DESIGN DESIGNATION

A.A.D.T. - 2026 = 283 A.A.D.T. - 2046 = 313 D.H.V. = 23.30% V = 45 M.P.H.

D = 48.4%/51.6%

FUNCTIONAL CLASSIFICATION- MAJOR COLLECTOR

NO NEW RIGHT OF WAY OR EASEMENTS REQUIRED

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CONVENTIONAL SYMBOLS

	EXISTING	NEW
BUILDINGS AND STRUCTURES GUARD RAIL GUARD CABLE CONCRETE RIGHT-OF-WAY MARKER STEEL RIGHT-OF-WAY MARKER LOCATION SURVEY MARKER UTILITIES	0000	••••
FIBER OPTICS OVERHEAD CABLE TV UNDERGROUND CABLE TV OVERHEAD TELEPHONE UNDERGROUND TELEPHONE OVERHEAD POWER UNDERGROUND POWER SANITARY SEWER STORM SEWER GAS WATER	- FOOTVUTV OT UT OE UE S S S G W SAN	-F0- -OTV- -UTV- -OT- -UT- -OE- -VE- -S- -SS- -G- -W
MANHOLE	HYD.)
FIRE HYDRANT	wv C)
WATER VALVE	ww.)
WATER METER)
DROP INLET		
DITCH BLOCK	=	₽
GROUND MOUNTED SIGN	SIGN	-
LIGHT POLE		
H-FRAME POWER POLE		
TELEPHONE PEDESTAL FENCE CHAIN LINK WOVEN WIRE GATE POST	PED V	·
BENCHMARK	™≪)

NOTE: DASHED OR OPEN SYMBOLS INDICATE EXISTING FEATURES

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

PLANS FOR PROPOSED STATE HIGHWAY



PEMISCOT COUNTY

A6033 26 25	30 29	Ditch umber 16 K	26
34 ± 432 T North Deering 35	32 ₄₃₂ OAKVIL 80737	umber 16 (£, 2)	35 440 E 36
3 2 1 1 448	3510005 Ditch Number 15	442 4 3	2 Z 1
3330017 S07 10 11 12/ 3310008 452 S07	7 B \$0713 8	9 ⁴⁴ 10 GIBSO	11 12 12 N
15 14 13 80004 14 454	B S0715 17	16 ₄₅ 4 2870001 15 458 COVINGTON	14 13 456 L
22 23 5 24		TON STEELE	23 24 1G0458 B A523
A4923 26 25	30 472 29 SAMFORD 474	28 472 B 27	25 0 61 26 A1939
PROJECT		TP. A.T.	
PROJECT BRIDGE REPLACEM PAVEM	ENT, CULVERT,	A17+45-25	

THE EXISTENCE AND APPROXIMATE LOCATION OF UTILITY FACILITIES KNOWN TO EXIST, AS SHOWN ON THE PLANS, ARE BASED ON THE BEST INFORMATION AVAILABLE TO THE COMMISSION AT THIS TIME. THIS INFORMATION IS PROVIDED BY THE COMMISSION "AS-IS" AND THE COMMISSION EXPRESSLY DISCLAIMS ANY REPRESENTATION OR WARRANTY AS TO THE COMPLETENESS, ACCURACY, OR SUITABILITY OF THE INFORMATION FOR ANY USE. RELIANCE UPON THIS INFORMATION IS DONE AT THE RISK AND PERIL OF THE USER, AND THE COMMISSION SHALL NOT BE LIABLE FOR ANY DAMAGES THAT MAY ARISE FROM ANY ERROR IN THE INFORMATION. IT IS, THEREFORE, THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY THE EXISTENCE, LOCATION AND STATUS OF ANY FACILITY. SUCH VERIFICATION INCLUDES DIRECT CONTACT WITH THE LISTED UTILITIES.

INDEX OF SHEETS

DESCRIPTION	SHEET NUMBER
TITLE SHEET	1
TYPICAL SECTIONS (TS)	2
QUANTITIES (QU) (2 SHEETS)	3
PLAN-PROFILE (PP)	4
COORDINATE & REFERENCE POINTS (CP)-	5
SPECIAL SHEETS (SS) (2 SHEETS)	6 - 7
TRAFFIC CONTROL SHEETS (TC)	8
EROSION CONTROL SHEETS (EC)	9
SIGNING & PAVEMENT MARKING (SN)	10-11
CULVERT SECTIONS (CS)	12
CROSS SECTIONS (XS)	XS01-XS04
BRIDGE DRAWINGS (B)	
A9720	1-23



LENGTH OF PROJECT

BEGINNING OF PROJECT STA. 412+65.59 STA. 417+45.25 END OF PROJECT APPARENT LENGTH 479.66 FEET

EQUATIONS AND EXCEPTIONS:

TOTAL CORRECTIONS

STATE LENGTH

NET LENGTH OF PROJECT

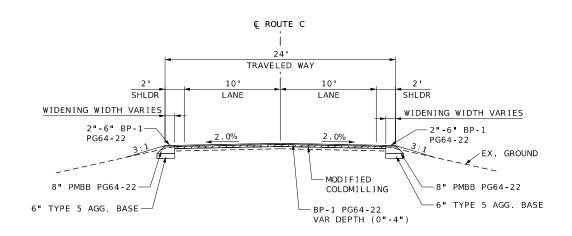
FOR INFORMATION ONLY ESTIMATED DISTURBED ACRES



5220 Oakland Ave. St. Louis, MO 63110 (314) 863-5570

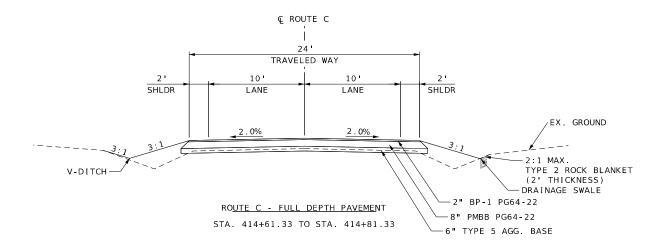


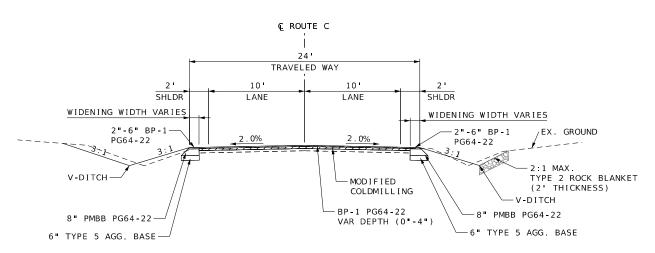
0 FEET 479.66 FEET 0.091 MILES 1 ACRES



ROUTE C - MILL & OVERLAY

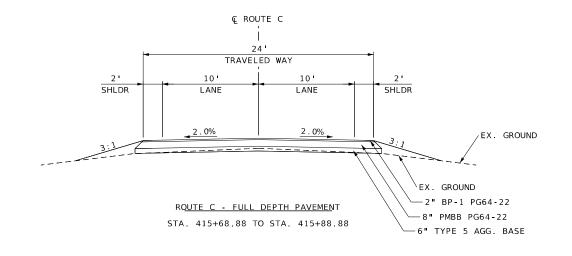
STA: 412+65.59 TO STA: 413+50.00 STA: 415+88.88 TO STA: 417+45.25





ROUTE C - MILL & OVERLAY

STA. 413+50.00 TO STA. 414+61.33



APPLICATION RATES:

BP-1 (PG64-22) 1.948 TON/CUYD COMPACTED MIXTURE BP-1 (PG64-22) 1.943 TON/SY BASE COURSE TACK COAT 0.10 GAL/SQYD

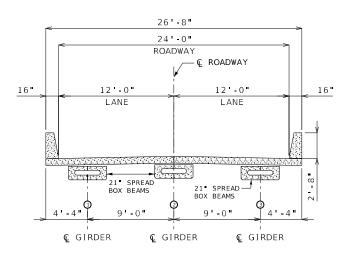
NOTE

MODIFIED COLDMILLING MUST BE APPLIED AS DIRECTED BY THE ENGINEER TO ENSURE A SMOOTH TRANSITION FROM EXISTING PAVEMENT TO THE PROPOSED FULL DEPTH PAVEMENT.

BRIDGE APPROACH SLAB (MINOR) STA. 414+81.33 TO STA. 415+00.83 STA. 415+49.38 TO STA. 415+68.88

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ROUTE C - BRIDGE STA 415+00.83 TO STA. 415+49.38

TYPICAL SECTIONS
SHEET 1 OF 1



	REMOVAL OF IMPROVEMENTS									
ROUTE	PLAN SHEET	BEGIN STATION	END STATION	OFFSET	DESCRIPTION	QUANTITY				
RTE C	10	N/A		RT	ONE LANE BRIDGE SIGN (36X36)	1 EA				
RTE C	10	N/A		RT	WEIGHT LIMIT SIGN (36X48)	1 EA				
RTE C	10	412+74		RT	YIELD SIGN (48X48) TO ONCOMING TRAFFIC SIGN (36X30)	1 EA				
RTE C	4	414+61	414+96	LT/RT	BITUMINOUS PAVEMENT	82 SY				
RTE C	4	414+18	414+67	RT	24" PIPE	50 LF				
RTE C	4	414+19	414+59	LT	24" PIPE	40 LF				
RTE C	10	414+26		LT	STOP SIGN (36X36)	1 EA				
RTE C	10	414+96		RT	OBJECT MARKER SIGN (12X36)	3 EA				
RTE C	10	414+94		LT	OBJECT MARKER SIGN (12X36)	3 EA				
RTE C	4	415+46	415+89	LT/RT	BITUMINOUS PAVEMENT	93 SY				
RTE C	10	415+47		RT	OBJECT MARKER SIGN (12X36)	3 EA				
RTE C	10	415+46		LT	OBJECT MARKER SIGN (12X36)	2 EA				
RTE C	10	417+68		LT	YIELD SIGN (48X48)	1 EA				
					TO ONCOMING TRAFFIC SIGN (36X30)					
RTE C	10	N/A		LT	WEIGHT LIMIT SIGN (36X48)	1 EA				
RTE C	10	N/A		LT	ONE LANE BRIDGE SIGN (36X36)	1 EA				
					TOTAL	1 LUMP SUM				

EARTHWORK								
	CUT	FILL	MODIFIED LINEAR					
LOCATION			GRADING CLASS II					
	CY	CY	STA					
NORTH	36	242						
ABUTMENTS	19	18	4.8					
SOUTH	64	52						
SUBTOTAL	119	312	4.8					
TOTAL	TOTAL FOR INFORMATION ONLY 4.8							
SHRINKAGE FACTOR = 1.00								

POROUS BACKFILL									
ROUTE	OUTE PLAN BEGIN END POROUS								
	SHEET	STATION	STATION	BACKFILL					
				CY					
RTE C	4	414+95.83	415+00.83	27					
RTE C	4	415+49.38	415+54.38	27					
			TOTAL	54					

THE INEAR SEA ASS II ONL DIS

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CONTRACTOR FURNISHED
SURVEYING AND STAKING
TOTAL = 1 LUMP SUM

MOBILIZATION							
	TOTAL	=	1	LUMP	SUM		

SEEDING							
SEEDING AND MULCHING -							
COOL SEASON GRASSES							
TOTAL = 1 LUMP SUM							

						PAVEMENT				
					BITUMINOUS	BITUMINOUS	MODIFIED	TYPE 5 AGGREGATE	TACK	GRAVEL (A) OR
ROUTE	PLAN	BEGIN	END	OFFSET	PAVEMENT MIXTURE	BASE COURSE	COLDMILLING	FOR BASE	COAT	CRUSHED
	SHEET	STATION	STATION		PG64-22 (BP-1)	PG64-22	(DEPTH TRANSITIONS)	(6 IN. THICK)		STONE (B)
					SY	SY	SY	SY	GALLON	SY
RTE C	4	412+65.59	414+61.33	LT/RT	562.0		562.0		56.2	
RTE C	4	412+65.59	414+17.74	LT	31.3	31.3		31.3	3.1	
RTE C	4	414+38.75		RT						3.9
RTE C	4	414+38.75		LT						4.3
RTE C	4	414+61.33	414+81.33	LT/RT	53.3	53.3		53.3	5.3	
RTE C	4	415+68.88	415+88.88	LT/RT	53.3	53.3		53.3	5.3	
RTE C	4	415+88.88	417+45.25	LT/RT	372.4		372.4		37.2	
RTE C	4	415+88.88	417+32.83	LT	21.4	21.4		21.4	2.1	
RTE C	4	415+88.88	416+23.42	RT	6.4	6.4		6.4	0.6	
RTE C	4	416+50.86		RT						7.2
RTE C	4	416+58.93	414+45.25	RT	22.4	22.4		22.4	2.2	
				SUBTOTAL	1122.5	188.1	934.4	188.1	112.0	15.4
					2" DEPTH	8" DEPTH				
					X 1.948 TONS/CY	X 1.943 TONS/SY				X 1.4 TONS/CY
					TONS	TONS	SY	SY	GALLON	TONS
		•		TOTAL	121.5	81.2	934	188	112	22

	SIGNING & PAVEMENT MARKING									
	CLASS 1 PAVEMENT MARKING CLASS 1 PAVEMENT MARKING									
ROUTE	PLAN	PLAN	END	OFFSET	(18-MIL, TYPE P BEADS)	(18-MIL, TYPE P BEADS)				
	SHEET STATION STATION			4 IN. WHITE	4 IN. YELLOW					
			LF	LF						
RTE C	RTE C 10 412+65.59 417+45.25 LT/RT		960	2694						
	TOTAL				960	2694				

	PERMANENT EROSION CONTROL											
					FURNISHING	PLACING	PERMANENT					
ROUTE	PLAN	BEGIN	END	OFFSET	TYPE 2	TYPE 2	EROSION CONTROL					
	SHEET	STATION	STATION		ROCK BLANKET	ROCK BLANKET	GEOTEXTILE					
					CY	CY	SY					
RTE C	4	413+35.59	414+17.54	RT	17	17	26					
RTE C	4	414+90.70	415+20.18	LT/RT	100	100	150					
RTE C	4	415+29.81	415+59.22	LT/RT	86	86	129					
				TOTAL	186	186	279					

			TEMPORARY	EROSIO	N CONT	ΓROL	
ROUTE	PLAN	BEGIN	END	OFFSET	SILT	SEDIMENT	ALTERNATE
	SHEET	STATION	STATION		FENCE	REMOVAL	DITCH CHECK
					LF	CY	LF
RTE C	9	412+65.59	413+00.00	RT	31	1	
RTE C	9	413+00.00	414+17.46	RT			20
RTE C	9	412+65.59	414+19.09	LT			30
RTE C	9	415+82.90	416+50.86	LT			10
RTE C	9	416+50.86	417+45.25	LT	95	1	
RTE C	9	415+86.84	416+96.40	RT			30
RTE C	9	416+96.40	417+45.25	RT	51	1	
		•		TOTAL	177	3	90

	PIPE CULVERTS											
ROUTE	PLAN	BEGIN	END	OFFSET	PIPE	FLARED END SECTION	FLAP GATE	CLASS 3				
	SHEET	STATION	STATION		36" GROUP C	36" GROUP C	36"	EXCAVATION				
					LF	EA	EA	CY				
RTE C	4	414+17.54	415+17.80	RT	92	1	1	63				
RTE C	4	414+19.12	415+13.36	LT	86	1	1	77				
	•	•	•	TOTAL	178	2	2	140				

SUMMARY OF QUANTITIES
SHEET 1 OF 2



THIS SHEET HAS SIGNED, SEALED, DATED ELECTRONIC

B/29/2025

ROUTE STATE C MO
DISTRICT SHEET NO.
SE 3

COUNTY
PEMISCOT
JOB NO.

J9S3770 CONTRACT ID

PROJECT NO.

BRIDGE NO.

I HIGHWAYS AND TRANSPORTATION DECOMMISSION

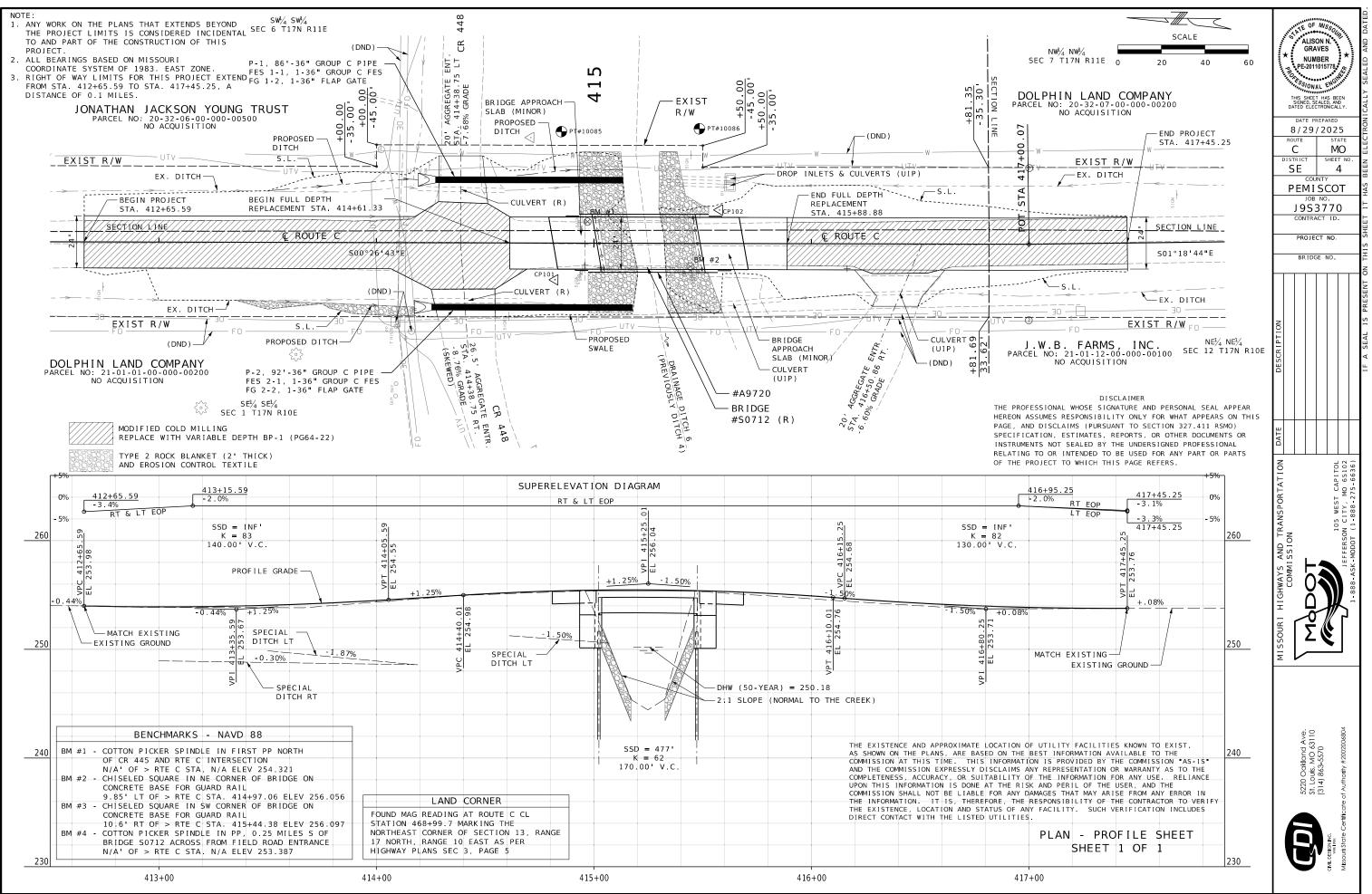
COMMISSION

105 WEST CAPITOL

JEFFERSON CITY, MO 65102

5220 Oakland Ave. St. Louis, MO 63110 (314) 863-5570 (314) 863-5570 styne. Istate Certificate of Authority #2002006804

		1	ITOTALL OTY ITO	- A . I		1		1	1	1	071/ [7074]		П			
			TOTAL QTY TOT	- 1 - 1							QTY TOTAL			EFFECTIVE: 07-01-2025		
SIGN	SIZE	AREA QTY	AREA RELOCREL	OC SIGN		SIGN	SIZE				ELOC RELOC SIG		ITEM	TOTAL	, regard	E OF MISSOUR
	IN.	SQ.FT. EACH	SQ.FT. EACH SQ.	FT NO	DESCRIPTION		IN.	SQ.FT.	EACH	I SQ FT	EACH SQ.FT. NO	. DESCRIPTION	NUMBE	R QTY DESCRIPTION		ALISON N
			WARNING SIGN	S						GUID	E SIGNS		6122008	B IMPACT ATTENUATOR 40 MPH (SAND BARRELS)	l : + :	GRAVES *
WO1-1L		16.00			TURN (SYMBOL LEFT)	E05-1		12.00				GORE EXIT	6122009			NUMBER
WO1 - 1R		16.00			TURN (SYMBOL RIGHT)	E05-2		12.00				EXIT OPEN	6122010		, 70x	· · · · · · · · · · · · · · · · · · ·
WO1 - 2L		16.00			CURVE (SYMBOL LEFT)		48X36					EXIT CLOSED	6122012		7,7,8	SONAL ENGLIS
WO1 - 2R		16.00			CURVE (SYMBOL RIGHT)		60X24					ROAD WORK NEXT XX MILES	6122014		THIS	SHEET HAS BEEN NED, SEALED, AND D ELECTRONICALLY.
WO1 - 3L		16.00			REVERSE TURN (SYMBOL LEFT)		48X24					END ROAD WORK	6122017		DATEC) ELECTRONICALLY.
WO1 - 3R		16.00			REVERSE TURN (SYMBOL RIGHT)		36X18					PILOT CAR FOLLOW ME	6122019			ATE PREPARED
WO1 - 4L		16.00			REVERSE CURVE (SYMBOL LEFT)	GO20-4a						PILOT CAR IN USE WAIT & FOLLOW				29/2025
WO1 - 4R		16.00			REVERSE CURVE (SYMBOL RIGHT)	GO20 - 4a						PILOT CAR IN USE WAIT & FOLLOW			ROUTE	
WO1 - 4bL		16.00		-	DOUBLE ARROW REVERSE CURVE (SYMBOL LEFT)	GO20-5aP			-	6.00	52	WORK ZONE (PLAQUE)	6122040		DISTRI	CT SHEET NO.
WO1-4bR WO1-4cL		16.00			DOUBLE ARROW REVERSE CURVE (SYMBOL RIGHT) TRIPLE ARROW REVERSE CURVE (SYMBOL LEFT)	MO4-8a MO4-9L		3.00	2	6.00	52	DETOUR (LEFT)	6122041		SE	
WO1-4CL WO1-4CR					TRIPLE ARROW REVERSE CURVE (SYMBOL RIGHT)		48X36					DETOUR (RIGHT)	6161012			COUNTY
WO1 - 6		12.50			HORIZONTAL ARROW (SYMBOL)	MO4 - 9P	48X12			+ +		STREET NAME (PLAQUE)	6161013		PE	MISCOT
WO1-6a		18.00			HORIZ. ARROW (SYMBOL ON PERMANENT BARRICADE)	MO4-10L						DETOUR ARROW (LEFT)	6161014			JOB NO.
WO1 - 7		12.50			DOUBLE HEAD HORIZONTAL ARROW (SYMBOL)	MO4 - 10R						DETOUR ARROW (RIGHT)	6161020			9S3770
WO1-7a		18.00			DOUBLE HEAD HORIZ. ARROW (SYMBOL ON PERM. BARR.)					REGULA	FORY SIGNS	,	6161022			ONTRACT ID.
WO1-8		3.00			CHEVRON (SYMBOL)	R1-1	48X48	13.25				STOP	6161025		PF	ROJECT NO.
WO1-8a	30X36	7.50			CHEVRON (SYMBOL FOR DIVIDED HIGHWAYS)	R1-2	48TRI.	6.93				YIELD	6161026	CHANNELIZER (VERTICAL PANEL)		
WO3 - 1	48X48	16.00			STOP AHEAD (SYMBOL)	R1-2a	36X36	9.00				TO ONCOMING TRAFFIC (PLAQUE)	6161030	0 20 TYPE 3 MOVEABLE BARRICADE	В	BRIDGE NO.
WO3 - 2	48X48	16.00			YIELD AHEAD (SYMBOL)	R1-3P	30X12	2.50				ALL WAY (PLAQUE)	6161033	DIRECTION INDICATOR BARRICADE		
WO3 - 3	48X48	16.00			SIGNAL AHEAD (SYMBOL)	R2-1		12.00				SPEED LIMIT XX	6161040	FLASHING ARROW PANEL	. □□□	
WO3 - 4		16.00			BE PREPARED TO STOP	R3-1		16.00				NO RIGHT TURN (SYMBOL)	6161047]	
WO3 - 5		16.00			SPEED LIMIT AHEAD	R3-2		16.00				NO LEFT TURN (SYMBOL)	6161055]	
WO4-1L		16.00			MERGE (SYMBOL FROM LEFT)	R3-3	36X36					NO TURNS	6161070]	
WO4-1R		16.00			MERGE (SYMBOL FROM RIGHT)	R3-4		16.00				NO U-TURN (SYMBOL)	6161095		[종]	
WO4-1aL	48X48			-	MERGE (LEFT)	R3-7L	30X30					LEFT LANE MUST TURN LEFT	4	CHANGEABLE MESSAGE SIGN,		
WO4-1aR		16.00			MERGE (RIGHT)	R3-7R	30X30					RIGHT LANE MUST TURN RIGHT	6161096			
WO5 - 1		16.00			ROAD/BRIDGE/RAMP NARROWS	R4-1		12.00				DO NOT PASS		CHANGEABLE MESSAGE SIGN WITHOUT COMM.	SC	
WO5 - 3		16.00			ONE LANE BRIDGE	R4-2		12.00				PASS WITH CARE	6161098			
WO5 - 5		16.00			NARROW LANES	R4-7a		12.00				KEEP RIGHT (HORIZONTAL ARROW)	1	CHANGEABLE MESSAGE SIGN WITH COMM.		
WO6 - 1		16.00		-	DIVIDED HIGHWAY (SYMBOL)	R4-8a		12.00				KEEP LEFT (HORIZONTAL ARROW)	6161099		!	
WO6 - 2 WO6 - 3		16.00			DIVIDED HIGHWAY END (SYMBOL)	R5-1	30X30			+		DO NOT ENTER WRONG WAY	6162000			
WO0-3 WO7-3a		16.00 5.00			TWO WAY TRAFFIC (SYMBOL) NEXT XX MILES (PLAQUE)	R5-1a R6-1L	36X24 54X18					ONE WAY ARROW (LEFT)	6162002	TEMPORARY LONG-TERM RUMBLE STRIPS TEMPORARY TRAFFIC BARRIER,		
WO7-3a WO8-1		16.00			BUMP	R6-1R	54X18					ONE WAY ARROW (LEFT) ONE WAY ARROW (RIGHT)	6173600	· ·	ш	
WO8 - 2		16.00			DIP	R6-2L	24X30					ONE WAY ARROW (RIGHT)	10173000	TEMP. TRAFFIC BARRIER ANCHORED,	TA	
WO8 - 3		16.00			PAVEMENT ENDS	R6-2R	24X30					ONE WAY (RIGHT)	6173700	· ·		
WO8 - 4		16.00			SOFT SHOULDER	R9-9	24X12					SIDEWALK CLOSED	10273700	TEMP. TRAFFIC BARRIER STIFFNESS TRANSITION	Z) 22 5) 22
WO8 - 5		16.00			SLIPPERY WHEN WET (SYMBOL)							SIDEWALK CLOSED AHEAD,	6173706		l:	T CAPITOL MO 65102 275-6636)
WO8 - 6		16.00			TRUCK CROSSING	R9-11L	24X18	3.00				(ARROW LEFT) CROSS HERE		TEMP TRAFFIC BARRIER HEIGHT TRANSITION,	Ā	CAP 5-6
WO8-6c	48X48	16.00			TRUCK ENTRANCE							SIDEWALK CLOSED AHEAD,	6174000	CONTRACTOR FURNISHED/RETAINED	<u> </u>	T 0
WO8 - 7	36X36	9.00			LOOSE GRAVEL	R9-11R	24X18	3.00				(ARROW RIGHT) CROSS HERE	6175010	RELOCATING TEMPORARY TRAFFIC BARRIER	1 6	VES ΓΥ, 88
WO8-7a	36X36	9.00			FRESH OIL / LOOSE GRAVEL	R10-6	24X36	6.00				STOP HERE ON RED (45^ ARROW)	6175011	1B RELOCATING TEMP. TRAFFIC BARRIER ANCHORED	l Š	105 v SON CI
WO8 - 9	48X48	16.00			LOW SHOULDER	R11-2	48X30	10.00	2	20.00	29	ROAD CLOSED	6175013	RELOCATING TEMP. TRAFFIC BARRIER STIFFNESS	<u> </u>	O N T
WO8 - 11	48X48	16.00			UNEVEN LANES							ROAD CLOSED XX MILES AHEAD	6175020	RELOCATING TEMP. TRAFFIC BARRIER HEIGHT	<u> </u>	RSC
WO8-12	48X48	16.00			NO CENTER LINE	R11-3a	60X30	12.50	2	25.00	55A,	/B LOCAL TRAFFIC ONLY	6208064	TEMPORARY RAISED PAVEMENT MARKER	ANI SS	MO F
WO8 - 15					GROOVED PAVEMENT	R11-4				50.00	550	C ROAD CLOSED TO THRU TRAFFIC	9029400		SIN	— — <u>— ×</u>
WO8 - 15P					MOTORCYCLE (PLAQUE)	CONST - 3A						FINE SIGN	9029401	1 TEMPORARY TRAFFIC SIGNALS AND LIGHTING	∀O	
WO8 - 17L					SHOULDER DROP-OFF (SYMBOL LEFT)	CONST - 3X	4 56X12	4.67	<u> </u>	41.665	NEOUS STEELS	SPEEDING/PASSING (PLATE)	1		ÌŽŬ	
WO8 - 17R					SHOULDER DROP-OFF (SYMBOL RIGHT)	CONST	4000	10.00		MISCELLA	NEOUS SIGNS	DOINT OF PRESSURE	4	DISCLAIMER	9	\\\\\
WO8 - 17P					SHOULDER DROP-OFF (PLAQUE)	CONST - 5				+ +		POINT OF PRESENCE	THE PE	ROFESSIONAL WHOSE SIGNATURE AND PERSONAL	I≖	7//// I
W10-1	42RND.			-	RAILROAD CROSSING	CONST - 5		_	-			POINT OF PRESENCE	1	APPEAR HEREON ASSUMES RESPONSIBILITY	_~	~ / / / / / / / / / / / / / / / / / / /
WO12-1					DOUBLE DOWN ARROW (SYMBOL)	CONST-8	48836	12.00		+ +		WORK ZONE NO PHONE ZONE		FOR WHAT APPEARS ON THIS PAGE, AND	S	> (2
WO12-2					LOW CLEARANCE (SYMBOL)					+ +			1	AIMS (PURSUANT TO SECTION 327.411 RSMO)	SS	- 🗸 📗
W012-2x W012-2a	_				LOW CLEARANCE (PLAQUE) OVERHEAD LOW CLEARANCE (FEET AND INCHES)			-		+ +				FICATION, ESTIMATES, REPORTS, OR OTHER	🛮 レ	
WO12-2a					LOW CLEARANCE (FEET AND INCHES)			-		+ +				ENTS OR INSTRUMENTS NOT SEALED BY THE	<u> </u>	
WO12-4					WIDTH RESTRICTION XX FT XX IN XX MILES AHEAD					+ +				SIGNED PROFESSIONAL RELATING TO OR	l	
WO12-3					ADVISORY SPEED (PLAQUE)					+ +			-	DED TO BE USED FOR ANY PART OR PARTS OF	l	
WO16-2	_			-	XXX FEET (PLAQUE)					+ +			-1	ROJECT TO WHICH THIS PAGE REFERS.	l	
WO16-3	_				X MILE (PLAQUE)								' ' ' '	ACTOR TO WITCH THIS TACE REFERST	l	
WO20-1					ROAD/BRIDGE/RAMP WORK AHEAD								1		l	[
			192.00	18	DETOUR AHEAD								1			.ve.
	_	16.00 2	32.00		ROAD CLOSED 500 FT	616-10	.05			TOTAL		•	_			0 0 × 0 × 0 × 0 × 0 × 0 × 0 × 0 × 0 × 0
WO20-3		16.00 2	32.00		ROAD CLOSED AHEAD	CONSTR		N SIGN	IS	357					l <u>i</u>	Oakland Ave. Juis, MO 63110 863-5570 Joiity #2002006804
WO20-4	48X48	16.00			ONE LANE ROAD AHEAD	616-10					TOTAL]	Oak Jis, 1 363-
WO20-5	48X48	16.00			RIGHT/CENTER/LEFT LANE CLOSED AHEAD	RELOCA		<u>I GNS</u>			0				9	So t Lou ithor
WO20-5a	48X48	16.00			2 RIGHT/CENTER/LEFT LANES CLOSED AHEAD										8	5220 St. Lo St. Lo (314)
WO20-6a	48X48	16.00			RIGHT/CENTER/LEFT LANE CLOSED										l	9
WO20-7a	48X48	16.00			FLAGGER (SYMBOL)											iii iii
WO21-2					FRESH OIL											5 [
WO21-5					SHOULDER WORK / SHOULDER WORK AHEAD									SUMMARY OF QUANTITIES		Z # Z
WO22-1	_				BLASTING ZONE AHEAD									SHEET 2 OF 2	1	Ti St William
WO22-2					TURN OFF 2-WAY RADIO AND PHONE									-	T	■ So Line A
WO22-3	_				END BLASTING ZONE											₩ S
GO22-1	21X15	2.19			WET PAINT (ARROW PIVOTS)										l	
I																



ALL PROJECT COORDINATES HAVE BEEN PROJECTED FROM THE MISSOURI STATE PLANE COORDINATE (SPC) SYSTEM OF 1983 USING AN AVERAGE PROJECTION (GRID TO GROUND) FACTOR. TO GET BACK TO STATE PLANE COORDINATES MULTIPY THE PROJECT COORDINATES BY THE AVERAGE GRID FACTOR AS SHOWN IN THE "REFERENCE CONTROL INFORMATION" PORTION OF THIS TABLE.

SHEET NO

N/A

4

4

N/A

ALIGNMENTS

STATION

N/A

414+82.12

415+57.74

N/A

409+23.20

417+00.07

425+00.52

441+79.23

417+00.07

418+00.00

ROUTE C - CONSTRUCTION CENTERLINE 411+00.00

PROJECT CONTROL POINTS

ROUTE C - EXISTING CENTERLINE

LOCATION

N/A

RT

LT

N/A

(USFT)

N/A

16.8931

15.3773

PROJECT COORD	I NA	TE INFOR	MAT I ON						
COORDINATE SYSTE	м м	O SPC							
HORIZONTAL DATUM	N.	AD83 2011							
VERTICAL DATUM	N.	NAVD88							
GEOID MODEL	2	018							
ELEVATIONS		IEEEDENTI	AL LEVELING						
DETERMINED BY	٦	DIFFERENTIAL LEVELING							
PROJECT PROJECTI	ON F	ACTOR	1.00000000						
REFERENCE CON	TROI	DL INFORMATION							
COORDINATE SYSTE	м м	MO SPC							
CONTROL STATION	M	OKE							
DESIGNATION	MODO	OT KENNET	T CORS ARP						
CORS_ID	MOKE	∃							
PID	DL68	392							
LATITUDE	3613	317.10465							
LONGITUDE	9004	124.46779							
NORTHING (M)	4314	3143.1560							
		17 0000							
EASTING (M)	2883	347.8830							

EXAMPLE OF PROJECT COORDINATE TO S.P.C.

PROJECT NORTHING X AVERAGE GRID FACTOR = STATE PLANE NORTHING PROJECT EASTING X AVERAGE GRID FACTOR = STATE PLANE EASTING

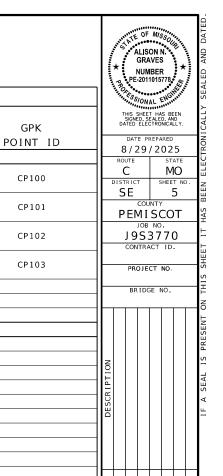
PROJECT AVERAGE GRID FACTOR 1.00000000

N _____ X _ . _ _ = N ____ . _ E ___ X _ . _ _ = E ____ . _ _ .

LINEAR UNIT CONVERSION

1 METER = 3.280833333 US SURVEY FEET (USFT)

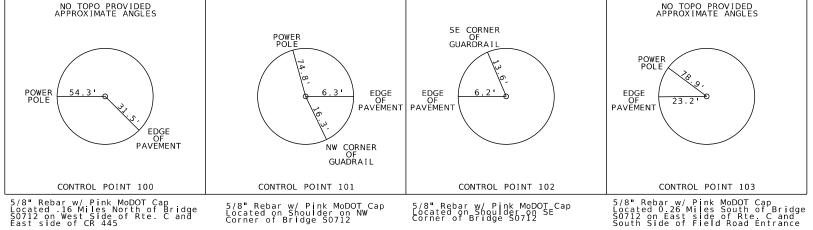
EDGE



5220 Oakland Ave. St. Louis, MO 63110 (314) 863-5570

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COORDINATE POINT LISTING

(US SURVEY FT) (US SURVEY FT

ELEVATION

252.653

253.710

253.814

253.295

MODIFIED STATE PLANE (GROUND)

EASTING

993097.506

993060.567

993093.424

993136.450

993073.1159

993079.1533

993097.4840

993103.8728

993074.4899

993079.1533

993081.4417

NORTHING

110340.863

109473.347

109397.984

108061.246

110032.3848

109255.5363

108455.3002

106776.5963

109855.5901

109255.5363

109155.6345

(US SURVEY FT)

COORDINATE POINT AND

REFERENCE POINT SHEET

SHEET 1 OF 1

DESCRIPTION

5/8" Rebar w/ Pink MoDOT Cap Located 0.16 Miles North of Bridge S0712 on West Side of Rte. C and East Side of CR $445\,$

5/8" Rebar w/ Pink MoDOT Cap Located on Shoulder on NW Corner of Bridge S0712

5/8" Rebar w/ Pink MoDOT Cap Located on Shoulder on SE Corner of Bridge S0712

5/8" Rebar w/ Pink MoDOT Cap Located 0.26 Miles South of Bridge S0712 on East side of Rte. C and South Side of Field Road Entrance

POR

PΙ

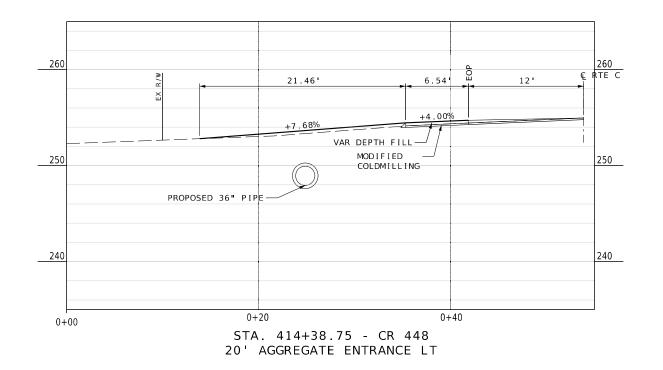
PΙ

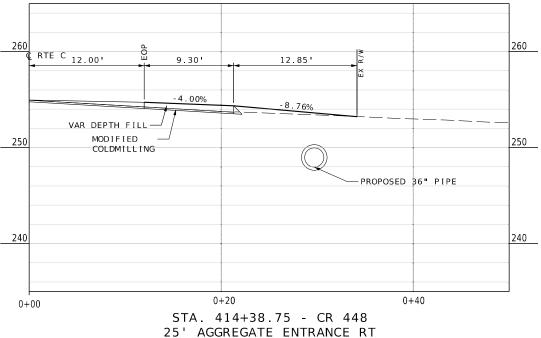
POE

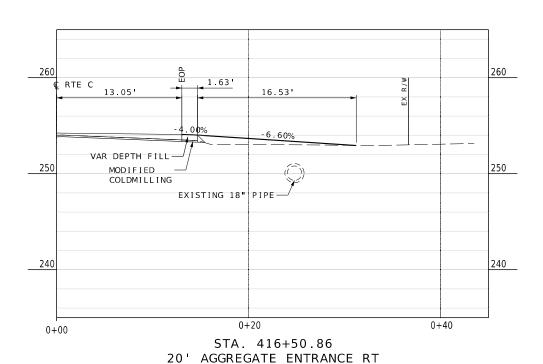
POB

PΙ

POE

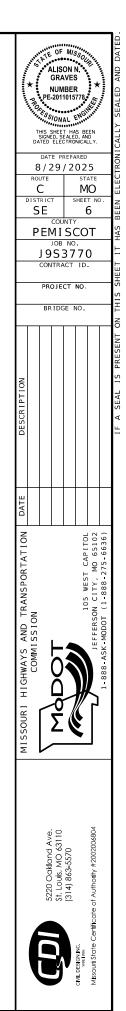


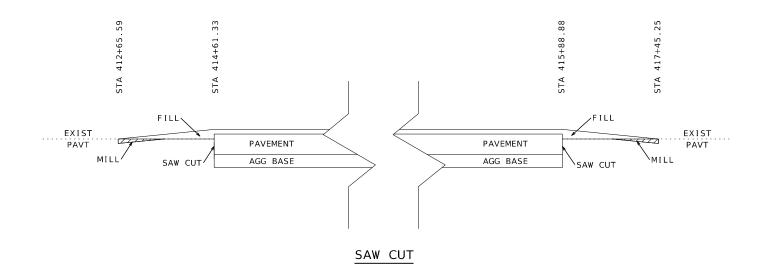


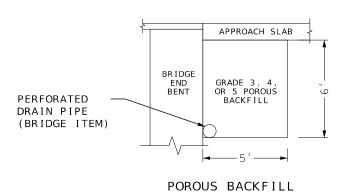


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SPECIAL SHEET DRIVEWAY PROFILES SHEET 1 OF 2



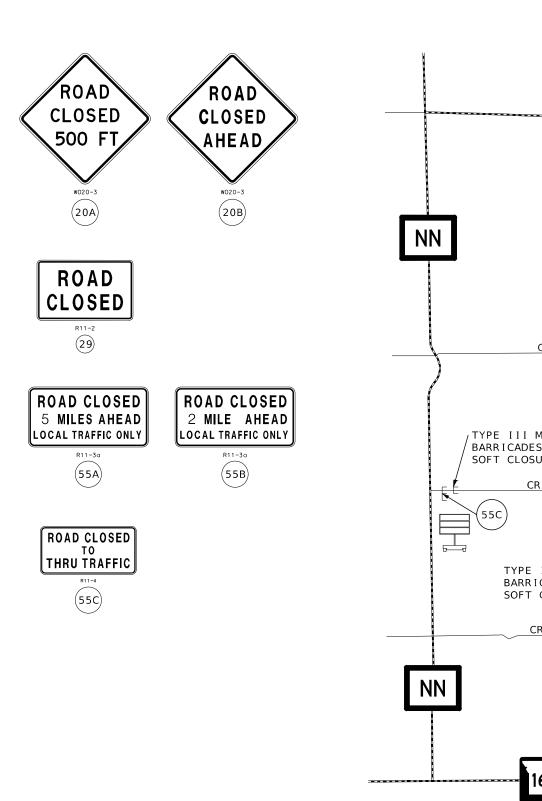




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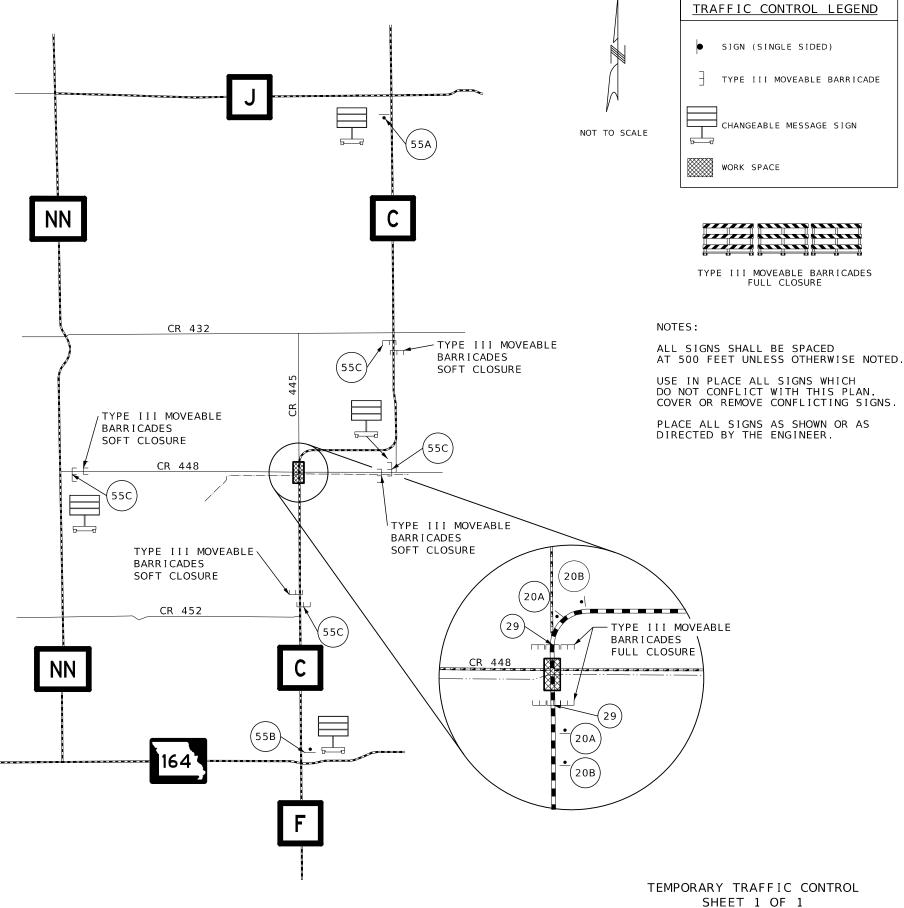
SPECIAL SHEETS
POROUS BACKFILL DETAIL
SHEET 2 OF 2





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SIGNED, SEALED, AND DATED ELECTRONICALL DATE PREPARED

8/29/2025
ROUTE STATE
C MO
DISTRICT SHEET NO
SE 8

PEMISCOT

J9S3770 CONTRACT ID.

PROJECT NO.

BRIDGE NO.

DESCRIPTION

LIGHWAYS AND TRANSPORTATION
COMMISSION

DOT

105 WEST CAPITOL
JEFFERSON CITY, MO 65102

MISSOURI HIGHWAYS.

COMMI

5220 Oakland Ave. St. Louis, MO 63110 (314) 863-5570

CAL DESIGNING.

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 $\langle A \rangle$

88

TEMPORARY EROSION CONTROL LEGEND

TEMPORARY BERM TYPE B

TEMPORARY BERM TYPE C

TEMPORARY SEDIMENT BASIN

PERMANENT SEDIMENT BASIN

SILT FENCE

ALTERNATE DITCH CHECK

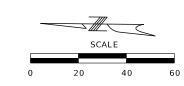
CURB INLET CHECK

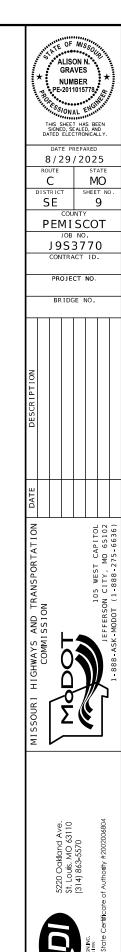
ROCK DITCH CHECK

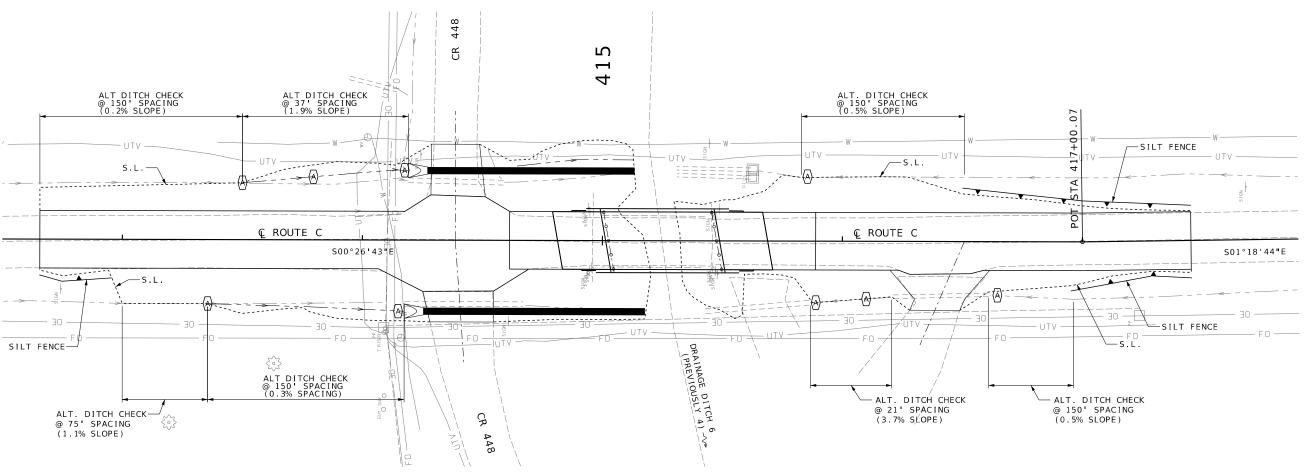
ENERGY DISSIPATOR

SEDIMENT TRAP

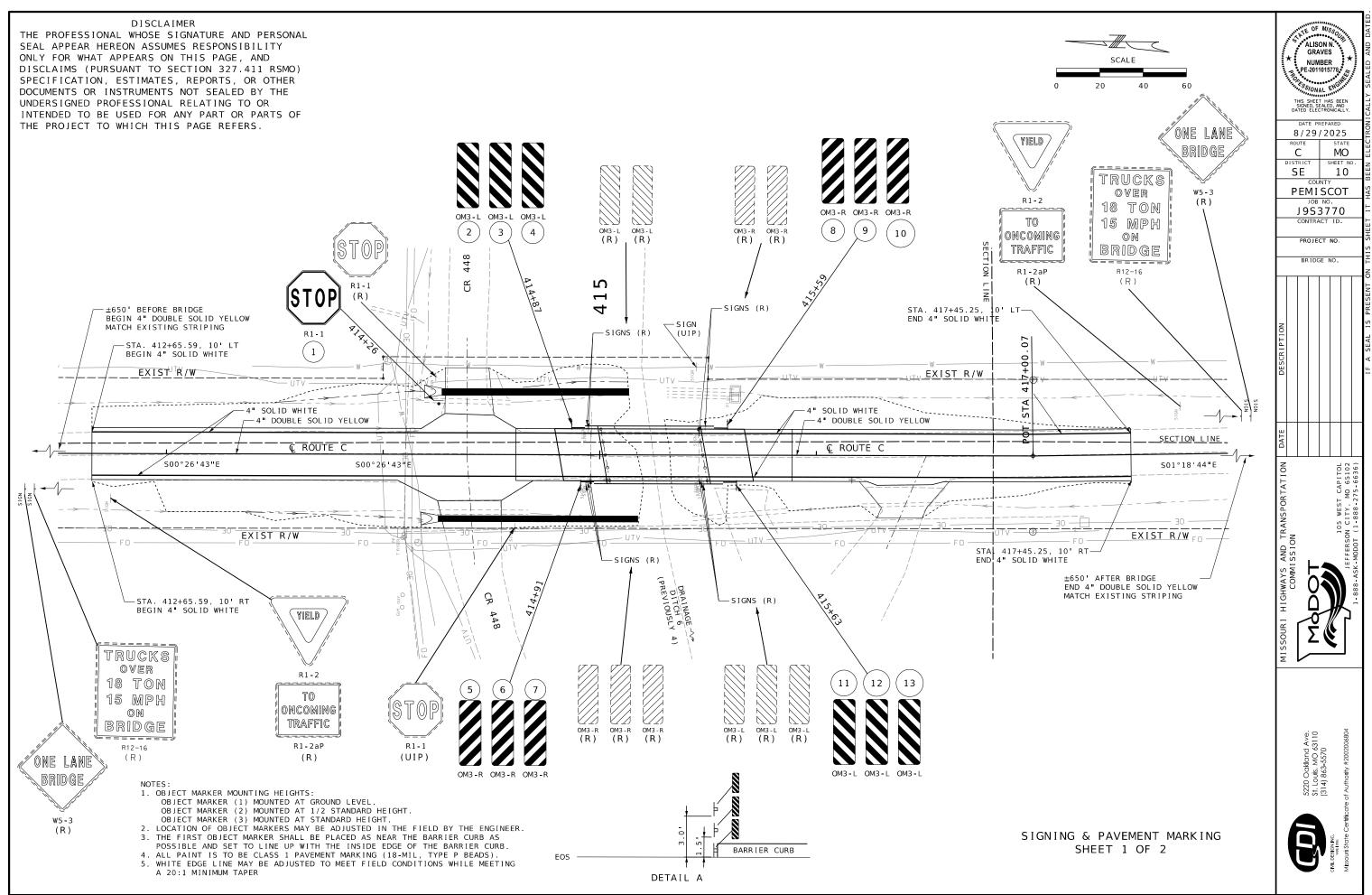
ROCK BLANKET







EROSION CONTROL SHEET SHEET 1 OF 1



		SIGN	S			CONCRETE	S	TRUC	CTUF	RAL S	TEEL	F	PIPE P	OST	S*		BACI	K I NG		U-				PE	RFORA	TED S	QUAF	RE S	STEEL	TUBE				EFFECTIVE: 07-01	-2024		
						FOOTINGS	5			STS*							BAR	S**		CHANNEL				NCH P					2.5	- I NCH	POST		BREAK -			, retrest 1 E O	F MISSO
902	SIGNAL SI	GNS TABULA			Т	EMBEDDED)													POST			TOTAL		ANCHORS		」			2.25"		HORS	AWAY			ALI:	ISON N. RAVES
			HOR I Z								TOTA				TOTAL						POST				1	CONCRET				1	1	CONCRETE	ASSEMBL'				
SIG		STATION			SIGN	١		1 1		POST LB	1		POST POS			1	X 3/8				NO.1	NO . 2		12 GA.		7 GA.	1			(6 FT)	1	7 GA.		AND		\$ 0 DE 20	JMBER 7
NO.	SIZE	OR	ΙF	LOCATIO	1				NO. 2				NO.1 NO.				2.55			ITEM NO.					ITEM NO.	ITEM NO.				1	ITEM NO.			OTHER REQUIRED	ITEMS		
		LOG MILE			SHT		NO.	\vdash			903121				9031220					9031250A		-			9031273A	9031274	1	-			9031281A	9031285				O,ESS/01	NAL ENGLINA
<u> </u>			STD		NO.	CY		LF	LF	LF	LBS	IN	LF LF		LBS	EACH	IN.	LF	LBS	LF	LF	LF	LF	EACH	EACH	EACH	LF	LF	LF	EACH	EACH	EACH	EACH			*****	*******
1	36X36	414+26		LT	-											-					11.0	1	11.0	1							-					SIGNED. DATED ELI	EET HAS BEEN , SEALED, AND LECTRONICALLY.
2	12X36	414+87		LT	_											1					10.6	1	10.6	1							1					DATE	PREPARED
3	12X36	414+87		LT	-																10.9	1	10.9	1													9/2025
4	12X36	414+87		LT																	11.2		11.2	1			_									ROUTE	STATE
5	12X36	414+91		RT	-		+	\vdash								+	\vdash	-+			10.5		10.5	1			+				-					_ C	MO
6	12X36	414+91		RT	+		_	\vdash								+	\vdash	-	-+		10.9	-	10.9	1			+				-					DISTRICT	SHEET NO
<u>'</u>	12X36	414+91		RT LT	+		-									+			-+		11.3		11.3	1							-					SE	11
8	12X36 12X36	415+59 415+59		LT			_									-		-			10.8	1	10.8	1							-						I SCOT
10	12X36	415+59		LT	_		_	\vdash						_		_					10.5		10.5	1			+										B NO.
10	12X36	415+63		RT	_		_	\vdash						_		_					9.8	1	9.8	1			+										3770
1.2	12X36	415+63		RT	+		+	\vdash								+	\vdash	-+	-+		9.6	\vdash	9.6	1			+				-						RACT ID.
12	12X36	415+63		RT	+		+	\vdash								+		-+	-+		9.5	-	9.5	1			+				+					1	
13	12/30	413+03		N I	+		+	\vdash								+		-+	-+		9.5	-	9.5	1			+				+					PROJ	IECT NO.
					+		_									+															 						
-				SUBT			 	\bowtie		XXX		XXXX		XXXX		 		XXXX	-		XXXX		137 0	13			×××							+		BRID	DGE NO.
							- XXX	$\times\!\!\times\!\!\times\!\!\times$	$\times\!\!\times\!\!\times\!\!\times$		⊗	–‱		$\times\!\!\times\!\!\times\!\!\times$				XXX I-			\ggg	$\times\!\!\times\!\!\!\times$	137.0 137	1.5				$\times\!\!\times\!\!\times$						4		\vdash	
					OTAL			$\times\!\!\times\!\!\times$			<u> </u>			$\times\!\!\times\!\!\times$	TID ALL C				**		\bowtie	$\times\!\!\times\!\!\times\!\!$	137	13				$\times\!\!\times\!\!\times$						_			
													AL FOR S						015.																		

**BACKING BARS ARE TOTALED WITH STRUCTURAL STEEL OR PIPE POSTS.

ROUN	ID PIP	E POST	AND FOO	TING	DATA 1	ABLE
NOM.SIZE	WE I	GHT	STUB	FO	OT I NG	CONCRETE
(IN.)	LBS/FT	LBS/IN	LENGTH	DIA.	DEPTH	C.Y.
21/2	5.79	0.48	4 3½	12"	4~6 "	0.13
4	10 70	0.90	5 31/	18	5~6"	0.36

		ST	RUCTU	RAL ST	TEEL	POST	AND F	OOTIN	G DA	TA TABI	_E					
		POST				FOOTING										
POST DES.	NOM.	WEI	GHT	STUB	DIA.	LEVEL (GROUND	6:1 G	RADE	4:1 GR	ADE	3:1 OR 2	:1 GRADE			
NO.	SIZE	LBS/FT	LBS/IN	LENGTH		DEPTH	C.Y.	DEPTH	C.Y.	DEPTH	C.Y.	DEPTH	C.Y.			
1	W6	9.0	0.75	3 0	15"	3'-0"	0.14	3'-2"	0.15	3'-3"	0.16	3'-6"	0.17			
2	W6	15.0	1.25	4 0	24"	4'-0"	0.47	4'-2"	0.50	4 - 3	0.51	4'-6"	0.54			
3	W8	18.0	1.50	4 6	28"	4'-6"	0.71	4'-8"	0.73	4 - 9	0.74	5'-0"	0.78			
4	W10	22.0	1.83	5 0	36"	5'-0"	1.31	5'-2"	1.36	5'-3"	1.39	5'-6"	1.45			
5	W10	26.0	2.17	5 0	36"	5'-0"	1.31	5'-3"	1.37	5 - 5	1.43	5'-9"	1.52			
6	W12	35.0	2.92	5 6	36"	5'-6"	1.44	5'-9"	1.52	5 - 11"	1.56	6'-3"	1.65			

STANDARD SIGN ASSEMBLIES SIGN, TYPE, DESIGNATION, SIZE & NUMBER OF EACH STATION OR LOCATION NO. LOG MILE (STOP) 1 415+63 2 414+87 LT 414+87 4 414+87 LT 5 414+91 RT 6 414+91 7 414+91 RT 8 415+59 9 415+59 LT 10 415+59 11 415+63 RT 12 415+63 RT 13 415+63 RT TOTALS 1

			SIGN	SUMMARY		EFFECTIV	E: 10-01-2016
					TYPE & S	QUARE FEE	Т
	SIGN	ı			FLAT SHEET		STRUCTURAL
STANDARD OR SPECIAL	DTL.	NO.	SIGN	FLAT SHEET	FLUORESCENT	STRUCTURAL	FLUORESCENT
SIGN DESIGNATION	SHT.	EACH	SIZE	SH	SHF*	ST	STF*
	NO.			ITEM NO.	ITEM NO.	ITEM NO.	ITEM NO.
				9035004A	9035069A	9035011A	9035071A
R1-1		1	36X36		9.0		
OM3 - R		6	12X36		18.0		
OM3 - L		6	12X36		18.0		
	İ						
			TOTALS		45.0		
		,		•	*ORANGE YI	TIOW AND Y	ELLOW CDEEN

*ORANGE, YELLOW AND YELLOW-GREEN

SIGNING & PAVEMENT MARKING SHEET 2 OF 2

D-30

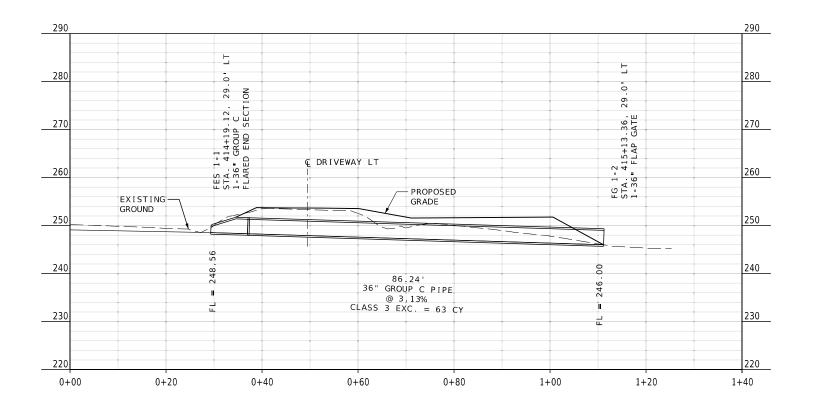
D-29

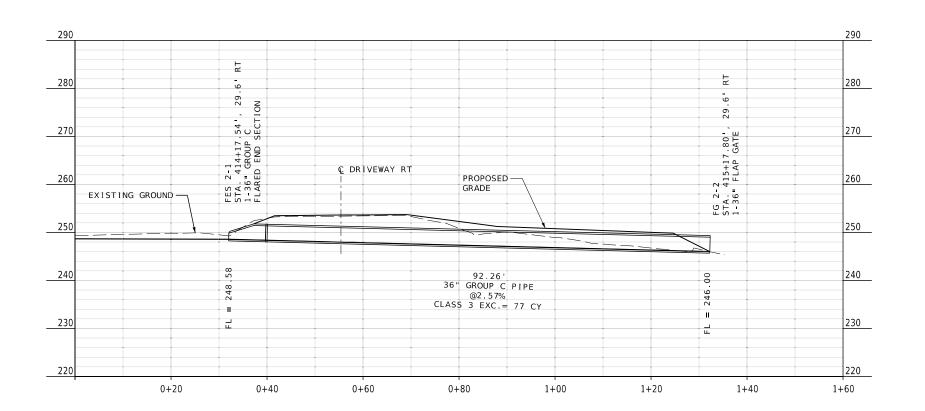
SPECIFICATION, ESTIMATES, REPORTS, OR OTHER DOCUMENTS OR INSTRUMENTS NOT SEALED BY THE UNDERSIGNED PROFESSIONAL RELATING TO OR INTENDED TO BE USED FOR ANY PART OR PARTS OF

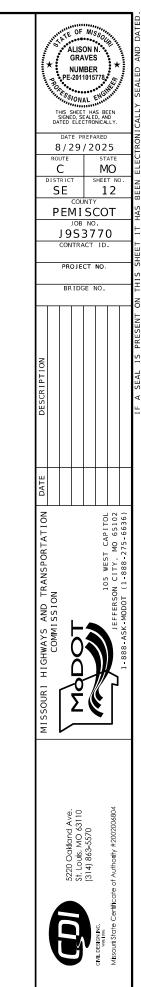
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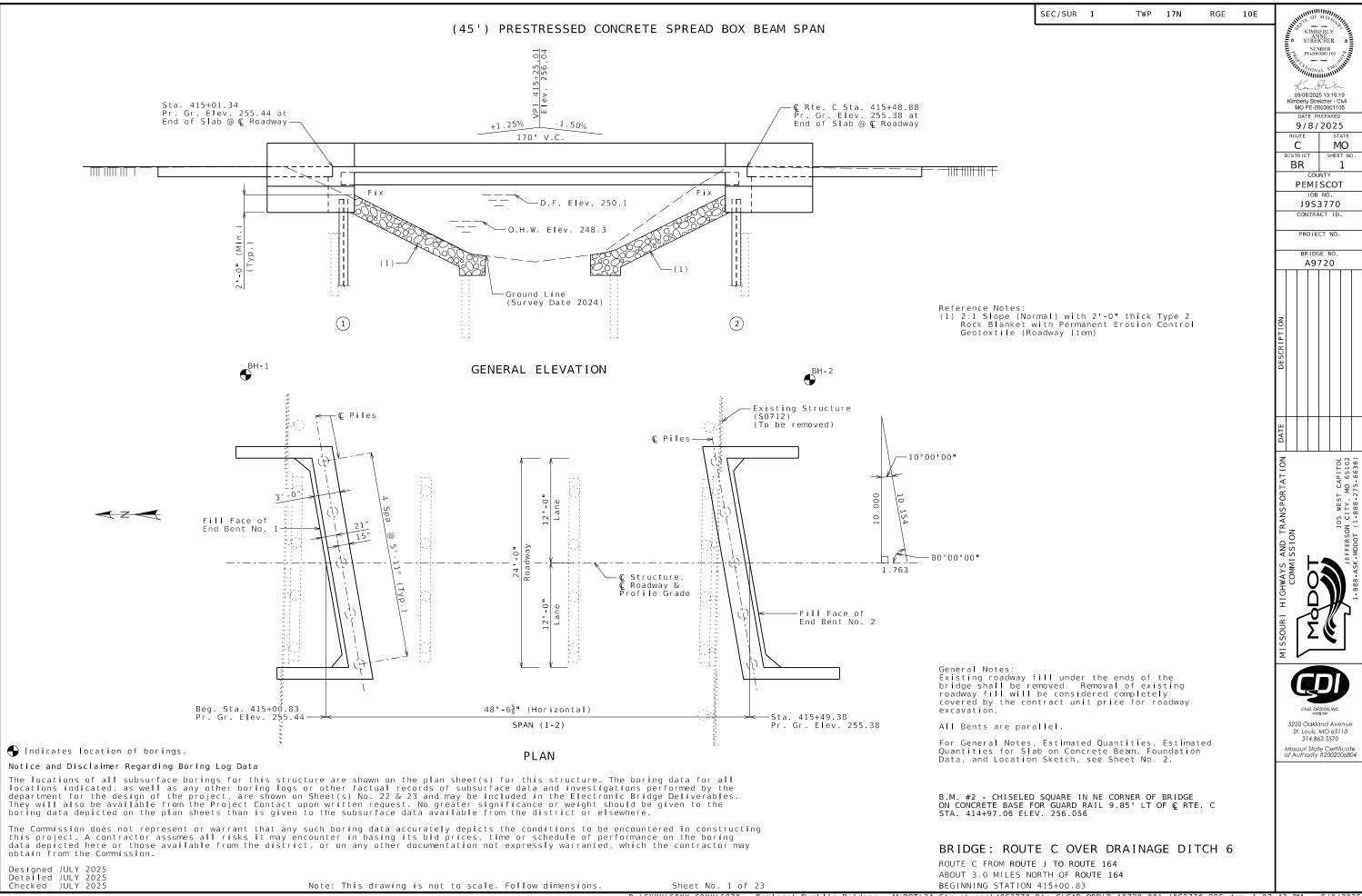
P:\6XXX\68XX-69XX\6870 - Pemiscot Dunklin Bridges - MoDOT\28-Trans\CAD\J9S3770_RTE C_S0712\011_SN_02_J9S3770_I1.dgn 11:45:32 AM 8/29/2025







CULVERT SECTIONS
SHEET 1 OF 1



Estimated Q	uantities			
I t em		Substr.	Superstr.	Total
Class 1 Excavation	cu. yard	60		60
Removal of Bridges (S0712)	lump sum			1
Bridge Approach Slab (Minor)	sq. yard		108	108
Galvanized Cast-In-Place Concrete Piles (14 in.)	linear foot	500		500
Dynamic Pile Testing	each	2		2
Class B Concrete (Substructure)	cu. yard	22.2		22.2
Type H Barrier	linear foot		121	121
Slab on Concrete Beam	sq. yard		141	141
21 in., Prestressed Concrete Spread Box Beam	linear foot		137	137
Slab Drain	each		8	8
Vertical Drain at End Bents	each			2
Plain Neoprene Bearing Pad	each		6	6

All concrete above the construction joint in the end bents is included in the Estimated Quantities for Slab on Concrete Beam.

All reinforcement in the end bents and cast-in-place pile is included in the Estimated Quantities for Slab on Concrete Beam.

Estimated Quantities for Slab on Concrete Beam	
I t em	Total
Class B-2 Concrete cu. yard	44.4
Reinforcing Steel (Epoxy Coated) pound	13,150

The table of Estimated Quantities for Slab on Concrete Beam represents the quantities used by the State in preparing the cost estimate for concrete slabs. The area of the concrete slab will be measured to the nearest square yard longitudinally from end of slab to end of slab and transversely from out to out of bridge slab (or with the horizontal dimensions as shown on the plan of slab). Payment for prestressed panels, stay-in-place corrugated steel forms, conventional forms, all concrete and epoxy coated reinforcing steel will be considered completely covered by the contract unit price for the slab. Variations may be encountered in the estimated quantities but the variations cannot be used for an adjustment in the contract unit price.

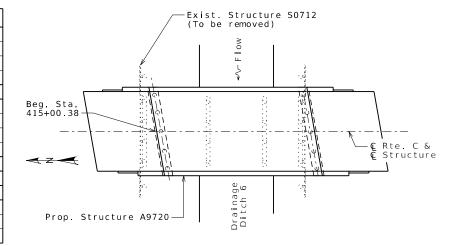
Method of forming the slab shall be as shown on the plans and in accordance with Sec 703. All hardware for forming the slab to be left in place as a permanent part of the structure shall be coated in accordance with ASTM A123 or ASTM B633 with a thickness class SC 4 and a finish type I, II or III.

Class B-2 Concrete quantity is based on minimum top flange thickness and minimum joint material thickness.

The Estimated Quantities for Slab on Concrete Beam are based on skewed precast prestressed panels.

The prestressed panel quantities are not included in the table of Estimated Quantities for Slab on Concrete Beam.

Detailed JULY 2025



LOCATION SKETCH

Note: This drawing is not to scale. Follow dimensions

General Notes:

Design Specifications:

2020 AASHTO LRFD Bridge Design Specification (9th Ed.) 2023 AASHTO Guide Specifications for LRFD Seismic Bridge Design (3rd Ed.) Seismic Design Category D (Seismic Details) Design earthquake response spectral acceleration coefficient at 1.0 second period, $S_{01}=1.062$ Acceleration Coefficient (effective peak ground acceleration coefficient), $A_{\rm S}=0.948$

Design Loading:

Vehicular = HL-93
Future Wearing Surface = 35 lb/sf
Earth = 120 lb/cf
Equivalent Fluid Pressure = 45 lb/cf (Min.)
Superstructure: Simply-Supported, Non-Composite for dead load.
Composite for live load.

Design Unit Stresses:

Class B Concrete (Substructure)

Class B-2 Concrete (Superstructure, except Prestressed Beams and Barrier)

Class B-1 Concrete (Barrier)

Reinforcing Steel (ASTM A706 Grade 60)

Welded or Seamless steel shell (pipe) for CIP pile (ASTM A252 Modified Grade 3) fy = 50,000 psi

For prestressed box beam stresses, see Sheet No. 11.

Neoprene Pads:

Neoprene bearing pads shall be 60 durometer and shall be in accordance with Sec 716.

Joint Filler:

All joint filler shall be in accordance with Sec 1057 for preformed sponge rubber expansion and partition joint filler, except as noted.

Reinforcing Steel:

Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise shown.

Traffic Handling:

Structure to be closed during construction. Traffic to be maintained on other routes during construction See roadway plans for traffic control.

 ${\tt MoDOT}$ Construction personnel will indicate the type of joint filler option used under the precast panels for this structure:

Constant Joint Filler

── Variable Joint Filler

	Foundation Da	ata		
			Bent I	Numb e r
Туре	Design Data		1	2
	Pile Type and Size		CECIP 14"	CECIP 14"
	Numb e r	ea	5	5
	Approximate Length Per Each	ft	50	50
	Pile Point Reinforcement	e a	=	-
	Min. Galvanized Penetration (Elev.)	ft	Full Length	Full Length
Load	Est. Max. Scour Depth 100 (Elev.)	ft	-	-
Bearing Pile	Minimum Tip Penetration (Elev.)	ft	202	202
' ' ' '	Criteria for Min. Tip Penetration		Liquefaction	Liquefaction
	Pile Driving Verification Method		DT	DT
	Resistance Factor		0.65	0.65
	Minimum Nominal Axial Compressive Resistance	kip	208	208

DT = Dynamic Testing

CECIP = Closed Ended Cast-in-Place Concrete Pile

Load Bearing Pile:
Minimum Nominal Axial Compressive Resistance = Maximum Factored Loads
Resistance Factor

Estimated Maximum Scour Depth (Elevation) shown is for verifying Minimum Nominal Axial Compressive Resistance using dynamic testing only where pile resistance contribution above this elevation shall not be considered.

All piles shall be galvanized down to the minimum galvanized penetration (elevation).

The Contractor shall make every effort to achieve the minimum galvanized penetration (elevation) shown on the plans for all piles. Deviations in penetration less than 5 feet of the minimum will be considered acceptable provided the Contractor makes the necessary corrections to ensure the minimum penetration is achieved on subsequent piles.

GENERAL NOTES & QUANTITIES

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** STRECHER **

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** NUMBER

** PE-2003001105

** DATE PREPARED

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ROUTE

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DISTRICT

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2

COUNTY

CONTRACT ID.

PROJECT NO.

PEMISCOT

J9S3770

BRIDGE NO A9720

COMMISSION

COMMISSION

TO DOT

105 WEST CAPITOL

JEFFERSON CITY, MO 65102



GALVANIZED CLOSED ENDED CAST-IN-PLACE (CECIP) CONCRETE PILE

WITHOUT PILE POINT REINFORCEMENT

Α

Vertical Bars (Equally spaced)

and Pile Cap Footing

Bottom of Beam Cap or Pile Cap Footing—

Min. Galvanized

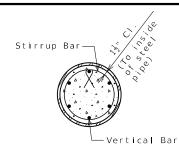
Penetration (Flev.) (See Foundation Data) — (See bent sheets)

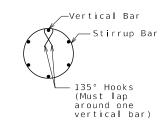
Welded or Seamless

Steel Pipe Cast-In-Place Pile

Nominal Wall Thickness

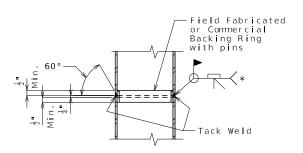
Closure Plate





SECTION A-A

DETAIL OF SEISMIC STIRRUP BAR



STEEL PIPE PILE SPLICE

* Galvanizing material shall be omitted or removed one inch clear of weld locations in accordance with Sec 702.

Galvanized Closed Ended Cast-In-	-Place (CECIP) Con	crete Pile Data
Bent Number	1	2
D1, CECIP Pile (O.D.)	14"	14"
Min. Nominal Wall Thickness	1/2"	1/2"
Closure Plate Thickness	3/4"	3/4"
Pile Point Reinforcement	none	none
Vertical Bars	6-#5-V103	6-#5-V203
L1, Length of Vertical Bars	5'-3"	5'-3"
Upper Stirrup Bars	3-#4-P100	3-#4-P200
Lower Stirrup Bars	5-#4-P100	5-#4-P200

Notes:

Welded or seamless steel shell (pipe) shall be ASTM A252 Modified Grade 3 (fy = 50,000 psi) with physical and chemical requirements that meet ASTM A572 Grade 50. Pipe certification and source material certification shall be required.

Concrete for cast-in-place pile shall be Class B-1.

Steel for closure plate shall be ASTM A709 Grade 50.

Steel for cruciform pile point reinforcement shall be ASTM A709

Steel casting for conical pile point reinforcement shall be ASTM A148 Grade 90-60.

The minimum wall thickness of any spot or local area of any type shall not be more than 12.5% under the specified nominal wall

The contractor shall determine the pile wall thickness required to avoid damage from all driving activities, but wall thickness shall not be less than the minimum specified. No additional payment will be made for furnishing a thicker pile wall than specified on the plans.

Closure plate shall not project beyond the outside diameter of the pipe pile. Satisfactory weldments may be made by beveling tip end of pipe or by use of inside backing rings. In either case, proper gaps shall be used to obtain weld penetration full thickness of pipe. Payment for furnishing and installing closure plate will be considered completely covered by the contract unit price for Galvanized Cast-In-Place Concrete Piles.

Splices of pipe for cast-in-place concrete pile shall be made watertight and to the full strength of the pipe above and below the splice to permit hard driving without damage. Pipe damaged during driving shall be replaced without cost to the state. Pipe sections used for splicing shall be at least 5 feet in

The hooks of vertical bars embedded in the beam cap should not be turned outward, away from the pile core.

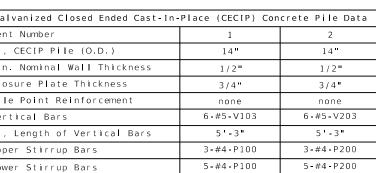
The hook of vertical bars embedded in the pile cap footing should be oriented outward for all seismic categories.

Closure plate need not be galvanized.

Reinforcing steel for cast-in-place piles is included in the Bill of Reinforcing Steel.

All reinforcement for cast-in-place pile at end bents is included in the Estimated Quantities for Slab on Concrete Beam.

For Foundation Data table, see Sheet No. 2.





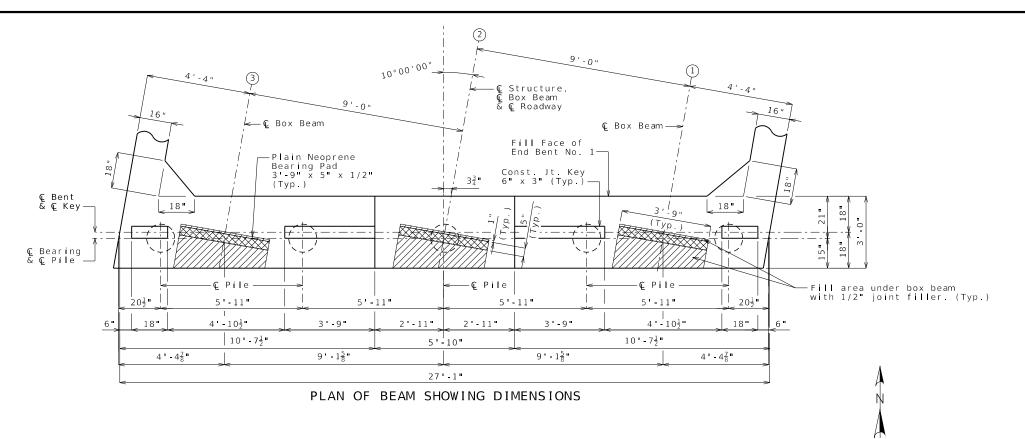
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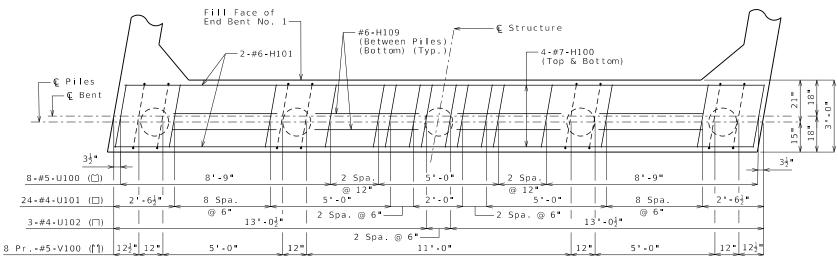
NUMBER PF-200300116



Missouri State Certificate of Authority #2002006804

GALVANIZED CLOSED ENDED CAST-IN-PLACE (CECIP) CONCRETE PILE



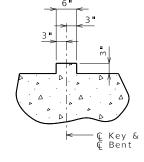


PLAN OF BEAM SHOWING REINFORCEMENT

(Keys not shown for clarity)

Substructure Quantity Table for Bent	No. 1
I t em	Quantity
Class 1 Excavation cu. yard	30
Galvanized Cast-in-Place Concrete Pile (14 in.) linear foo	t 250
Class B Concrete (Substructure) cu. yard	11.1

These quantities are included in the Estimated Quantities table on Sheet No. 2.



SECTION THRU KEY

Notes:

Work this sheet with Sheets No. 5 & 6.

Reinforcing steel shall be shifted to clear piles. U bars shall clear piles by at least 1 1/2 inches.

All U bars and pairs of V bars shall be placed parallel to centerline of roadway.

DETAILS OF END BENT NO. 1

Detailed JULY 2025 Checked JULY 2025

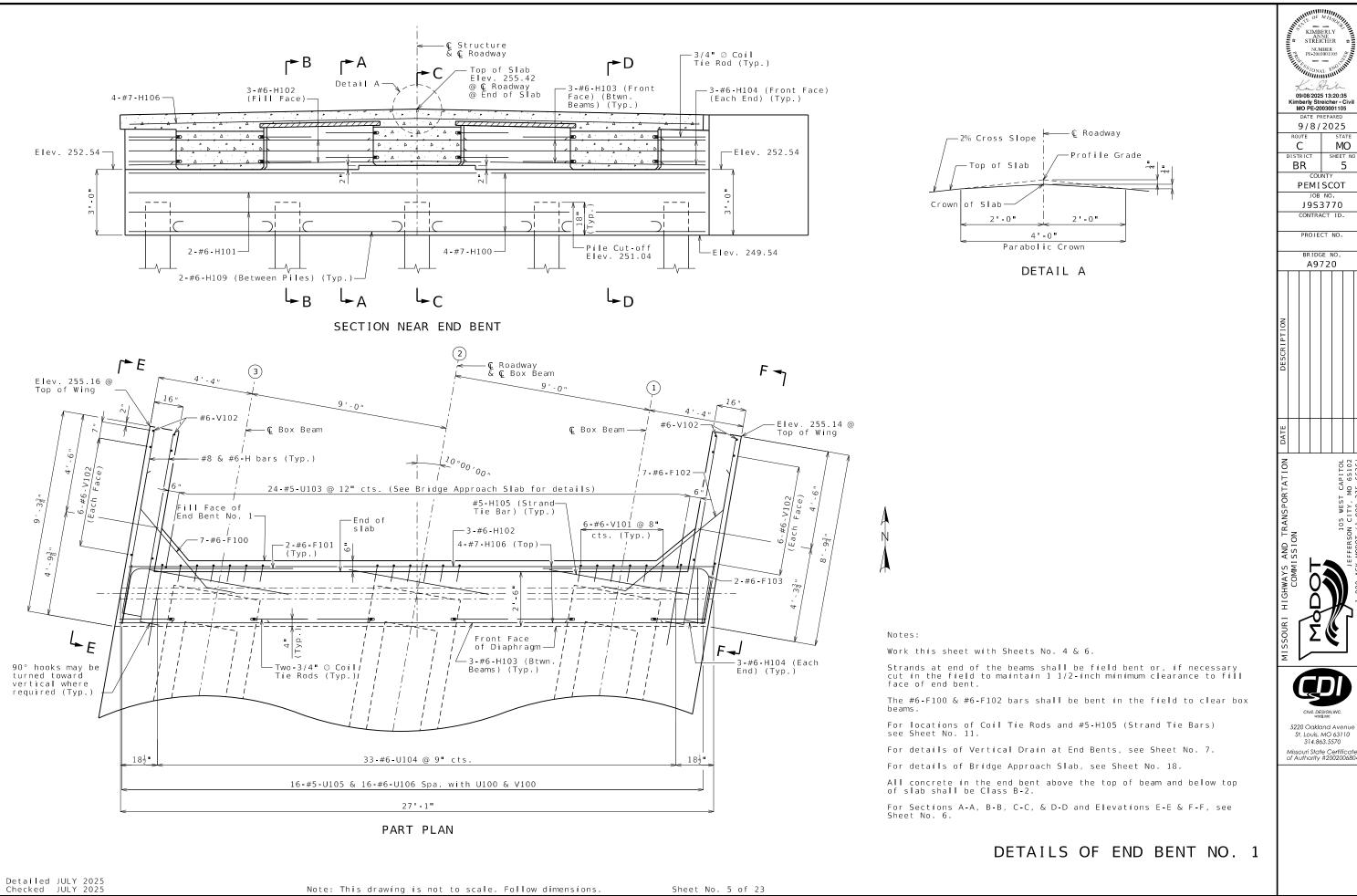
Note: This drawing is not to scale. Follow dimensions.

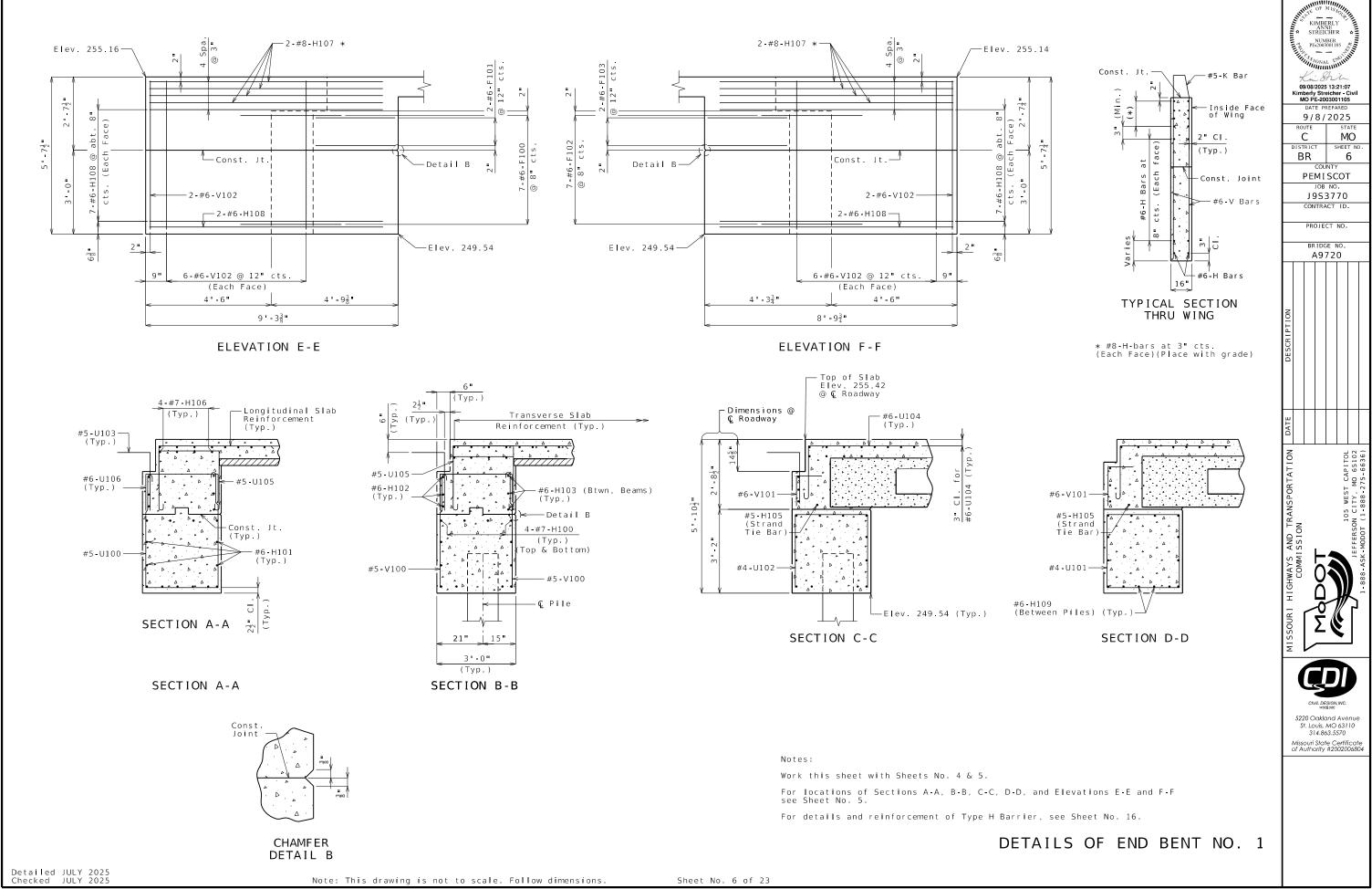
Sheet No. 4 of 23





Missouri State Certificate of Authority #2002006804





Σ

- Unperforated

Drain Pipe

ELEVATION OF WING

-Cut coupler flush

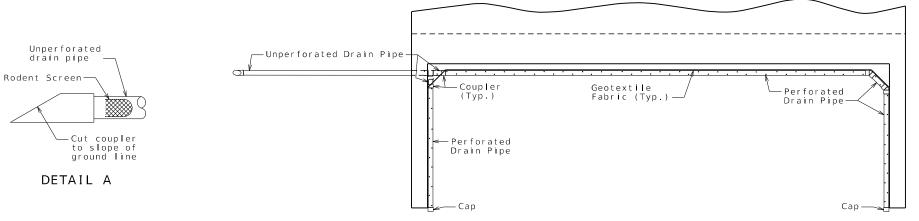
with ground line-

– Cap

Beam-

ELEVATION OF END BENT

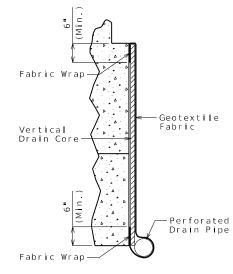
Perforated Drain Pipe



— Coupler

PLAN OF END BENT

(Squared end bent shown, skewed end bent similar)



NUMBER PE-200300110

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PEMISCOT

J9S3770

CONTRACT ID.

PROJECT NO.

A9720

5220 Oakland Avenue St. Louis, MO 63110 314.863.5570

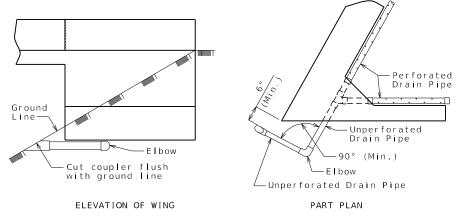
Missouri State Certificate of Authority #2002006804

MO SHEET NO 7

C.

BR

PART SECTION A-A (Section thru wing similar)



OPTIONAL TURNED DRAIN

(Use only when straight drain is not practical.)

General Notes:

All drain pipe shall be sloped 1 to 2

Drain pipe may be either 6-inch diameter corrugated metallic-coated steel pipe underdrain, 4-inch diameter corrugated polyvinyl chloride (PVC) drain pipe, or 4-inch diameter corrugated polyethylene (PE) drain pipe.

Drain pipe shall be placed at fill face of end bent and inside face of wings. The pipe shall slope to lowest grade of ground line, also missing the lower beam of end bent by a minimum of 1 1/2 inches.

Perforated pipe shall be placed at fill face side and inside face of wings at the bottom of end bent and plain pipe shall be used where the vertical drain ends to the exit at ground line.

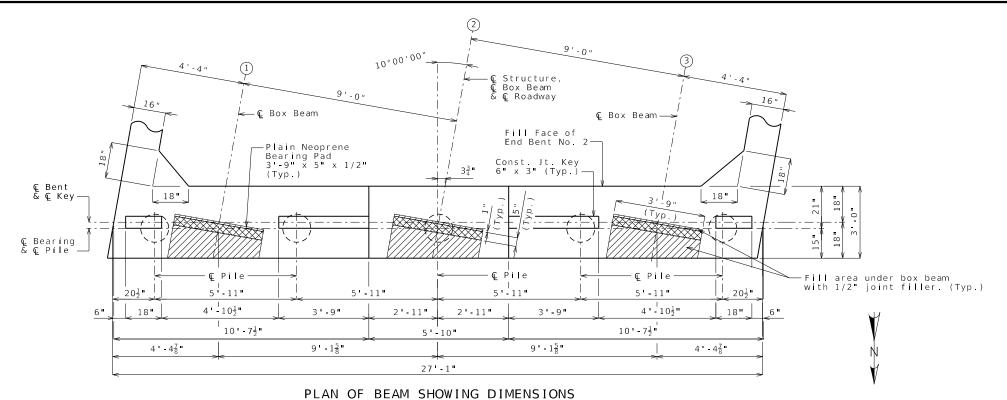
VERTICAL DRAIN AT END BENTS

percent.

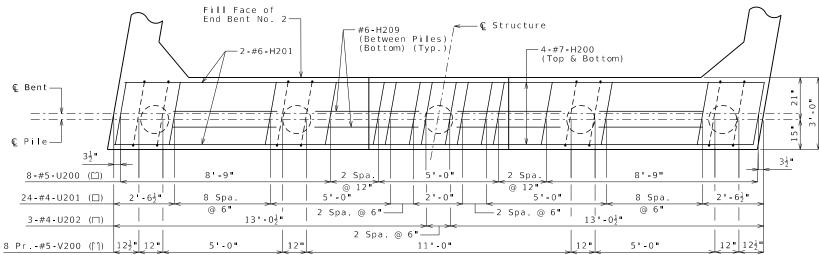
Detailed JULY 2025 Checked JULY 2025

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 7 of 23



FEAN OF BEAM SHOWING BIMENSTONS

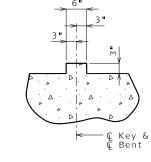


PLAN OF BEAM SHOWING REINFORCEMENT

(Keys not shown for clarity)

Substructure Quantity Table for Bent N	lo. 2
I t em	Quantity
Class 1 Excavation cu. yard	30
Galvanized Cast-in-Place Concrete Pile (14 in.) linear foot	250
Class B Concrete (Substructure) cu. yard	11.1

These quantities are included in the Estimated Quantities table on Sheet No. 2.



SECTION THRU KEY

Notes

Work this sheet with Sheets No. 9 & 10.

Reinforcing steel shall be shifted to clear piles. U bars shall clear piles by at least 1 1/2 inches.

All U bars and pairs of V bars shall be placed parallel to centerline of roadway.

DETAILS OF END BENT NO. 2

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BR 8
COUNTY
PEMI SCOT

JOB NO.
J9S3770
CONTRACT ID.

PROJECT NO.

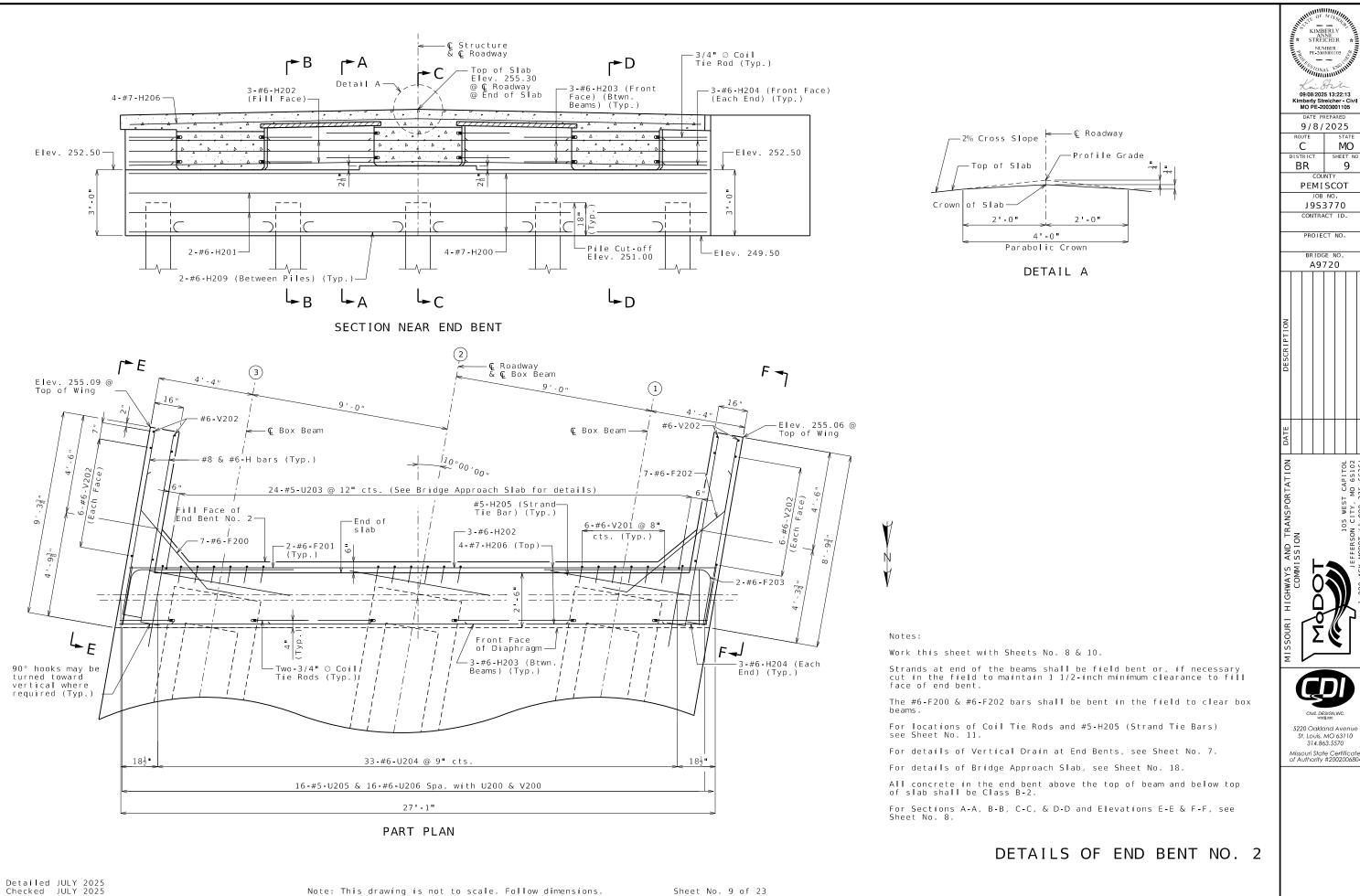
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NO 1141

NO 124

ISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION
TO TELEBREASON CITY, MO 65102

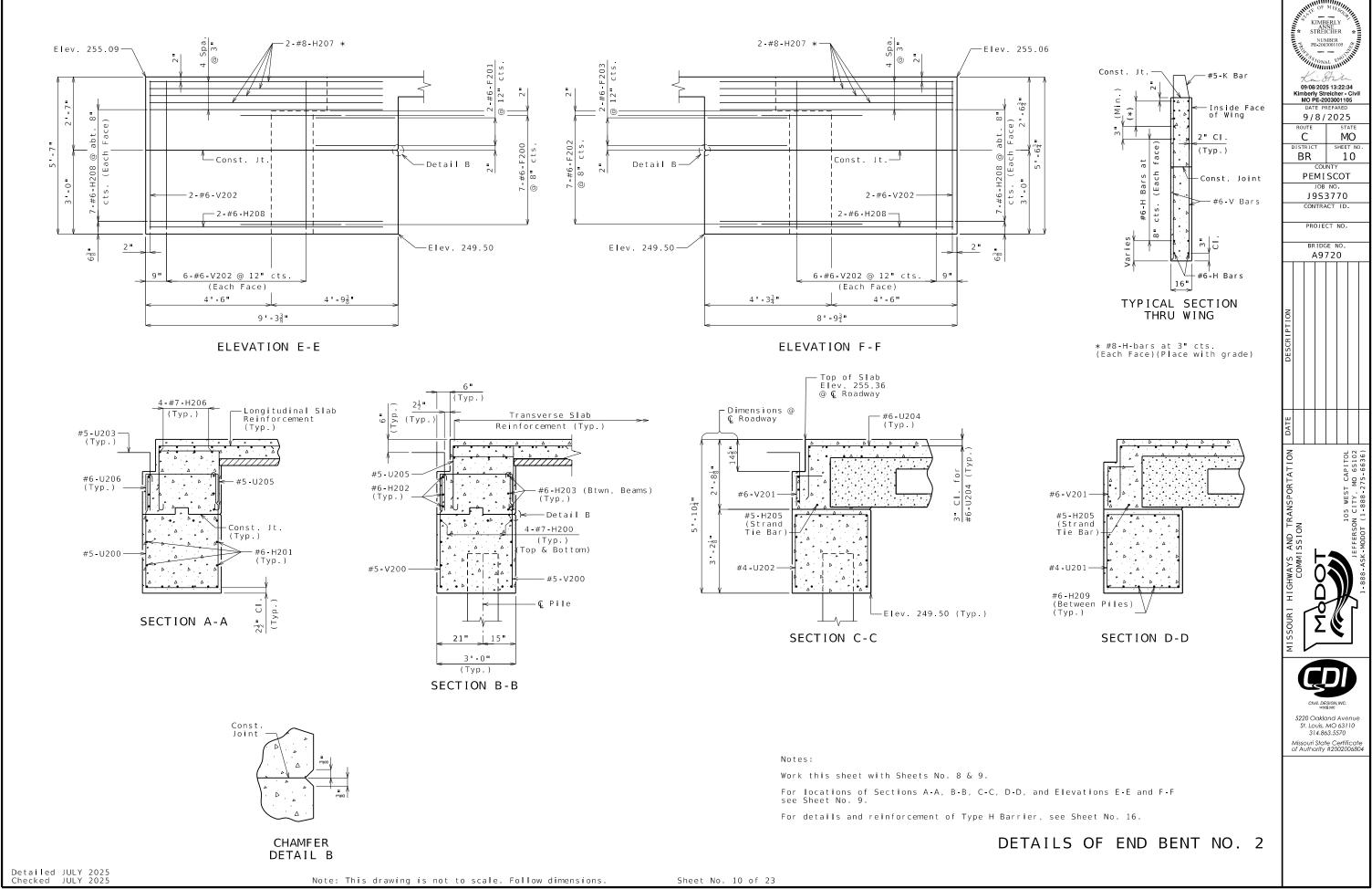


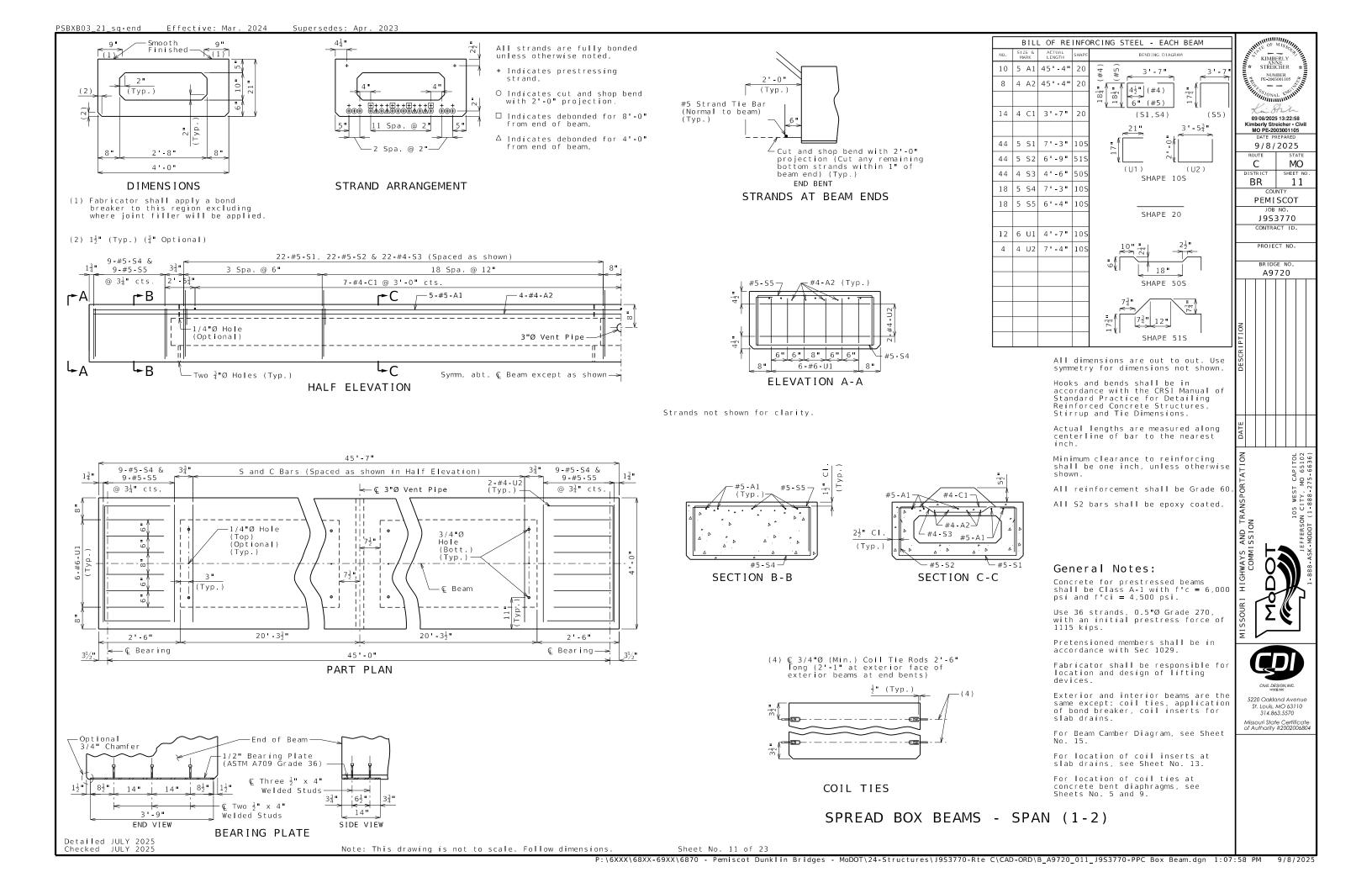


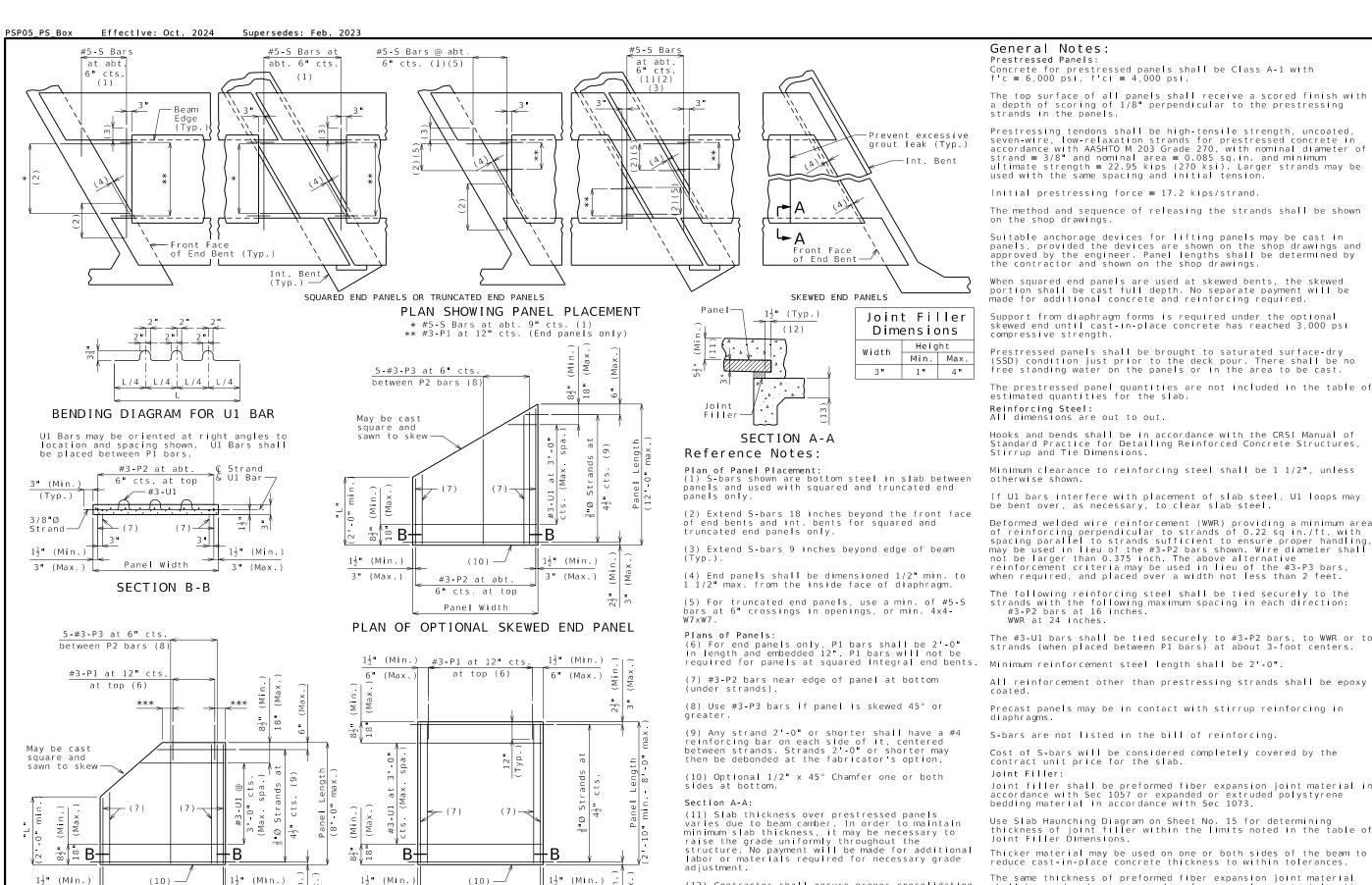
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SHEET NO

9







<u>.−</u> ∑ |

3 (Max.

#3-P2 at abt.

6" cts. at top

Panel Width

PLAN OF SOUARED PANEL

Note: This drawing is not to scale. Follow dimensions

3" (Max.)

#3-P2 at abt.

6" cts. at top

Panel Width

PLAN OF OPTIONAL TRUNCATED END PANEL

*** 3" (Min.), 6" (Max.)

3" (Max.)

Detailed JULY 2025

PRESTRESSED PANELS

M M

3" (Max.)

(12) Contractor shall ensure proper consolidation

(13) At the contractor's option, the variation in

slab thickness over prestressed panels may be eliminated or reduced by increasing and varying

the beam top flange thickness. Dimensions shall

under and between panels.

be shown on the shop drawings.

Concrete for prestressed panels shall be Class A-1 with

The top surface of all panels shall receive a scored finish with a depth of scoring of 1/8" perpendicular to the prestressing

Prestressing tendons shall be high-tensile strength, uncoated, seven-wire, low-relaxation strands for prestressed concrete in accordance with AASHTO M 203 Grade 270, with nominal diameter of strand = 3/8" and nominal area = 0.085 sq.in. and minimum ultimate strength = 22.95 kips (270 ksi). Larger strands may be used with the same spacing and initial tension.

The method and sequence of releasing the strands shall be shown

Suitable anchorage devices for lifting panels may be cast in panels, provided the devices are shown on the shop drawings and approved by the engineer. Panel lengths shall be determined by the contractor and shown on the shop drawings.

When squared end panels are used at skewed bents, the skewed portion shall be cast full depth. No separate payment will be made for additional concrete and reinforcing required.

Support from diaphragm forms is required under the optional skewed end until cast-in-place concrete has reached 3,000 psi

Prestressed panels shall be brought to saturated surface-dry (SSD) condition just prior to the deck pour. There shall be no free standing water on the panels or in the area to be cast.

The prestressed panel quantities are not included in the table of

Hooks and bends shall be in accordance with the CRSI Manual of Standard Practice for Detailing Reinforced Concrete Structures,

Minimum clearance to reinforcing steel shall be 1 1/2", unless

If U1 bars interfere with placement of slab steel, U1 loops may be bent over, as necessary, to clear slab steel.

Deformed welded wire reinforcement (WWR) providing a minimum area of reinforcing perpendicular to strands of 0.22 sq in /ft, with spacing parallel to strands sufficient to ensure proper handling, may be used in lieu of the #3-P2 bars shown. Wire diameter shall not be larger than 0.375 inch. The above alternative reinforcement criteria may be used in lieu of the #3-P3 bars,

The following reinforcing steel shall be tied securely to the strands with the following maximum spacing in each direction: #3-P2 bars at 16 inches.

The #3-U1 bars shall be tied securely to #3-P2 bars, to WWR or to strands (when placed between P1 bars) at about 3-foot centers.

Minimum reinforcement steel length shall be 2'-0".

All reinforcement other than prestressing strands shall be epoxy

Precast panels may be in contact with stirrup reinforcing in

S-bars are not listed in the bill of reinforcing.

Cost of S-bars will be considered completely covered by the

Joint filler shall be preformed fiber expansion joint material in accordance with Sec 1057 or expanded or extruded polystyrene bedding material in accordance with Sec 1073

Thicker material may be used on one or both sides of the beam to reduce cast-in-placé concrete thickness to within tolerances.

The same thickness of preformed fiber expansion joint material shall be used under any one edge of any panel except at locations where top flange thickness may be stepped. The maximum change in thickness between adjacent panels shall be 1/2 inch. polystyrene bedding material may be cut with a transition to match haunch height above top of flange.

Joint filler shall be glued to the beam. When thickness exceeds 1 1/2 inches, the joint filler shall be glued top and bottom. The glue used shall be the type recommended by the joint filler manufacturer.

Edges of panels shall be uniformly seated on the joint filler before slab reinforcement is placed.

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PEMISCOT

J9S3770 CONTRACT ID

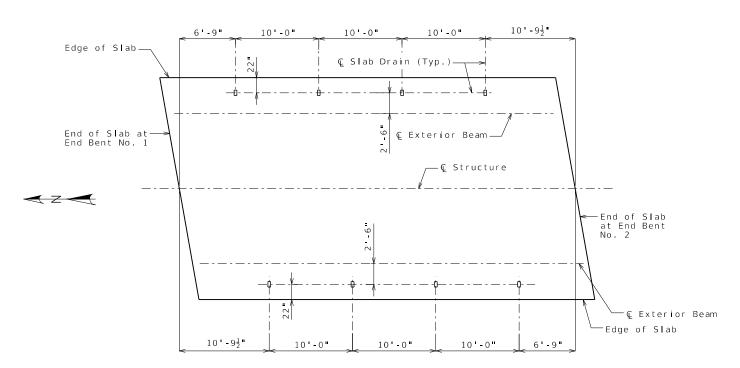
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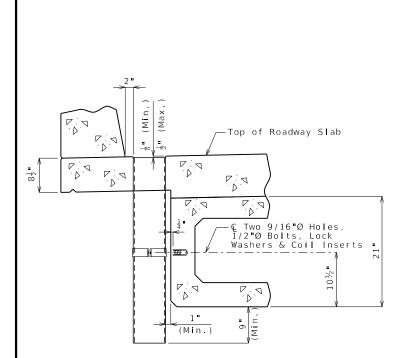


5220 Oakland Avenu St. Louis, MO 63110

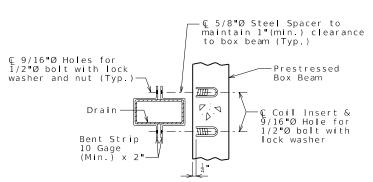
314.863.5570 Missouri State Certificate of Authority #2002006804



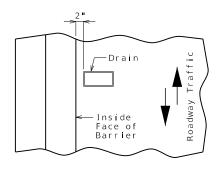
PLAN OF SLAB SHOWING SLAB DRAIN LOCATIONS



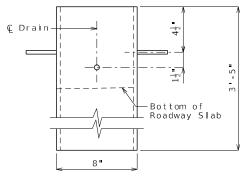
PART SECTION NEAR DRAIN



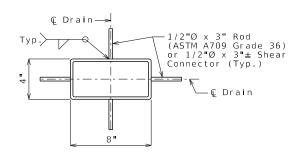
PART SECTION SHOWING BRACKET ASSEMBLY



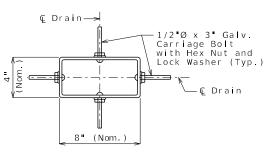
PART PLAN OF SLAB AT DRAIN



ELEVATION OF DRAIN



PLAN OF STEEL DRAIN OPTION



PLAN OF FRP DRAIN OPTION

General Notes:

Contractor shall have the option to construct either steel or FRP slab drains.

Slab drain bracket assembly shall be ASTM A709 Grade 36 steel

Locate drains in slab by dimensions shown in Part Section Near Drain.

Reinforcing steel shall be shifted to clear drains.

The coil inserts and bracket assembly shall be galvanized in accordance with ASTM A123.

All bolts, hardened washers, lock washers and nuts shall be galvanized in accordance with AASHTO M 232 (ASTM A153),

All 1/2 Ø bolts shall be ASTM A307.

Shop drawings will not be required for the slab drains and the bracket assembly.

The coil inserts required for the bracket assembly attachment shall be located on the prestressed beam shop drawings.

Coil inserts shall have a concrete pull-out strength (ultimate load) of at least 2,500 pounds in 5,000 psi concrete.

The bolts required to attach the slab drain bracket assembly to the prestressed beam shall be supplied by the prestressed

Notes for Steel Drain:

Slab drains may be fabricated of either 1/4" welded sheets of ASTM A709 Grade 36 steel or from 1/4" structural steel tubing ASTM A500 or A501

Outside dimensions of drains are 8" x 4".

The drains shall be galvanized in accordance with ASTM A123.

Notes for FRP Drain:

Drains shall be machine filament-wound thermosetting resin tubing meeting the requirements of ASTM D2996 with the following exceptions:

Shape of drains shall be rectangular with outside nominal dimensions of 8" x 4".

Minimum reinforced wall thickness shall be

The resin used shall be ultraviolet (UV) resistant and/or have UV inhibitors mixed throughout. Drains may have an exterior coating for additional UV resistance.

The color of the slab drain shall be gray (Federal Standard #26373). The color shall be uniform throughout the resin and any coating used.

The combination of materials used in the manufacture of the drains shall be tested for UV resistance in accordance with ASTM D4329 Cycle A. The representative material shall withstand at least 500 hours of testing with only minor discoloration and without any physical deterioration. The contractor shall furnish the results of the required ultraviolet testing prior to acceptance of the slab drains.

At the contractor's option, drains may be field cut. The method of cutting FRP slab drain shall be as recommended by the manufacturer to ensure a smooth, chip free



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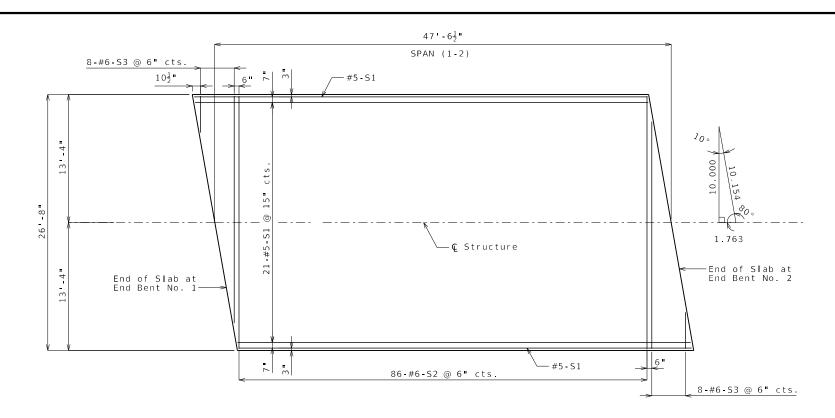
J9S3770 CONTRACT ID. PROJECT NO

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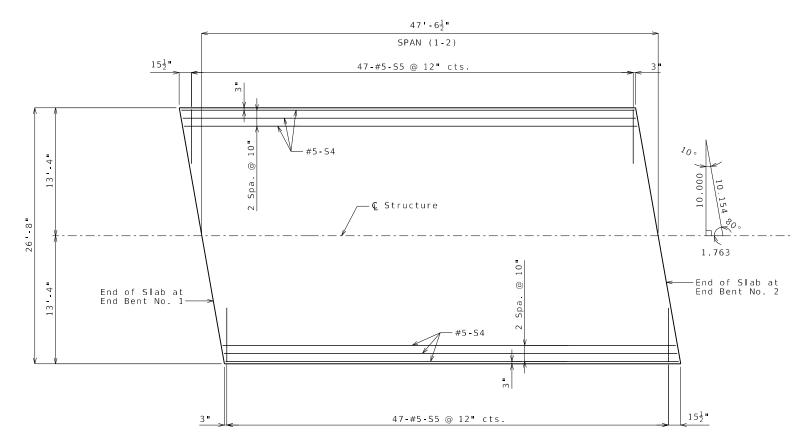
SLAB DRAINS



$\forall z \blacktriangleleft$

Detailed JULY 2025 Checked JULY 2025

PLAN OF SLAB SHOWING TOP REINFORCEMENT



PLAN OF SLAB SHOWING BOTTOM REINFORCEMENT

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 14 of 23

NUMBER PRODUCTION OF THE PROJECT NO.

BRIDGE NO. A9720

BRIDGE NO. A9720

MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION

MADOT

105 WEST CAPITOL

JEFFERSON CITY, MO 65102



St. Louis, MO 63110 314.863.5570 Missouri State Certificate of Authority #2002006804

Notes:

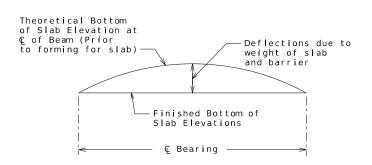
Longitudinal slab dimensions are measured horizontally.

For Section Thru Slab, Theoretical Slab Haunching Diagram, Theoretical Bottom of Slab Elevations and Box Beam Camber Diagram, See Sheet No. 15.

For details and reinforcement of Type H Barrier, see Sheets No. 16 & 17.

For details and locations of Slab Drains, see Sheet No. 13.

For details of Precast Prestressed Panels, see Sheet No. 12.

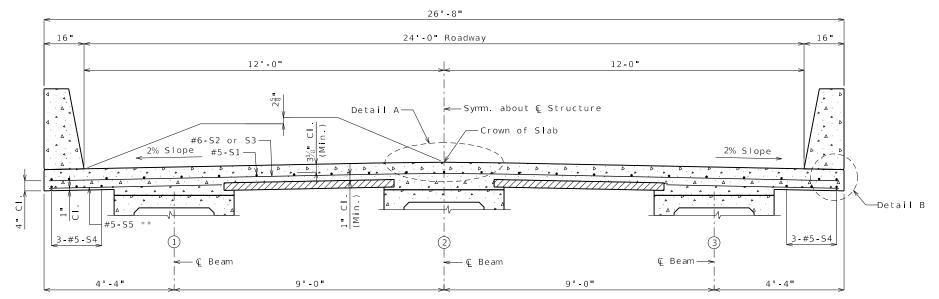


TYPICAL SLAB ELEVATIONS DIAGRAM

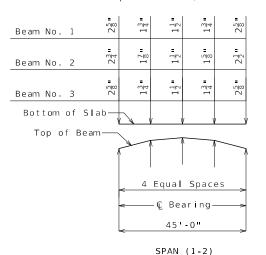
Theoretical Bottom of Slab Elevations at Centerline of Beam (Prior to forming for slab) (Estimated at 90 days)

Beam	Span ((1-2) (4	·5'-0" @	Brg C	Brg.)
Number	© Brg.	. 25	. 50	. 75	€ Brg.
1	254.55	254.61	254.63	254.59	254.51
2	254.73	254.79	254.80	254.76	254.68
3	254.55	254.61	254.63	254.59	254.51

Elevations are based on a constant slab thickness of 8 1/2" and include allowance for theoretical dead load deflections due to weight of slab (including precast panel) and barrier.



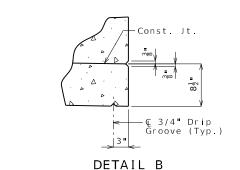
** Alternate bar shape available, see Sheet No. 16.



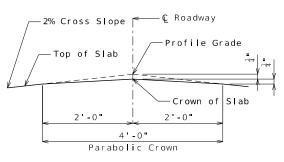
THEORETICAL SLAB HAUNCHING DIAGRAM (ESTIMATED AT 90 DAYS)

If beam camber is different from that shown in the camber diagram, in order to maintain minimum slab thickness, an adjustment of the slab haunches, an increase in slab thickness or a raise in grade uniformly throughout the structure shall be necessary. No payment will be made for additional labor or materials required for variation in haunching, slab thickness or grade adjustment.

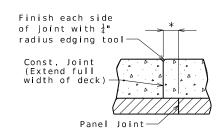
Concrete in the slab haunches is included in the Estimated Quantities for Slab on Concrete Beam.



SECTION THRU SLAB

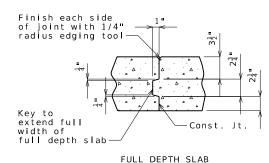




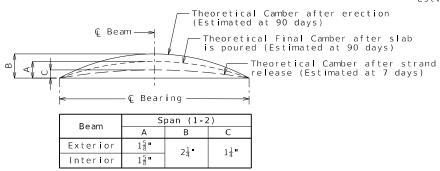


* Adjust the construction joint to a clearance of 6 inches minimum from the panel joint.

SLAB ON PANELS



SLAB CONSTRUCTION JOINT



BEAM CAMBER DIAGRAM

Conversion Factors for Beam Camber (Estimated at 90 days):

 $0.25 \text{ pt.} = 0.7125 \times 0.5 \text{ pt.}$

Contractor may shift or swap bars as needed to tie R3 bar in barrier (4" min. bar spacing)
bar as needed to tie R2 bar in barrier

OPTIONAL SHIFTING TOP BARS AT BARRIER

Notes:

For details and reinforcement of Type H Barrier, see Sheets No. 16 & 17. For Plan of Slab Showing Reinforcement, see Sheet No. 14.

For details of Precast Prestressed Panels, see Sheet No. 12.

SLAB DETAILS



09/08/2025 13:24:37 Kimberly Streicher - Civil MO PE-2003001105

9/8/2025

PEMISCOT

J9S3770

CONTRACT ID.

PROJECT NO.

A9720

MO

SHEET NO

15

C

BR

Detailed JULY 2025 Checked JULY 2025

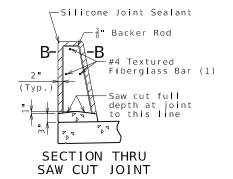
Note: This drawing is not to scale. Follow dimensions.

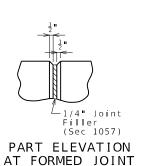
Sheet No. 15 of 23

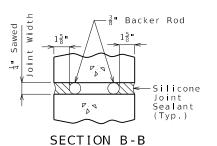
SPAN (1-2)

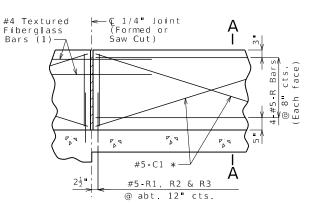
ELEVATION OF BARRIER

(Left barrier shown, right barrier similar) Longitudinal dimensions are horizontal.





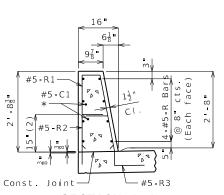




PART ELEVATION OF BARRIER

(1) Four feet long, centered on joint,

slip-formed option only

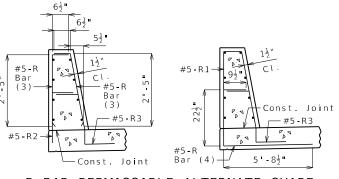


SECTION A-A

Use a minimum lap of 2'-6" for #5 horizontal barrier bars.

The cross-sectional area above the slab is 2.89 square feet.

(2) To top of bar



R-BAR PERMISSIBLE ALTERNATE SHAPE

- (3) The R1 bar may be separated into two bars as shown, at the contractor's option, only when slip forming is not used. (All dimensions are out to out.)
- (4) The R2 bar and #5 bottom transverse slab bar in cantilever (prestressed panels only) combination may be furnished as one bar as shown, at the contractor's option.

General Notes:

* Slip-formed option only.

Conventional forming or slip forming may be used. Saw cut joints may be used with conventional forming.

Top of barrier shall be built parallel to grade and barrier joints (except at end Ďents) normal to grade.

All exposed edges of barrier shall have either a 1/2-inch radius or a 3/8-inch bevel, unless otherwise noted.

Payment for all concrete and reinforcement, complete in place, will be considered completely covered by the contract unit price for Type H Barrier per linear foot.

Concrete in barrier shall be Class B-1.

Measurement of barrier is to the nearest linear foot for each structure, measured along the outside top of slab from end of wing to end of wing.

Concrete traffic barrier delineators shall be placed on top of the barrier as shown on Missouri Standard Plan 617.10 and in accordance with Sec 617. Delineators on bridges with two-lane, two-way traffic shall have retroreflective sheeting on both sides. Concrete traffic barrier delineators will be considered completely covered by the contract unit price for Type H Barrier.

Joint sealant and backer rods shall be in accordance with Sec 717 for silicone joint sealant for saw cut and formed joints.

For slip-formed option, both sides of barrier shall have a vertically broomed finish and the top shall have a transversely broomed finish.

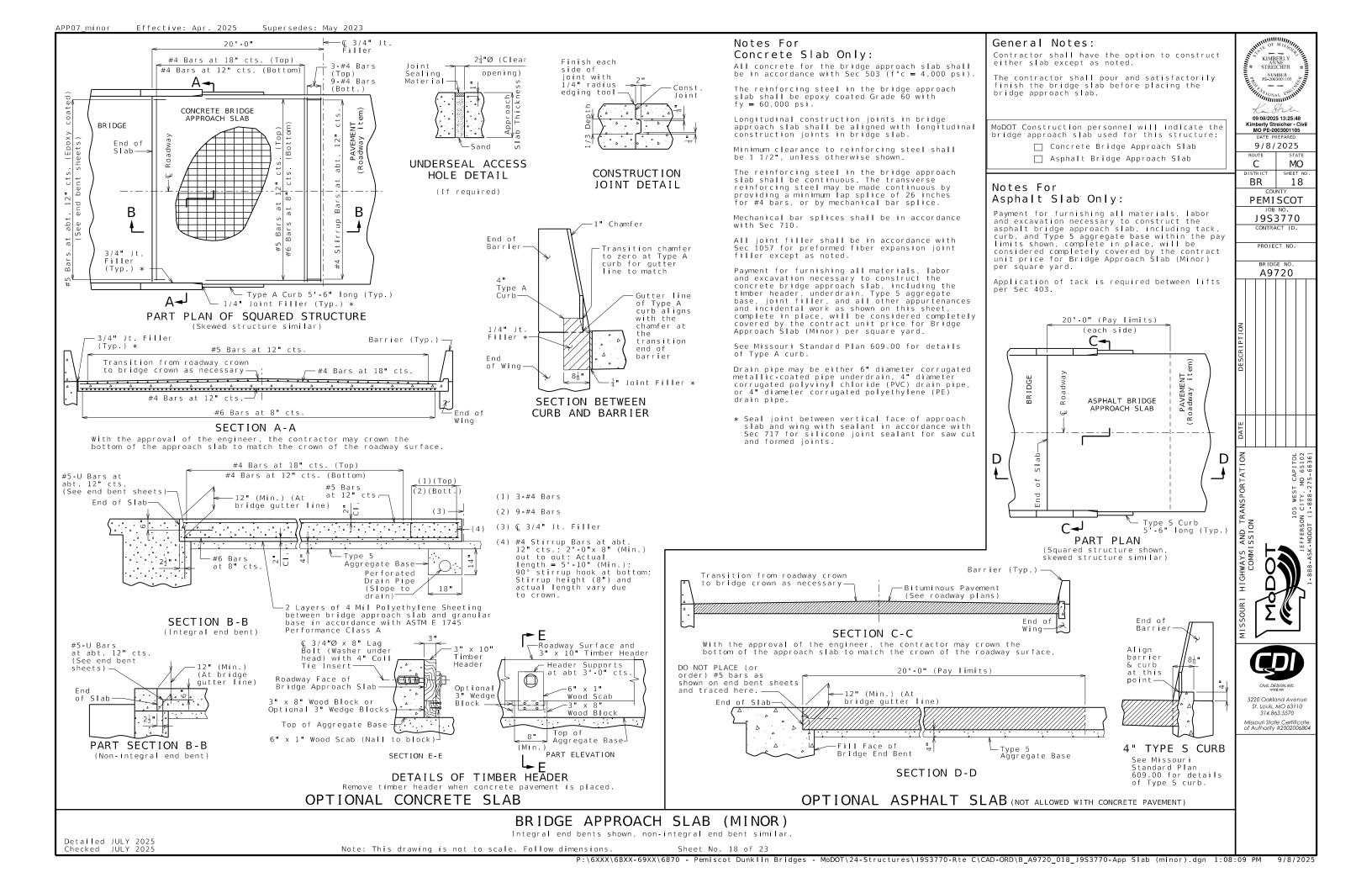
TYPE H BARRIER

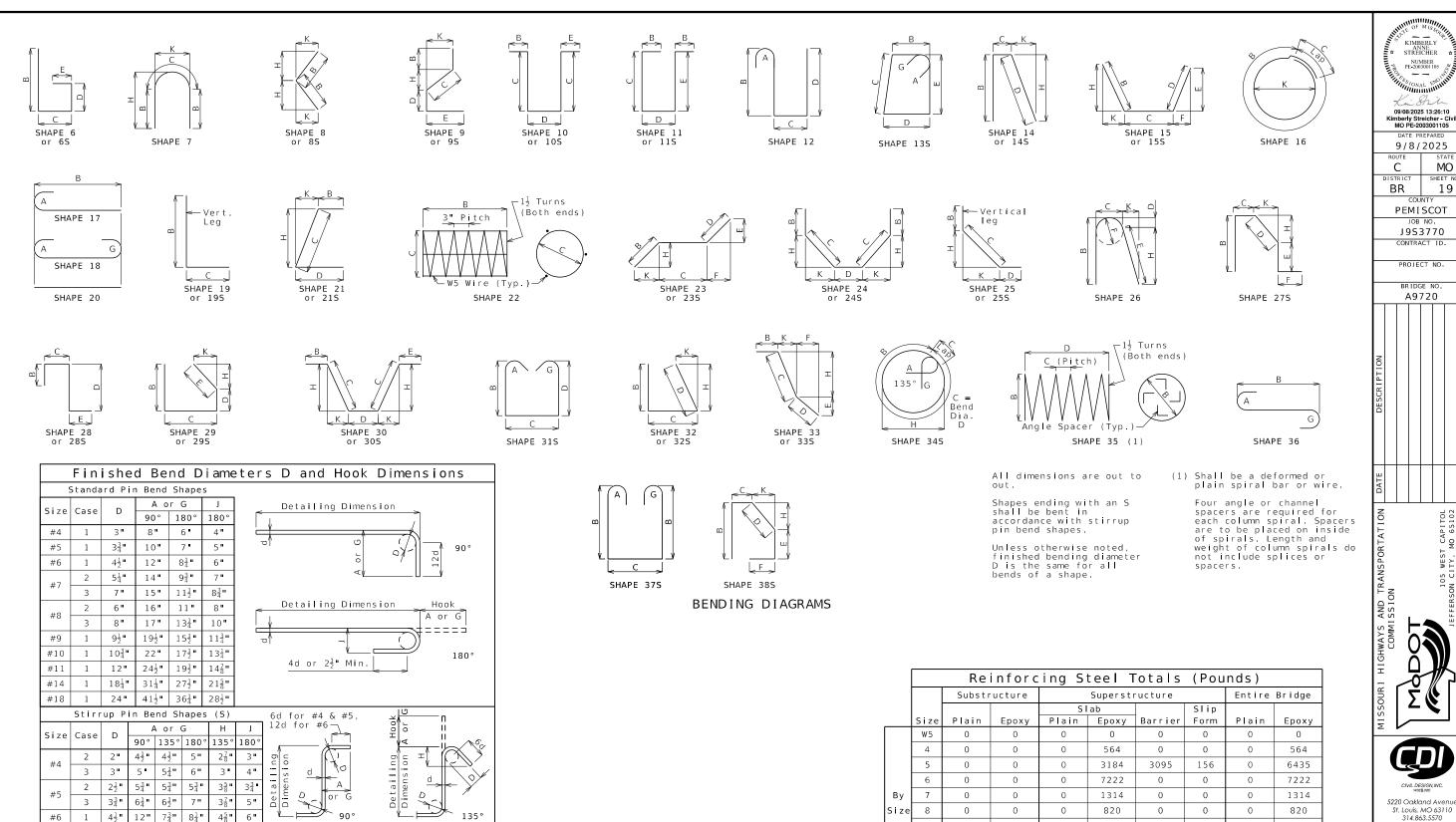
09/08/2025 13:24:59 Kimberly Streicher - Civil MO PE-2003001105 9/8/2025 C MO SHEET NO BR 16 **PEMISCOT** J9S3770 CONTRACT ID. PROJECT NO. A9720





Detailed JULY 2025 Checked JULY 2025





		Substr	ructure		Supersti		Entire Bridge			
				S	lab		Slip			
	Size	Plain	Epoxy	Plain	Epoxy	Barrier	Form	Plain	Epoxy	
	W5	0	0	0	0	0	0	0	0	
	4	0	0	0	564	0	0	0	564	
	5	0	0	0	3184	3095	156	0	6435	
	6	0	0	0	7222	0	0	0	7222	
Ву	7	0	0	0	1314	0	0	0	1314	
Size	8	0	0	0	820	0	0	0	820	
	9	0	0	0	0	0	0	0	0	
	10	0	0	0	0	0	0	0	0	
	11	0	0	0	0	0	0	0	0	
	14	0	0	0	0	0	0	0	0	
	18	0	0	0	0	0	0	0	0	
Bv	Type	0	0	0	13104	3095	156	0	16355	

ANNE STREICHER

MO SHEET NO

19

J9S3770 CONTRACT ID. PROJECT NO.

A9720

Missouri State Certificate of Authority #2002006804

All superstructure reinforcing steel shall be epoxy coated unless otherwise specified.

BENDING DIAGRAMS AND REINFORCING STEEL TOTALS

Detailed JULY 2025 Checked JULY 2025

Applicable for all grades of steel

Case 1 applies to all reinforcement. Case 2 applies to all reinforcement except for galvanized bars. Case 3 applies to

ğalvanized bars only.

Hook

ÀorG

180°

Detailing Dimension

4d or $2\frac{1}{2}$ Min.

BILLO3 data tables

New: Mar. 2024

			Bil	of Rei	<u>infor (</u>	cing St	eel											Bill o	f Re	infor	cing	Stee					
			5		Dimensi	ions		1/	Nom.	Act										Dimens	ions	- 1			Nom.	Actual	
Size/ J. Mark	Location	Codes	B C		ft in	ft in	ft in	ft in	Length ft in			lb	No. Req	Size/ Mark	Location	Codes	B V ft in.	ft in	ft in	f+ i) f+	in ft	H in f	K		Length ft in	
. Mark	Location	511 1		11.	110 111		1 (111.	1 2 111.	1 0 111	+ ' '		- 10	incq.	Mark	Location	0 311	V 1 C 1111.	16 1111.	1 2 111		1. 1.	111.		- 111.		1 1 1 111	1. 10
S	SUPERSTRUCTURE				\perp										SLAB												
	END DENT 1									_			22	5 S1	SLAB	E 20	47 2 000								47 3	17 3	3 11
+ +	END BENT 1				+								23 86		SLAB	E 20	47 3.000 26 5.000								26 5	47 3 26 5	
7 6 F100 W	ING BRACE	E 25	20.0004 8.	.000 20.00	00		2 11.00	003 7.000	8 0	7	11	83		6 S3			2 3 10.000								3 10		
2 6 F101 D		E 21 2	7.5007 7.				7 5.50			10	0	30			INCR=2'-10"		23 8.000								23 8	23 8	3
7 6 F102 W		E 25	20.0005 8.				4 5.25					93	6		SLAB	E 20	47 3.000								47 3	47 3	3 2
2 6 F103 D	IAPHRAGM	E 21	7 7.	.000 2 7.50	10		7 5.50	00 15.750	10 3	10	2	30	94	5 S5	SLAB	E 20	5 9.000								5 9	5 9	5
8 7 H100 BE	FΔM	E 20 2	6 9.000						26 9	26	a	438			BARRIER											+	
4 6 H101 B			6 9.000		+				26 9			161			DANNILN									+		+	
3 6 H102 D			6 9 000						26 9			121	20	5 K1	BARRIER CURB	E 275	3 5.000	9.250		2 11.	750		5.250	1.000	7 7	7 4	
6 6 H103 D			10.000						4 10		10	44			BARRIER CURB	E 27S	3 5.000	9.250		2 2.	750	1	14.250	2.750	7 8	7 5	j :
6 6 H104 D			1.000						2 1	2	1	19		5 K3		E 27S	22.500			2 2.	50 12	1.000 1	14.250	2.750	5 6	5 3	3
3 5 H105 S		E 20 6	6.000						6 6		6	20	-	5 K4	BARRIER CURB	E 19S	2 5.000	10.000							3 3	3 2	2
4 7 H106 D		-	6 9.000						26 9		9	219	$\overline{}$	5 K5	BARRIER CURB	E 38S			10.0		8 000		18.750	4.250		3 0)
16 8 H107 W 32 6 H108 W		E 19 8	5.500 16 0.000 12						9 9	_	10	410 377		5 K6	BARRIER CURB	E 21S	8 8.500		10.00	00		2	4.250	6.000	8 9	8 9	2 2
8 6 H109 BE		E 18 4		000						5	7	67			BARRIER CURB	E 20	8 8.000									8 11	
0 0 11103 81	LAIT	10 7	3.000						,	+ -		07	20	3 1(7	BARRIER CORB	20	0.000									+	-
40 4 P100 P	ILE	E 34S			+		10.00	00	3 5	3	3	89	86	5 R1	BARRIER CURB	E 14S	2 5.000	6.500				2	5.000	5.500	5 5	5 3	3 4
													86	5 R2	BARRIER CURB	E 19S	20.500	9.500							2 6	2 4	1 2
8 5 U100 BE		E 37S 4	3.0002 8.						12 5			102			BARRIER CURB	E 27S		9.500		5.	000 12	.000 1	15.000	3.000		3 3	3 2
24 4 U101 BE		E 13S 2	8.5002 7		002 7.0	00			11 4		1	178	16	5 R4	BARRIER CURB	E 20	42 1.000								42 1	42 1	1 7
3 4 U102 BE		E 105	2 7.		10				7 11		8	15														+	
24 5 U103 AF 33 6 U104 D		E 19S 2 E 19S 2	0.000 15 4.5004 3						3 3 6 8		1	77 322			SLIP FORM											+	
16 5 U105 BE		E 37S 2	4.5002 2							8	0	134			SLIP FURIN											+	
16 6 U106 BE		E 10S		0002 8.50	ا ا				5 3		1	122	8	5 C1	SLIP FORM	E 20	12 0.000								12 0	12 0) :
					1					1					SLIP FORM	E 20	6 9.000								6 9		_
16 5 V100 BE	EAM	E 17 4	3.000						4 10	4	10	79															
18 6 V101 D				. 000					2 0	_	10	50															
28 6 V102 W			2.500						5 3		3	221															
30 5 V103 P	ILE	E 17 5	3.000						5 10	5	10	183															
	END BENT 2				+					+			\vdash											\longrightarrow		+	
	END BEINT 2				+																					+	
7 6 F200 W		E 25	20.0004 8	.000 20.00	0		2 11.00	003 7.000	8 0	7	11	83														L	
2 6 F201 D		-	7.5007 7.	. 000			7 5.50			10		30															
7 6 F202 W		E 25	20.0005 8.				4 5.25			8		93				\Box											
2 6 F203 D	TAPHRAGM	E 21	7 7.	000 2 7.50	10		7 5.50	15.750	10 3	10	2	30															
8 7 H200 BE	E AM	E 20 2	6 0 000						26 9	26	0	438														+	
	EAM		6 9.000							26		161														+	
3 6 H202 D			6 9.000						26 9			121														+	
6 6 H203 D			10.000		+				4 10	_	10	44														+	
6 6 H204 D		E 20 2			1					2	1	19														+	
3 5 H205 S		E 20 6	6.000							6	6	20															
4 7 H206 D		E 20 2							26 9	_		219															
16 8 H207 W			5.500 16.				1			9	7	410	\square			\perp											
32 6 H208 W		E 19 7		000	+					7	10	377	\vdash			+								\longrightarrow		+	
8 6 H209 BE	EAIVI	E 18 4	3.000		+				5 7	5	/	67														+	
40 4 P200 P	ILE	E 34S			+		10.00	00	3 5	3	3	89												\longrightarrow		+	+
1		-			+		10.00			Ť		- 55												$\overline{}$		+	
8 5 U200 BE	EAM	E 37S 4	3.0002 8.	500	+				12 5	12	3	102														T	
24 4 U201 BE	EAM	E 13S 2	8.5002 7.	.0002 8.50		100			11 4			178															
3 4 U202 BE		E 10S		0002 8.50	10				7 11		8	15															
24 5 U203 AF		E 19S 2	0.000 15							3	1	77															
33 6 U204 D		E 195 2	4.5004 3.				1		6 8		6	322	\vdash			\perp										+	
16 5 U205 BE		E 37S 2			10					8	0	134	\vdash			+										+	
16 6 U206 BE	CAIVI	E 10S	15.	0002 8.50					5 3	5	1	122													 	+	1
16 5 V200 BE	FAM	E 17 4	3.000	+	+		+		4 10	4	10	79	\vdash													+	-
18 6 V201 D				.000	+					1		50	\vdash											\longrightarrow		+	1
28 6 V202 W			2.500		+				5 3		3	221			1										ſ	†	
30 5 V203 P			3.000						5 10	_	10	183															
																		-									
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			1					1																			

Nominal lengths are based on out to out dimensions shown in bending diagrams and are listed to the nearest inch for fabricator's use. Actual lengths are measured along centerline bar to the nearest inch. Weights are based on actual lengths.

For bending diagrams and steel reinforcing totals, see Sheet No. 19.

SH = Required shape, see bending diagrams.

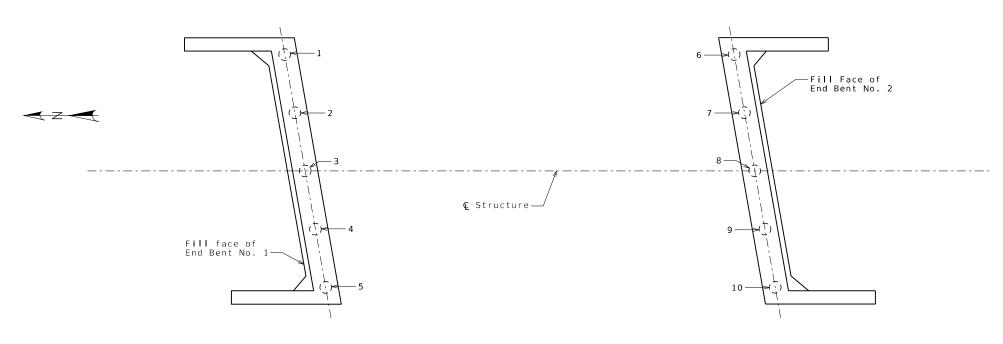
BILL OF REINFORCING STEEL

V = Sets of varied bars and number of bars of each length. Bar dimensions vary in equal increments between dimensions shown on this line and the following line and the actual length dimension shown on this line and the following line vary by the specified increment.

09/08/2025 13:26:29 Kimberly Streicher - Civi MO PE-2003001105 9/8/2025 С MO DISTRICT SHEET NO BR 20 PEMISCOT JOB NO. J9S3770 CONTRACT ID. PROJECT NO.

BRIDGE NO. A9720





PART PLAN SHOWING PILE NUMBERING FOR RECORDING AS-BUILT PILE DATA

	As-Built Pile Data						
Pile No.	Length in Place (ft)	PDA Nom. Axial Compressive Resistance (kips)	PDA End of Drive Blow Count (blows/in.)	Actual End of Drive Blow Count (blows/in.)	R emarks		
					End Bent No. 1		
1							
2							
3					•		
4							
5							
					End Bent No. 2		
6							
7							
8							
9							
10							

Note: Indicate in remarks column: A. Pile type and grade B. Batter C. Driven to practical refusal D. PDA test pile E. Minimum tip elevation controlled (Use when actual blow count is less than PDA blow count due to minim tip elevation requirement. A plus sign (+) shall be placed after the PDA nominal axial compressive resistance value indicating actual val is higher than PDA value.)	
is higher than PDA value.) This sheet to be completed by MoDOT construction personnel.	

09/08/2025 13:26:53 Kimberly Streicher - Civil MO PE-2003001105 9/8/2025 С MO BR 21 PEMISCOT JOB NO. J9S3770

PROJECT NO.

A9720



Missouri State Certificate of Authority #2002006804

Detailed JULY 2025 Checked JULY 2025

Note: This drawing is not to scale. Follow dimensions.

Job No.: <u>\$0712</u> Design: <u>J9S3770</u>

Offset: NE Corner

Elevation: 253.0

Requested Station:

Requested Offset:

Missouri Department of Transportation	BORING
Construction and Materials	

Missouri Department of Transportat	tion BORING NO. BH Page 1 of
County: Pemiscot	Route: C
Skew:	Location: Pemiscot County
Logged By: Smith&Co MBF	Operator: Smith&Co JAM
Northing: <u>108471.035</u>	Date of Work: 12/13/24-12/13/24
Easting: 993128.715	Depth to Water: 21.0
Requested Northing:	Depth Hole Open: 56
Requested Easting:	Time Change: 0 hours
Equipment: CME 750, Split-Spoon Sampler	

Requested Elevation: Location Note: 42'E & 11'N of the NE corner of existing structure

-		elevation: Location Note: _42		IVOI	IIIC IVE C					
Drill N	lo.:	Hammer Efficiency:		Drilling Method: HSA/Mud Rotary						
O Depth (ft)	Graphic	Description	Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N_{60})	Field Tests	Specimen		
		0.0-15.0' (CH) SANDY FAT CLAY	250							
-			-	×	94	4-4-5 (0)	PP = 1.50 tsf	5.0 - 5.0: 0-5.0 5.0 - 6.0: 4.5-6.0		
10			-	\times	106	1-2-2 (0)	PP = 1.50 tsf			
- - -		15.0-20.0' (SP-SC) POORLY GRADED SAND WITH	240		89	5-7-8 (0)		15.0 - 16.0: 14.5-16.0		
20		CLAY 19.5' added drilling fluid 20.0-56.0' (SP) POORLY GRADED SAND	230	-						
-				×	72	1-1-1 (0)				
30			220	-						
-				×	61	8-11-12 (0)				
40			-	-						
-			210	\times	67	4-9-7 (0)				
50 50										
-			200		78	12-13-12				

 N_{00} = (Em/60)Nm N_{00} - Corrected N value for standard 60% SPT efficiency; Em - Measured hammer efficiency in percent; Nm - Observed N-value (1) = Assumed, (2) = Actual

Coordinate System: Modified U.S. State Plane 1983 Coordinate Zone: Missouri East Coordinate Proj. Factor: Coordinate Datum: NAD 83 (CONUS) Coordinate Units: U.S. Survey Feet

*Persons using this information are cautioned that the materials shown are determined by the equipment noted and accuracy of the "log of materials" is limited thereby and by judgement of the operator. THIS INFORMATION IS FOR DESIGN PURPOSES ONLY.

Missouri Department of Transportation Construction and Materials

BORING NO. BH1 Page 2 of 2

	Constituction and materials	
Job No.: <u>S0712</u>	County: Pemiscot	Route: C
Design :	Skew:	Location: Pemiscot County
Bent:	Logged By: Smith&Co MBF	Operator: Smith&Co JAM
Station:	Northing: <u>108471.035</u>	Date of Work: 12/13/24-12/13/24
Offset: NE Corner	Easting: 993128.715	Depth to Water: 21.0
Elevation: 253.0	Requested Northing:	Depth Hole Open: 56
Requested Station:	Requested Easting:	Time Change: 0 hours
Requested Offset:	Equipment: CME 750,Split-Spoon Sampler	
Requested Elevation:	Location Note: 42'E & 11'N of the NE corner	of existing structure
Drill No.:	Hammer Efficiency: 93.7%	Drilling Method: HSA/Mud Rotary

			_					
00 00 00 00 00 00 00 00	Graphic	Description	Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N _{E0})	Field Tests	Specimen Info
<u> </u>		56.0-80.0' (SP) Poorly Graded Sand	-			(0)		
60 E			ļ -					
		,	ļ -	\times	67	7-11-16 (0)		
<u> </u>			190					
<u> </u>		•	ļ -	$\overline{}$	72	9-10-15		
<u>"</u>			ļ -			(0)		
원 5 70			ļ -					
- 70 -				\times	72	10-16-24 (0)		
5			180					
<u> </u>			-	$\overline{}$	100	14-19-17 (0)		
3						(0)		
80			-					
5		Bottom of borehole at 80.0 feet.		\times	83	22-33-39 (0)		
<u>-</u>								
25								
=								
2 CE								
240Z1								
7								
1 62/6								
9								
28.6								
70610								
۲ 5								
Ď N								
90								
<u> </u>								

- 1	N = /E (CO)N	N Commeted	NI value for standard COO/	CDT officiency Fo	. Manasumad bananaan	efficiency in accepts Non	Observed M. volu
	$IN_{BO} = (EIII/OU)INIII$	N ₈₀ - Corrected	N value for standard 60%	SPT efficiency, En	ı - ivleasured nammer	eniciency in percent, ivin	- Observed IN-Value
	(4)	~		• • • • • • • • • • • • • • • • • • • •			
- 1	(1) = Assumed, (2)	= Actual					

Coordinate System:	Modified U.S. State Plane 1983	Coordinate Zone:	Missouri East	Coordinate Proj. Factor: _	
Coordinate Datum:	NAD 83 (CONUS)	Coordinate Units:	ILS Survey Feet		

Lni Stin 09/08/2025 13:27:16 Kimberly Streicher - Civi MO PE-2003001105 9/8/2025

C. MO 22 BR

PEMISCOT JOB NO. J9S3770

PROJECT NO.

		72		
DESCRIPTION				
DATE				
NO			ō	0.2

MISSOURI HIGHWAYS AND TRANSPOR COMMISSION MODOT 105 WEST JEFFERSON CITY, IN THE STANSPORT AND THE S



Missouri State Certificate of Authority #2002006804

BORING DATA

^{*} Persons using this information are cautioned that the materials shown are determined by the equipment noted and accuracy of the "log of materials" is limited thereby and by judgement of the operator. THIS INFORMATION IS FOR DESIGN PURPOSES ONLY.

Job No.: <u>S0712</u> Design: J9S3770

Offset: SE Corner

Elevation: 253.0

Requested Station:

Requested Offset:

Missouri Department of Transportation	BORING NO. BH2
Construction and Materials	Page 1 of 2

Construction and Materials	rage
County: Pemiscot	Route: C
Skew:	Location: Pemiscot County
Logged By: Smith&Co MBF	Operator: Smith&Co JAM
Northing: <u>109407.607</u>	Date of Work: 12/12/24-12/12/24
Easting: 993130.671	Depth to Water: 20.0
Requested Northing:	Depth Hole Open: 51
Requested Easting:	Time Change: 0 hours
Equipment: CME 750,Split-Spoon Sampler	

Location Note: 52' E & 3' S of the SE corner of existing structure

Requested Elevation:

	Drill N	lo.:	Hammer Efficiency:		, ,		Drilling I	/lethod: <u>HSA/M</u> ւ	ud Rotary
J LOG - MODO I 20150/28/GDI - 6/6/25 15/19 - P:/PZ40Z13/CIVIL DESIGN-MODO I KOU I E C & Z GEOTECH/ENGINEPORT SIGEOTECH LOGSIROU I E CISU/12/GPJ	O Depth (ft)	Graphic	Description	Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N _{so})	Field Tests	Specimen Info
S'ROUIE			0.0-15.0' (CH) SANDY FAT CLAY	250					
) ECH LOC	 			-	\times	100	1-3-4		
OKIS/GEC	10			-		100	1-2-2		
1/ENG/REF				240			(0)		
GEOLECT			15.0-20.0' (SP-SC) POORLY GRADED SAND WITH CLAY	-	X	89	2-2-7 (0)		15.0 - 16.0: 14.5-16.0
JUIE C & 2	20		∑ 20.0-56.0' (SP) POORLY GRADED SAND	-	\times	83	3-2-2 (0)		
MODOI R	 			230					
L DESIGN-	 			_					
40213 CIVI	30			220	X	61	4-5-6 (0)		
5:19 - P:\PZ					-				
-6/5/25 1	 40			- - -			7-10-9		
50728.GD				210		78	(0)		
10DO 201	 			-					
- POO - P	 50			-		72	14-18-21		

 N_{00} = (Em/60)Nm N_{00} - Corrected N value for standard 60% SPT efficiency; Em - Measured hammer efficiency in percent; Nm - Observed N-value (1) = Assumed, (2) = Actual

Coordinate System: Modified U.S. State Plane 1983 Coordinate Zone: Missouri East Coordinate Proj. Factor: Coordinate Datum: NAD 83 (CONUS) Coordinate Units: U.S. Survey Feet

* Persons using this information are cautioned that the materials shown are determined by the equipment noted and accuracy of the "log of materials" is limited thereby and by judgement of the operator. THIS INFORMATION IS FOR DESIGN PURPOSES ONLY.

Missouri Department of Transportation Construction and Materials

BORING NO. BH2 Page 2 of 2

09/08/2025 13:27:50 Kimberly Streicher - Civil MO PE-2003001105

9/8/2025

PEMISCOT JOB NO. J9S3770

CONTRACT ID. PROJECT NO.

A9720

5220 Oakland Avenue

St. Louis, MO 63110 314.863.5570

Missouri State Certificate of Authority #2002006804

MO

SHEET NO 23

C.

BR

Job No.: 80712	County: Pemiscot	Route: C
Design: _J9S3770	Skew:	Location: Pemiscot County
Bent:	Logged By: Smith&Co MBF	Operator: Smith&Co JAM
Station:	Northing: 109407.607	Date of Work: 12/12/24-12/12/24
Offset: SE Corner	Easting : 993130.671	Depth to Water: 20.0
Elevation: 253.0	Requested Northing:	Depth Hole Open: 51
Requested Station:	Requested Easting:	Time Change: 0 hours
Requested Offset:	Equipment: CME 750,Split-Spoon Sa	ampler
Requested Elevation:	Location Note: 52' E & 3' S of the SE	corner of existing structure
Drill No.:	Hammer Efficiency: 93.7%	Drilling Method: HSA/Mud Rotary

Drill N	o.:	Hammer Efficiency	: <u></u>			Drilling i	/lethod: HSA/Mu	d Rotary
Depth (ft)	Graphic	Description	Elevation (ft)	Sample Type	REC % (RQD %)	Blow Counts (N ₆₀)	Field Tests	Specimen Info
		20.0-56.0' (SP) POORLY GRADED SAND (continued)				(0)		
	1.404(81	Bottom of borehole at 51.0 feet.						

, N_{so} = (Em/60)Nm, N_{so} - Corrected N value for standard 60% SPT efficiency; Em - Measured hammer efficiency in percent; Nm - Observed N-value

Coordinate System: Modified U.S. State Plane 1983 Coordinate Zone: Missouri East _ Coordinate Proj. Factor: __ Coordinate Units: U.S. Survey Feet Coordinate Datum: NAD 83 (CONUS)

* Persons using this information are cautioned that the materials shown are determined by the equipment noted and accuracy of the "log of materials" is limited thereby and by judgement of the operator. THIS INFORMATION IS FOR DESIGN PURPOSES ONLY.

BORING DATA

Note: For locations of borings, see Sheet No. 1.

Detailed JULY 2025 Checked JULY 2025

Note: This drawing is not to scale. Follow dimensions.