DESIGN DESIGNATION

A.A.D.T. - 2023 = 450 A.A.D.T. - 2034 = 450 D.H.V. = 52 T = 10.7% V = 55 M.P.H.

FUNCTIONAL CLASSIFICATION - SUPPLEMENTARY

NO RIGHT OF WAY ACQUISITION NEEDED

CONVENTIONAL SYMBOLS

(0020 110 12/110	.,	
	EXISTING	NEW
BUILDINGS AND STRUCTURES GUARD RAIL GUARD CABLE CONCRETE RIGHT-OF-WAY MARKER STEEL RIGHT-OF-WAY MARKER LOCATION SURVEY MARKER UTILITIES	0000 0000 111 121	•••• ••••
FIBER OPTICS OVERHEAD CABLE TV UNDERGROUND CABLE TV OVERHEAD TELEPHONE UNDERGROUND TELEPHONE OVERHEAD POWER UNDERGROUND POWER SANITARY SEWER STORM SEWER GAS WATER	- FO OTV UTV OT UT OE UE S S G W SAN	-OTV -UTV -OT -UT -OE -UE -S
MANHOLE	SAN HYD)
FIRE HYDRANT	wv C	Ì
WATER VALVE	wv_C)
WATER METER	****)
DROP INLET	°[=	
DITCH BLOCK	=	⊨
GROUND MOUNTED SIGN	SIGN	-
LIGHT POLE		
H-FRAME POWER POLE		
TELEPHONE PEDESTAL FENCE CHAIN LINK WOVEN WIRE	PEDV	·
GATE POST	BM ←	7
BENCHMARK	8)

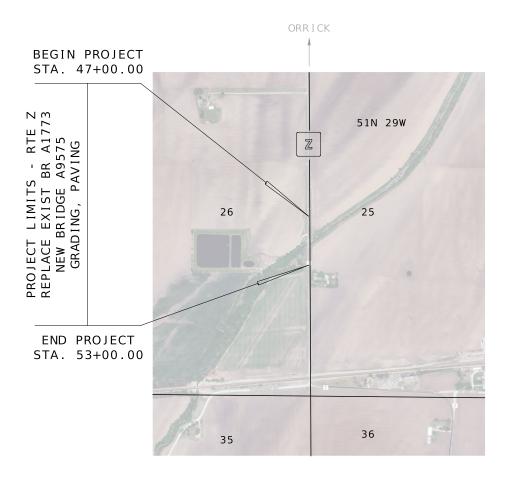
NOTE: DASHED OR OPEN SYMBOLS INDICATE EXISTING FEATURES

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

PLANS FOR PROPOSED STATE HIGHWAY



RAY COUNTY



NOT TO SCALE

THE EXISTENCE AND APPROXIMATE LOCATION OF UTILITY FACILITIES KNOWN TO EXIST, AS SHOWN ON THE PLANS, ARE BASED ON THE BEST INFORMATION AVAILABLE TO THE COMMISSION AT THIS TIME. THIS INFORMATION IS PROVIDED BY THE COMMISSION "AS-IS" AND THE COMMISSION EXPRESSLY DISCLAIMS ANY REPRESENTATION OR WARRANTY AS TO THE COMPLETENESS, ACCURACY, OR SUITABILITY OF THE INFORMATION FOR ANY USE. RELIANCE UPON THIS INFORMATION IS DONE AT THE RISK AND PERIL OF THE USER, AND THE COMMISSION SHALL NOT BE LIABLE FOR ANY DAMAGES THAT MAY ARISE FROM ANY ERROR IN THE INFORMATION. IT IS, THEREFORE, THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY THE EXISTENCE, LOCATION AND STATUS OF ANY FACILITY. SUCH VERIFICATION INCLUDES DIRECT CONTACT WITH THE LISTED UTILITIES.

INDEX OF SHEETS

DESCRIPTION	SHEET NUMBER
TITLE SHEET	1
TYPICAL SECTIONS (TS) (2 SHEETS)	2
QUANTITIES (QU) (2 SHEETS)	3
PLAN-PROFILE (PP)	4 - 5
REFERENCE POINTS (RP)	6
COORDINATE POINTS (CP)	7
TRAFFIC CONTROL SHEETS (TC)	8 - 9
EROSION CONTROL SHEETS (EC)	10
BRIDGE DRAWINGS (B) (23 SHEETS)	
A9575	1-23
CROSS SECTIONS (XS)	1 - 7

BENJAMN STEPHEN MCABE SR NUMBER PE-2003015005 30 NAL WALL MONE STEPHEN MODE 2003015005							
	2025						
ROUTE Z	STATE MO						
DISTRICT	SHEET NO.						
KC	1						
COU	NTY						
R/	AΥ						
JOB	NO.						
JKR	0103						
CONTRA	CT ID.						
PROJE	CT NO.						
	SE NO. 575						
111							
:1 1 1							

OF Mary

LENGTH OF PROJECT

BEGINNING OF PROJECT STA. 47 + 00.00
END OF PROJECT STA. 53 + 00.00

APPARENT LENGTH 600.00 FEET

EQUATIONS AND EXCEPTIONS:



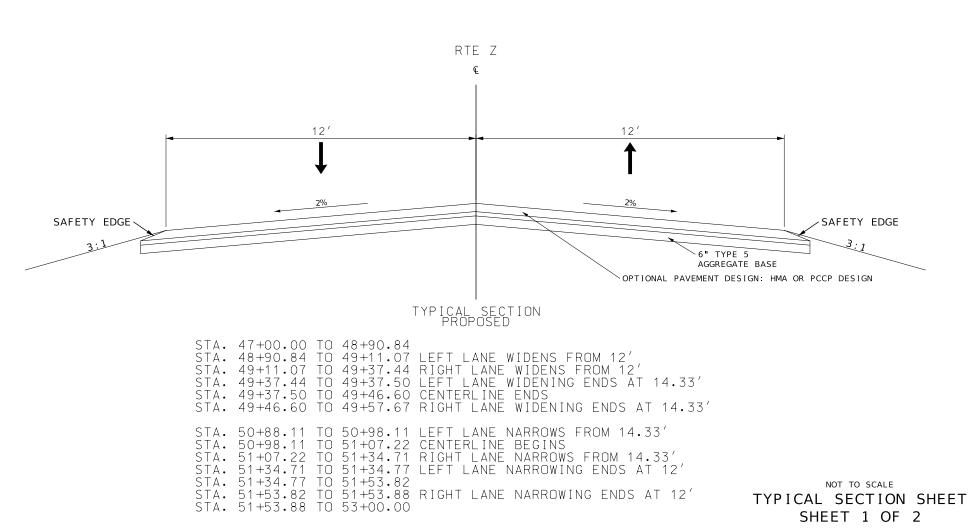
TOTAL CORRECTIONS 0 FEET

NET LENGTH OF PROJECT 600.00 FEET

STATE LENGTH 0.114 MILES

FOR INFORMATION ONLY
ESTIMATED DISTURBED ACRES 0.2466 ACRES

OPTIONAL PAVEMENT HMA DESIGN RTE Z 9" HMA 2" BP-1 w/PG 64-22 (1.984 TON/CY) OVER 7" BB w/PG 64-22 (1.997 TON/CY) 6" TYPE 5 AGGREGATE BASE SHOULDERS 12' FULL DEPTH AS PART OF THE TRAVELWAY PCCP DESIGN 8.5" JPCP NON-WIDEN SLAB 15 FT JOINT SPACING, 1 1/4" DOWELS FULL DEPTH AS PART OF THE TRAVEL WAY BASE 6" TYPE 5 AGGREGATE BASE EARTHWORK QUANTITIES ARE CALCULATED BASED ON HMA OPTIONAL PAVEMENT SECTION STA. 47+00.00 TO 49+67.00 STA. 50+77.75 TO 53+00.00



BENJAMIN STEPHEN MCCABE SR NUMBER PE-2003015005

3/4/2025

RAY

LOB NO

JKR0103 CONTRACT ID.

BRIDGE NO

A9575

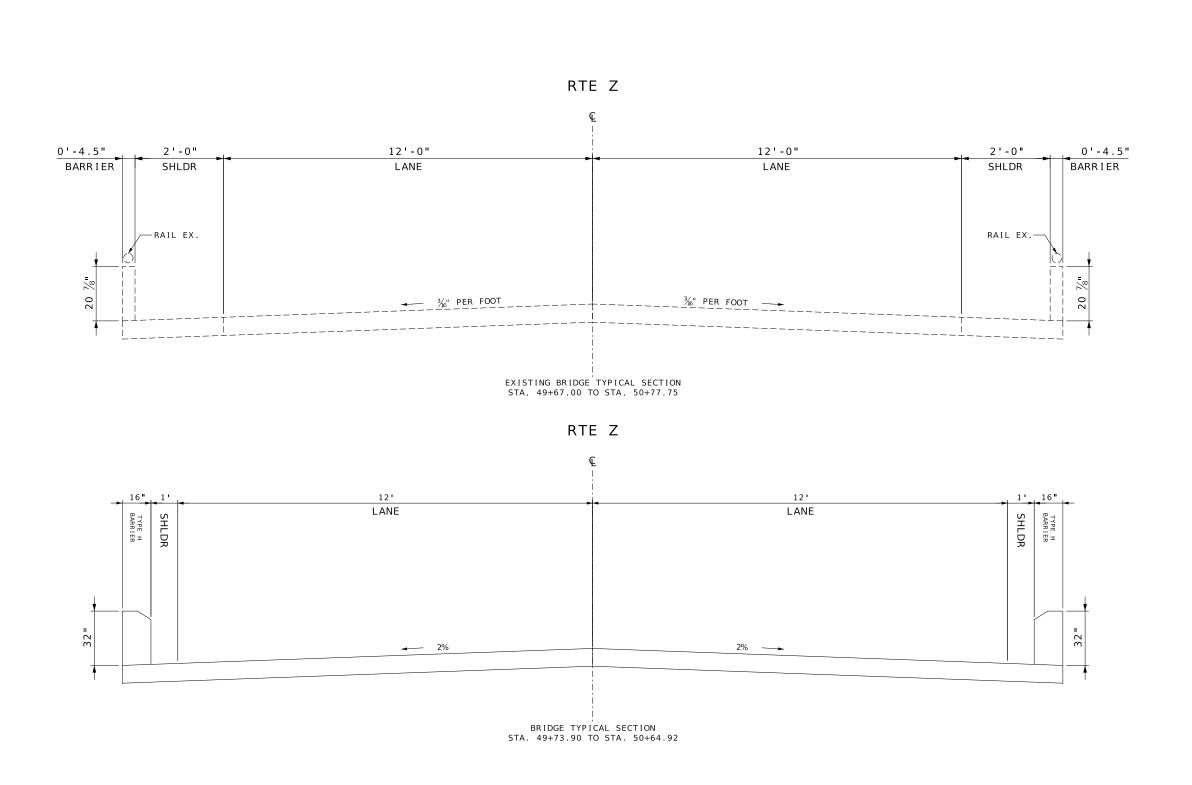
MO SHEET NO

2

Z

KC

PAVEMENT



BENJAMIN STEPHEN MCCABE SR NUMBER PE-2003015005 3/4/2025 ROUTE Z STATE DISTRICT KC SHEET NO. RAY JOB NO.
JKR0103
CONTRACT ID. PROJECT NO. BRIDGE NO.

TYPICAL SECTION SHEET SHEET 2 OF 2

	PAVEMENT AND BASE								
LOCA	TION	LENGTH	OPTIONAL	6 IN TYPE 5	TYPE 5 AGGREGATE	NOTES			
START	END	FT	PAVEMENT	AGG BASE	FOR ENTRANCE				
STA	STA		SY	SY	TON				
47+00.	48+90.84	190.84	509	509	0	PROJECT BEGINNING			
48+90.84	49+11.07	20.23	55	55	0	LEFT LANE WIDENS FROM 12'			
49+11.07	49+37.44	26.37	77	77	0	RIGHT LANE WIDENS FROM 12'			
49+37.44	49+37.5	0.06	0.2	0.2	0	LEFT LANE WIDENING ENDS AT 14.33			
49+37.5	49+46.6	9.10	21	21	0	CENTERLINE ENDS			
49+46.6	49+57.67	11.07	7	7	0	RIGHT LANE WIDENING ENDS AT 14.33'			
50+88.11	50+98.11	10.00	8	8	0	LEFT LANE NARROWS FROM 14.33'			
50+98.11	51+07.22	9.11	23	23	0	CENTERLINE BEGINS			
51+07.22	51+34.71	27.49	78	78	0	RIGHT LANE NARROWS FROM 14.33'			
51+34.71	51+34.77	0.06	0.2	0.2	0	LEFT LANE NARROWING ENDS AT 12'			
51+34.77	51+53.82	19.05	52	52	0				
51+53.82	51+53.88	0.06	0.2	0.2	0	RIGHT LANE NARROWING ENDS AT 12'			
51+53.88	53+00.	146.12	390	390	0	PROJECT END			
49+1	5.00	33.46	0	0	1.5	ORRICK WWTP GRAVEL ENTRANCE, WEST SIDE NEAR BRIDGE			
		TOTAL	1,220.6	1,220.6	1.5				
		PAYTOTAL	1220	1220	1.5				

MOBILIZATION 1 LUMP SUM

	PAVEMENT MARKINGS										
STA	STA	LENGTH	4 " WHITE STANDARD	4" YELLOW STANDARD	NOTES						
START	END	FT	WATERBORNE PAVEMENT	WATERBORNE PAVEMENT							
			MARKING PAINT,	MARKING PAINT							
			TYPE P BEADS	TYPE P BEADS							
			LF	LF							
47+00.00	48+75.00	175.0	350.0	218.8	WHITE EOP, SOLID AND INTERMITTENT YELLOW						
48+75.00	51+75.00	300.0	600.0	600.0	WHITE EOP, DOUBLE CENTER YELLOW						
51+75.00	53+00.00	125.0	250.0	156.3	WHITE EOP, SOLID AND INTERMITTENT YELLOW						
		TOTAL	1,200	975							

REMOVAL OF IMPROVEMENTS							
START	END	LOCATION	UNITS	QUANTITIES	ITEM		
STA	STA						
47+00.00	49+77.56	NORTHS I DE	SY	669	EX. PAVEMENT		
50+64.22	53+00.00	SOUTHS I DE	SY	551	EX. PAVEMENT		
49+46.76	50+95.73	NORTH AND SOUTHSIDE	EA	4	EX. BRIDGE SIGNS FOR DELINEATION		
47+00.00	47+00.00	NORTHS I DE	LF	24	NORTHSIDE SAWCUT		
53+00.00	53+00.00	SOUTHS I DE	LF	24	SOUTHSIDE SAWCUT		
49+52.84	49+81.01	NE OVERBANK	LS	1	CONCRETE DEBRIS REMOVAL (23 SY)		
		_		1 LUMP SUM			

	GUARDRA I L										
START	END	MGS CRASHWORTHY	MGS BRIDGE APPROACH	MGS GUARDRAIL, 8 FT POSTS,	TYPE A GUARDRAIL, 8 FT POSTS	ASYMMETR I CAL	END	RUN			
STA	STA	END TERMINAL (MASH)	SECTION (THRIE-BEAM)	6 FT 3 IN. SPACING	6 FT 3 IN. SPACING	TRANSITION SECTION	ANCHOR	LOCATION			
		EACH	EACH	LF	LF	EACH	EACH	ı			
47+71.90	49+46.90	1	1	75	0	0	0	NE			
49+29.43	49+64.97	0	1	0	50	1	1	NW			
50+77.40	52+52.40	1	1	75	0	0	0	SE			
50+95.24	52+70.24	1	1	75	0	0	0	SW			
	TOTAL	3	4	225	50	1	1	TOTAL			

	TEMPORARY EROSION CONTROL								
LOCATION		ALTERNATE	SILT	SEDIMENT	NOTES				
START	END	DITCH CHECK	FENCE	REMOVAL					
STA	STA	LF	LF	CY					
47+00.00	48+92.00		200	2	ALONG NORTHWEST ROADWAY				
47+00.00	50+00.00		300	3	ALONG NORTHEAST ROADWAY				
49+31.00	50+22.00		98	1	ALONG NORTHWEST ROADWAY BEAR END BENT				
50+34.00	53+00.00		220	2	ALONG SOUTHWEST ROADWAY				
50+47.00	52+67.00		266	3	ALONG SOUTHEAST ROADWAY				
47+0	0.00	25		0.3	NORTHWEST DITCH CHECKS				
48+7	7.00	25		0.3	NORTHWEST DITCH CHECKS				
49+6	2.00	25		0.3	NORTHWEST DITCH CHECKS				
51+1	1.00	10		0.1	SOUTHWEST DITCH CHECK				
47+0	0.00	20		0.2	NORTHEAST DITCH CHECKS				
48+0	0.00	20		0.2	NORTHEAST DITCH CHECKS				
48+9	5.00	35		0.4	NORTHEAST DITCH CHECKS				
50+63.00		35		0.4	SOUTHEAST DITCH CHECK				
TOTAL		195	1084	13.2					
PAY TOTAL		195	1084	14					

	CEEDING										
	SEEDING										
START	END	LOCATION	SIDE	AREA	EROS I ON	SEEDING	REMARKS				
STA	STA			SF	CONTROL	COOL SEASON					
					BLANKET	MIXTURES					
					SY	ACRES					
47+00.00	49+67.92	RTE Z	LT	2596.06	290	0.06	NW SIDE OF BRIDGE				
47+00.00	49+67.92	RTE Z	LT	1588.27	194	0.04	NE SIDE OF BRIDGE				
50+74.23	53+00.00	RTE Z	RT	803.85	97	0.02	SW SIDE OF BRIDGE				
50+74.23	53+00.00	RTE Z	RT	283.91	48	0.01	SE SIDE OF BRIDGE				
				TOTAL	629	0.13					
				PAY TOTAL	629	0.2					

	EARTHWORK								
LOCATION	*TOTAL	*UNSUITABLE	CLASS A	*NECESSARY	COMPACTING	*EXCESS USABLE			
	EXCAVATION	MATERIAL	EXCAVATION	FILL	EMBANKMENT	MATERIAL			
	CY	CY	CY	CY	CY	CY			
RTE Z	1079.9	499	580.6	88.1	105.7	474.9			
PAY TOTAL 581 106									

^{*}INFORMATIONAL PURPOSES ONLY

	ROCK BLANKET								
START	END	LOCATION	AREA	FURNISHING	PLACING	PERMANENT	NOTES		
STA	STA		SF	TYPE 2 ROCK	TYPE 2 ROCK	EROSION CONROL			
				BLANKET	BLANKET	GEOTEXTILE			
				CY	CY	SY			
49+11.50	50+19.36	RTE Z	3176.30	235.28	235.28	352.92	NORTH SIDE OF BRIDGE		
50+26.73	51+20.00	RTE Z	2294.40	169.96	169.96	254.93	SOUTH SIDE OF BRIDGE		
			TOTAL	405.24	405.24	607.85			
		PAY TOTAL	406	406	608	_			

SUMMARY OF QUANTITIES SHEET 1 OF 2

13 / 4 / 2025

ROUTE
Z
DISTRICT
KC SHEET NO.

COUNTY JOB NO.

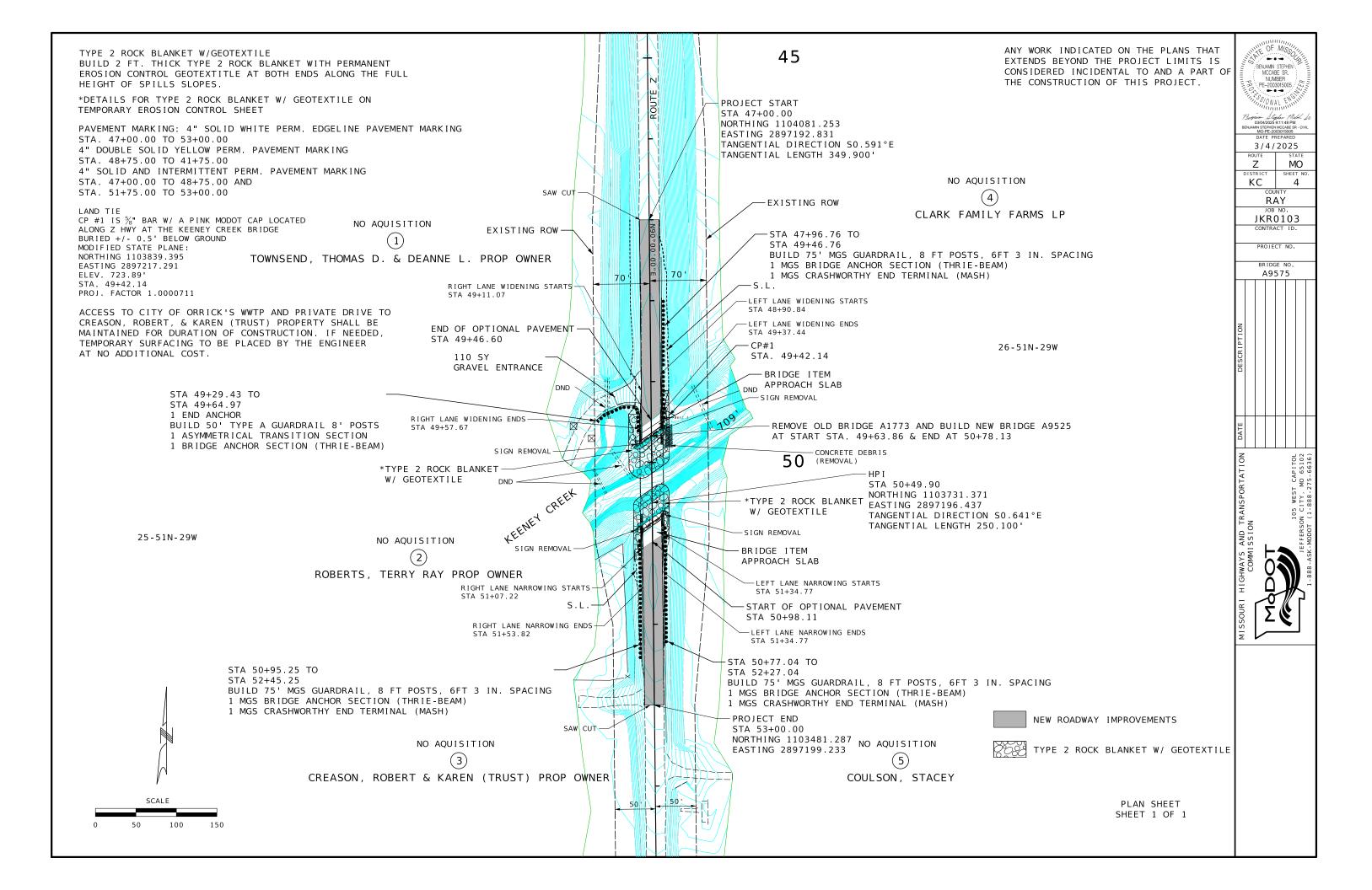
JKR0103

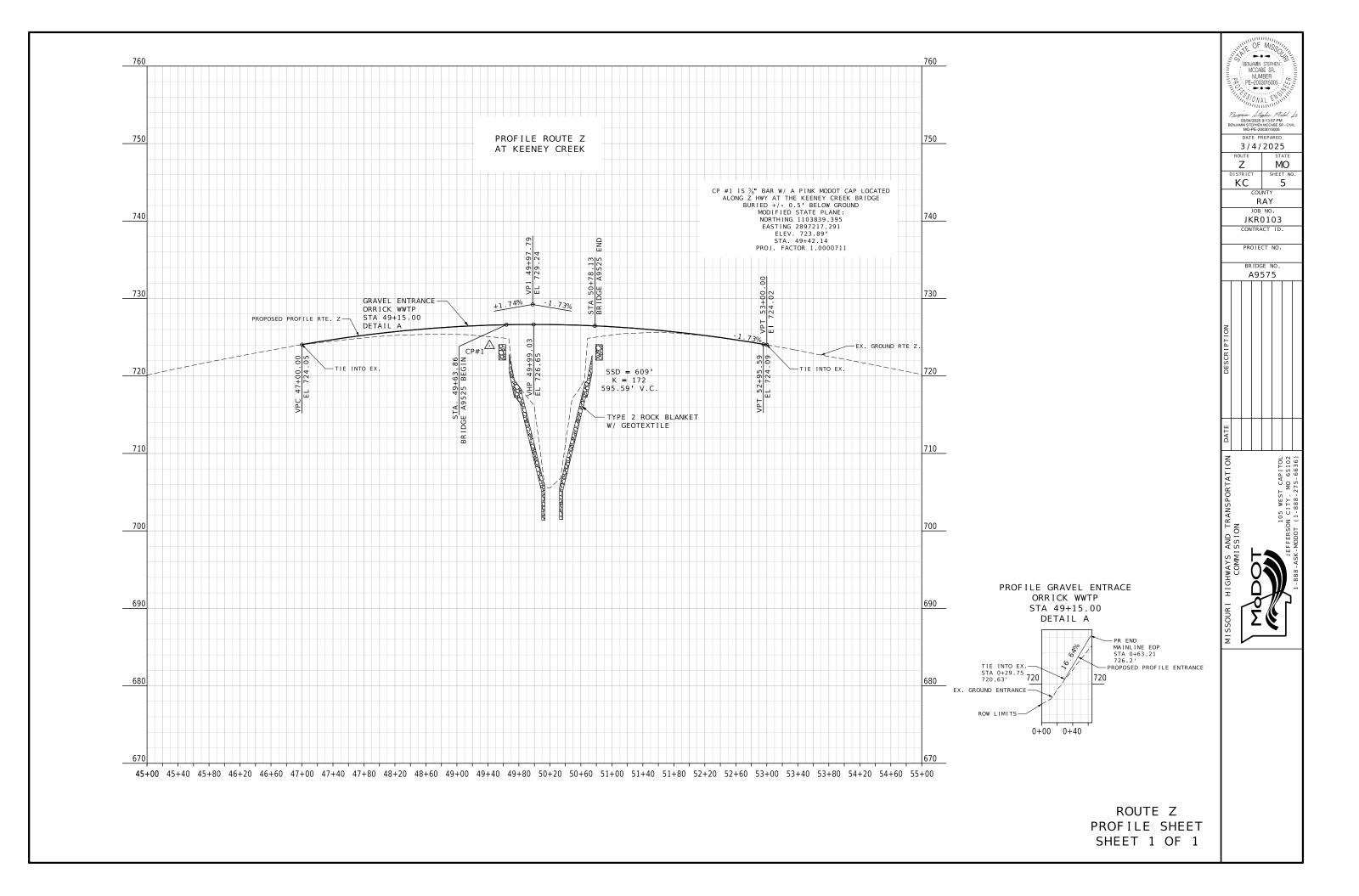
CONTRACT ID.

PROJECT NO.

BRIDGE NO.

		TOTAL QTY T	OTAL SIGN					QTY TOTAL	SIGN		Т		EFFECTIVE: 07-01-2024	.www.
	SIZE ARE	A QTY AREA RELOCK	ELOC NUM.			SIZE	AREA QTY	TOTAL RELOC RELOC	NUM.		ITEM	тота		THE OF MISSO
SIGN	IN. SQ.F	T. EACH SQ.FT. EACH S	Q FT	DESCRIPTION	SIGN	IN.	SQ.FT. EACH	H SQ.FT. EACH SQ.FT.		DESCRIPTION	NUMBER	QTY	DESCRIPTION	BENJAMIN STEPHEN
		WARNING SIC	GNS					GUIDE SIGNS			6122008		IMPACT ATTENUATOR 40 MPH (SAND BARRELS)	MCCABE SR : E
	48X48 16.0			TURN (SYMBOL LEFT)	E05-1		12.00			GORE EXIT	6122009		IMPACT ATTENUATOR 45 MPH (SAND BARRELS)	PE-2003015005
WO1 - 1R	48X48 16.0			TURN (SYMBOL RIGHT)	E05-2		12.00			EXIT OPEN EXIT CLOSED	6122010)	IMPACT ATTENUATOR 50 MPH (SAND BARRELS)	THE SOUNAL ENGILLE
WO1 - 2L WO1 - 2R	48X48 16.0			CURVE (SYMBOL LEFT) CURVE (SYMBOL RIGHT)	E05 - 2a GO20 - 1		12.00			ROAD WORK NEXT XX MILES	6122012		IMPACT ATTENUATOR 55 MPH (SAND BARRELS) IMPACT ATTENUATOR 60 MPH (SAND BARRELS)	"Thimmin"
WO1-2K WO1-3L	48X48 16.0			REVERSE TURN (SYMBOL LEFT)	GO20-2		8.00			END ROAD WORK	6122017	,	IMPACT ATTENUATOR 65 MPH (SAND BARRELS)	Buyan Stepher Mild In 03/26/2025 8:34:52 AM BEN IAMIN STEPHEN MCCABE SP. CIVII
WO1 - 3R	48X48 16.0			REVERSE TURN (SYMBOL RIGHT)	GO20-4		4.50			PILOT CAR FOLLOW ME	6122019)	IMPACT ATTENUATOR 70 MPH (SAND BARRELS)	BENJAMIN STEPHEN MCCABE SR - CIVIL MO-PE-2003015005 DATE PREPARED
WO1-4L	48X48 16.0	00		REVERSE CURVE (SYMBOL LEFT)	GO20-4a	42X30	8.75			PILOT CAR IN USE WAIT & FOLLOW	6122020)	REPLACEMENT SAND BARREL	3/25/2025
WO1-4R	48X48 16.0			REVERSE CURVE (SYMBOL RIGHT)	GO20-4a	_				PILOT CAR IN USE WAIT & FOLLOW	6122030)	IMPACT ATTENUATOR (RELOCATION)	ROUTE STATE
WO1-4bL	48X48 16.0			DOUBLE ARROW REVERSE CURVE (SYMBOL LEFT)	GO20-5aP					WORK ZONE (PLAQUE)	6123001	L	TRUCK MOUNTED ATTENUATOR (TMA)	Z MO
WO1 - 4bR	48X48 16.0			DOUBLE ARROW REVERSE CURVE (SYMBOL RIGHT) TRIPLE ARROW REVERSE CURVE (SYMBOL LEFT)	MO4 - 8 a MO4 - 9 L		3.00			END DETOUR	6161008	3	ADVANCED WARNING RAIL SYSTEM	KC SHEET NO.
WO1-4cL WO1-4cR	48X48 16.0			TRIPLE ARROW REVERSE CURVE (SYMBOL RIGHT)	MO4 - 9L MO4 - 9R		12.00			DETOUR (LEFT) DETOUR (RIGHT)	6161012 6161013	1	BUOYS (BOATS KEEP OUT) BUOYS (NO WAKE)	COUNTY
WO1 - 6	60X30 12.5			HORIZONTAL ARROW (SYMBOL)	MO4 - 9P	_	4.00			STREET NAME (PLAQUE)	6161014	,	SPECIAL SIGN ASSEMBLY (BOATS KEEP OUT)	RAY
WO1-6a	72X36 18.0	00		HORIZ. ARROW (SYMBOL ON PERMANENT BARRICADE)	MO4 - 10L	48X18	6.00			DETOUR ARROW (LEFT)	6161025	i	CHANNELIZER (TRIM LINE)	јов NO. KR0103
WO1-7	60X30 12.5	50		DOUBLE HEAD HORIZONTAL ARROW (SYMBOL)	MO4 - 10R	48X18	6.00			DETOUR ARROW (RIGHT)	6161030)	TYPE III MOVEABLE BARRICADE	CONTRACT ID.
WO1-7a	72X36 18.0			DOUBLE HEAD HORIZ. ARROW (SYMBOL ON PERM. BARR.)	 		T	REGULATORY SIGNS			6161033	3	DIRECTION INDICATOR BARRICADE	
WO1 - 8	18X24 3.0			CHEVRON (SYMBOL)	R1-1	_	13.25			STOP	6161040)	FLASHING ARROW PANEL	PROJECT NO.
WO1-8a WO3-1	30X36 7.5			CHEVRON (SYMBOL FOR DIVIDED HIGHWAYS) STOP AHEAD (SYMBOL)	R1-2 R1-2a		9 00			YIELD TO ONCOMING TRAFFIC (PLAQUE)	6161047 6161055		TYPE III OBJECT MARKER SEQUENTIAL FLASHING WARNING LIGHT	BRIDGE NO.
WO3 - 1	48X48 16.0			YIELD AHEAD (SYMBOL)	R1-2a		2.50			ALL WAY (PLAQUE)	6161033		TUBULAR MARKER	A9575
WO3 - 3	48X48 16.0			SIGNAL AHEAD (SYMBOL)	R2-1		12.00			SPEED LIMIT XX	6161095		RADAR SPEED ADVISORY SYSTEM	
WO3 - 4	48X48 16.0	00		BE PREPARED TO STOP	R3-1	48X48	16.00			NO RIGHT TURN (SYMBOL)	1		CHANGEABLE MESSAGE SIGN,	
WO3 - 5	48X48 16.0			SPEED LIMIT AHEAD	R3-2		16.00			NO LEFT TURN (SYMBOL)	6161096	5	COMMISSION FURNISHED/RETAINED	
WO4 - 1L	48X48 16.0			MERGE (SYMBOL FROM LEFT)	R3-3		9.00			NO TURNS			CHANGEABLE MESSAGE SIGN WITHOUT COMM.	<u> </u>
WO4 121	48X48 16.0			MERGE (SYMBOL FROM RIGHT) MERGE (LEFT)	R3-4 R3-7L		16.00			NO U-TURN (SYMBOL)	6161098/	4	INTERFACE- CONTRACTOR FURNISHED/RETAINED	<u> 6 </u>
WO4-1aL WO4-1aR	48X48 16.0			MERGE (RIGHT)	R3 - 7L		6.25			RIGHT LANE MUST TURN LEFT RIGHT LANE MUST TURN RIGHT	6161099	3	CHANGEABLE MESSAGE SIGN WITH COMM. INTERFACE- CONTRACTOR FURNISHED/RETAINED	H
WO4 - 1 a K	48X48 16.0			ROAD/BRIDGE/RAMP NARROWS	R4-1		12.00			DO NOT PASS	6162000		WORK ZONE TRAFFIC SIGNAL SYSTEM	[종]
WO5 - 3	48X48 16.0			ONE LANE BRIDGE	R4-2		12.00			PASS WITH CARE	6162002		TEMPORARY LONG-TERM RUMBLE STRIPS	DES
WO5 - 5	48X48 16.0	00		NARROW LANES	R4-7a	36X48	12.00			KEEP RIGHT (HORIZONTAL ARROW)			TEMPORARY TRAFFIC BARRIER	
WO6 - 1	48X48 16.0			DIVIDED HIGHWAY (SYMBOL)	R4-8a		12.00			KEEP LEFT (HORIZONTAL ARROW)	6173600	D	CONTRACTOR FURNISHED/RETAINED	
WO6 - 2	48X48 16.0			DIVIDED HIGHWAY END (SYMBOL)	R5-1		6.25			DO NOT ENTER		_	TEMPORARY TRAFFIC BARRIER	
WO6-3 WO7-3a	48X48 16.0			TWO WAY TRAFFIC (SYMBOL)	R5-1a R6-1L		6.00			WRONG WAY	6173602	_	CONTRACTOR FURNISHED/COMMISSION RETAINED TEMP. TRAFFIC BARRIER HEIGHT TRANSITION	
WO7 - 3 a WO8 - 1	48X48 16.0			NEXT XX MILES (PLAQUE) BUMP	R6-1R		6.75			ONE WAY ARROW (LEFT) ONE WAY ARROW (RIGHT)	6175010		RELOCATING TEMPORARY TRAFFIC BARRIER	
WO8 - 2	48X48 16.0			DIP	R6-2L		5.00			ONE WAY (LEFT)	101730107		TEMPORARY TRAFFIC BARRIER	TAG
WO8 - 3	48X48 16.0	00		PAVEMENT ENDS	R6-2R	24X30	5.00			ONE WAY (RIGHT)	61760001	В	COMMISSION FURNISHED/RETAINED	
WO8 - 4	48X48 16.0	00		SOFT SHOULDER	R9-9	24X12	2.00			SIDEWALK CLOSED	1		TEMP. TRAFFIC BARRIER HEIGHT TRANSITION	TOL 102
WO8 - 5	48X48 16.0			SLIPPERY WHEN WET (SYMBOL)						SIDEWALK CLOSED AHEAD,	61770001		COMMISSION FURNISHED/RETAINED	TI (P.17)
WO8 - 6	48X48 16.0			TRUCK CROSSING TRUCK ENTRANCE	R9-11L	24X18	3.00			(ARROW LEFT) CROSS HERE	6208064		TEMPORARY RAISED PAVEMENT MARKER	CA CA MO 75-
WO8 - 6 c WO8 - 7	36X36 9 0			LOOSE GRAVEL	- _{R9-11R}	24X18	3 00			SIDEWALK CLOSED AHEAD, (ARROW RIGHT) CROSS HERE	9029400)	TEMPORARY TRAFFIC SIGNALS TEMPORARY TRAFFIC SIGNALS AND LIGHTING	POF FST Y,
WO8 - 7a	36X36 9.0			FRESH OIL / LOOSE GRAVEL	R10-6		6.00			STOP HERE ON RED (45^ ARROW)	6161030	10	TYPE III MOVEABLE BARRICADE	NS > 1
WO8 - 9	48X48 16.0	00		LOW SHOULDER	R11-2		10.00 3	30.00		ROAD CLOSED	1			AR 100 NO 100 NO 100 LO
WO8 - 11	48X48 16.0	00		UNEVEN LANES						ROAD CLOSED XX MILES AHEAD	1			I ON TOO
	48X48 16.0			NO CENTER LINE			12.50 2			LOCAL TRAFFIC ONLY	1			SS SS MOE
	48X48 16.0			GROOVED PAVEMENT			12.50 3	37.5		ROAD CLOSED TO THRU TRAFFIC	.⊢——			SK - SK-
	30X24 5.0 48X48 16.0			MOTORCYCLE (PLAQUE) SHOULDER DROP-OFF (SYMBOL LEFT)	CONST - 3A					FINE SIGN SPEEDING/PASSING (PLATE)	ı ├			
	48X48 16.0			SHOULDER DROP-OFF (SYMBOL RIGHT)	1 001131 37	JONIZ		MISCELLANEOUS SIGN		STEEDING/TASSING (TEATE)				長 🗶 🐪 🖁
	30X24 5.0			SHOULDER DROP-OFF (PLAQUE)	CONST - 5	48X36				POINT OF PRESENCE	i			
W10-1	42RND. 9.6	2		RAILROAD CROSSING	CONST - 5					POINT OF PRESENCE	i			= \Q\\\\\\
	24X24 4.0			DOUBLE DOWN ARROW (SYMBOL)	CONST-8	48X36	12.00			WORK ZONE NO PHONE ZONE	i			3) 7 g
	48X48 16.0			LOW CLEARANCE (SYMBOL)	1					BR I DGE	i			SS 4 %
	24X18 3.0 84X24 14.0			LOW CLEARANCE (PLAQUE) OVERHEAD LOW CLEARANCE (FEET AND INCHES)	-						i			Σ
	120X60 50.0			LOW CLEARANCE XX FT XX IN XX MILES AHEAD	1		+ + + -				i		-	
	120X60 50.0		1	WIDTH RESTRICTION XX FT XX IN XX MILES AHEAD	1						i			
	30X30 6.2			ADVISORY SPEED (PLAQUE)							i			
	30X24 5.0			XXX FEET (PLAQUE)							i			
	30X24 5.0			X MILE (PLAQUE)	4	1					i			
		00 2 32.00		ROAD/BRIDGE/RAMP WORK AHEAD	-						i			
	48X48 16.0	00 2 32.00		ROAD CLOSED AHEAD	616-10	05		TOTAL						
	48X48 16.0			ONE LANE ROAD AHEAD	_		N SIGNS	157						
	48X48 16.0			RIGHT/CENTER/LEFT LANE CLOSED AHEAD	616-10			TOTAL						
WO20-5a	48X48 16.0	00		2 RIGHT/CENTER/LEFT LANES CLOSED AHEAD	RELOCA	TED S	I GNS	0						
	48X48 16.0			RIGHT/CENTER/LEFT LANE CLOSED										
	48X48 16.0			FLAGGER (SYMBOL)	4									
	36X36 9.0			FRESH OIL	4									
	48X48 16.0			SHOULDER WORK / SHOULDER WORK AHEAD BLASTING ZONE AHEAD	+									
				TURN OFF 2-WAY RADIO AND PHONE	1								CLIMMAD V OF OUR VITATION	
WO22-1	42X36 10.5				-1								SUMMARY OF QUANTITIES	1
WO22-2	42X36 10.5	50		END BLASTING ZONE										
WO22-2 WO22-3				END BLASTING ZONE WET PAINT (ARROW PIVOTS)	_								SHEET 2 OF 2	





RA-17 IS A NAIL & SHINER IN UTILITY POLE WITH CARSONITE WITNESS POST AT 3.9'

MODIFIED STATE PLANE:

NORTHING 1102895.215

EASTING 2904646.952

ELEV. 717.71

PROJ. FACTOR 1.0000711

CL SAND
PIT ROAD

17.5'

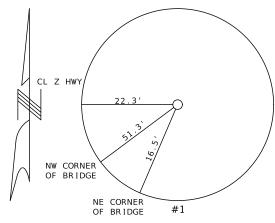
NAIL & SHINER
IN RAILROAD
CROSSING SIGN
POST

RA-17

RAILROAD

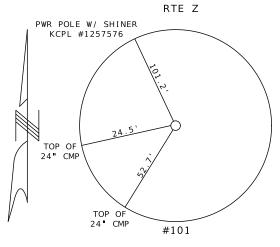
#1 IS A $\frac{5}{8}$ " BAR W/ A PINK MODOT CAP LOCATED ALONG Z HWY AT THE KEENEY CREEK BRIDGE BURIED +/- 0.5' BELOW GROUND

RTE Z



MODIFIED STATE PLANE:
NORTHING 1103839.395
EASTING 2897217.291
ELEV. 723.89'
PROJ. FACTOR 1.0000711

#101 IS A SECTION CORNER THAT LIES
NEAR THE CENTERLINE OF Z HWY WITH
AN ALUMINUM CAP. IT IS FLUSH WITH THE SURFACE
OF THE ROAD. THE MONUMENT IS ABOUT
900' +\- NORTH OF THE KEENEY CREEK BRIDGE



MODIFIED STATE PLANE: NORTHING 1104731.158 EASTING 2897187.172 ELEV. 714.60' PROJ. FACTOR 1.0000711



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3/4/2025

ROUTE STATE

Z MO

DISTRICT SHEET NO.

KC 6

JOB NO.
JKR0103
CONTRACT ID.

PROJECT NO.

DATE DESCRIPTION

SSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION
TO THE TO TH

	INATES HAVE BEEN PROJECTED FROM						COORDINATE	POINT LISTIN	IG
	E PLANE COORDINATE (SPC) SYSTEM AVERAGE PROJECT PROJECTION					MODIFIE	D STATE PLANE	(GROUND)	
	FACTOR. TO GET BACK TO STATE				OFFSET	NORTH I NG	EASTING	ELEVATION	
	, MULTIPY THE PROJECT	SHEET NO	STATION	LOCATION	(USFT)	(US SURVEY FT)	(US SURVEY FT)	(US SURVEY FT	DESCRIPTIO
COORDINATES BY TH	E AVERAGE GRID FACTOR AS SHOWN	PROJECT CO	ONTROL POINT	S					
	CONTROL INFORMATION" PORTION	RA-17		SAND PIT RD	17.5	1,102,895.22	2,904,646.95	717.71	NAIL & SHINER IN UTILITY POLE
OF THIS TABLE.		#1		RTE Z	22.25	1,103,839.40	2,897,217.29	723.89	5/8" BAR W/ PINK MODOT CAP
PROJECT COORDI	NATE INFORMATION	#101		RTE Z	0	1,104,731.16	2,897,187.17	714.60	SECTION CORNER NEAR CENTERLINE
COORDINATE SYSTEM	MODIFIED STATE PLANE(GROUND)								
HORIZONTAL DATUM	NAD83	AL I GNMENTS	5						
VERTICAL DATUM	NAVD88		47+00	RTE Z	0	1,104,081.25	2,897,192.83	724.05	NORTH LIMITS
GEOID MODEL	GEOID18		53+00	RTE Z	0	1,103,481.29	2,897,199.23	724.02	SOUTH LIMITS
ELEVATIONS									
DETERMINED BY	GPS OBSERVATION								
PROJECT PROJECTIO	N FACTOR 1.00007110								
REFERENCE CONT	ROL INFORMATION								
COORDINATE SYSTEM	MO COORDINATE SYSTEM OF 1983								
CONTROL STATION	MO GEOGRAPHIC REFERENCE SYSTEM								
DESIGNATION P	RA 17								
CORS_ID									
P I D	DK 7485								
LATITUDE 3	39°11'40.80955 "								
LONGITUDE)94°05'29.92080"								
NORTHING (M) 3	336139.2320								
EASTING (M) 8	385275.2030								
ZONE	/EST								
PROJECT AVERAGE G	RID FACTOR 0.99992891								
EXAMPLE OF PRO	JECT COORDINATE TO S.P.C.								
= STATE PLANE NO	AVERAGE GRID FACTOR								

EXAMPLE: CONTROL POINT #1
N 1,103,839.40 X 0.99992891 = N 1,103,760.93
E 2,897,217.29 X 0.99992891 = E 2,897,011.33

1 METER = 3.280833333 US SURVEY FEET (USFT)

LINEAR UNIT CONVERSION

BENJAMN STEPHEN

CCARE SR

MCCARE SR

MCARE SR

MCCARE SR

MCCARE

GPK POINT ID

MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION

MADOT

105 WEST CAPITOL
LEFFERSON CITY, MO 65102

Traffic Control Legend, Sign Spacing, Device Spacing, Channelizing Taper Lengths And Recommended Maximum Speed Reductions

SIGN SP	ACING FOR A	DVANCE SIGN SERIES (1) (2)				
PERMANENT						
POSTED SPEED	UNDIVIDED	DIVIDED				
MPH	HIGHWAYS (S)	HIGHWAYS (S)				
0-35	200′	200′				
40-45	350′	500′				
50-55	500′	1000′				
60-70	1000′	SA - 1000' SB - 1500' SC - 2640'				

TAPE	R LENGTHS AI	ND END TREAT	TMENTS FOR	CONCRETE BARRIER
PERMANENT				
POSTED SPEED	MINIMUM LANE			
MPH	10′	11′	12′	END TREATMENT (4)
<40	160′	168′	176′	BARRIER HEIGHT TRANSITION
>40	160′	168′	176′	APPROVED CRASH CUSHION

	TAPER LENGTHS AND SPACING FOR CHANNELIZERS									
PERMANENT				MINIMUM SHOULDER	BUFFER	MAXIMUM CHANNELI	ZER SPACING			
POSTED SPEED	MINIMUM LANE TAPER LENGTH (L) (3)			TAPER LENGTH (T1)	LENGTH	THROUGH	THROUGH			
MPH	10′	11′	12′	BASED ON 10' SHOULDER	FT	TAPER	WORK AREA			
0-35	205′	225′	245′	70′	280'	35′	40′			
40-45	450′	495′	540′	150′	400′	40′	80′			
50-55	550′	605′	660′	185′	560′	50′	80′			
60-70	700′	770′	840′	235′	840'	60′	120′			

THE SA DIMENSION IS THE DISTANCE FROM THE TRANSITION OR POINT OF RESTRICTION TO THE FIRST SIGN.

THE SB DIMENSION IS THE DISTANCE BETWEEN THE FIRST AND SECOND SIGNS.

THE SC DIMENSION IS THE DISTANCE BETWEEN THE SECOND AND THIRD SIGNS.

(THE "FIRST SIGN" IS THE SIGN IN A THREE-SIGN SERIES THAT IS CLOSEST TO THE TEMPORARY TRAFFIC CONTROL ZONE. THE "THIRD SIGN" IS THE SIGN THAT IS FURTHEST UPSTREAM FROM THE TEMPORARY TRAFFIC CONTROL ZONE)

DETAILS NOTES:

- (1) SPACING BETWEEN SIGNS AND SPACING BETWEEN LAST SIGN AND FLAGGER, BEGINNING OF TAPER, OF SIGNED CONDITION.
- (2) SPACING MAY BE ADJUSTED AS NECESSARY TO MEET FIELD CONDITIONS.
- (3) TAPER LENGTHS SHOWN INCLUDE LENGTH REQUIRED FOR LANE AND 10' SHOULDER
- (4) CONCRETE BARRIER MAY BE INSTALLED AT 8:1 FLARE RATE FROM THE SHOULDER POINT OF THE LIMITS OF THE CLEAR ZONE WHERE THE SIDE SLOPE IS 6:1 OR FLATTER

TRAFFIC CONTROL LEGEND

FLAGGER

TAPER LENGTH (L)

L = W X P FOR 40 MPH OR MORE

 $L = \frac{WP^2}{60}$ FOR 35 MPH OR LESS

L = TAPER LENGTH IN FEET

W = LATERAL SHIFT IN FEET

P = POSTED SPEED PRIOR TO ROAD WORK IN MPH

SIGN (SINGLE SIDED)

FLASHING ARROW PANEL

CHANGEABLE MESSAGE BOARD

CHANNELIZER - BARRICADE

PROTECTIVE VEHICLE WITH
WORK SIGN, FLASHING ARROW
PANEL AND REAR-MOUNTED

LEGEND NOTE:

THE PROTECTIVE VEHICLE SIGN SHALL BE MOUNTED AT A RECOMMENDED HEIGHT OF 48 IN. ABOVE THE ROAD SURFACE.

GENERAL NOTES:

- SEE STANDARD PLAN 616.10 FOR DETAILS AND ITEMS NOT SHOWN.
- 2. EXISTING SIGNS SHALL BE COVERED DURING WORKING HOURS ONLY IF IN CONFLICT WITH TRAFFIC CONTROL PLANS.
- 3. NO DIRECT PAYMENT WILL BE MADE FOR RELOCATING, COVERING, UNCOVERING OR REMOVING SIGNS.
- 4. CONES ALLOWABLE FOR DAYTIME OPERATIONS ON MINOR ROUTES ONLY.
- 5. LOCATE FLASHING ARROW PANEL AT BEGINNING OF TAPER WHEN FEASIBLE, ARROW PANELS ARE ALWAYS LOCATED BEHIND CHANNELIZERS OR CONES.

EPG TABLE 616.12 RECOMMENDE	D MAXIMUM SPEED REDUCTIONS
ACTIVITY (I.E. WORKERS, EQUIPMENT OR MATERIAL) LOCATION	RECOMMENDED WORK ZONE SPEED REDUCTION (WHEN APPLICABLE)
10 FT. BEYOND EDGE OF TRAVELWAY TO EDGE OF RIGHT OF WAY	NO SPEED REDUCTION
IN TRAFFIC LANE OR WITHIN 10FT. OF THE TRAFFIC LANE	10 MPH
HEAD-TO-HEAD ON MULTILANE	10 MPH

SPECIAL CIRCUMSTANCES WITHIN A TEMPORARY TRAFFIC CONTROL WORK ZONE MAY WARRANT A LOWER SPEED LIMIT THAN RECOMMENDED ABOVE. ALL SPEED LIMIT REDUCTIONS GREATER THAN 10 MPH SHALL BE DOCUMENTED, SUBMITTED TO AND APPROVED BY THE DISTRICT WORK ZONE COORDINATOR.

TEMPORARY
TRAFFIC CONTROL
SHEET 1 OF 2

BENJAMN STEPHEN

BENJAMN STEPHEN

MCCABE SR

NUMBER

BENJAMN STEPHEN

BENJAMN STEPHEN

BENJAMN STEPHEN MCCABE SR

GOOGLEOGE SR

DATE PREPARED

3 / 4 / 2025

ROUTE

Z

MO

DISTRICT

SHEET NO.

KC

8

COUNTY

RAY

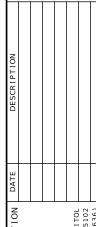
JOB NO.

J KR0103

CONTRACT ID.

BRIDGE NO.

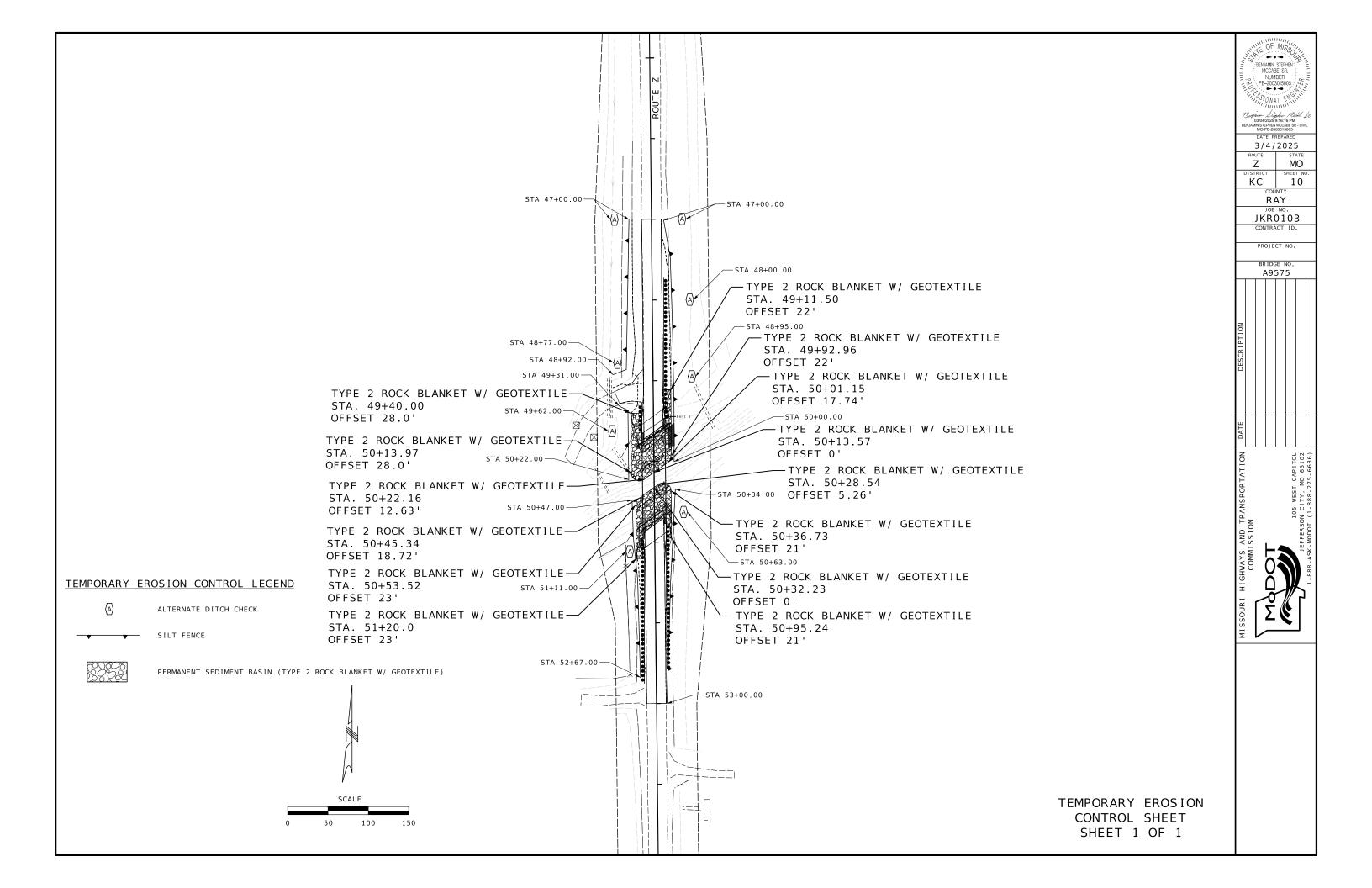
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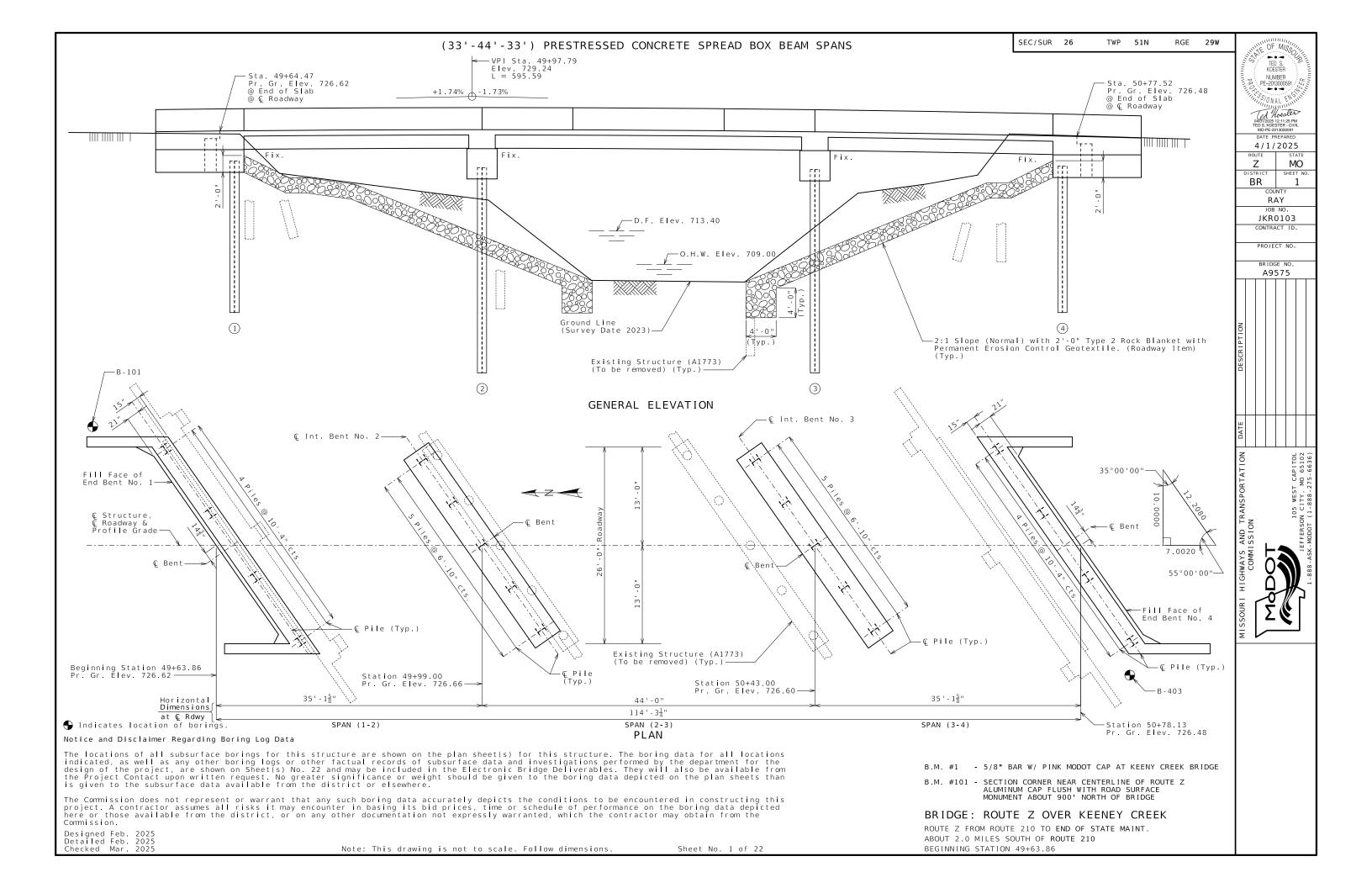






MO SHEET NO 9





	Foundation Data								
			Bent Number						
Туре	Design Data		1	2	3	4			
	Pile Type and Size		HP 14x73	HP 14×73	HP 14×73	HP 14x73			
	Numb e r	e a	4	5	5	4			
	Approximate Length Per Each	ft	103	102	101	99			
	Pile Point Reinforcement	еa	ALL	ALL	ALL	ALL			
	Min. Galvanized Penetration (Elev.)	ft	700.80	691.39	690.00	700.66			
Load	Est. Max. Scour Depth 100 (Elev.)	ft	-	701.4	700.0	-			
Bearing	Minimum Tip Penetration (Elev.)	ft	670	668	675	670			
''''	Criteria for Min. Tip Penetration		Lateral Stability	Lateral Stability	Lateral Stability	Lateral Stability			
1	Pile Driving Verification Method		DF	DF	DF	DF			
	Resistance Factor		0.40	0.40	0.40	0.40			
	Minimum Nominal Axial Compressive Resistance	kip	350	415	415	350			

DF = FHWA-Modified Gates Dynamic Formula

Minimum Nominal Axial Compressive Resistance = <u>Maximum Factored Loads</u> Resistance Factor

All piles shall be galvanized down to the minimum galvanized penetration

Pile point reinforcement need not be galvanized. Shop drawings will not be required for pile point reinforcement.

The contractor shall make every effort to achieve the minimum galvanized penetration (elevation) shown on the plans for all piles. Deviation in penetration less than 5 feet of the minimum will be considered acceptable provided the contractor makes the necessary corrections to ensure the minimum penetration is achieved on subsequent piles.

General Notes:

Design Specifications:

2020 AASHTO LRFD Bridge Design Specifications (9th Ed.)

2023 AASHTO Guide Specifications for LRFD Seismic Bridge Design (3rd ED.)

Seismic Design Category = A (Nonseismic Details)

Design earthquake response spectral acceleration coefficient at 1.0 second period, SD1 < 0.15 = 0.129gAcceleration Coefficient (effective peak ground acceleration coefficient), As

Design Loading: Vehicular = HL-93 Future Wearing Surface = 35 lb/sf Earth = 120 lb/cfEquivalent Fluid Pressure = 45 lb/cf (Min.) Superstructure:

Simply-Supported, Non-Composite for dead load. Continuous Composite for live load.	
Design Unit Stresses: Class B Concrete (Substructure)	f'c = 3,000 psi

Class B-1 Concrete (Barrier) f'c = 4,000 ps iClass B-2 Concrete (Superstructure, except Prestressed Beams f'c = 4,000 ps iand Barrier)

Reinforcing Steel (ASTM A615 Grade 60) fy = 60,000 ps iStructural Steel HP Pile (ASTM A709 Grade 50) fy = 50,000 psi

For prestressed box beam stresses, see Sheets No. 9 & 10.

Neoprene Pads:

Neoprene bearing pads shall be 60 durometer and shall be in accordance with Sec 716.

All joint filler shall be in accordance with Sec 1057 for preformed sponge rubber expansion and partition joint filler, except as noted.

Reinforcing Steel: Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise shown.

Minimum clearance between galvanized piles and uncoated (plain) reinforcing steel including bar supports shall be 1 1/2". Nylon, PVC, or polyethylene spacers shall be used to maintain clearance. Nylon cable ties shall be used to bind the spacers to the reinforcement.

Traffic Handling: Structure to be closed during construction. Traffic to be maintained on other routes during construction. See roadway plans for traffic control.

Outline of existing work is indicated by light dashed lines. Heavy lines

I t em		Substr.	Superstr	Tota
Removal of Miscellaneous ACM (Non-Friable)	sq. foot			9
Class 1 Excavation	cu. yard	70		70
Removal of Bridges (A1773)	lump sum			1
Bridge Approach Slab (Minor)	sq. yard		122	122
Galvanized Structural Steel Piles (14 in.)	linear foot	1823		1823
Pile Point Reinforcement	each	18		18
Class B Concrete (Substructure)	cu. yard	65.6		65.6
Type H Barrier	linear foot		261	261
Slab on Concrete Beam	sq. yard		360	360
21 in., Prestressed Concrete Spread Box Beam	linear foot		328	328
Reinforcing Steel (Bridges)	pound	5,200		5,20
Slab Drain	each		16	16
Vertical Drain at End Bents	each			2
Plain Neoprene Bearing Pad	each		6	6
Laminated Neoprene Bearing Pad	each		12	12

Estimated Quantities

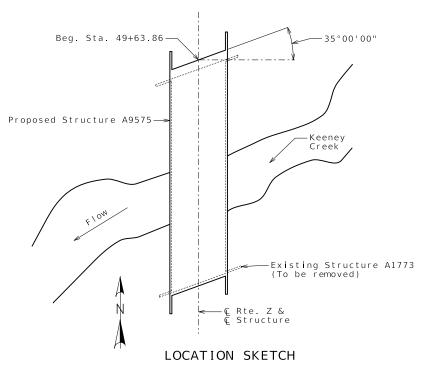
All reinforcement in the end bents is included in the Estimated Quantities for Slab on Concrete Beam.

All reinforcement in the intermediate bent concrete diaphragms except reinforcement embedded in the beam cap is included in the Estimated Quantities for Slab on Concrete Beam.

All concrete above the intermediate beam cap is included in the Estimated Quantities for Slab on Concrete Beam.

Drainage Area = 7.3 mi³	
Design Flood Frequency = 50 ye	ears
Design Flood Discharge = 2300	c f s
Design Flood (D.F.) Elevation	= 713.4 (ft)
Base Flood (100-yea	r)
Base Flood Elevation = 719.6	(ft)
Base Flood Discharge = 2800 c	fs
Estimated Backwater = 0.2 ft	
Average Velocity thru Opening	= 8.7 ft/s
Freeboard (50-year)
Freeboard = 4.6 ft	
Roadway Overtoppin	g
Overtopping Flood Discharge =	3600 cfs
Overtopping Flood Frequency =	392.6 years
500-year Flood Elevation = 714	1.6

Hydrologic Data



Estimated Quantities for Slab on Concrete Beam						
I t em						
Class B-2 Concrete cu. yard	113					
Reinforcing Steel (Epoxy Coated) pound						
The table of Catinated Quantities for Clab as Consult Base						

The table of Estimated Quantities for Slab on Concrete Beam represents the quantities used by the State in preparing the cost estimate for concrete slabs. The area of the concrete slab will estimate for concrete slabs. The area of the concrete slab will be measured to the nearest square yard longitudinally from end of slab to end of slab and transversely from out to out of bridge slab (or with the horizontal dimensions as shown on the plan of slab). Payment for prestressed panels, conventional forms, all concrete and epoxy coated reinforcing steel will be considered completely covered by the contract unit price for the slab. Variations may be encountered in the estimated quantities but the variations cannot be used for an adjustment in the contract unit

Method of forming the slab shall be as shown on the plans and in accordance with Sec 703. All hardware for forming the slab to be left in place as a permanent part of the structure shall be coated in accordance with ASTM A123 or ASTM B633 with a thickness class SC 4 and a finish type I, II or III.

Slab shall be cast-in-place with conventional forms or stay-in-place corrugated steel forms. Precast prestressed panels will not be permitted.

TED S. KOESTER PE-2010 NUMBER 04/02/2025 7:03:57 AM TED S. KOESTER - CIVIL MO-PE-2013000591

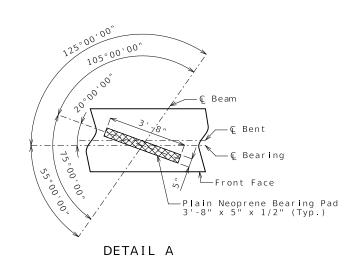
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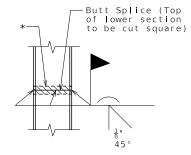
SHEET NO

BR 2 RAY LOB NO JKR0103 CONTRACT ID.

PROJECT NO. BRIDGE NO

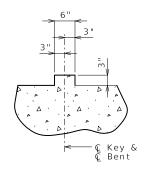
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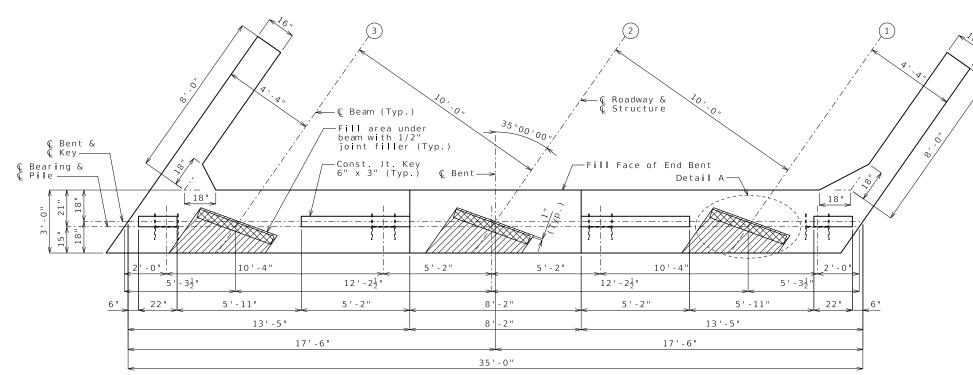


STEEL PILE SPLICE (If required)

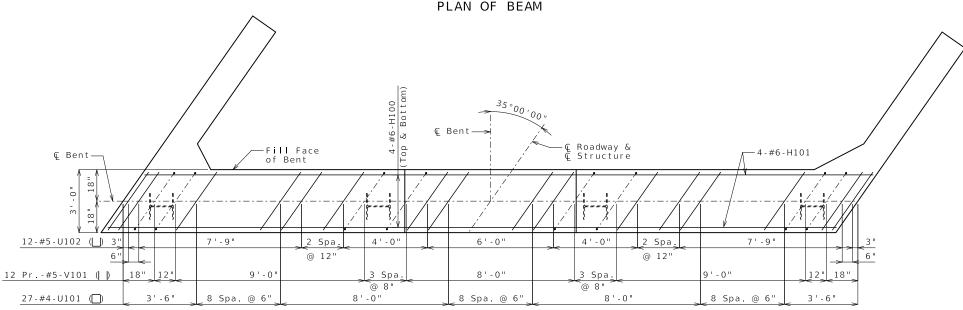
* Galvanizing material shall be omitted or removed one inch clear of weld locations in accordance with Sec 702.



SECTION THRU KEY



PLAN OF BEAM



PLAN OF BEAM SHOWING REINFORCEMENT

* 2 Spa. @ 6"

Notes:

Work this sheet with Sheets No. 4 & 5.

Reinforcing steel shall be shifted to clear piles. U bars shall clear piles by at least 1 1/2 inches.

The U bars and pairs of V bars shall be placed parallel to centerline of roadway.

END BENTS NO. 1 & 4

Substructure Quantity Table for	Bent No.	1 & 4	
I t em		Bent 1 Quantity	Bent 4 Quantity
Class 1 Excavation	cu. yard	35	35
Galvanized Structural Steel Piles (14 in.)	linear foot	412	396
Pile Point Reinforcement	each	4	4
Class B Concrete (Substructure)	cu. yard	14.6	14.6

TED S. KOESTER NUMBER PE-2013000591

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JKR0103 CONTRACT ID. PROJECT NO.

BRIDGE NO

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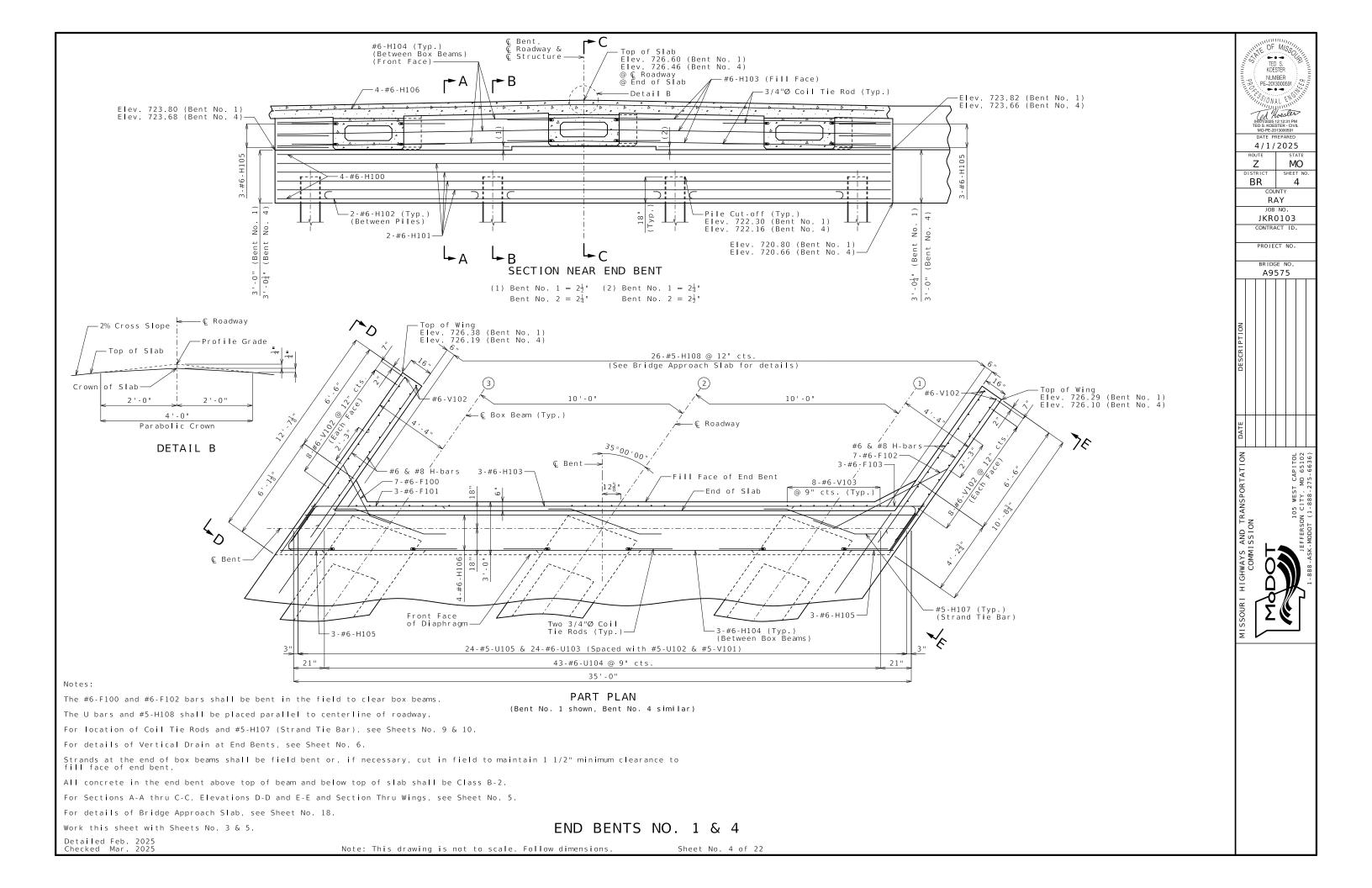
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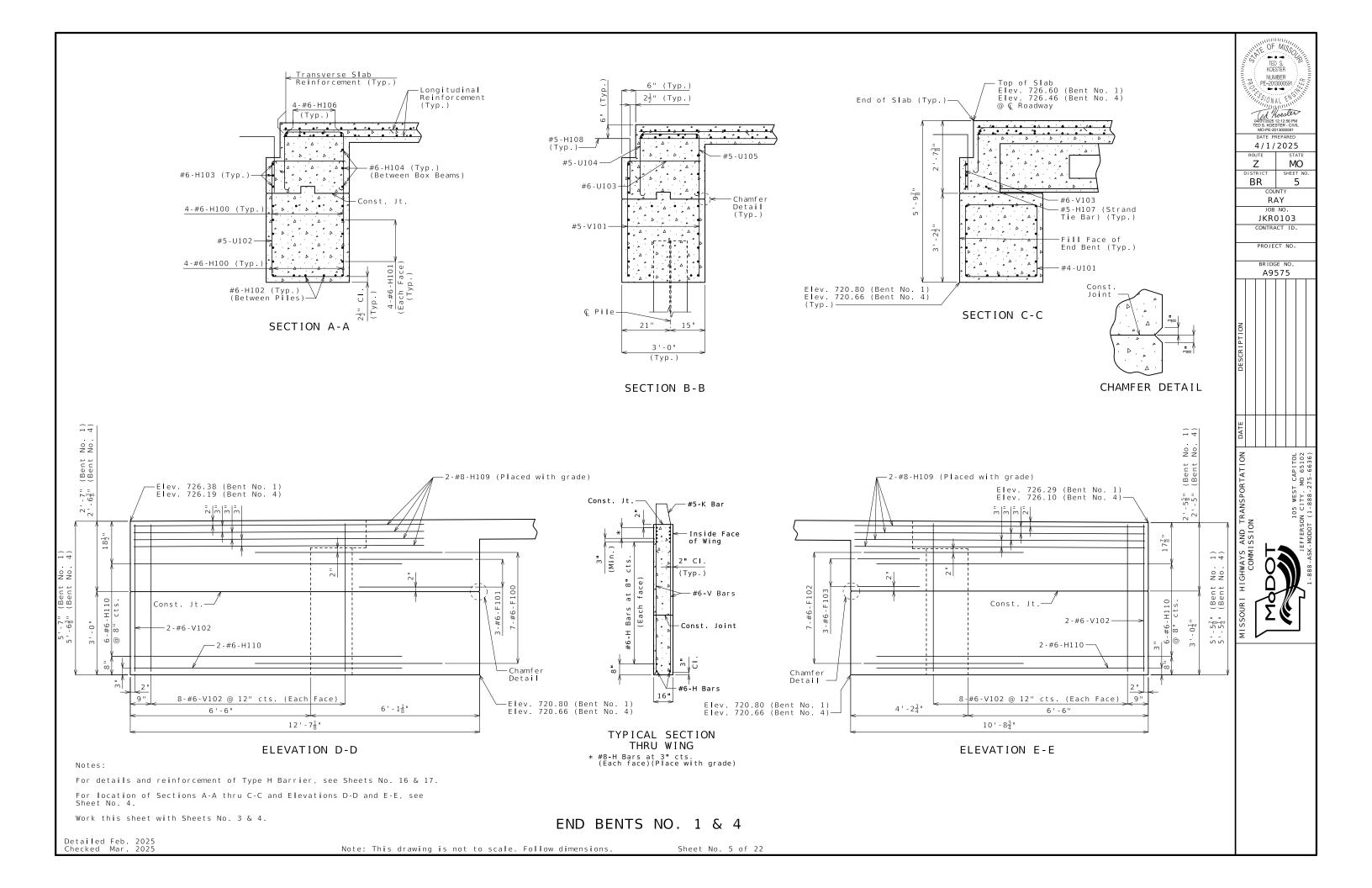
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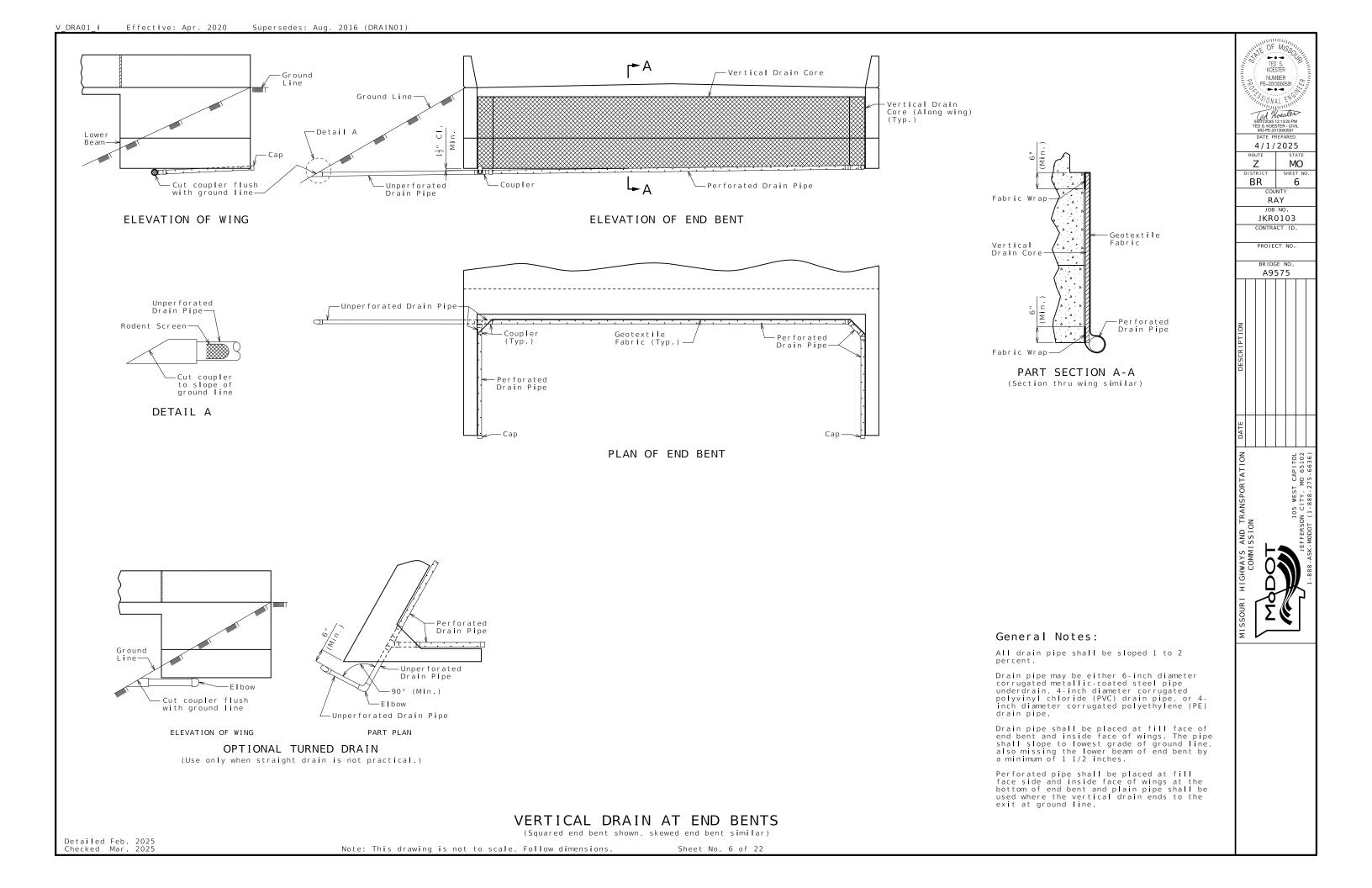
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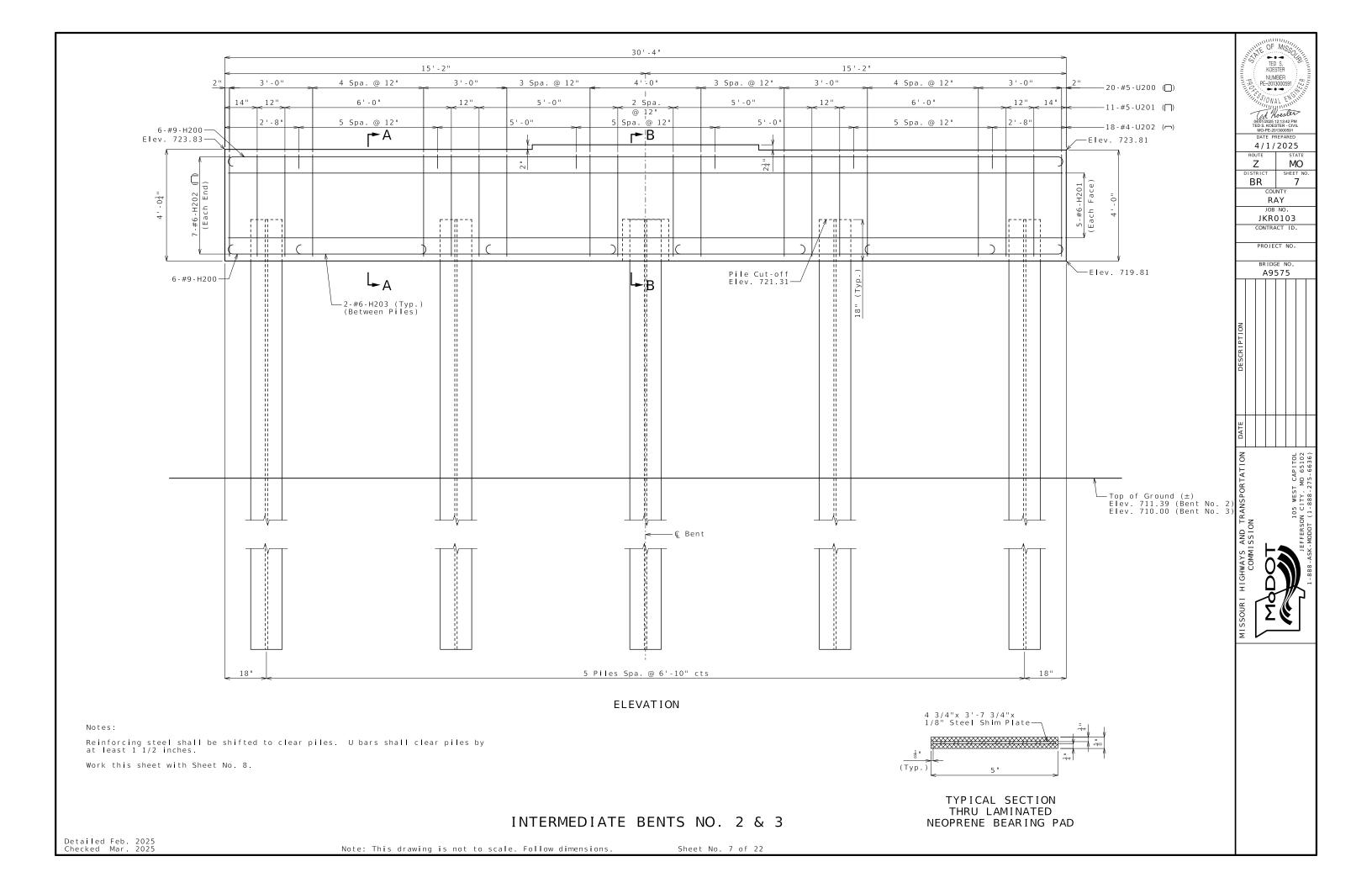
These quantities are included in the Estimated Quantities table on Sheet No. 2.

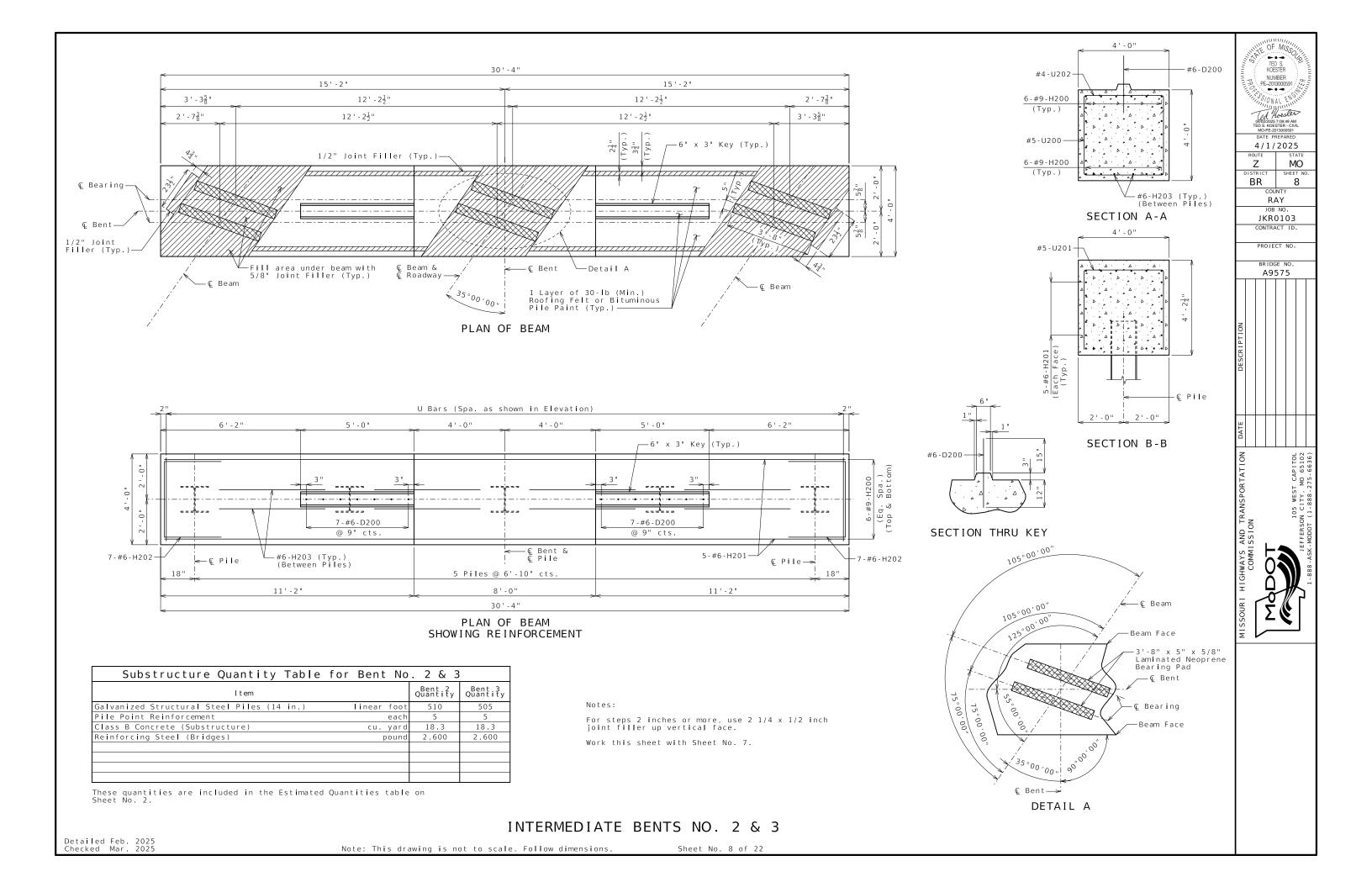
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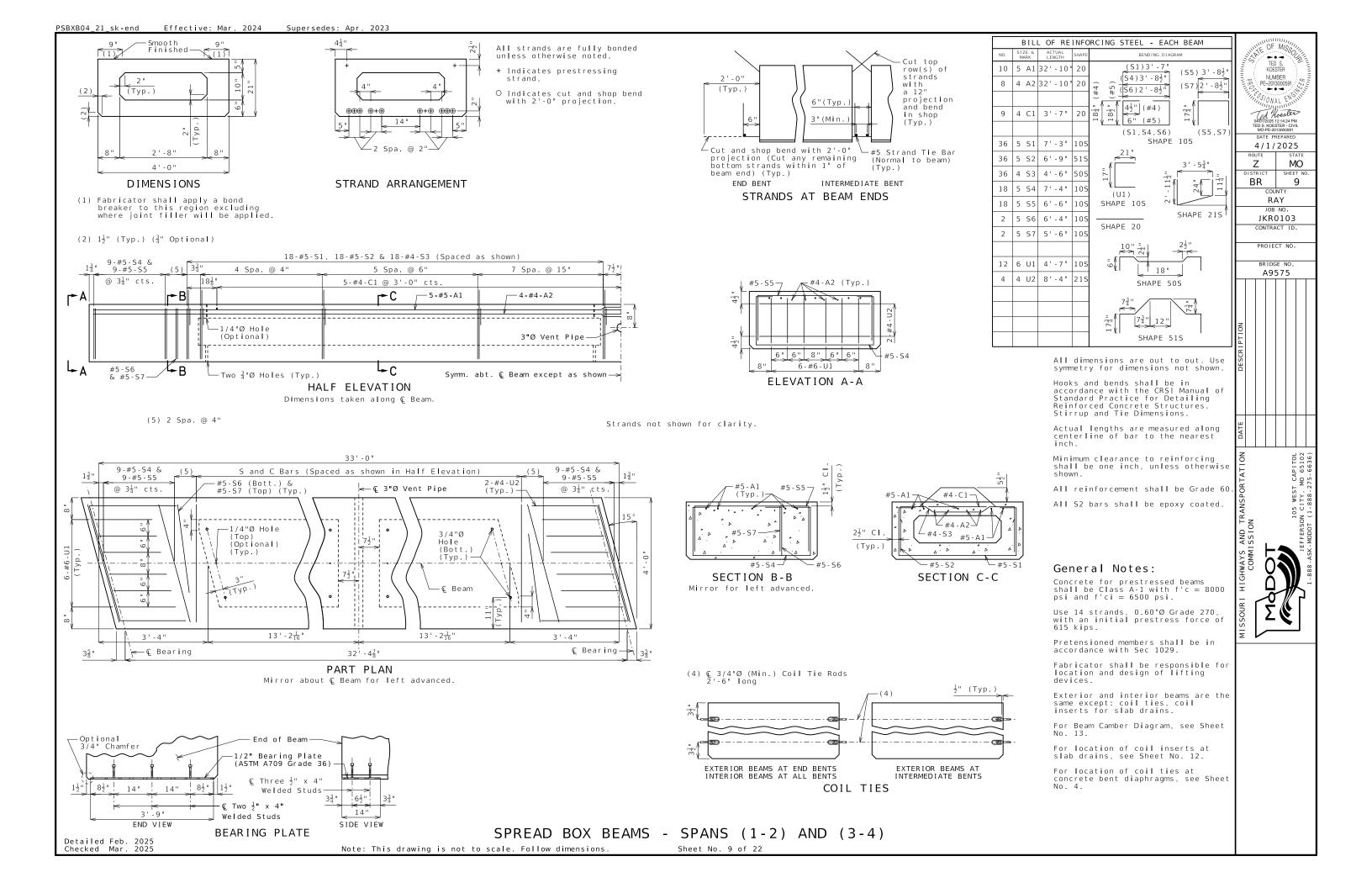


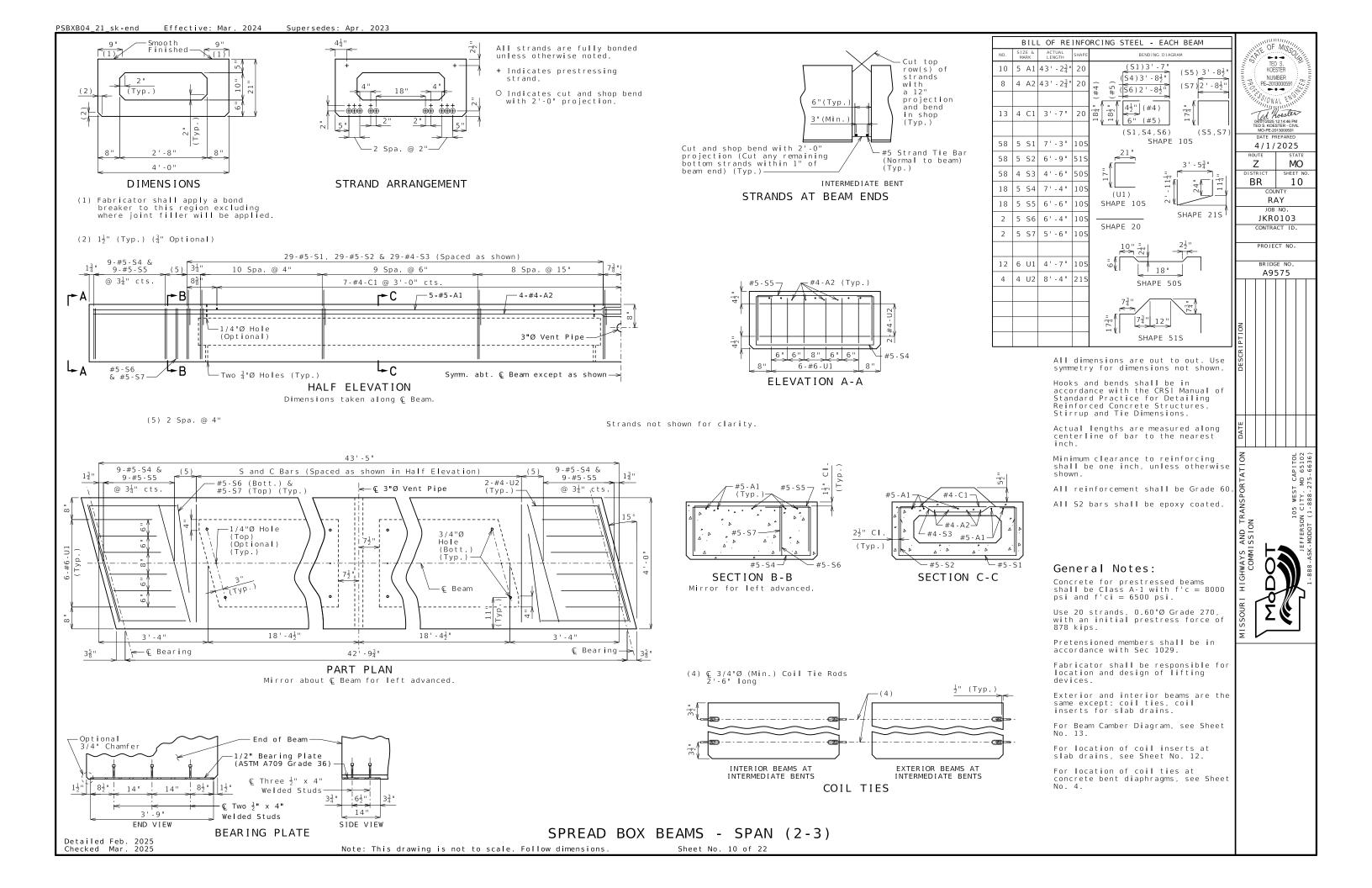


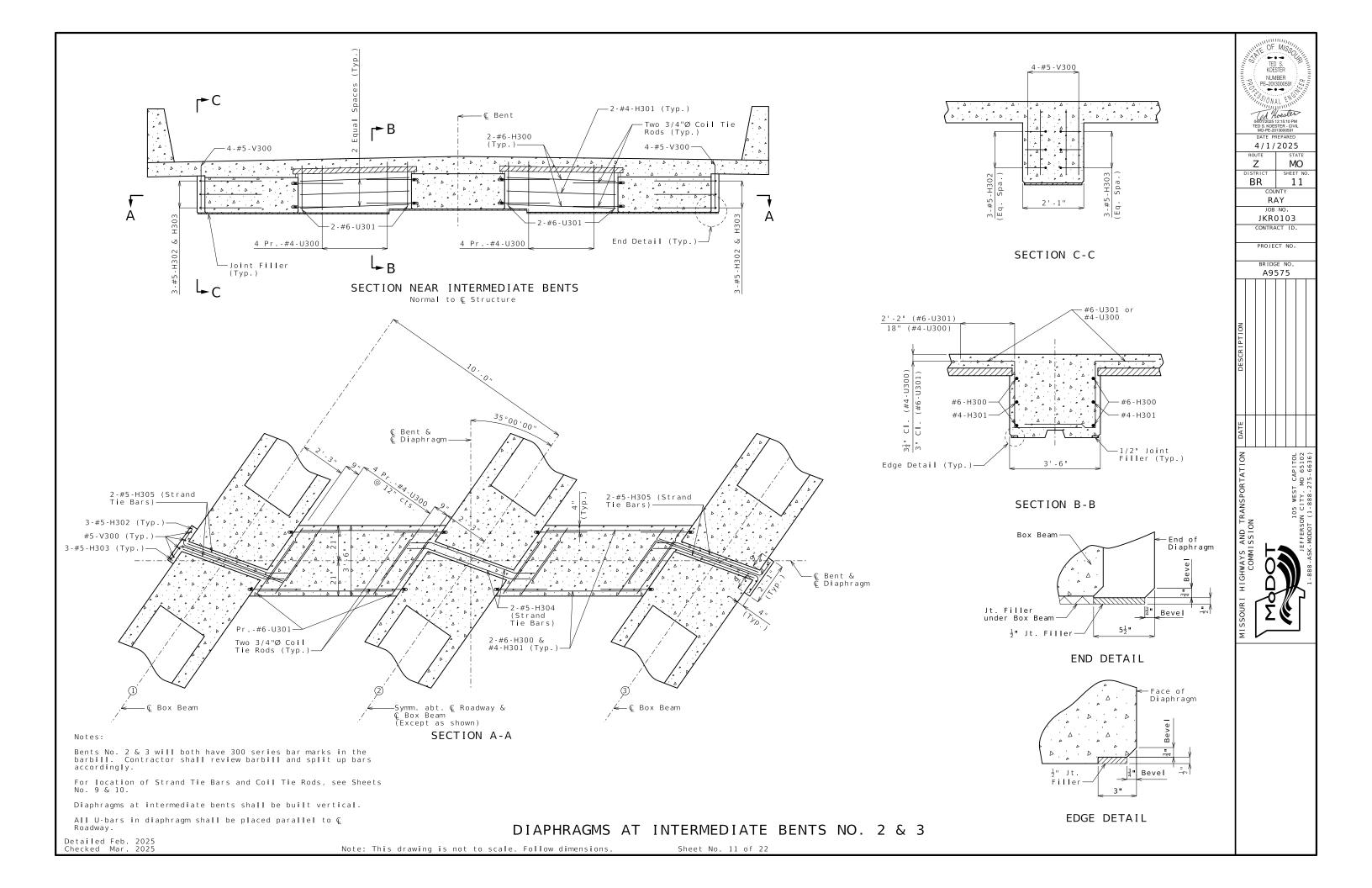


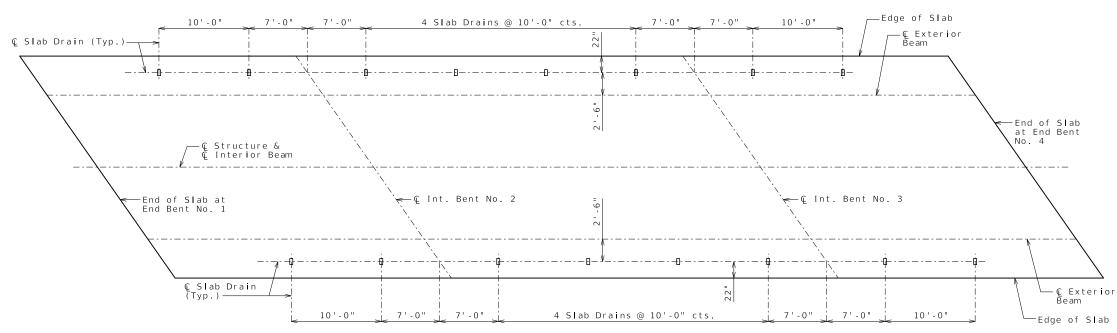




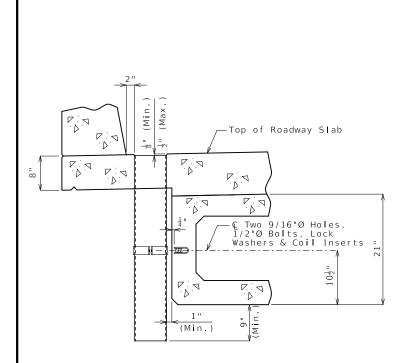




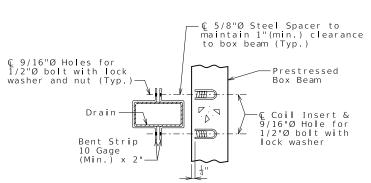




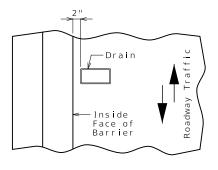
PLAN OF SLAB SHOWING SLAB DRAIN LOCATIONS



PART SECTION NEAR DRAIN

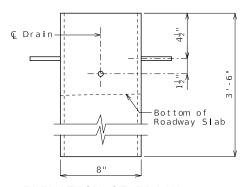


PART SECTION SHOWING BRACKET ASSEMBLY

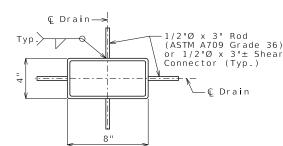


PART PLAN OF SLAB AT DRAIN

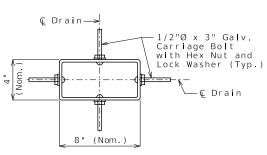
SLAB DRAINS



ELEVATION OF DRAIN



PLAN OF STEEL DRAIN OPTION



PLAN OF FRP DRAIN OPTION

General Notes:

Contractor shall have the option to construct either steel or FRP slab drains. All drains shall be of same type.

Slab drain bracket assembly shall be ASTM A709 Grade 36 steel.

Locate drains in slab by dimensions shown in Part Section Near Drain.

Reinforcing steel shall be shifted to clear drains.

The coil inserts and bracket assembly shall be galvanized in accordance with ASTM A123.

All bolts, hardened washers, lock washers and nuts shall be galvanized in accordance with AASHTO M 232 (ASTM A153), Class C.

AII 1/2"Ø bolts shall be ASTM A307.

Shop drawings will not be required for the slab drains and the bracket assembly.

The coil inserts required for the bracket assembly attachment shall be located on the prestressed beam shop drawings.

Coil inserts shall have a concrete pullout strength (ultimate load) of at least 2,500 pounds in 5,000 psi concrete.

The bolts required to attach the slab drain bracket assembly to the prestressed beam shall be supplied by the prestressed beam fabricator.

Notes for Steel Drain:

Slab drains may be fabricated of either 1/4" welded sheets of ASTM A709 Grade 36 steel or from 1/4" structural steel tubing ASTM A500 or A501.

Outside dimensions of drains are 8" x 4".

The drains shall be galvanized in accordance with ASTM A123.

Notes for FRP Drain:

Drains shall be machine filament-wound thermosetting resin tubing meeting the requirements of ASTM D2996 with the following exceptions:

Shape of drains shall be rectangular with outside nominal dimensions of $8\,^{\prime\prime}$ x $4\,^{\prime\prime}$.

Minimum reinforced wall thickness shall be 1/4 inch.

The resin used shall be ultraviolet (UV) resistant and/or have UV inhibitors mixed throughout. Drains may have an exterior coating for additional UV resistance.

The color of the slab drain shall be gray (Federal Standard #26373). The color shall be uniform throughout the resin and any coating used.

The combination of materials used in the manufacture of the drains shall be tested for UV resistance in accordance with ASTM D4329 Cycle A. The representative material shall withstand at least 500 hours of testing with only minor discoloration and without any physical deterioration. The contractor shall furnish the results of the required ultraviolet testing prior to acceptance of the slab drains.

At the contractor's option, drains may be field cut. The method of cutting FRP slab drain shall be as recommended by the manufacturer to ensure a smooth, chip free cut. TEO S.
KOESTER
NUMBER
PE-201000059

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COUNTY
RAY

CONTRACT ID.

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BRIDGE NO.
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11 SS ION

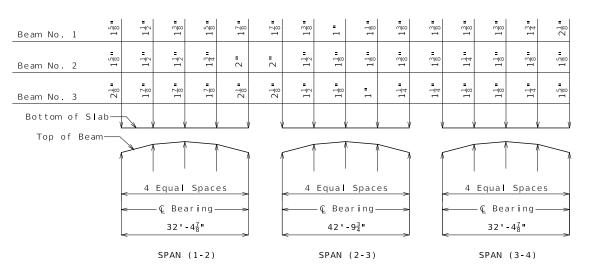
11 SS ION

11 SS ION

10 WEST CAPITOL

10 WEST CAPITOL

10 WEST CAPITOL



THEORETICAL SLAB HAUNCHING DIAGRAM (ESTIMATED AT 90 DAYS)

If beam camber is different from that shown in the camber diagram, in order to maintain minimum slab thickness, an adjustment of the slab haunches, an increase in slab thickness or a raise in grade uniformly throughout the structure shall be necessary. No payment will be made for additional labor or materials required for variation in haunching, slab thickness or grade adjustment.

Concrete in the slab haunches is included in the Estimated Quantities for Slab on Concrete Ream

		Ę I	Beam —→	, / '	(Estimat — Theore	ed at 9 tical C ated at — Theore	0 days) amber a 7 days etical I		and re	fter slab	
	Beam	Span (1-2)			Sı	oan (2-3)	Span (3-4)			
ı beam		_	_		_	_		_			

BEAM CAMBER DIAGRAM

 $1\frac{1}{4}$ "

3 11

1 "

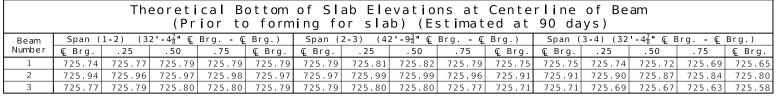
<u>3</u> 11

Conversion Factors for Beam Camber (Estimated at 90 days):

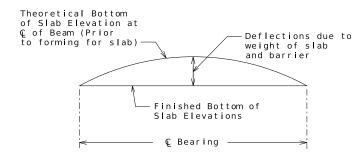
 $0.25 \text{ pt.} = 0.7125 \times 0.5 \text{ pt.}$

Exterior

Interior



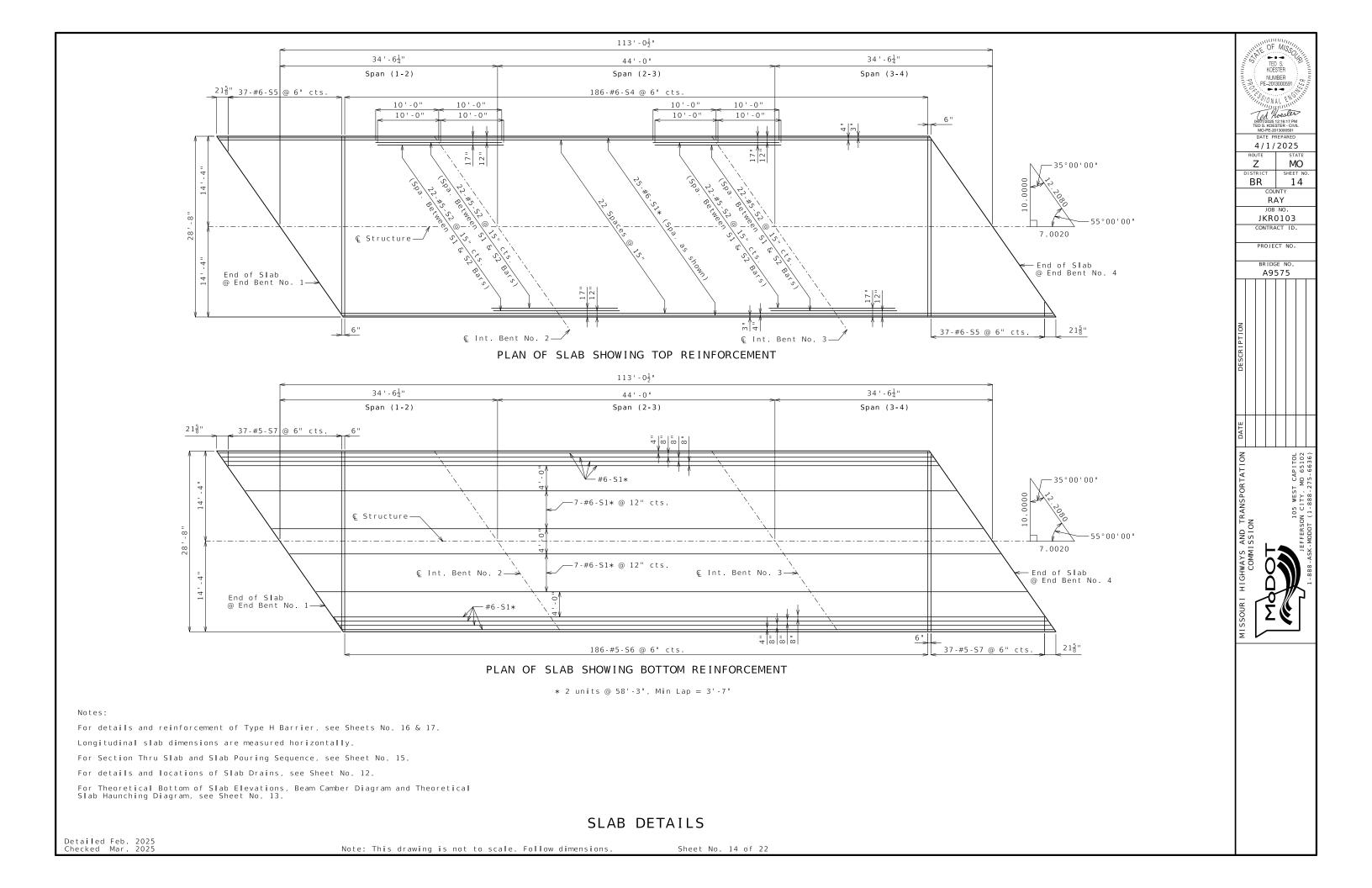
Elevations are based on a constant slab thickness of 8" and include allowance for theoretical dead load deflections due to weight of slab and barrier.



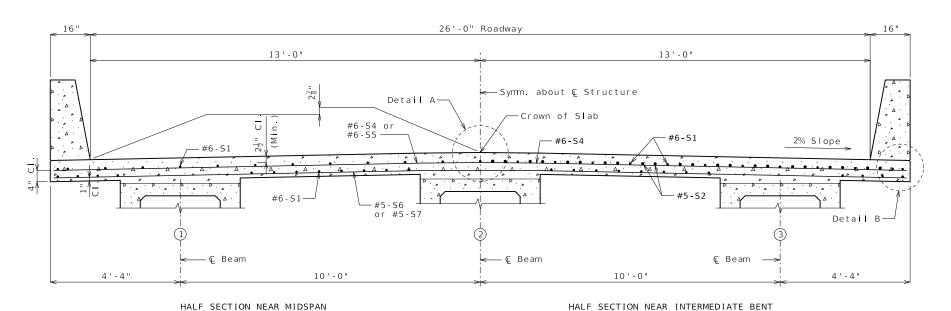
TYPICAL SLAB ELEVATIONS DIAGRAM

TED S. KOESTER NUMBER PE-2013000591 THE SOUNAL EN Ted Hoester 04/01/2025 12:15:57 PM TED S. KOESTER - CIVIL MO-PF-2013000591 4/1/2025 7 MO BR 13 RAY JKR0103 CONTRACT ID PROJECT NO. BRIDGE NO A9575

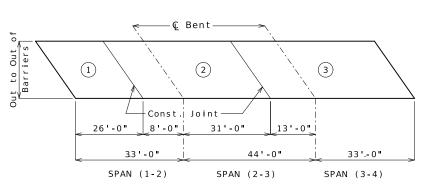
Detailed Feb. 2025 Checked Mar. 2025



OPTIONAL SHIFTING TOP BARS AT BARRIER



SECTION THRU SLAB

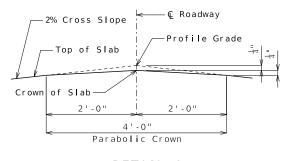


	Sed	Min. Rate of Pour Cu. Yds./Hr.			
		With Retarder			
Basic	1	2	3	25	
Sequence	End to 2	1 to 3	2 to End	25	
	rs to the basic s with Sec 703.	equence are subje	ct to the approva	l of the engineer	
Alternate A	1 .	+ 2	3	25	
Pours	End	to 3	2 to End		
Alternate B		1 + 2 + 3		25	
Pours		1 25			

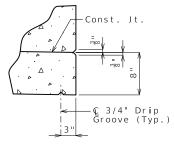
The contractor shall furnish an approved retarder to retard the set of the concrete to 2.5 hours, and shall pour and satisfactorily finish the slab pours at the rate given.

The concrete diaphragm at the intermediate bents and integral end bents shall be poured a minimum of 30 minutes and a maximum of 2 hours before the slab is poured.

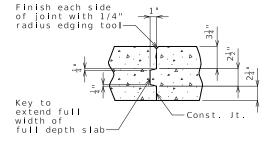
SLAB POURING SEQUENCE



DETAIL A



DETAIL B



FULL DEPTH SLAB

SLAB CONSTRUCTION JOINT

Notes:

For reinforcement of barrier not shown, see Sheets No. 16 & 17.

For Theoretical Bottom of Slab Elevations, Beam Camber Diagram and Theoreti Slab Haunching Diagram, see Sheet No. 13.

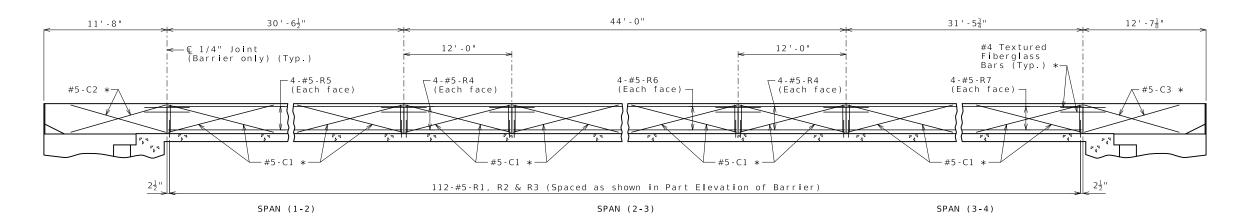
For Plan of Slab Showing Reinforcement, see Sheet No. 14.

SLAB DETAILS

Detailed Feb. 2025 Checked Mar. 2025

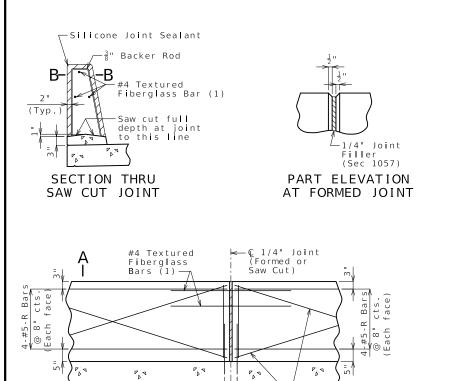






ELEVATION OF BARRIER

(Left barrier shown, right barrier similar) Longitudinal dimensions are horizontal.



PART ELEVATION OF BARRIER

#5-R1, R2 & R3

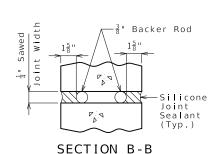
@ abt. 12" cts.

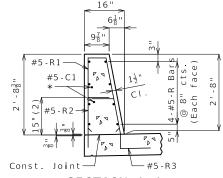
Detailed Feb. 2025 Checked Mar. 2025 -#5-C1 (Typ.) *

#5-R1, R2 & R3

@ abt. 12" cts.

(1) Four feet long, centered on joint, slip-formed option only



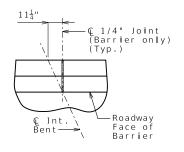


SECTION A-A

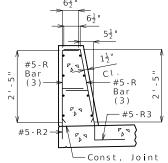
Use a minimum lap of 3'-1" for #5 horizontal barrier bars.

The cross-sectional area above the slab is 2.89 square feet.

(2) To top of bar



PART PLAN SHOWING JOINT LOCATION



R-BAR PERMISSIBLE ALTERNATE SHAPE

(3) The R1 bar may be separated into two bars as shown, at the contractor's option, only when slip forming is not used. (All dimensions are out to out.)

General Notes:

st Slip-formed option only.

Conventional forming or slip forming may be used. Saw cut joints may be used with conventional forming.

Top of barrier shall be built parallel to grade and barrier joints (except at end bents) normal to grade.

All exposed edges of barrier shall have either a 1/2-inch radius or a 3/8-inch bevel, unless otherwise noted.

Payment for all concrete and reinforcement, complete in place, will be considered completely covered by the contract unit price for Type H Barrier per linear foot

Concrete in barrier shall be Class B-1.

Measurement of barrier is to the nearest linear foot for each structure, measured along the outside top of slab from end of wing to end of wing.

Concrete traffic barrier delineators shall be placed on top of the barrier as shown on Missouri Standard Plan 617.10 and in accordance with Sec 617. Delineators on bridges with two-lane, two-way traffic shall have retroreflective sheeting on both sides. Concrete traffic barrier delineators will be considered completely covered by the contract unit price for Type H Barrier.

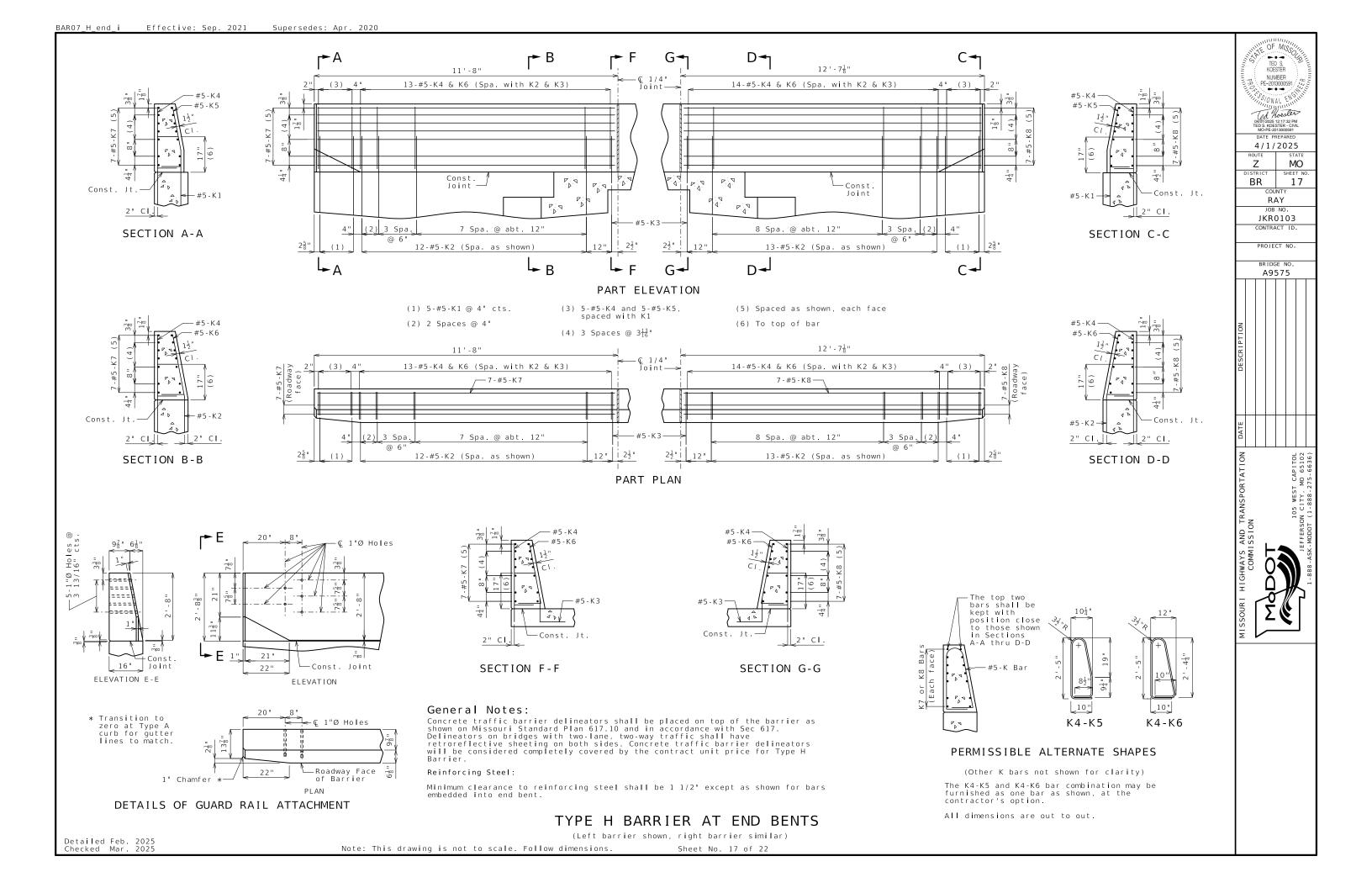
Joint sealant and backer rods shall be in accordance with Sec 717 for silicone joint sealant for saw cut and formed joints.

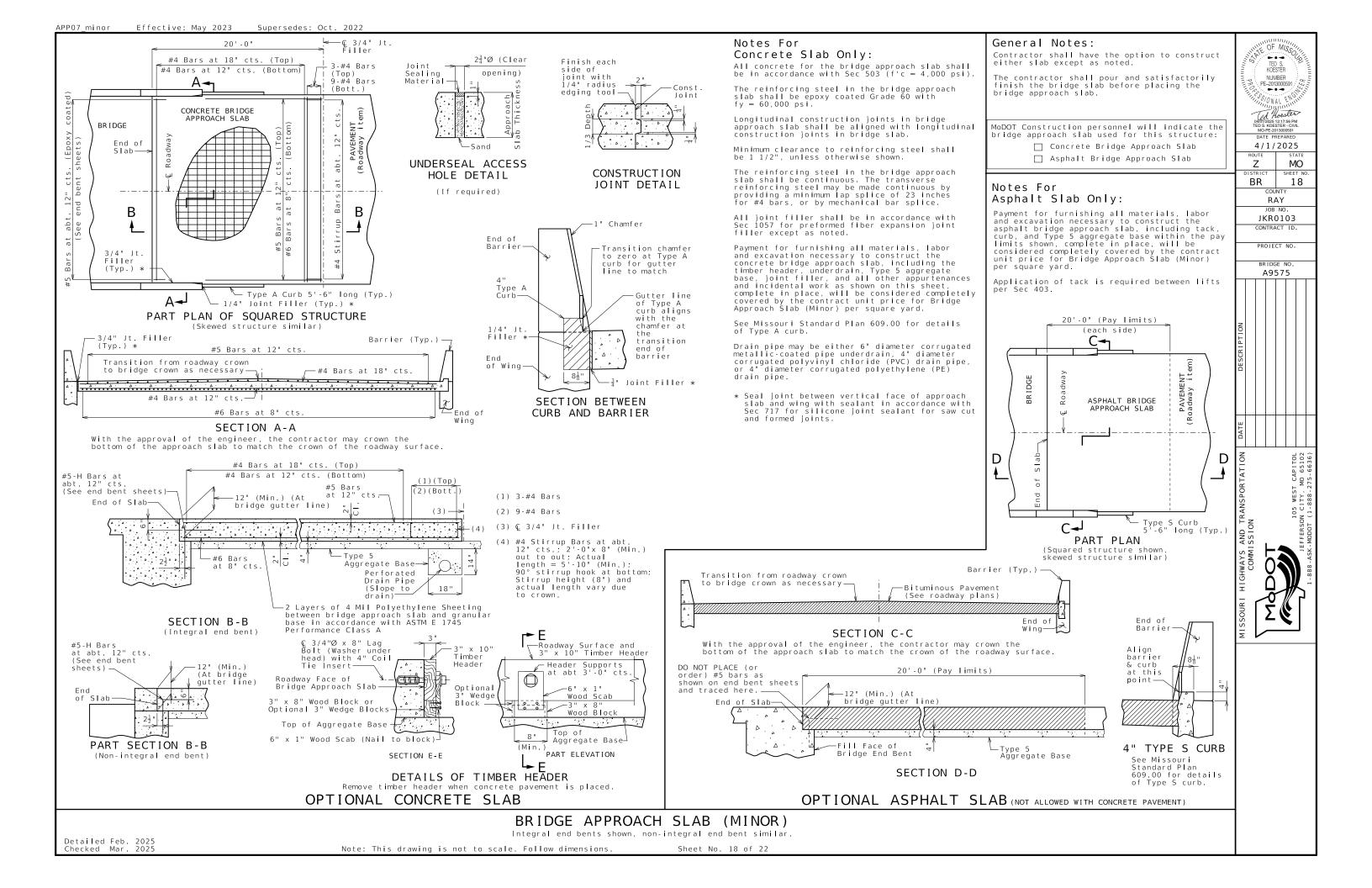
For slip-formed option, both sides of barrier shall have a vertically broomed finish and the top shall have a transversely broomed finish.

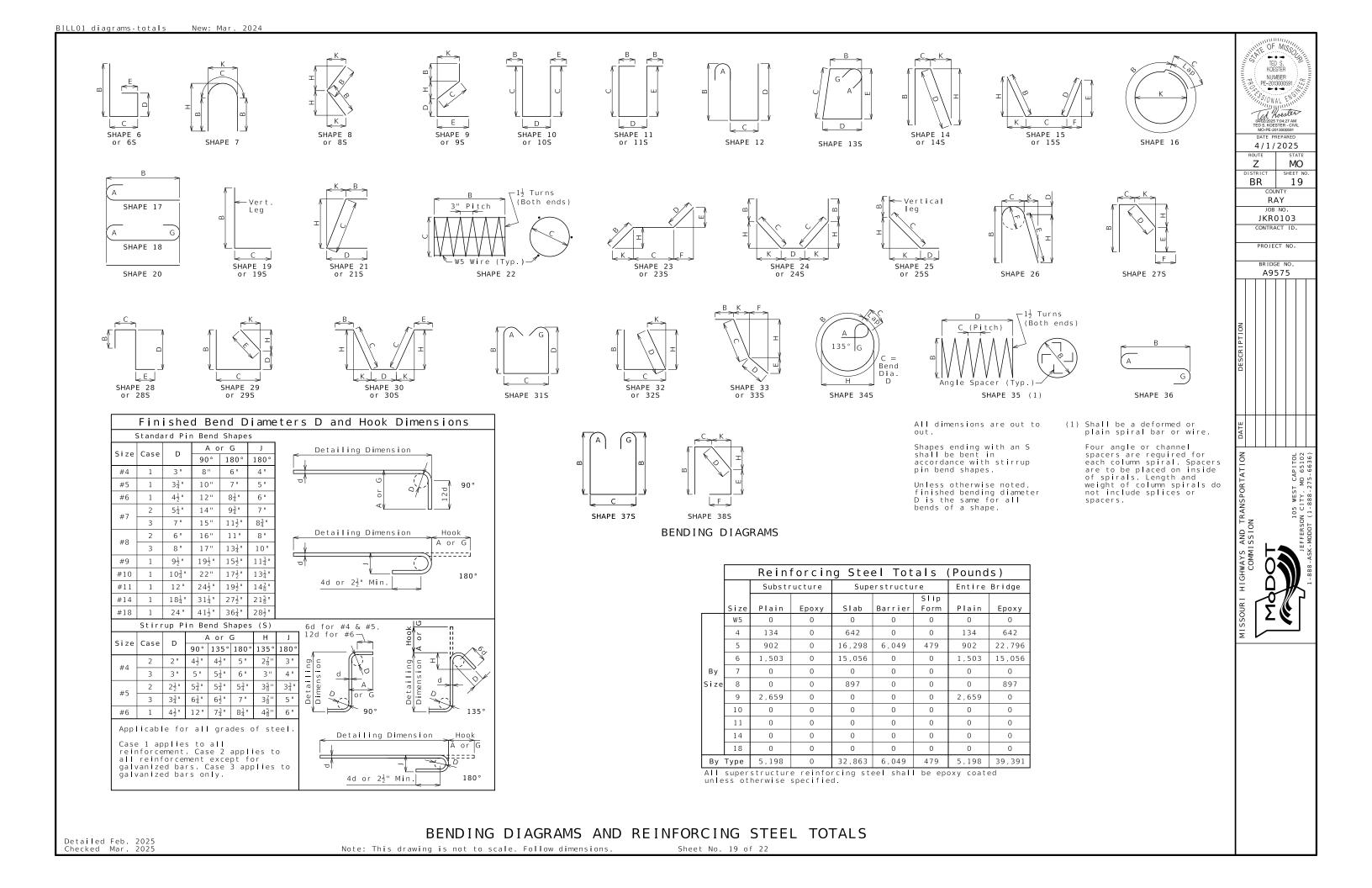
TED S. KOESTER NUMBER PE-2013000591 TIESSIONAL ENG 04/01/2025 12:17:07 PM TED S. KOESTER - CIVIL MO-PE-2013000591 4/1/2025 7 MO BR 16 RAY JKR0103 CONTRACT ID. PROJECT NO. BRIDGE NO A9575



TYPE H BARRIER







BILLO2 data New: Mar. 2024

Bill of Reinforcing Steel								Bill of Reinforcing Steel						OF MISSO									
					imensions			Nom.	Actual								Dimension			Nom.	Actual		TED S GETTE
No. Size/		Codes	ВС	D	E	F H	K	Length	Length					Codes	В	C D	E	F H	K		Length	Weight	I ∃ : KOESTER : ∃
Req. Mark	Location	C SH V	ft in. ft in.	ft in.	ft in. f	ft in ft in.	ft in.	ft in	. ft in	. Ib	Req.	Mark 5 K4	Location BARRIER	E 195	ft in 2 5.00	ft in. ft in. 10.00	ft in.	ft in. ft in.	ft in.	ft in.	-	1 b 244	NUMBER PE-2013000591
	Substructure										20	5 K5	BARRIER	E 38S	8.25	9.50 19.25		18.75	4.25	3 1	2 9	57	THE SOUNAL ENGLISH
											54		BARRIER	E 21S		2 5.00 10.00		2 4.25	6.00			178	
28 6 D200	Int Bent 2 BEAM	20	2 6.00					2 6	2 6	105	28	5 K7 5 K8	BARRIER BARRIER		11 5.00 12 4.00					11 5 12 4		333 360	04/01/2025 12:18:47 PM TED S. KOESTER - CIVIL
28 0 0 0	BLAM	20	2 0.00					2 0	2 0	103	20	3 80	DARRIER	120	12 4.00					12 4	12 4	300	MO-PE-2013000591 DATE PREPARED
24 9 H200	BEAM		30 1.00					32 7	32 7	2,659		5 R1	BARRIER	E 14S	2 5.00	6.50 2 5.50		2 5.00	5.50			1,227	4/1/2025
20 6 H201	BEAM BEAM	20	30 1.00	3 7.50				30 1 8 8	30 1 8 4	904		5 R2 5 R3	BARRIER BARRIER	E 19S	20.00	9.50	4 50	12 00 15 00	3 00	2 6		545 759	Z MO
28 6 H202 16 6 H203	BEAM	105	4 8.00	3 7.30				6	6	350 144		5 R4	BARRIER		11 9.00	9.50 15.25	4.50	12.00 15.00	3.00		11 9	392	DISTRICT SHEET NO.
											16	5 R5	BARRIER	E 20	30 3.00					30 3	30 3	505	BR 20
40 5 U200	BEAM BEAM	135	3 9.00 3 9.00		3 9.00			15 11	15 7	650	16	5 R6 5 R7	BARR I ER BARR I ER	E 20	19 9.00					19 9	19 9	330	COUNTY
22 5 U201 36 4 U202	BEAM	10S		3 9.00				11 3 5 9	11 5 7	252 134	10	3 K/	DARKIEK	E 20	31 2.00					31 2	31 2	520	JOB NO.
													Slip-Form										JKR0103
	Superstructure										32		SLIP FORM		12 0.00					12	12	401	CONTRACT ID.
	End Bent 1										4	5 C2 5 C3	SLIP FORM SLIP FORM	E 20	9 9.00					8 9		37 41	PROJECT NO.
14 6 F100	WING BRACE	E 15S	14.75 3 2.75		2 0.00	12.50 10.00	11.00	6 9	6 8	140													DD LDG5 NO
6 6 F101	DIAPHRAGM	E 19S	3 2.00 7 3.00		11 00	10.00 12.50	2 0 00	10 5	10 3	92				+									BRIDGE NO. A9575
14 6 F102 6 6 F103	WING BRACE DIAPHRAGM	E 15S	2 3.00 7 3.00 3 2.00 7 3.00		11.00	10.00 12.50	2 0.00	10 9	10 8	224 92	\vdash	-		++-+									
16 6 H100	BEAM		34 8.00					34 8	34 8	833													
16 6 H101 12 6 H102	BEAM BEAM	E 20 E 18	34 8.00 8 8.00					34 8	34 8	833 180													
6 6 H103	DIAPHRAGM		34 8.00					34 8	34 8	312													01.
12 6 H104	DIAPHRAGM		6 6.00					6 6	6 6	117													TAI
12 6 H105 8 6 H106	DIAPHRAGM DIAPHRAGM	E 19	2 5.00 2 11.00 34 8.00)				5 4 34 8	5 2 34 8	93													SCR
6 5 H107	STRAND TIE	E 23S	12.00 4 5.00	12.00	4.25	11.25 4.25	11.25	6 5	6 5														
52 5 H108	APPROACH NOTCH	E 19S	2 0.00 15.00)				3 3	3 2														
32 8 H109 56 6 H110	WING WING	E 20	10 6.00 9 8.00					10 6		897 813													
30 0 11110	WING	20	9 8.00					9 0	1 9 0	013													
54 4 U101	BEAM	E 13S	3 4.00 2 7.00		2 7.00			12 7	12 4	445													
24 5 U102 48 6 U103	BEAM DIAPHRAGM	E 10S	14.00 3 4.00	3 4.00				11 8	11 5	286 312													ATE
86 5 U104	DIAPHRAGM	E 12	20.00 4 6.00					6 9	6 7	591													
48 5 U105	DIAPHRAGM	E 37S	2 0.00 2 11.00					7 11	7 8	384													ON 015
48 5 V101	BEAM	E 17	4 2.00					4 9	4 9	238													UD TRANSPORTATION SION 105 WEST CAPITOL ERSON CITY, MO 65102 DOOT (1.888-275-6636)
72 6 V102	WING	E 20	5 2.00					5 2		559													CA CA MO
48 6 V103	DIAPHRAGM	E 20	12.00					1	1	72													POI
	Int Diaphragms																						3 NS S W 5 W 5 W 5 W 5 W 6 W 6 W 6 W 6 W 6 W 6
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16 5 H305	STRAND TIE	E 23S	12.00 4 5.00			4.25																	HIGHWAYS COMM 1-888-AS
32 4 U300	DIAPHRAGM	E 28S	19 00	2 2.00	4 0 00			7 8	7 6	160				+									
16 6 U301	DIAPHRAGM	E 28S		2 2.00				8 4		192													
16 5 V300	DIAPHRAGM	E 20	2 2.00					2 2	2 2	36													
	Slab																						
94 5 S1	SLAB		58 3.00							5,711													F
88 5 S2 186 6 S4	SLAB SLAB		20 0.00					20 28 5	20	1,836 7,939													
74 6 S5	SLAB		27 11.00						27 11														
	Incr. = 8.625"		2 2.00					2 2		1,672													
186 5 S6	SLAB	E 30	28 5.00					28 5	20 5	5,513	-			+									
74 5 S7	SLAB		28 5.00						28 5														
	Incr = 8.625"		2 2.00							1,161													
	D:													\Box									
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20 5 K1	BARRIER		3 8.00 9.25		3 2.75	5.25			7 11					+									
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	gths are based or						•		1 2 1	1 41			I		2	- Poguired coat	•			C C- I			

Nominal lengths are based on out to out dimensions shown in bending diagrams and are listed to the nearest inch for fabricator's use. Actual lengths are measured along centerline bar to the nearest inch. Weights are based on actual lengths.

All bars shall be Grade 60.

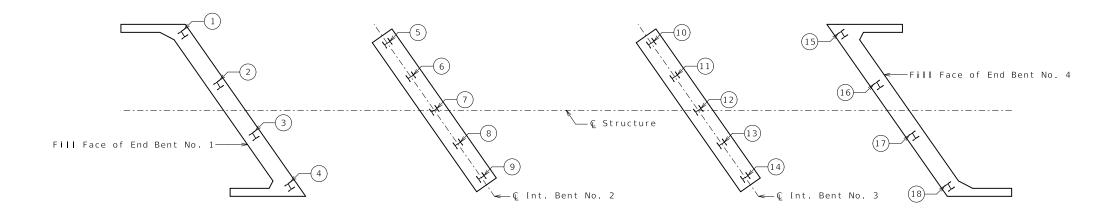
SH = Required shape, see bending diagrams.

BILL OF REINFORCING STEEL

V = Sets of varied bars and number of bars of each length. Bar dimensions vary in equal increments between dimensions shown on this line and the following line and the actual length dimension shown on this line and the following line vary by the specified increment.

 $\label{eq:codes:Codes:$

For bending diagrams and steel reinforcing totals, see Sheet No. 19. Detailed Feb. 2025 Checked Mar. 2025



PART PLAN SHOWING PILE NUMBERING FOR RECORDING AS-BUILT PILE DATA

			As-Built Pile Data
Pile No.	Length in Place (ft)	Computed Nominal Axial Compressive Resistance (kips)	R ema r k s
			End Bent No. 1
1			
2			
3			
4			
			Int. Bent No. 2
5			
6			
7			
8			
9			

			As-Built Pile Data
Pile No.	Length in Place (ft)	Computed Nominal Axial Compressive Resistance (kips)	Remarks
			Int. Bent No. 3
10			
11			
12			
13			
14			
			End Bent No. 4
15			
16			
17			
18			

Not	te:		
Ind	dicate in	remarks co	umn :
Α.	Pile type	and grade	
В.	Batter	_	
\sim	Drivon to	practical	rofucal

This sheet to be completed by MoDOT construction personnel.

TED S. NUMBER NUMBER PE-2013000591													
TED S. KOESTER - CIVIL MO-PE-2013000591 DATE PREPARED													
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				иту ЧҮ	,								
			JOB	NO									
		CON		01 CT	ID								
		PRO	OJE	СТ	NO.								
	PROJECT NO.												
	BRIDGE NO. A9575												
DESCRIPTION													
DAIE													
AYS AND TRANSPORTATION	OMMISSION		⊢ C		105 WEST CAPITOL	JEFFERSON CITY, MO 65102	-ASK-MODOT (1-888-275-6636)						