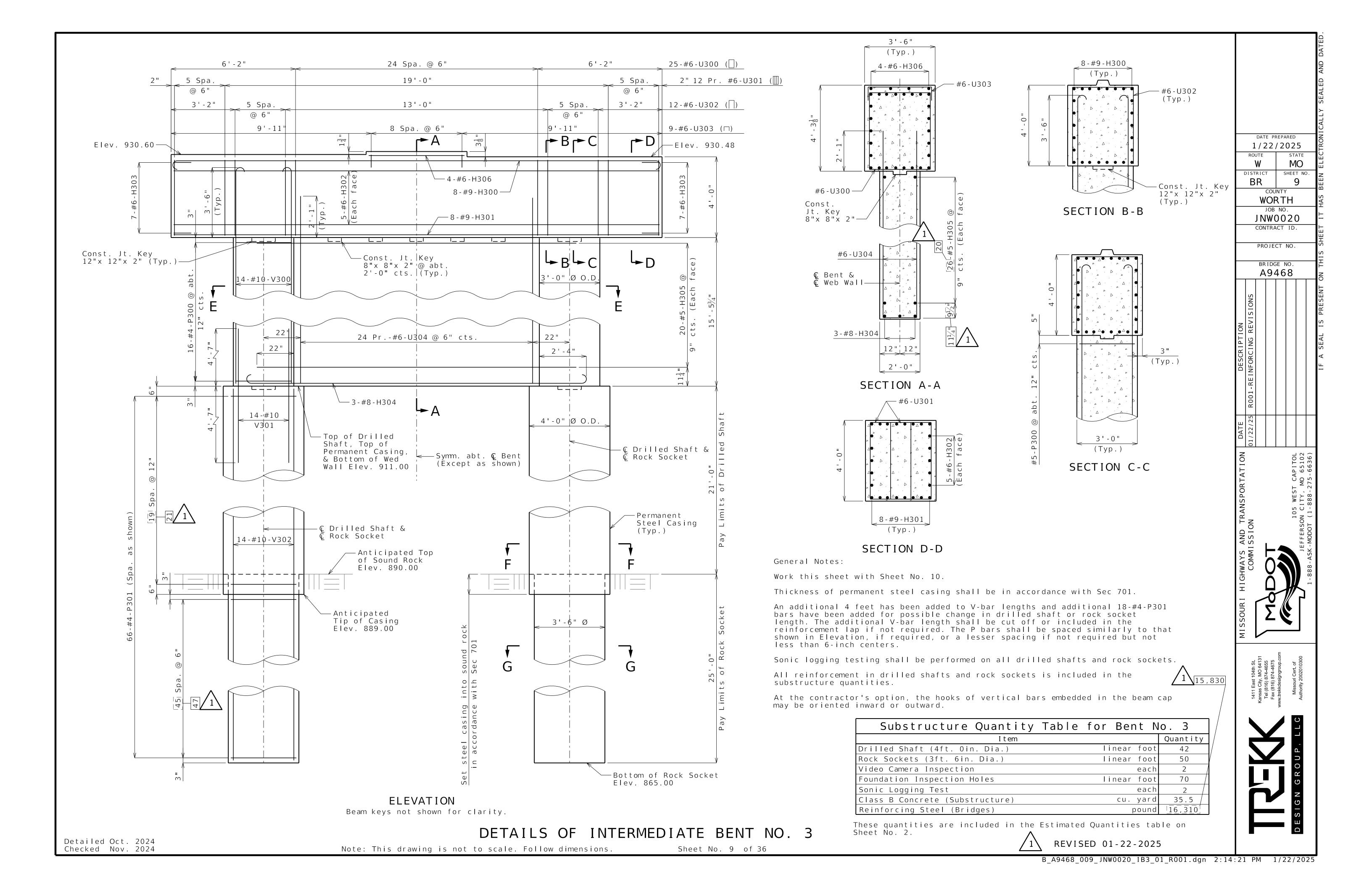
A9468

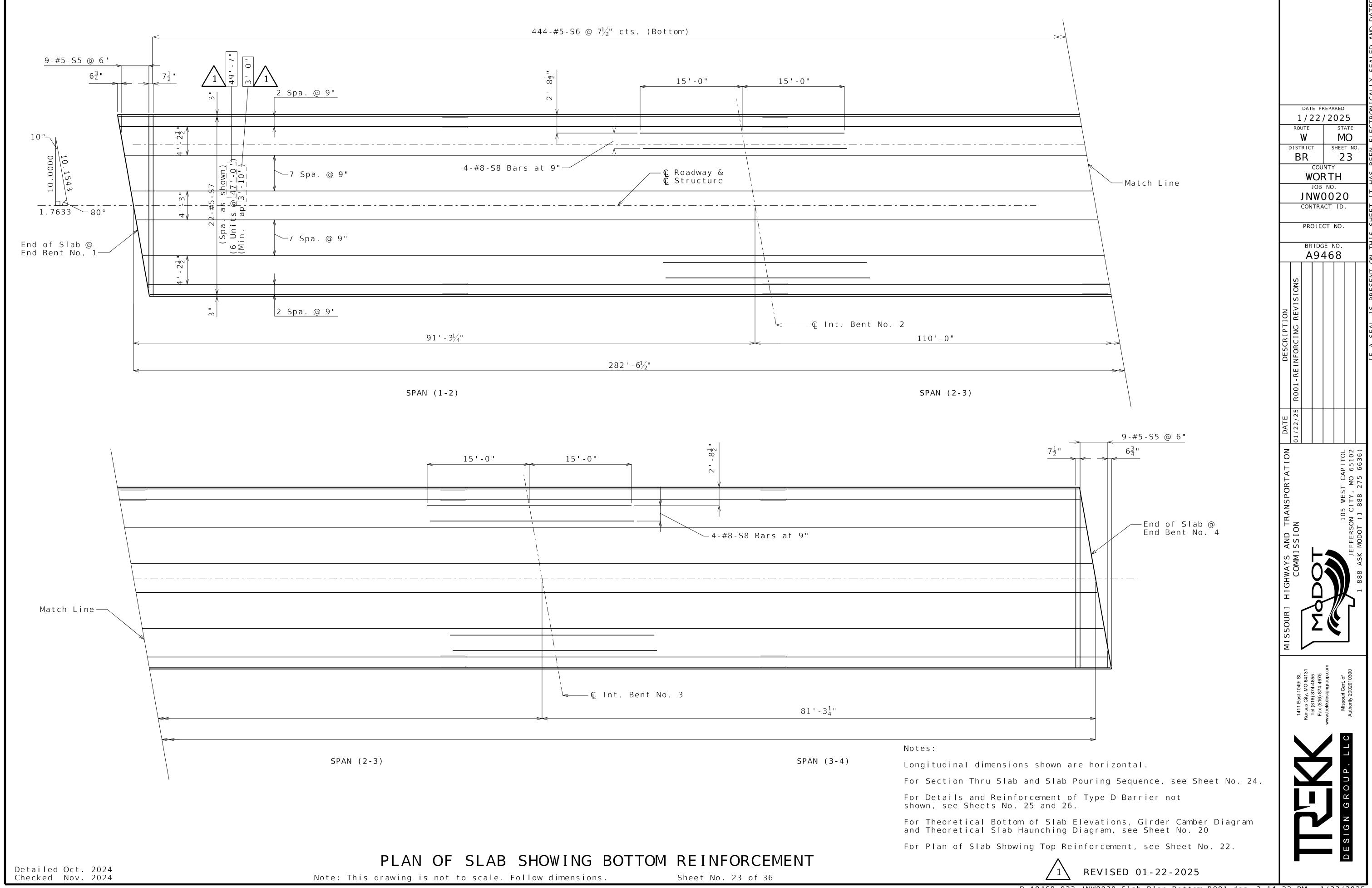
MO SHEET NO.

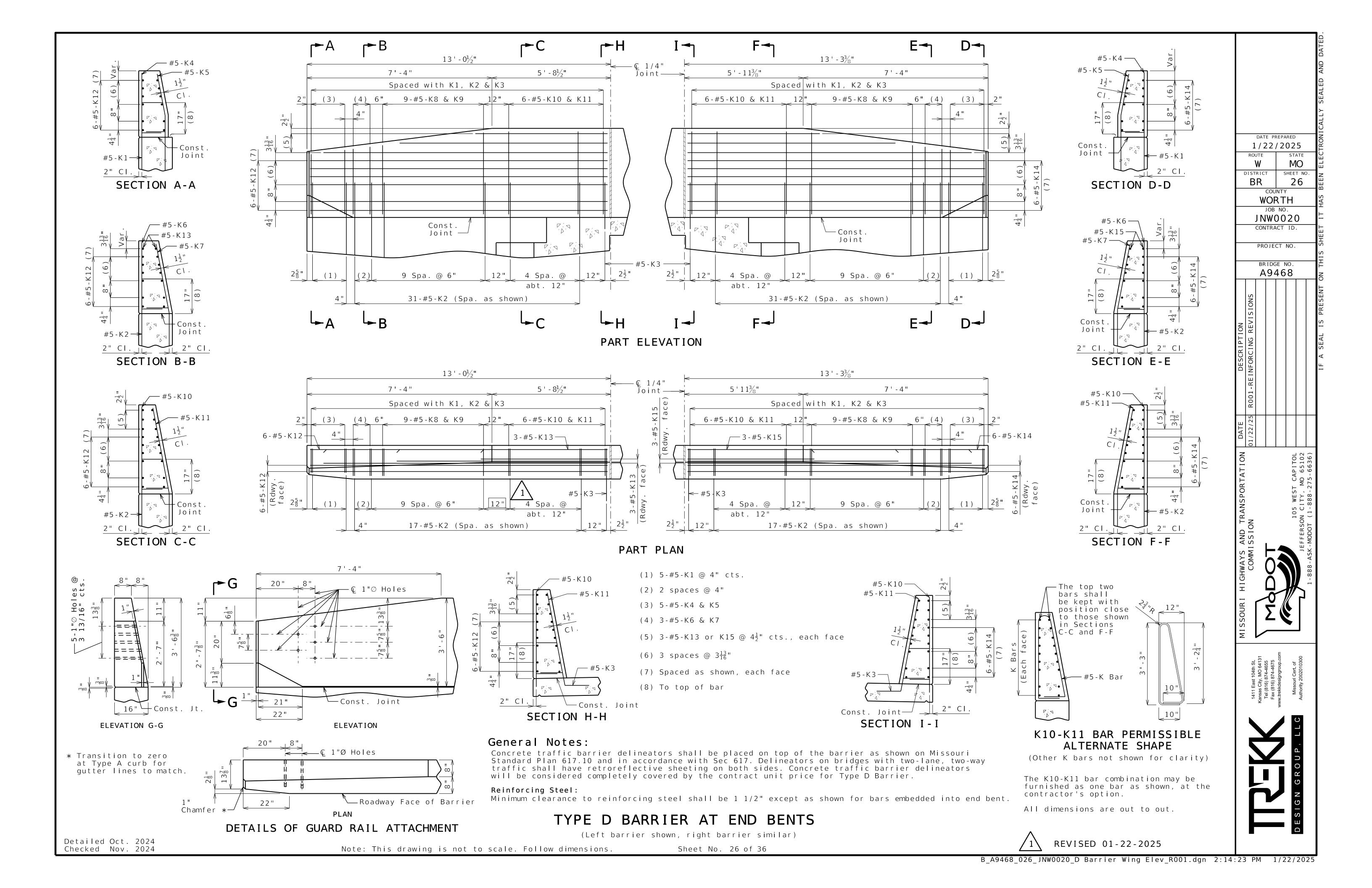
ROUTE

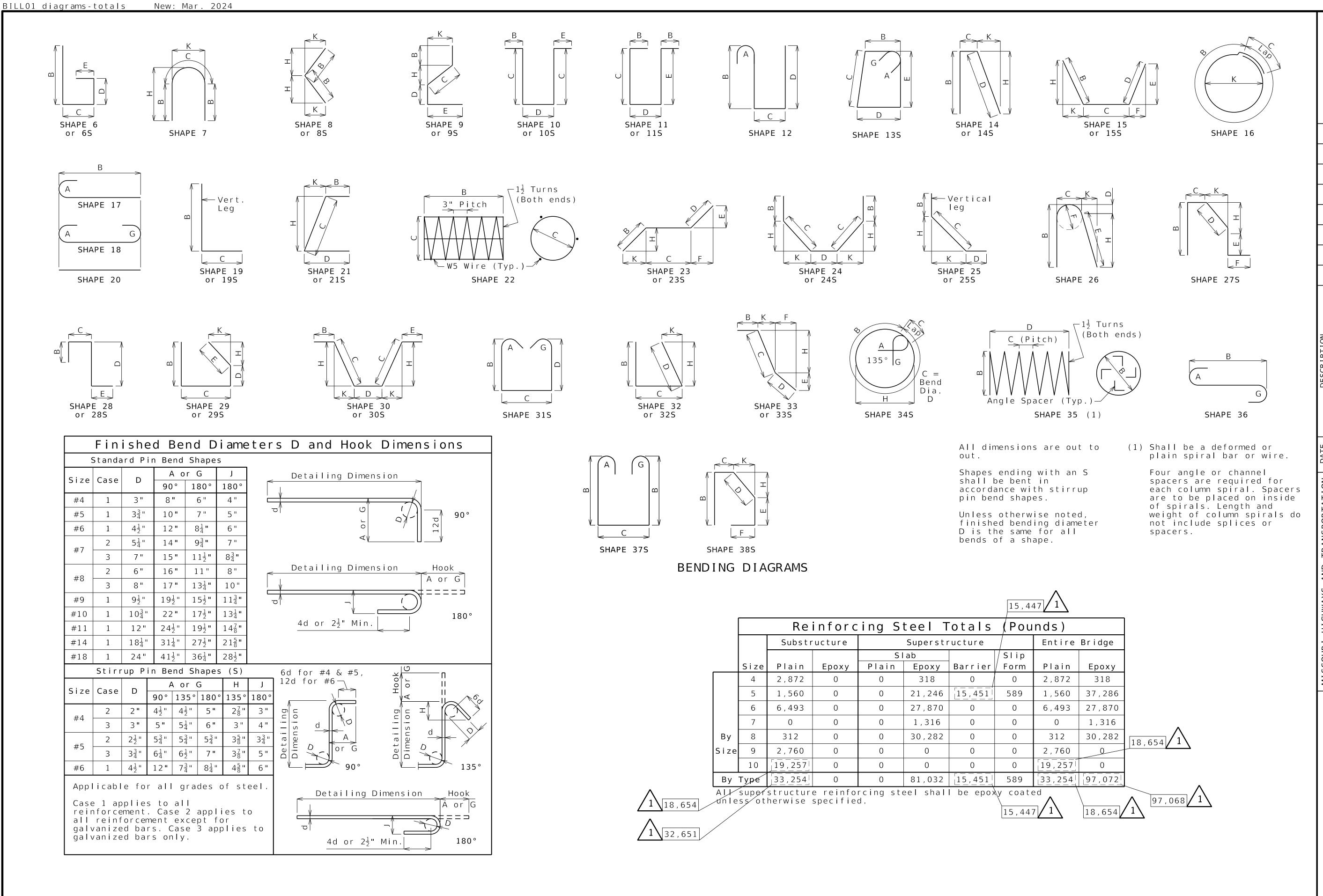
DISTRICT











Detailed Oct. 2024

Checked Nov. 2024

DATE PREPARED

1/22/2025

COUNTY

JOB NO.

JNW0020

PROJECT NO.

BRIDGE NO.

A9468

DISTRICT

 BR

MO

SHEET NO

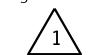
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							mensions	<u> </u>		Non	n. Actual									imensions				Nom.	Actual		$\exists 1$
No.	Size/	Codes	В	ТС		D	E	F	Н	K Leng		Weight	No.	Size/		Codes	В	С	D	E	F	Н	K	Length	Length		nt
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-	INT. BENTING. 2												6		DIA PHRA GM	E 20	8 10.500							8 10		10	80
11	6 DOOD BEAM KEY	1 20	2 600)O						2	6 2 6	53												6 10	5	10	18
14	6 D200 BEAM KEY	20	2 6.00	101							0 2 0	55	2		DIA PHRA GM	E 20	6 1.500							0 1	3		10
	0 11000 55111	10		\								705	6		DIA PHRA GM	E 20	3 4.500							3 4	3	4	30
8	9 H200 BEAM	18	24 1.00							26	8 26 8	725	16		DIA PHRA GM	E 20	2 0.000							2 0	2	0	85
8	9 H201 BEAM	20	24 1.00							24	1 24 1	655	16	8 H106		E 20	12 6.750								12		534
10	6 H202 BEAM	20	24 1.00							24	1 24 1	362	40	6 H107		E 20	11 9.500							11 10	11	10 /	711
14	6 H203 BEAM	10			000 3	1.500				5	2 4 10	102	24		DIA PHRA GM	E 19	2 0.000 1	3.000						3 3	3	2	79
3	8 H204 WEB WALL	18	16 10.00							19	5 19 5	156	3	5 H109	STRAND TIE	E 23	1 3.000 3	2.500 1	3.000	2.500	1 2.750	2.500 1	2.750	5 9	5	8	18
52	5 H205 WEB WALL	20	15 10.00	00						15	10 15 10	859															
4	5 H206 BEAM	20	4 9.00	00						4	9 4 9	20	10	5 U100	BEAM	E 31	4 6.125 2	9.500 4	6.125					12 9	12	7 1	131
													21	5 U101	BEAM	E 13	2 9.500 2	8.000 2	9.500 2	8.000				11 10	11	_	252
28	10 V200 COLUMN	17	23 2.00	00 /	\sum					24	8 24 8	2972	33	6 U103	DIA PHRA GM	E 19	3 2.875 4	7.000						7 10	7	8 ?	380
28	10 V201 SHAFT	20	9 2.00	00 45						1 45 9	2 9 2	1104	18	6 U104	DIAPHRAGM	E 19	3 1.625 2	9.500						5 11	5	9 ′	155
28	10 V202 SOCKET/SHAFT	20	46 9.0	10						46	9 46 9	5633	18	5 U105	DIA PHRA GM	E 31	3 11.000 2	3.375 3	11.000					11 1	10	10 7	203
										1 45																$\overline{}$	\neg
25	6 U200 BEAM	13	3 3.00	00 3 9.	000 3	3.000 3	9.000			15	3 14 9	554	16	5 V100	BEAM & DIAPHRAGM	E 17	4 6.125							5 1	5	1	85
24	6 U201 BEAM	13	2 2.00		000 2	2.000 3	9.000			13	1 12 7	454	15		DIA PHRA GM	E 20	3 0.625							3 0	3	0	68
12	6 U202 BEAM	10			000 3	3.000				10	9 10 5	188	2	6 V102		E 20	7 9.375							7 9	7	9	23
9	6 U203 BEAM	10		_	000 3	3.000				5	3 4 11	66	20	6 V103			V 7 3.750							7 3	7	3 7	224
24	6 U204 WEB WALL	10			000	21.000				45	3 45 0	1622			INC = 0.500 INCH		7 8.750							7 8	7	8	-
	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	+ +											2	6 V104		E 20	7 11.750							7 11	7	11	24
20	4 P200 COLUMN	16	8 7.7	50 2 5	000					2 9.000 11	1 11 1	148	20	6 V105		E 20	V 7 6.250							7 6	7	6 7	232
150	4 P201 SOCKET/SHAFT	16	10 2.50							2 9.000 12	8 12 8	1269			INC = 0.500 INCH		7 11.250							7 11	7	11	
100	1 1 20 1 GOOKE WATER	+ + 10 +	10 2.0	,0 2 0.						2 0.000 12	0 12 0				110 0.000 111011		7 11.200									''	\dashv
	INT. BENT NO. 3														II DIA PHRA GM A T INT	BENTS 2	<u> </u>										-
	#\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\																										-
14	6 D300 BEAM KEY	20	2 6.0	00						2	62 6	53	8	6 H500	DIA PHRA GM	E 20	6 1.750							6 1	6	1	73
'-	0 B300 BEAWINET	20	2 0.00	,0						2	0 2 0		16		DIA PHRA GM	E 20	8 6.500							8 6	2	-	91
	9 H300 BEAM	18	24 1.00	00						26	8 26 8	725	0		DIA PHRA GM	E 20	8 6.500							0 0	9	6	102
0	9 H301 BEAM	20	24 1.00		_					24	1 24 1	655			DIA PHRA GM	E 6	4 5.000	10.500						5 3	5	2	13
10	6 H302 BEAM	20	24 1.00							24	1 24 1	362	22		DIAPHRAGM	E 6	4 10.000	10.500						7 0	-	2 ,	225
10	6 H303 BEAM	10	24 1.00		000 3	1.500				5	24 1	102	0		STRAND TIE	E 20	4 5.500	10.300						7 6	7	9 2	38
2	8 H304 WEB WALL	18	16 10.00		000 3	1.500				19	5 19 5	156	0		STRAND TIE	E 20	5 8.500							5 0	+		24
10											10 15 10	661	4	5 11506	STRAIND HE	E 20	5 6.500							5 9		9	
40	5 H305 WEB WALL	20	15 10.00							15	10 15 10	20	10	4 1 1500		F 00	4	11 000 0	0.500	0.000				7 0		4 (227
4	5 H306 BEAM	20	4 9.00	101						4	9 4 9	20	48		DIA PHRA GM	E 28	1	11.000 3	9.500	6.000				7 3	7	1 2	220
	40 1/000 001 19 91	17	40 400							Λ	100	0.450	32		DIA PHRA GM	E 28	1	11.000 3	0.500 2	1.750				7 2	<u>3</u>	10 3	328
28	10 V300 COLUMN	17	18 10.00	 /	\longrightarrow					1 20	4 20 4	2450	16	6 0502	DIA PHRA GM	E 28	1	11.000 3	9.500 2	2 1.750				/ 11	<u>/</u>		182
28	10 V301 SHAFT	20	9 2.00	0 45	<u>- </u>				4	45 9	2 9 2	1104	40	5 1/500	DIA DI IDA CAA	- OO	4 200							4			
28	10 V302 SOCKET/SHAFT	20	49 9.0	101						49	9 49 9	5994	16	5 V 500	DIA PHRA GM	E 20	4 1.000							4 1	+	1	
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25	6 U300 BEAM	13	3 3.00		000 3	3.000 3	9.000			15	3 14 9	151	<u> </u>		END BENT 4		23/1				<u> </u>	I					\dashv [
24	6 U301 BEAM	13	2.00	_	000 2	2.000 3	9.000			13	1 12 7	454		0 5100		F 22		40.500 4	0.000	40.755	0.000 (0.750	F 055	0 2			110
12	6 U302 BEAM	10			000 3	3.000				10	9 10 5	188	9		WING BRACE	E 23	/	10.500 1	2.000	10.750	9.000 1	8.750 1	5.250		3	4	110
9	6 U303 BEAM	10			000 3	3.000				5	3 4 11	66	4		DIA PHRA GM	E 6_/	2 8.750 5	5.750	0.000	10 ===	2	8.250	5.750		5	1	49
24	6 U304 WEB WALL	10		17 6.	000	21.000				36	9 36 5	1313	9		WING BRACE	E 23	2 3.000 5	9.000 1	2.000	10.750	9.000 1	8.750 1	5.250		<i>y</i>	1 1	123
												,	4	6 F403	DIA PHRA GM	E _6_	2 8.750 5	5.750			2	8.250	5.750	8 3	3	0	48
16	4 P300 COLUMN	16	8 7.7							2 9.000 11	1 1 1	118															
158	4 P301 SOCKET/SHAFT	16	10 2.50	00 2 5.	000					2 9.000 12	8 12 8	1337	4		BEAM & DIAPHRAGM	E 20	26 10.000 2	3/ 1						26 10			161
													12		BEAM & DIAPHRAGM	E 20	26 10.000							26 10		10 F	658
	SUPERSTRUCTUR	RE											6		DIAPHRAGM	E 20	8 10.500							8 10	3	10	80
	END BENT 1		23/1	Δ			<u> </u>		-				2		DIAPHRAGM	E 20	6 1.500							6 1	3	1	18
													6		DIA PHRA GM	E 20	3 4.500							3 4	3	4	30
10	6 F100 WING BRACE	E 23	2 3.00	00 4 10.	500 1	2.000	10.750	9.000 1	8.750	1 5.250 8	3 8 2	123	16	8 H405	DIA PHRA GM	E 20	2 0.000							2 0	2	0	85
4	6 F101 DIAPHRAGM	E 6_/	2 8.7	50 5 5.	750			2	8.250	5.750 8	3 8 1	49	16	8 H406	WING	E 20	12 6.750							12 6	12		534
10	6 F102 WING BRACE	E 23	2 3.00	00 5 9.	000 1	2.000	10.750	9.000 1	8.750	1 5.250 9	2 9 1	136	36	6 H407	WING	E 20	11 9.500							11 10	11	10 F	640
4	6 F103 DIAPHRAGM	E [6_	2 8.7	50 5 5.	750			2	8.250	5.750 8	3 8 0	48	24	5 H408	DIAPHRAGM	E 19	2 0.000 1	3.000						3 3	3	2	79
													3	5 H409	STRAND TIE	E 23	1 3.000 3	2.500 1	3.000	2.500	1 2.750	2.500 1	2.750	5 9	5	8	18
			23/1															<u>-</u>									

All bars shall be Grade 60.

Codes: C = Required coatings, where E = Epoxy Coated and <math>G = Galvanized.

SH = Required shape, see bending diagrams.

V = Sets of varied bars and number of bars of each length. Bar dimensions vary in equal increments between dimensions shown on this line and the following line and the actual length dimension shown on this line and the following line vary by the specified increment.



REVISED 01-22-2025

DATE PREPARED 1/28/2025

VORTH

JOB NO.

JNW0020

CONTRACT ID.

PROJECT NO.

BRIDGE NO. A9468

SHEET NO.

centerline bar to the nearest inch. Weights are based on actual lengths.

For bending diagrams and steel reinforcing totals, see Sheet No. 28.

BILL OF REINFORCING STEEL

Detailed Oct. 2024 Checked Nov. 2024

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 29 of 36

				Ві	<u>ill o</u>			ing S	Stee			I	T -		_	T			1		<u>B i I</u>	<u>l of</u>		forci		eel		1 -			
							Dimensio	ns	ı		1.7	Nom.	Actual								<u> </u>			Dimensions		· · ·		Nom.	Actual	\	
o. Size/		Cod		В	С	D	<u>E</u>	F		H	K	Length							Codes				D	E	F	H	K	Length		Weight	
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	DIA PHRA GM	E 1	9 2	11.000 2	9.500							5	3 5	0	49																
8 5 U405 Γ	DIAPHRAGM	E 3	1 3	10.000 2	3.375	10.000						10 1	1 10	8 2	200																DA ⁻
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6 5 V400 F	BEAM & DIAPHRAGM	E 1	7 4	5.625								5	1 5	1	85																ROUTE
5 6 V401 r	DIAPHRAGM	E 2) 2	11.000								2 1	1 2 1	1	66																I w
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0 6 V403 V		-	V 7	0.125									0 7		214																BR
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5 K10 E	BARRIER	E 1	3	3.000	10.000							4	1 4	0	00																I S
5 K11 E	BARRIER	E 2	1 3	3.000	10.000				3	2.250	7.750	4	1 3 1	1	98																Σ
5 K12 E		E 2) 12									12	9 12	9 :	319																
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All bars shall be Grade 60.

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REVISED 01-22-2025

For bending diagrams and steel reinforcing totals, see Sheet No. 28.

Detailed Oct. 2024 Checked Nov. 2024

BILL OF REINFORCING STEEL Sheet No. 30 of 36