Estimated Quantities							
I t em		Substr.	Superstr.	Total			
Class 1 Excavation	cu. yard	400		400			
Removal of Bridges (G0248)	lump sum			1			
Bridge Approach Slab (Minor)	sq. yard		116	116			
Galvanized Structural Steel Piles (12 in.) linear foot	652		652			
Pile Point Reinforcement	each	34		34			
Class B Concrete (Substructure)	cu. yard	154.2		154.2			
Type D Barrier	linear foot		308	308			
Slab on Concrete NU-Girder	sq. yard		338	338			
NU 43, Prestressed Concrete NU-Girder	linear foot		402	402			
Reinforcing Steel (Bridges)	pound	7,540		7,540			
Slab Drain	each		16	16			
Vertical Drain at End Bents	each			2			
Plan Neoprene Bearing Pad	each		8	8			
<u> </u>							
<u> </u>							
				-			

All concrete above the construction joint in the end bents is included in the Estimated Quantities for Slab on Concrete NU-Girder

All reinforcement in the end bents (except detached wing walls) is included in the Estimated Quantities for Slab on Concrete NU-Girder.

Cost of L4x4 ASTM A709 Grade 36 HP pile anchors and 3/4-inch diameter ASTM F3125 Grade A325 Type 1 bolts, complete in place, will be considered completely covered by the contract unit price for Galvanized Structural Steel Piles (12 in.).

Estimated Quantities for Slab on Concrete NU Girder		
I t em	Total	
Class B-2 Concrete cu. yar		
Reinforcing Steel (Epoxy Coated) poun	d (14,610)	37,450

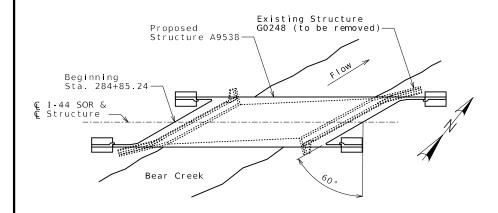
The table of Estimated Quantities for Slab on Concrete NU Girder represents the quantities used by the State in preparing the cost estimate for concrete slabs. The area of the concrete slab will be measured to the nearest square yard longitudinally from end of slab to end of slab and transversely from out to out of bridge slab (or with the horizontal dimensions as shown on the plan of slab). Payment for prestressed panels, conventional forms, all concrete and epoxy coated reinforcing steel will be considered completely covered by the contract unit price for the slab. Variations may be encountered in the estimated quantities but the variations cannot be used for an adjustment in the contract unit price.

Method of forming the slab shall be as shown on the plans and in accordance with Sec 703. All hardware for forming the slab to be left in place as a ermanent part of the structure shall be coated in accordance with ASTM A123 or ASTM B633 with a thickness class SC 4 and a finish type I, II or III.

The Estimated Quantities for Slab on Concrete NU-Girder are based on skewed precast prestressed end panels.

Class B-2 Concrete quantity is based on minimum top flange thickness and

The prestressed panel quantities are not included in the table of Estimated Ouantities for Slab on Concrete NU-Girder.



LOCATION SKETCH

Hydrologic Data Drainage Area = 7.9 mi² Design Flood Frequency = 50 years Design Flood Discharge = 3,370 cfs Design Flood (D.F.) Elevation = 1074.9 Base Flood (100-year) Base Flood Elevation = 1075.8Base Flood Discharge = 4,390 cfs Estimated Backwater = 0.0 ft Average Velocity thru Opening = 7.6 ft/sFreeboard (50-vear) Freeboard = -1.3 ft Roadway Overtopping Overtopping Flood Discharge > 5,950 cfs

Overtopping Flood Frequency > 500 years

500-year Flood Elevation = 1077.3

General Notes:

Design Specifications: 2020 AASHTO LRFD Bridge Design Specifications (9th Ed.) 2011 AASHTO Guide Specifications for LRFD Seismic Bridge Design (2nd and 2014 Interim Revisions (Seismic Details) Seismic Design Category = B

Design Loading: Vehicular = HL-93 Future Wearing Surface = 35 lb/sf Earth = 120 lb/cf Equivalent Fluid Pressure = 45 lb/cf (Min.) Superstructure: Non-composite for dead load. Composite for live load.

Design Unit Stresses: Class B Concrete (Substructure) f'c = 3.000 psiClass B-2 Concrete (Superstructure, except Prestressed Girders and Barrier) f'c = 4,000 psiClass B-1 Concrete (Barrier) f'c = 4,000 psiReinforcing Steel (ASTM A706 Grade 60) fv = 60,000 psi

For prestressed panel stresses, see Sheet No. 14.

Structural Steel HP Pile (ASTM A709 Grade 50)

For prestressed girder stresses, see Sheets No. 12 & 13.

Neoprene Pads: Neoprene bearing pads shall be 60 durometer and shall be in accordance with Sec 716.

All joint filler shall be in accordance with Sec 1057 for preformed sponge rubber expansion and partition joint filler,

Reinforcing Steel: Minimum clearance to reinforcing steel shall be 1 1/2"

Structure to be closed during construction. See roadway plans for traffic control.

Miscellaneous: Payment for furnishing all materials, labor and excavation necessary to construct both detached wing walls at End Bents No. 1 and No. 2 including the Class 1 Excavation, Galvanized Structural Steel Pile, (12 in.), Class B Concrete (Substr.) and Reinforcing Steel (Bridges), will be considered completely covered by the contract unit price for these items.

MoDOT Construction personnel will indicate the type of joint filler option used under the precast panels for this structure:

Constant Joint Filler □ Variable Joint Filler

Foundation Data									
		Bent Number							
Туре	Design Data	1 (Detached Wing Walls Only)	1 (Except Detached Wing Walls)	2 (Detached Wing Walls Only)	2 (Except Detached Wing Walls)				
Load Bearing Pile	Pile Type and Size	HP 12x53	HP 12x53	HP 12x53	HP 12x53				
	Number e	10	7	10	7				
	Approximate Length Per Each f	21	24	12	22				
	Pile Point Reinforcement e	All	AII	AII	AII				
	Min. Galvanized Penetration (Elev.) f	: Full Length	Full Length	Full Length	Full Length				
	Pile Driving Verification Method	DF	DF	DF	DF				
	Resistance Factor	0.40	0.40	0.40	0.40				
	Minimum Nominal Axial Compressive Resistance ki	350	543	350	543				

DF = FHWA-modified Gates Dynamic Pile Formula

 $\label{eq:minimum Nominal Axial Compressive Resistance} \ = \ \frac{\text{Maximum Factored Loads}}{\text{Resistance Factor}}$

Pile point reinforcement need not be galvanized. Shop drawings will not be required for pile point reinforcement.

All piles shall be galvanized down to the minimum galvanized penetration (elevation).

The contractor shall make every effort to achieve the minimum galvanized penetration (elevation) shown on the plans for all piles. Deviations in penetration less than 5 feet of the minimum will be considered acceptable provided the contractor makes the necessary corrections to ensure the minimum penetration is achieved on subsequent piles.

HP piles are anticipated to be driven to refusal on rock. Review all borings for depth of rock and restrict driving as appropriate to comply with hard rock driving criteria in accordance with Sec 702. When pile refusal on rock occurs, as approved by the engineer, the minimum nominal axial compressive resistance is verified and no additional pile driving verification method is required.

GENERAL NOTES AND QUANTITIES Sheet No. 2 of 25

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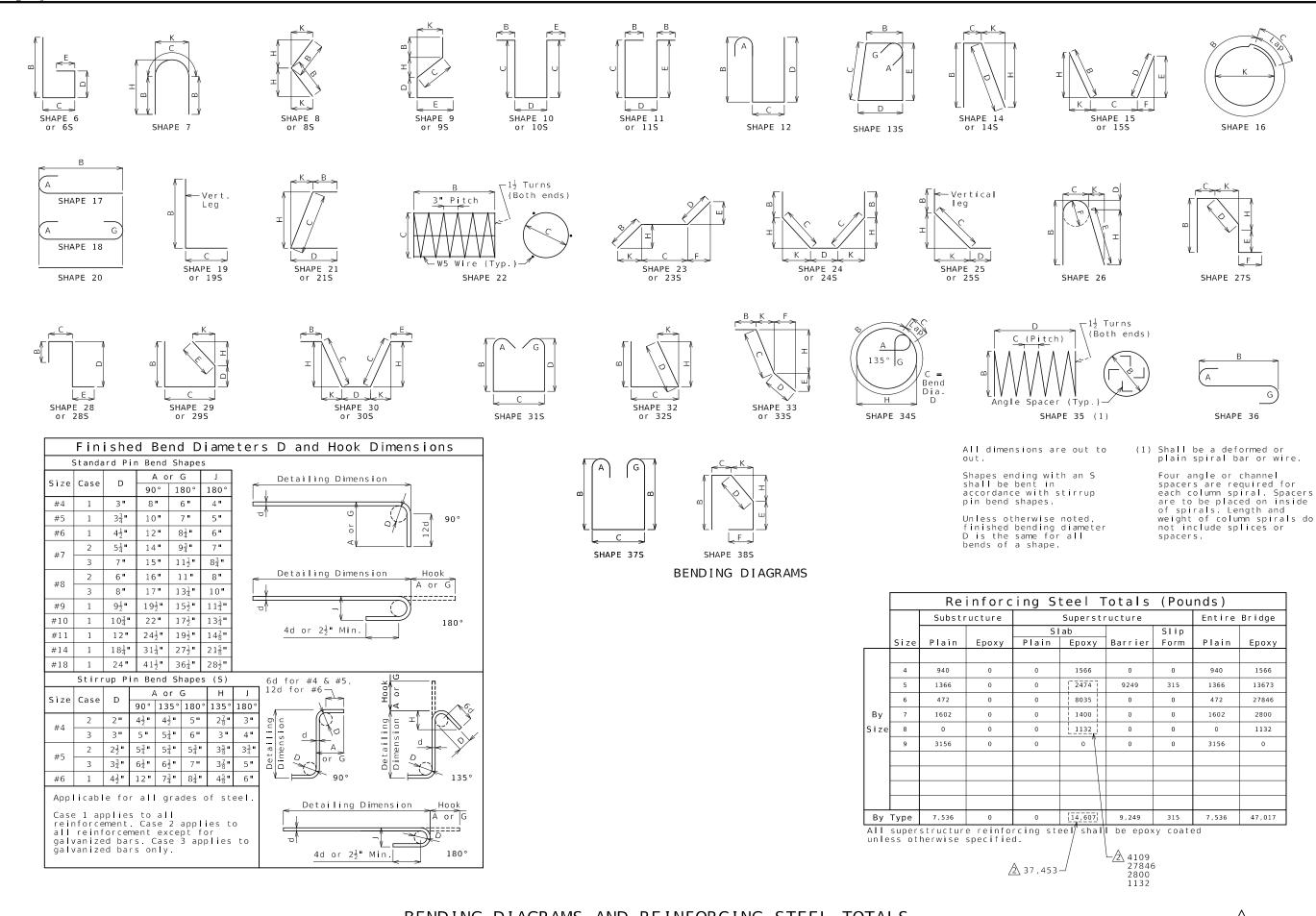
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Alex Benz - Civil

PROJECT NO. A9538

fv = 50,000 psi

Civil Engineering Design 13523 Barrett Parkway Dr Suite 250 St. Louis, MO 63021 Fax 314.



<u></u> REVISED

BILLO1 diagrams-totals

Detailed Aug. 2024 Checked Aug. 2024 New: Mar. 2024

ALEX BENZ NUMBER PE-20180031

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PROJECT NO.

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