

DESIGN DESIGNATION

ROUTE U

A.A.D.T. - 2026 = 188

A.A.D.T. - 2046 = 207

D.H.V. = 11%

T = 11.96%

V = 55 M.P.H.

FUNCTIONAL CLASSIFICATION - MAJOR COLLECTOR

NORMAL RIGHT OF WAY

THE EXISTENCE AND APPROXIMATE LOCATION OF UTILITY FACILITIES KNOWN TO EXIST, AS SHOWN ON THE PLANS, ARE BASED ON THE BEST INFORMATION AVAILABLE TO THE COMMISSION AT THIS TIME. THIS INFORMATION IS PROVIDED BY THE COMMISSION "AS-IS" AND THE COMMISSION EXPRESSLY DISCLAIMS ANY REPRESENTATION OR WARRANTY AS TO THE COMPLETENESS, ACCURACY, OR SUITABILITY OF THE INFORMATION FOR ANY USE. RELIANCE UPON THIS INFORMATION IS DONE AT THE RISK AND PERIL OF THE USER, AND THE COMMISSION SHALL NOT BE LIABLE FOR ANY DAMAGES THAT MAY ARISE FROM ANY ERROR IN THE INFORMATION. IT IS, THEREFORE, THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY THE EXISTENCE, LOCATION AND STATUS OF ANY FACILITY. SUCH VERIFICATION INCLUDES DIRECT CONTACT WITH THE LISTED UTILITIES.

CONVENTIONAL SYMBOLS

(USED IN PLANS)

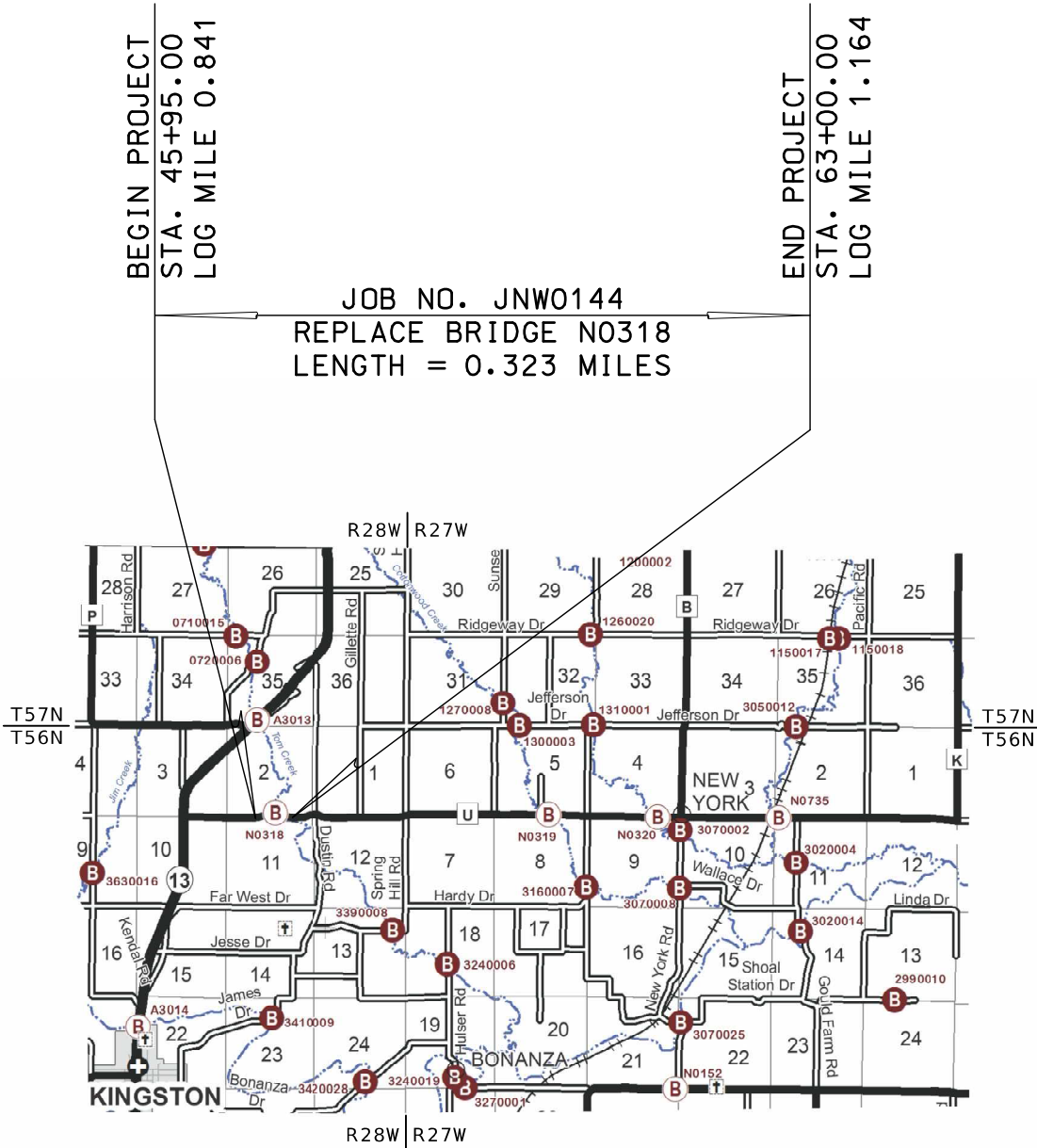
	EXISTING	NEW
BUILDINGS AND STRUCTURES		
GUARD RAIL		
GUARD CABLE		
CONCRETE RIGHT-OF-WAY MARKER		
STEEL RIGHT-OF-WAY MARKER		
LOCATION SURVEY MARKER		
UTILITIES		
FIBER OPTICS	-FO-	-FO-
OVERHEAD CABLE TV	-OTV-	-OTV-
UNDERGROUND CABLE TV	-UTV-	-UTV-
OVERHEAD TELEPHONE	-OT-	-OT-
UNDERGROUND TELEPHONE	-UT-	-UT-
OVERHEAD POWER	-OE-	-OE-
UNDERGROUND POWER	-UE-	-UE-
SANITARY SEWER	-S-	-S-
STORM SEWER	-SS-	-SS-
GAS	-G-	-G-
WATER	-W-	-W-
MANHOLE		
FIRE HYDRANT		
WATER VALVE		
WATER METER		
DROP INLET		
DITCH BLOCK		
GROUND MOUNTED SIGN		
LIGHT POLE		
H-FRAME POWER POLE		
TELEPHONE PEDESTAL		
FENCE		
CHAIN LINK		
WOVEN WIRE		
GATE POST		
BENCHMARK		

NOTE: DASHED OR OPEN SYMBOLS INDICATE EXISTING FEATURES

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION
PLANS FOR PROPOSED
STATE HIGHWAY
CALDWELL COUNTY



KEY MAP
LOCATION OF CALDWELL COUNTY



BEGIN PROJECT
STA. 45+95.00
LOG MILE 0.841

END PROJECT
STA. 63+00.00
LOG MILE 1.164

JOB NO. JNW0144
REPLACE BRIDGE N0318
LENGTH = 0.323 MILES

INDEX OF SHEETS

DESCRIPTION	SHEET NUMBER
TITLE SHEET -----	1
TYPICAL SECTIONS (TS) (1 SHEET)-----	2
QUANTITIES (QU) (2 SHEETS)-----	3
PLAN-PROFILE (PP)-----	4
COORDINATE POINTS (CP)-----	5
SPECIAL SHEETS (SS)-----	6-8
TRAFFIC CONTROL SHEETS (TC)-----	9
EROSION CONTROL SHEETS (EC)-----	10
CULVERT SECTIONS (CS)-----	11
BRIDGE DRAWINGS (B)	
A9660-----	1-30
CROSS SECTIONS (XS)-----	1-14

LENGTH OF PROJECT

BEGINNING OF PROJECT	STA. 45+95.00
END OF PROJECT	STA. 63+00.00
APPARENT LENGTH	1,705.00 FEET
EQUATIONS AND EXCEPTIONS:	

TOTAL CORRECTIONS	0.00 FEET
NET LENGTH OF PROJECT	1,705.00 FEET
STATE LENGTH	0.323 MILES

FOR INFORMATION ONLY
ESTIMATED DISTURBED ACRES 3 ACRES



THIS SHEET HAS BEEN
SIGNED, SEALED AND DATED
ELECTRONICALLY.

DATE PREPARED
12/1/2025

ROUTE U STATE MO

DISTRICT NW SHEET NO. 1

COUNTY
CALDWELL

JOB NO.
JNW0144

CONTRACT ID.

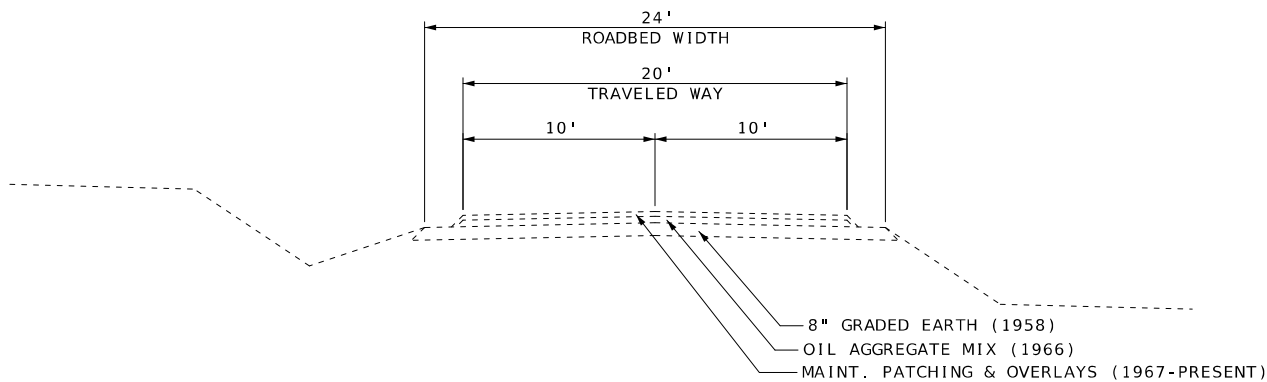
PROJECT NO.

BRIDGE NO.

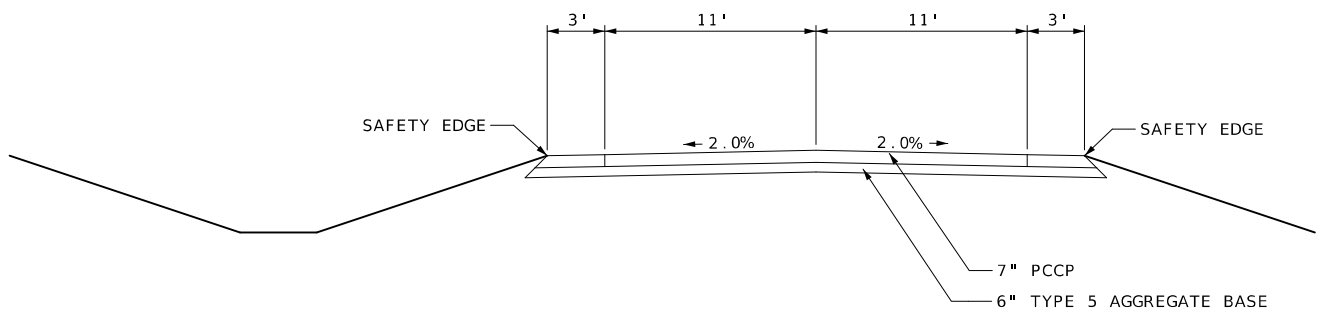
DESCRIPTION	DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

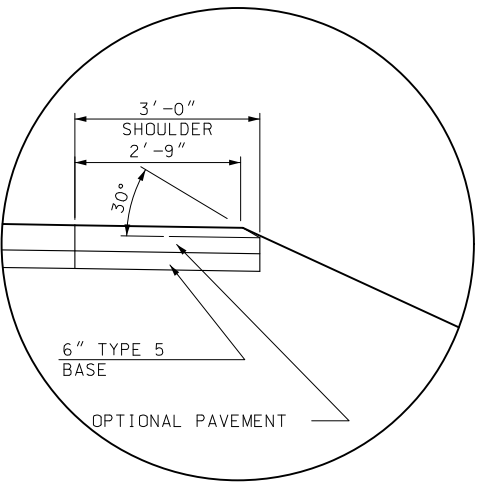
105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)



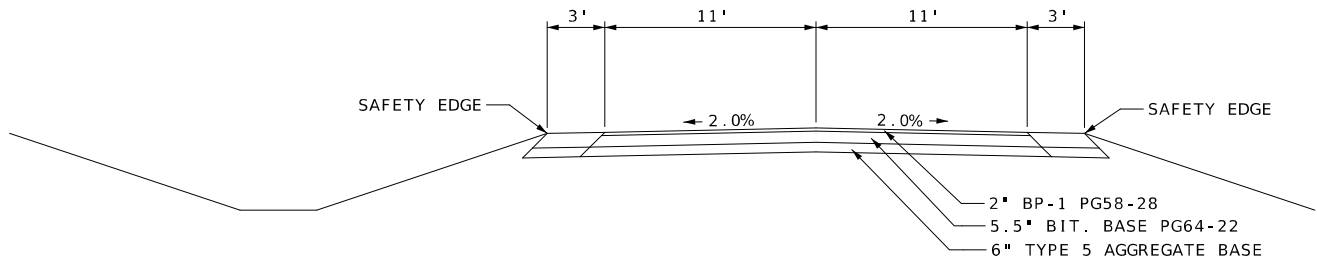
EXISTING TYPICAL SECTION
ROUTE U
STA. 45+95.00 TO STA. 63+00.00



PROPOSED TYPICAL SECTION
ROUTE U - OPTION A: PCCP
STA. 45+95.00 TO STA. 63+00.00



SAFETY EDGE DETAIL



PROPOSED TYPICAL SECTION
ROUTE U - OPTION B: HMA
STA. 45+95.00 TO STA. 63+00.00

NOT TO SCALE

STATE OF MISSOURI

RICHARD W. ORR

NUMBER
PE-2002003397

PROFESSIONAL ENGINEER

THIS SHEET HAS BEEN
SIGNED, SEALED AND DATED
ELECTRONICALLY.

DATE PREPARED 11/26/2025	
ROUTE U	STATE MO
DISTRICT NW	SHEET NO. 2
COUNTY CALDWELL	
JOB NO. JNW0144	
CONTRACT ID.	
PROJECT NO.	
BRIDGE NO.	

DESCRIPTION	DATE

MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION

MoDOT

105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)

REMOVAL OF IMPROVEMENTS						
STATION	STATION	LOCATION	DESCRIPTION	REMOVALS		
				EACH	L.F.	S.Y.
45+99.26		RTE. U	30" X 40' CMP		40	
46+28.62	49+03.96	RTE. U RT.	FENCE		337.3	
49+50.00	52+82.32	RTE. U LT.	FENCE		362.7	
53+70.88	54+12.51	RTE. U RT.	TYPE III OBJECT MARKER	3		
53+72.95	54+12.50	RTE. U LT.	TYPE III OBJECT MARKER	3		
55+10.46	55+50.34	RTE. U LT.	TYPE III OBJECT MARKER	3		
55+11.69	55+49.72	RTE. U RT.	TYPE III OBJECT MARKER	3		
SUBTOTAL				12	740	
PAY TOTAL				LUMP SUM = 1		

CLEARING AND GRUBBING				
STATION	STATION	LOCATION	CLEARING AND GRUBBING ACRE	REMARKS
45+50.00	56+50.00	ROUTE U LT.	1.30	TREES REMOVED BY OTHERS, STUMP REMOVAL ONLY
45+50.00	51+50.00	ROUTE U RT	0.92	TREES REMOVED BY OTHERS, STUMP REMOVAL ONLY
61+00.00	63+00.00	ROUTE U RT	0.09	TREES REMOVED BY OTHERS, STUMP REMOVAL ONLY
SUBTOTAL			2.31	
PAY TOTAL			2.3	

EARTHWORK						
STATION	STATION	LOCATION	CLASS A EXCAVATION C.Y.	COMPACTING EMBANKMENT C.Y.	EMBANKMENT IN-PLACE C.Y.	REMARKS
45+95.00	54+05.00	RTE. U	7739.9	3096.7		
54+05.00	54+27.84	RTE. U			500	SPILL/ FILL SLOPE
54+90.84	55+18.00	RTE. U			500	SPILL/ FILL SLOPE
55+18.00	63+00.00	RTE. U	134.7	94.3	3166.7	
46+22.00		RTE. U RT.	350.1	22.8		
50+25.00		RTE. U RT.	4.7	3.3	147.7	
57+50.00		RTE. U LT.	1.3	0.9	283.3	
57+50.00		RTE. U RT.	2.3	1.6	236.2	
SUBTOTAL			8233	3219.6	4833.9	
PAY TOTAL			8233	3219.6	4833.9	

PAVEMENT				
STATION	STATION	LOCATION	OPTIONAL PAVEMENT S.Y.	6" TYPE 5 AGGREGATE BASE S.Y.
45+95.00	47+00.00	ROUTE U	290.00	290.0
47+00.00	54+10.00	ROUTE U	2208.89	2208.9
55+20.00	62+00.00	ROUTE U	2115.56	2115.6
62+00.00	63+00.00	ROUTE U	277.78	277.8
SUBTOTAL			4892.23	4892.30
PAY TOTAL			4892.2	4892.0

GRAVEL (A) OR CRUSHED STONE (B)			
STATION	LOCATION	6" GRAVEL (A) OR CRUSHED STONE (B) TONS	REMARKS
46+22.00	ROUTE U RT.	33.8	TYPE I DRIVEWAY
50+20.00	ROUTE U RT.	29.4	TYPE I DRIVEWAY
57+50.00	ROUTE U LT.	36.2	TYPE I DRIVEWAY
57+50.00	ROUTE U RT.	29.4	TYPE I DRIVEWAY
SUBTOTAL		128.8	
PAY TOTAL		129	

SEEDING					
STATION	STATION	LOCATION	SEEDING COOL SEASON GRASSES ACRE	TEMPORARY SEEDING ACRE	TYPE 2C EROSION CONTROL BLANKET S.Y.
45+95.00	54+05.00	ROUTE U LT.	1.13	0.11	5469.2
45+95.00	54+05.00	ROUTE U RT.	0.73	0.07	3533.2
55+20.00	63+00.00	ROUTE U LT.	0.34	0.03	1645.6
55+20.00	63+00.00	ROUTE U RT.	0.34	0.03	1645.6
SUBTOTAL			2.54	0.24	12293.6
PAY TOTAL			2.5	0.2	12294

DRAINAGE STRUCTURES							
STATION	LOCATION	30" GROUP B PIPE L.F.	30" GROUP B F.E.S. EACH	15" GROUP C PIPE L.F.	18" CROUP C PIPE L.F.	CLASS 3 EXCAVATION C.Y.	REMARKS
45+99.26	ROUTE U	51	2			46.6	
46+22.00	ROUTE U RT.			50			DRIVEWAY PIPE
50+25.00	ROUTE U RT.				44		DRIVEWAY PIPE
SUBTOTAL		51	2	50	44	46.6	
PAY TOTAL		51	2	50	44	47	

EROSION CONTROL					
STATION	STATION	LOCATION	ROCK DITCH CHECK L.F.	SILT FENCE L.F.	SEDIMENT REMOVAL C.Y.
47+00.00		RTE. U RT.	18		1
47+50.00		RTE. U LT.	25.2		1
49+00.00		RTE. U LT.	20		1
49+75.00		RTE. U RT.	18.7		1
50+50.00		RTE. U LT.	20		1
50+49.68	54+07.48	RTE. U RT.		357.8	3.6
52+00.00		RTE. U LT.	20		1
53+50.00		RTE. U LT.	20		1
55+16.97	57+38.68	RTE. U LT.		221.7	2.2
55+17.31	57+37.25	RTE. U RT.		219.9	2.2
57+60.56	63+00.10	RTE. U RT.		539.5	5.4
57+60.62	62+93.46	RTE. U LT.		532.8	5.3
SUBTOTAL			141.9	1871.7	25.7
PAY TOTAL			142	1872	26

ROCK LINING				
STATION	STATION	LOCATION	ROCK LINING C.Y.	REMARKS
45+96.46	46+16.64	ROUTE U LT.	17.2	CULVERT OUTLET PROTECTION
SUBTOTAL			17.2	
PAY TOTAL			17	

TYPE 2 ROCK BLANKET					
STATION	STATION	LOCATION	FURNISHING TYPE 2 ROCK BLANKET C.Y.	PLACING TYPE 2 ROCK BLANKET C.Y.	PERMANENT EROSION CONTROL GEOTEXTILE S.Y.
54+11.05	54+27.84	ROUTE U	65.5	65.5	98.2
54+90.84	55+13.61	ROUTE U	88.8	88.8	133.2
SUBTOTAL			154.3	154.3	231.4
PAY TOTAL			154	154	231

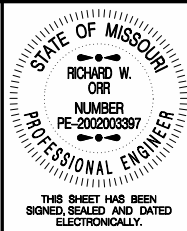
MOBILIZATION
LUMP SUM = 1

ADDITIONAL MOBILIZATION FOR SEEDING
4 EACH

CONTRACTOR FURNISHED SURVEYING AND STAKING
LUM SUM =1

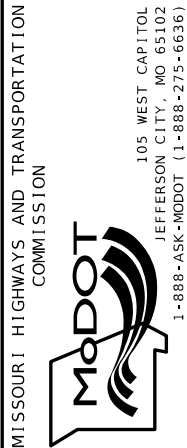
TYPE III OBJECT MARKERS			
STATION	LOCATION	U-CHANNEL POST, 3 LB. L.F.	SH-FLAT SHEET S.F.
53+67.88	ROUTE U LT	11.5	3
53+87.88	ROUTE U LT	11.5	3
54+07.88	ROUTE U LT	11.5	3
53+66.18	ROUTE U RT.	11.5	3
53+86.18	ROUTE U RT.	11.5	3
54+06.18	ROUTE U RT.	11.5	3
55+16.72	ROUTE U LT	11.5	3
55+36.72	ROUTE U LT	11.5	3
55+56.72	ROUTE U LT	11.5	3
55+18.43	ROUTE U RT.	11.5	3
55+38.43	ROUTE U RT.	11.5	3
55+58.43	ROUTE U RT.	11.5	3
SUBTOTAL		138	36
PAY TOTAL		138	36

PAVEMENT MARKING				
STATION	STATION	LOCATION	4" YELLOW CLASS 1 PAVEMENT MARKING PAINT (18-MIL, TYPE P BEADS) L.F.	REMARKS
45+95.00	63+00.00	ROUTE U	3410	DOUBLE YELLOW
SUBTOTAL			3410	
PAY TOTAL			3410	

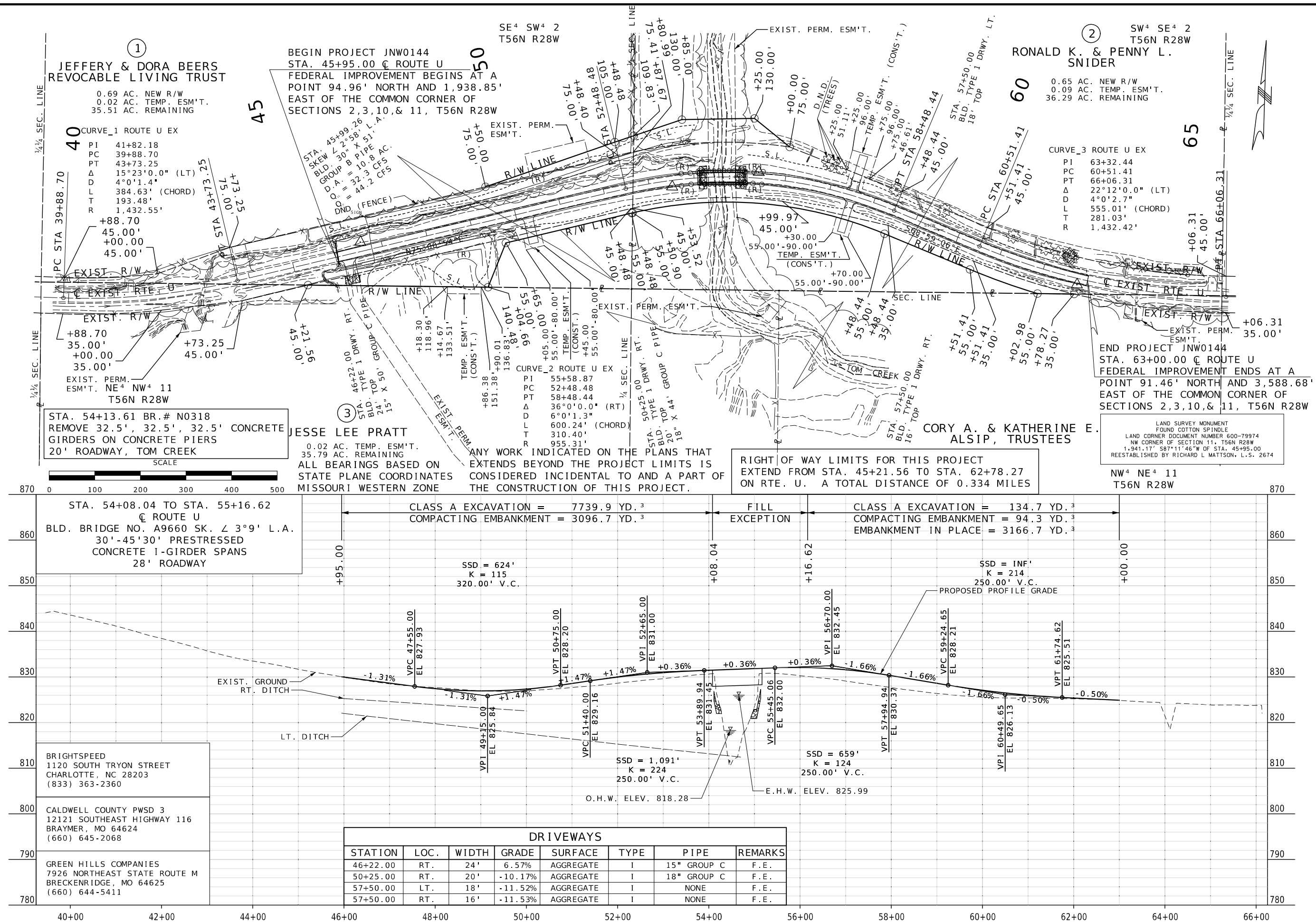


DATE PREPARED 12/23/2025	
ROUTE U	STATE MO
DISTRICT NW	SHEET NO. 3
COUNTY CALDWELL	
JOB NO. JNW0144	
CONTRACT ID.	
PROJECT NO.	
BRIDGE NO.	

DESCRIPTION	DATE								



SIGN								SIGN								SIGN								ITEM		TOTAL		EFFECTIVE: 07-01-2025											
SIZE	AREA	QTY	TOTAL	RELOC	RELOC	SIGN	DESCRIPTION	SIZE	AREA	QTY	TOTAL	RELOC	RELOC	SIGN	DESCRIPTION	NUMBER	QTY	DESCRIPTION	DESCRIPTION	STATE	DISTRICT	SHEET NO.																	
IN.	SQ. FT.	EACH	SQ. FT.	EACH	SQ. FT.	NO.		IN.	SQ. FT.	EACH	SQ. FT.	EACH	SQ. FT.	NO.						ROUTE																			
WARNING SIGNS								GUIDE SIGNS								REGULATORY SIGNS								MISCELLANEOUS SIGNS								SUMMARY OF QUANTITIES							
WO1-1L	48X48	16.00					TURN (SYMBOL LEFT)	E05-1	36X48	12.00					GORE EXIT	6122008		IMPACT ATTENUATOR 40 MPH (SAND BARRELS)		U	NW	3																	
WO1-1R	48X48	16.00					TURN (SYMBOL RIGHT)	E05-2	48X36	12.00					EXIT OPEN	6122009		IMPACT ATTENUATOR 45 MPH (SAND BARRELS)		MO																			
WO1-2L	48X48	16.00					CURVE (SYMBOL LEFT)	E05-2a	48X36	12.00					EXIT CLOSED	6122010		IMPACT ATTENUATOR 50 MPH (SAND BARRELS)																					
WO1-2R	48X48	16.00					CURVE (SYMBOL RIGHT)	GO20-1	60X24	10.00					ROAD WORK NEXT XX MILES	6122012		IMPACT ATTENUATOR 55 MPH (SAND BARRELS)																					
WO1-3L	48X48	16.00					REVERSE TURN (SYMBOL LEFT)	GO20-2	48X24	8.00					END ROAD WORK	6122014		IMPACT ATTENUATOR 60 MPH (SAND BARRELS)																					
WO1-3R	48X48	16.00					REVERSE TURN (SYMBOL RIGHT)	GO20-4	36X18	4.50					PILOT CAR FOLLOW ME	6122017		IMPACT ATTENUATOR 65 MPH (SAND BARRELS)																					
WO1-4L	48X48	16.00					REVERSE CURVE (SYMBOL LEFT)	GO20-4a	42X30	8.75					PILOT CAR IN USE WAIT & FOLLOW	6122019		IMPACT ATTENUATOR 70 MPH (SAND BARRELS)																					
WO1-4R	48X48	16.00					REVERSE CURVE (SYMBOL RIGHT)	GO20-4a	18X12	1.50					PILOT CAR IN USE WAIT & FOLLOW	6122020		REPLACEMENT SAND BARREL																					
WO1-4bL	48X48	16.00					DOUBLE ARROW REVERSE CURVE (SYMBOL LEFT)	GO20-5aP	36X24	6.00					WORK ZONE (PLAQUE)	6122030		IMPACT ATTENUATOR (RELOCATION)																					
WO1-4bR	48X48	16.00					DOUBLE ARROW REVERSE CURVE (SYMBOL RIGHT)	MO4-8a	24X18	3.00					END DETOUR	6122040		WORK ZONE CRASH CUSHION (NARROW)																					
WO1-4cL	48X48	16.00					TRIPLE ARROW REVERSE CURVE (SYMBOL LEFT)	MO4-9L	48X36	12.00					DETOUR (LEFT)	6122041		WORK ZONE CRASH CUSHION (RELOCATION)																					
WO1-4cR	48X48	16.00					TRIPLE ARROW REVERSE CURVE (SYMBOL RIGHT)	MO4-9R	48X36	12.00					DETOUR (RIGHT)	6123001	1 *	TRUCK MOUNTED ATTENUATOR (TMA)																					
WO1-6	60X30	12.50					HORIZONTAL ARROW (SYMBOL)	MO4-9P	48X12	4.00					STREET NAME (PLAQUE)	6161012		BUOYS (BOATS KEEP OUT)																					
WO1-6a	72X36	18.00					HORIZ. ARROW (SYMBOL ON PERMANENT BARRICADE)	MO4-10L	48X18	6.00					DETOUR ARROW (LEFT)	6161013		BUOYS (NO WAKE)																					
WO1-7	60X30	12.50					DOUBLE HEAD HORIZONTAL ARROW (SYMBOL)	MO4-10R	48X18	6.00					DETOUR ARROW (RIGHT)	6161014		SPECIAL SIGN ASSEMBLY (BOATS KEEP OUT)																					
WO1-7a	72X36	18.00					DOUBLE HEAD HORIZ. ARROW (SYMBOL ON PERM. BARR.)									6161020		CHANNELIZER (DRUM-LIKE)																					
WO1-8	18X24	3.00					CHEVRON (SYMBOL)	R1-1	48X48	13.25					STOP	6161022		CHANNELIZER (CONE)																					
WO1-8a	30X36	7.50					CHEVRON (SYMBOL FOR DIVIDED HIGHWAYS)	R1-2	48TRI.	6.93					YIELD	6161025		CHANNELIZER (TRIM-LINE)																					
WO3-1	48X48	16.00					STOP AHEAD (SYMBOL)	R1-2a	36X36	9.00					TO ONCOMING TRAFFIC (PLAQUE)	6161026		CHANNELIZER (VERTICAL PANEL)																					
WO3-2	48X48	16.00					YIELD AHEAD (SYMBOL)	R1-3P	30X12	2.50					ALL WAY (PLAQUE)	6161030	9 *	TYPE 3 MOVEABLE BARRICADE																					
WO3-3	48X48	16.00					SIGNAL AHEAD (SYMBOL)	R2-1	36X48	12.00					SPEED LIMIT XX	6161033		DIRECTION INDICATOR BARRICADE																					
WO3-4	48X48	16.00					BE PREPARED TO STOP	R3-1	48X48	16.00					NO RIGHT TURN (SYMBOL)	6161040		FLASHING ARROW PANEL																					
WO3-5	48X48	16.																																					



THIS SHEET HAS BEEN
SIGNED, SEALED AND DATED
ELECTRONICALLY.

DATE PREPARED
11/26/2025

ROUTE U	STATE MO
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DISTRICT NW	SHEET NO. 4
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COUNTY
CALDWELL

JOB NO.
JNW0144

CONTRACT ID.

PROJECT NO.

BRIDGE NO.

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[illegible][illegible]

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888-1-800-445-4453



ALL PROJECT COORDINATES HAVE BEEN PROJECTED FROM THE MISSOURI STATE PLANE COORDINATE (SPC) SYSTEM OF 1983 USING AN AVERAGE PROJECT PROJECTION (GRID TO GROUND) FACTOR. TO GET BACK TO STATE PLANE COORDINATES MULTIPLY THE PROJECT COORDINATES BY THE AVERAGE GRID FACTOR AS SHOWN IN THE "REFERENCE CONTROL INFORMATION" PORTION OF THIS TABLE.

PROJECT COORDINATE INFORMATION

COORDINATE SYSTEM	MISSOURI STATE PLANE, WEST ZONE
HORIZONTAL DATUM	NAD83
VERTICAL DATUM	NAVD88
GEOID MODEL	GEOID 18
ELEVATIONS DETERMINED BY	BARTLETT & WEST RAN 2 - 4 HOUR OPUS SOLUTIONS FOR CONTROL POINTS 100 AND 101. OPUS SOLUTIONS WERE AVERAGED AND HELD FOR CONTROL PTS 100 AND 101. LEVEL LOOPS WERE RAN FROM CONTROL POINT 100 THRU THE REMAINING CONTROL POINTS
PROJECT PROJECTION FACTOR	1.000083252

REFERENCE CONTROL INFORMATION

COORDINATE SYSTEM	MO COORDINATE SYSTEM OF 1983
CONTROL STATION	MISSOURI CORS
DESIGNATION	MODOT ALBANY CORS ARP
CORS_ID	MOAL
PID	DM4112
LATITUDE	N40°15' 20.75415"
LONGITUDE	W94°17' 58.90379"
NORTHING (M)	453884.890
EASTING (M)	867039.790
ZONE	WEST
PROJECT AVERAGE GRID FACTOR	0.999916755

EXAMPLE OF PROJECT COORDINATE TO S.P.C.

PROJECT NORTHING X AVERAGE GRID FACTOR
= STATE PLANE NORTHING
PROJECT EASTING X AVERAGE GRID FACTOR
= STATE PLANE EASTING

EXAMPLE: CONTROL POINT #100
N 1281220.193 X 0.999916755 = N 1281113.538
E 2924978.969 X 0.999916755 = E 2924730.480

LINEAR UNIT CONVERSION

1 METER = 3.280833333 US SURVEY FEET (USFT)

COORDINATE POINT LISTING							
SHEET NO	STATION	LOCATION	OFFSET (USFT)	MODIFIED STATE PLANE (GROUND)			DESCRIPTION
				NORTHING (US SURVEY FT)	EASTING (US SURVEY FT)	ELEVATION (US SURVEY FT)	
PROJECT CONTROL POINTS							
	38+83.73	RTE. U	26.38' RT.	1,281,220.19	2,924,978.97	845.38	CONTROL POINT
4	46+54.87	RTE. U	20.18' LT.	1,281,386.83	2,925,731.32	827.98	CONTROL POINT
4	53+49.93	RTE. U	14.25' RT.	1,281,527.73	2,926,411.42	828.95	CONTROL POINT
4	54+13.03	RTE. U	12.20' LT.	1,281,561.66	2,926,470.77	831.53	BENCHMARK
4	55+09.82	RTE. U	13.11' RT.	1,281,540.13	2,926,568.43	830.96	BENCHMARK
4	55+41.04	RTE. U	12.92' LT.	1,281,565.26	2,926,600.39	830.12	CONTROL POINT
4	62+79.55	RTE. U	13.55' RT.	1,281,339.17	2,927,304.84	823.91	CONTROL POINT
	68+97.97	RTE. U	26.93' LT.	1,281,354.97	2,927,923.38	822.10	CONTROL POINT
ALIGNMENTS							
	30+00.00	☒ RTE. U	0	1,281,252.72	2,924,095.45		POINT ON TANGENT
4	39+88.70	☒ RTE. U	0	1,281,245.84	2,925,084.13		P.C.
4	43+73.25	☒ RTE. U	0	1,281,294.52	2,925,464.50		P.T.
4	45+95.00	☒ RTE. U	0	1,281,351.86	2,925,678.71		BEGIN PROJECT
4	52+48.48	☒ RTE. U	0	1,281,520.82	2,926,309.96		P.C.
4	54+08.01	☒ RTE. U	0	1,281,549.05	2,926,466.86		FILL EXCEPTION
4	55+16.60	☒ RTE. U	0	1,281,553.13	2,926,575.37		FILL EXCEPTION
4	58+48.44	☒ RTE. U	0	1,281,489.77	2,926,899.56		P.T.
4	60+51.41	☒ RTE. U	0	1,281,416.98	2,927,089.03		P.C.
4	63+00.00	☒ RTE. U	0	1,281,348.36	2,927,327.68		END PROJECT
4	66+06.31	☒ RTE. U	0	1,281,322.01	2,927,632.33		P.T.
	69+70.57	☒ RTE. U	0	1,281,329.55	2,927,996.51		POINT ON TANGENT

STATE OF MISSOURI

RICHARD W. ORR

NUMBER PE-2002003397

PROFESSIONAL ENGINEER

THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY.

DATE PREPARED 11/26/2025

ROUTE U

DISTRICT NW

STATE MO

SHEET NO. 5

COUNTY CALDWELL

JOB NO. JNW0144

CONTRACT ID.

PROJECT NO.

BRIDGE NO.

DESCRIPTION

DATE

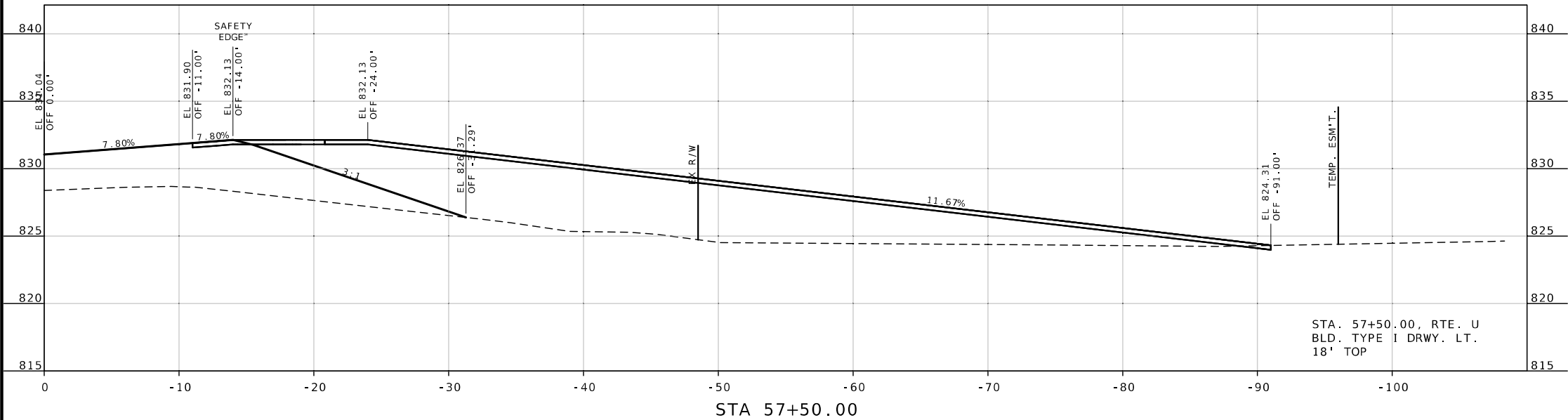
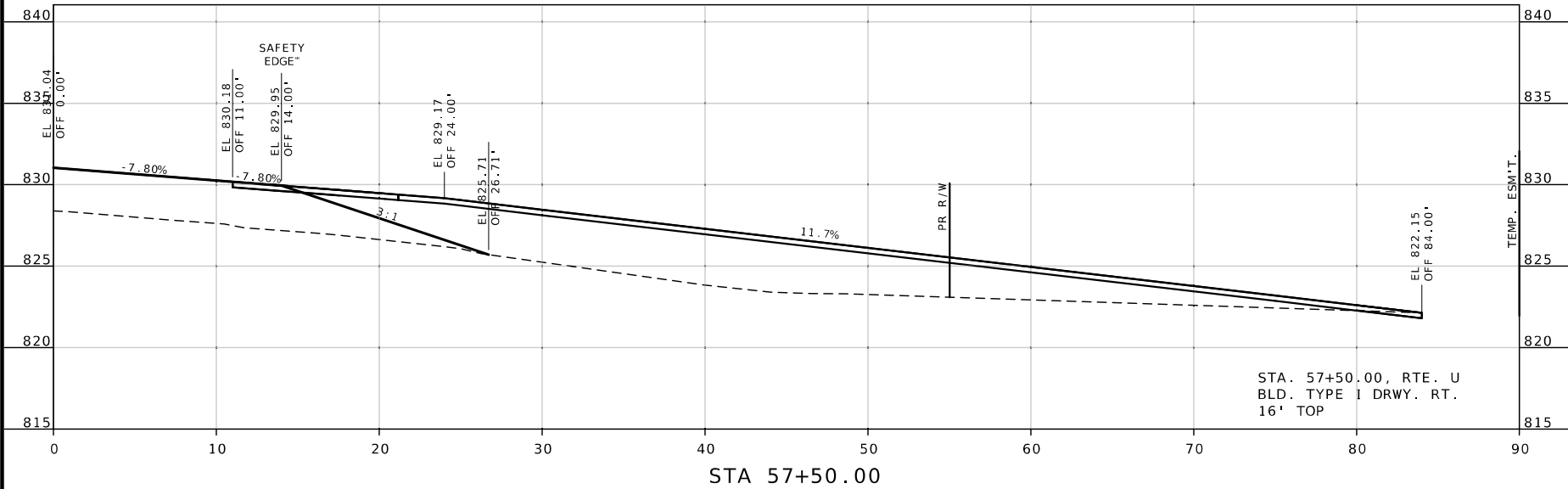
MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

MODOT

105 WEST CAPITOL JEFFERSON CITY, MO 65102 1-888-ASK-MODOT (1-888-275-6636)

COORDINATE POINTS

SHEET 1 OF 1



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DATE PREPARED
12/18/2025

ROUTE	STATE
U	MO
DISTRICT	SHEET NO.
NW	7

COUNTY
CALDWELL

JOB NO.
JNW0144

CONTRACT ID.

PROJECT NO.

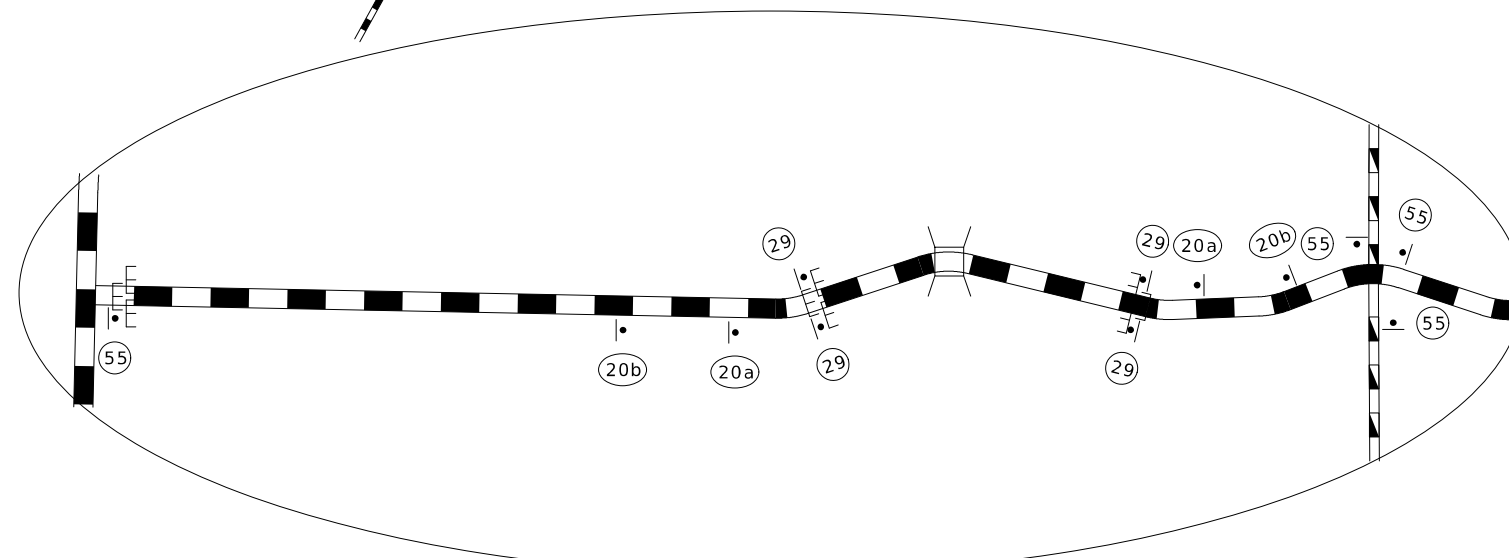
BRIDGE NO.

DATE	DESCRIPTION

MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION

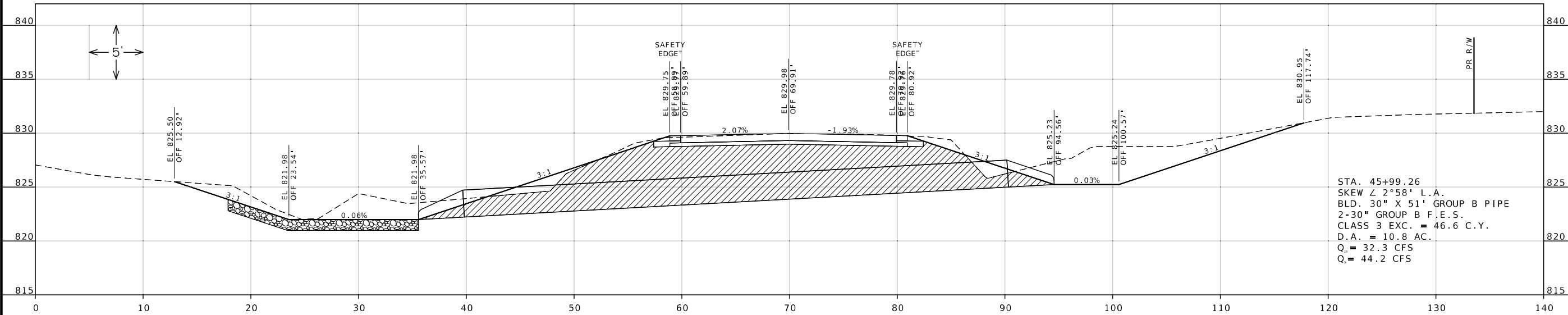


105 WEST CAPITOL
JEFFERSON CITY, MO 65102
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DETAIL A

SHEET 1 OF 1



STA 45+99.26



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11/26/2025

ROUTE U STATE MO

DISTRICT NW SHEET NO. 11

COUNTY
CALDWELL

JOB NO.
JNW0144

CONTRACT ID.

PROJECT NO.

BRIDGE NO.

DATE	DESCRIPTION

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JEFFERSON CITY, MO 65102
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CULVERT SECTION

SHEET 1 OF 1

Foundation Data					
Type	Design Data	Bent Number			
		1	2	3	4
Load Bearing Pile	Pile Type and Size	CECIP 16"	CECIP 16"	CECIP 16"	CECIP 16"
	Number ea	4	5	5	4
	Approximate Length Per Each ft	39	42	42	25
	Pile Point Reinforcement ea	ALL	ALL	ALL	ALL
	Min. Galvanized Penetration (Elev.) ft	Full Length	Full Length	Full Length	Full Length
	Est. Max. Scour Depth 100 (Elev.) ft	-	802.9	803.8	-
	Minimum Tip Penetration (Elev.) ft	803.46	783.56	783.73	802.86
	Criteria for Min. Tip Penetration	Lat Stability	Scour	Scour	Lat Stability
	Pile Driving Verification Method	WEAP	WEAP	WEAP	WEAP
	Resistance Factor	0.5	0.5	0.5	0.5
	Minimum Nominal Axial Compressive Resistance kip	295	319	319	294

CECIP= Closed Ended Cast-In-Place concrete pile

WEAP= Wave Equation Analysis of Piles

Load Bearing Pile:

Minimum Nominal Axial Compressive Resistance = Maximum Factored Loads/Resistance Factor

Pile point reinforcement need not be galvanized. Shop drawings will not be required for pile point reinforcement.

All piles shall be galvanized down to the minimum galvanized penetration (elevation).

The contractor shall make every effort to achieve the minimum galvanized penetration (elevation) shown on the plans for all piles. Deviations in penetration less than 5 feet of the minimum will be considered acceptable provided the contractor makes the necessary corrections to ensure the minimum penetration is achieved on subsequent piles.

Prebore for piles at Bents No. 2 and 3 to elevations 802.9 and 803.8, respectively.

Estimated Maximum Scour Depth (Elevation) shown is for verifying Minimum Nominal Axial Compressive Resistance using dynamic testing only where pile resistance contribution above this elevation shall not be considered.

Hydrologic Data
Drainage Area = 11 mi ²
Design Flood Frequency = 15.5 years
Design Flood Discharge = 2800 cfs
Design Flood (D.F.) Elevation = 822.5
Base Flood (100-year)
Base Flood Elevation = 826.1
Base Flood Discharge = 4600 cfs
Estimated Backwater = 0.6 ft
Average Velocity thru Opening = 5.4 ft/s
Freeboard (50-year)
Freeboard = 1.1 ft
Roadway Overtopping
Overtopping Flood Discharge = 2900 cfs
Overtopping Flood Frequency = 17.5 years
Overtopping Flood Elevation = 823.7

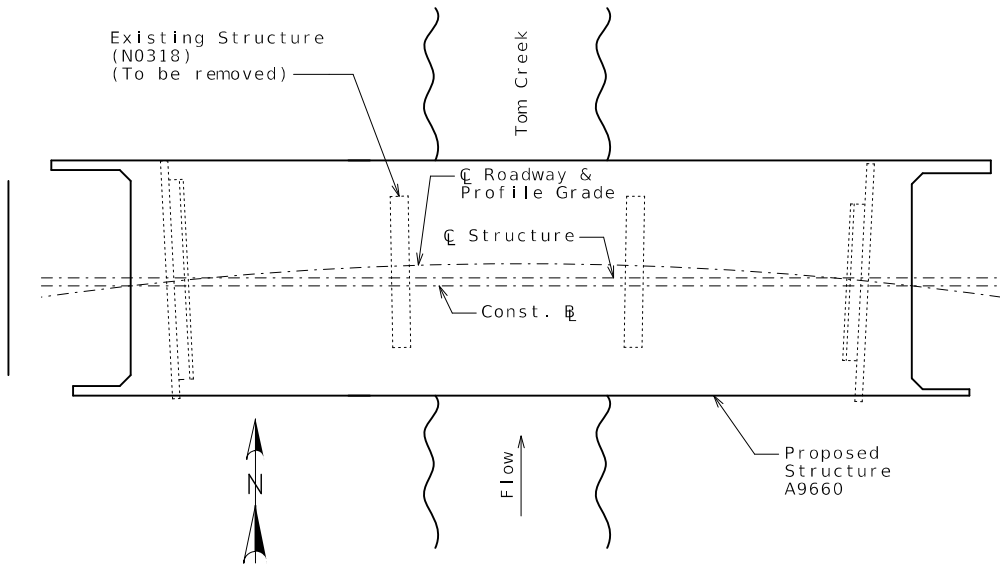
Estimated Quantities			
Item	Substr.	Superstr.	Total
Class 1 Excavation cu. yard	90		90
Removal of Bridges lump sum			1
Bridge Approach Slab (Minor) sq. yard		136	136
Galvanized Cast-In-Place Concrete Piles (16 in.) linear foot	676		676
Pile Wave Analysis each	4		4
Pre-Bore for Piling linear foot	135		135
Pile Point Reinforcement each	18		18
Class B Concrete (Substructure) cu. yard	55.1		55.1
Slab on Concrete I-Girder sq. yard		390	390
Type D Barrier linear foot		255	255
Type 2 (32 in.), P/S Concrete I-Girder linear foot		419	419
Reinforcing Steel (Bridges) pound	4190		4190
Slab Drain each		8	8
Vertical Drain at End Bents each			2
Plain Neoprene Bearing Pad each		8	8
Laminated Neoprene Bearing Pad each		16	16

All concrete above the construction joint in the end bents is included in the Estimated Quantities for Slab on Concrete I-Girder.

All reinforcement in the end bents and all reinforcement in cast-in-place pile at end bents is included in the Estimated Quantities for Slab on Concrete I- Girder.

All reinforcement in the intermediate bent concrete diaphragms except reinforcement embedded in the beam cap is included in the Estimated Quantities for Slab on Concrete I-Girder.

All concrete above the intermediate beam cap is included in the Estimated Quantities for Slab on Concrete I-Girder.



LOCATION SKETCH

Estimated Quantities for Slab on Concrete I-Girder	
Item	Total
Class B-2 Concrete cu. yard	112.4
Reinforcing Steel (Epoxy Coated) pound	28,600

The table of Estimated Quantities for Slab on Concrete I-Girder represents the quantities used by the State in preparing the cost estimate for concrete slabs. The area of the concrete slab will be measured to the nearest square yard longitudinally from end of slab to end of slab and transversely from out to out of bridge slab (or with the horizontal dimensions as shown on the plan of slab). Payment for prestressed panels, conventional forms, all concrete and epoxy coated reinforcing steel will be considered completely covered by the contract unit price for the slab. Variations may be encountered in the estimated quantities but the variations cannot be used for an adjustment in the contract unit price.

Method of forming the slab shall be as shown on the plans and in accordance with Sec 703. All hardware for forming the slab to be left in place as a permanent part of the structure shall be coated in accordance with ASTM A123 or ASTM B633 with a thickness class SC 4 and a finish type 1, II or III.

Class B-2 Concrete quantity is based on minimum top flange thickness and minimum joint material thickness.

The prestressed panel quantities are not included in the table of Estimated Quantities for Slab on Concrete I-Girder.

General Notes:

Design Specifications:

2023 AASHTO Guide Specifications for LRFD Seismic Bridge Design (3rd Ed.)
Seismic Design Category = A (Nonseismic)
Design earthquake response spectral acceleration coefficient at 1.0 second period, SD1 < 0.15
Acceleration Coefficient (effective peak ground acceleration coefficient), As = 0.048g

Design Loading:

Vehicular = HL-93
Future Wearing Surface = 35 lb/sf
Earth = 120 lb/cf
Equivalent Fluid Pressure = 45 lb/cf (Min.)
Superstructure: Simply-Supported, Non-Composite for dead load. Continuous Composite for live load.

Design Unit Stresses:

Class B Concrete (Substructure) f'c = 3,000 psi
Class B-1 Concrete (Barrier) f'c = 4,000 psi
Class B-2 Concrete (Superstructure, except Prestressed Girders and Barrier) f'c = 4,000 psi
Reinforcing Steel (ASTM A615 Grade 60) fy = 60,000 psi

Welded or Seamless steel shell (pipe) for CIP pile (ASTM A252 Modified Grade 3) fy = 50,000 psi

For precast prestressed panel stresses, see Sheet No. 16.

For prestressed girder stresses, see Sheets No. 13 & 14.

Neoprene Pads:

Neoprene bearing pads shall be 60 durometer and shall be in accordance with Sec 716.

Joint Filler

All joint filler shall be in accordance with Sec 1057 for preformed sponge rubber expansion and partition joint filler, except as noted.

Reinforcing Steel:

Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise shown.

Minimum clearance between galvanized piles and uncoated (plain) reinforcing steel including bar supports shall be 1 1/2". Nylon, PVC, or polyethylene spacers shall be used to maintain clearance. Nylon cable ties shall be used to bind the spacers to the reinforcement.

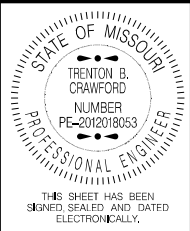
Traffic Handling:

Structure to be closed during construction. Traffic to be maintained on other routes. See roadway plans for traffic control.

Miscellaneous:

MoDOT Construction personnel will indicate the type of joint filler option used under the precast panels for this structure:

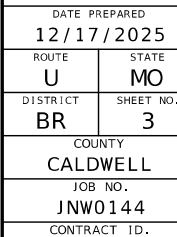
- ☐ Constant Joint Filler
☐ Variable Joint Filler



DATE PREPARED 12/17/2025	
ROUTE U	STATE MO
DISTRICT BR	SHEET NO. 2
COUNTY CALDWELL	
JOB NO. JNW0144	
CONTRACT ID.	
PROJECT NO.	
BRIDGE NO. A9660	

DESCRIPTION	DATE

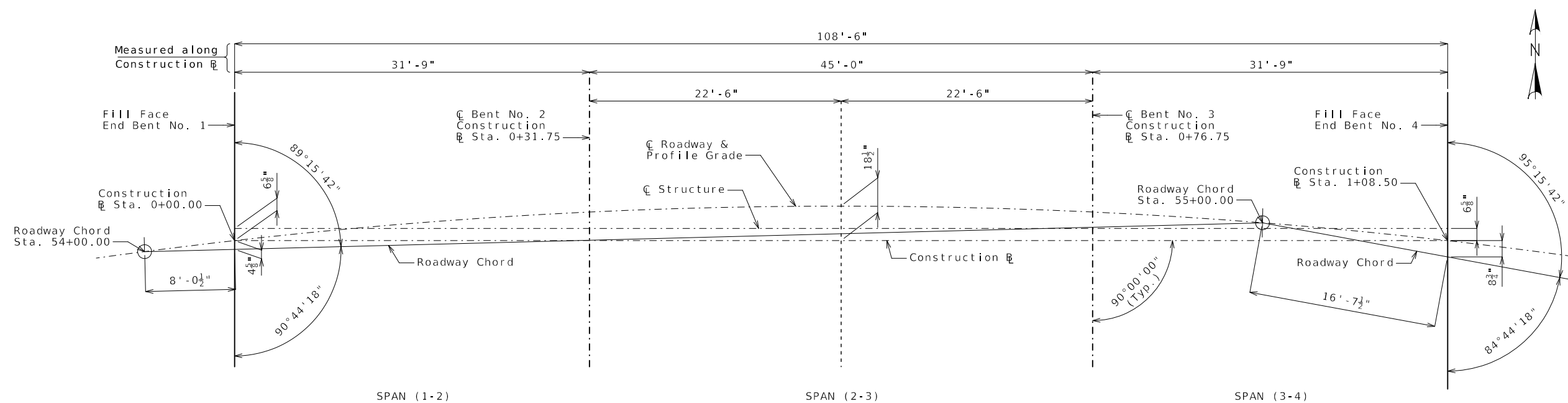




PROJECT NO.
BRIDGE NO. A9660

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PI	55+58.90
PC	52+48.48
PT	58+48.50
Δ	36°0'0.0" (RT)
D	5°59'59.9"
L	600.28' (CHORD)
T	310.42' (CHORD)
R	955.37'

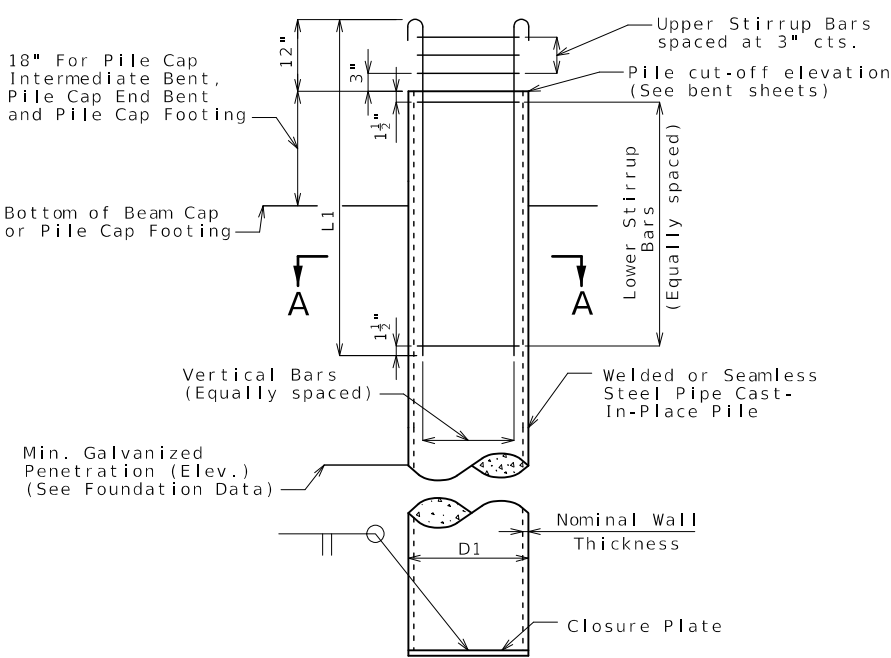
Note:
All dimensions are horizontal.

SUBSTRUCTURE LAYOUT

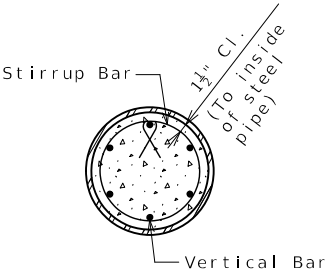
Detailed Nov. 2025
Checked Nov. 2025

Note: This drawing is not to scale. Follow dimensions.

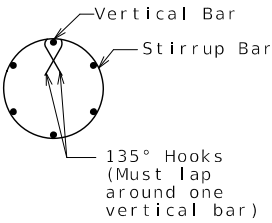
Sheet No. 3 of 30



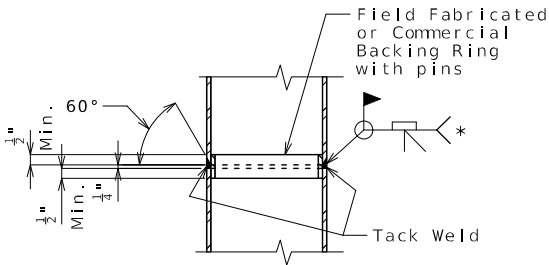
GALVANIZED CLOSED ENDED CAST-IN-PLACE (CECIP) CONCRETE PILE WITHOUT PILE POINT REINFORCEMENT



SECTION A-A

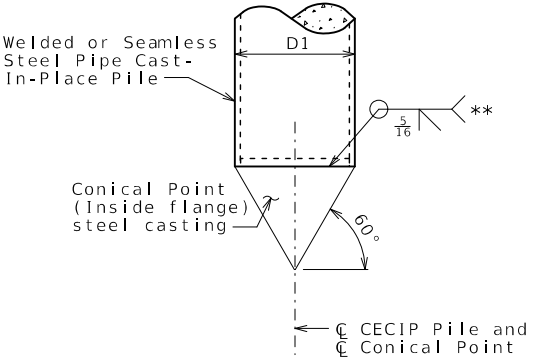


DETAIL OF SEISMIC STIRRUP BAR



STEEL PIPE PILE SPLICE

* Galvanizing material shall be omitted or removed one inch clear of weld locations in accordance with Sec 702.



MANUFACTURED CONICAL PILE POINT

(Omit closure plate)

** If the conical pile point is not pre-beveled, place a 3/8" bevel at 40 degrees on the pipe.

Notes:

Welded or seamless steel shell (pipe) shall be ASTM A252 Modified Grade 3 (fy = 50,000 psi) with physical and chemical requirements that meet ASTM A572 Grade 50. Pipe certification and source material certification shall be required.

Concrete for cast-in-place pile shall be Class B-1.

Steel for closure plate shall be ASTM A709 Grade 50.

Steel casting for conical pile point reinforcement shall be ASTM A148 Grade 90-60.

The minimum wall thickness of any spot or local area of any type shall not be more than 12.5% under the specified nominal wall thickness.

The contractor shall determine the pile wall thickness required to avoid damage from all driving activities, but wall thickness shall not be less than the minimum specified. No additional payment will be made for furnishing a thicker pile wall than specified on the plans.

Closure plate shall not project beyond the outside diameter of the pipe pile. Satisfactory weldments may be made by beveling tip end of pipe or by use of inside backing rings. In either case, proper gaps shall be used to obtain weld penetration full thickness of pipe. Payment for furnishing and installing closure plate will be considered completely covered by the contract unit price for Galvanized Cast-In-Place Concrete Piles.

Splices of pipe for cast-in-place concrete pile shall be made watertight and to the full strength of the pipe above and below the splice to permit hard driving without damage. Pipe damaged during driving shall be replaced without cost to the state. Pipe sections used for splicing shall be at least 5 feet in length.

At the contractor's option, the hooks of vertical bars embedded in the beam cap may be oriented inward or outward.

Closure plate need not be galvanized.

Reinforcing steel for cast-in-place piles is included in the Bill of Reinforcing Steel.

All reinforcement for cast-in-place pile at end bents is included in the Estimated Quantities for Slab on Concrete I-Girder. Reinforcement for cast-in-place pile at intermediate bents is included in the substructure quantity tables.

For Foundation Data table, see Sheet No. 2.

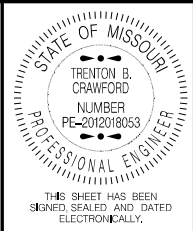
Galvanized Closed Ended Cast-In-Place (CECIP) Concrete Pile Data				
Bent Number	1	2	3	4
D1, CECIP Pile (O.D.)	16"	16"	16"	16"
Min. Nominal Wall Thickness	1/2"	1/2"	1/2"	1/2"
Closure Plate Thickness	3/4"	3/4"	3/4"	3/4"
Pile Point Reinforcement	Conical	Conical	Conical	Conical
Vertical Bars	6-#6-V105	6-#6-V200	6-#6-V300	6-#6-V405
L1, Length of Vertical Bars	6'-3"	6'-3"	6'-3"	6'-3"
Upper Stirrup Bars	3-#4-P100	3-#4-P200	3-#4-P300	3-#4-P400
Lower Stirrup Bars	5-#4-P100	5-#4-P200	5-#4-P300	5-#4-P400

GALVANIZED CLOSED ENDED CAST-IN-PLACE (CECIP) CONCRETE PILE

Detailed Aug. 2025
Checked Oct. 2025

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 4 of 30

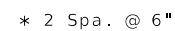


DATE PREPARED 12/17/2025	
ROUTE U	STATE MO
DISTRICT BR	SHEET NO. 4
COUNTY CALDWELL	
JOB NO. JNW0144	
CONTRACT ID.	
PROJECT NO.	
BRIDGE NO. A9660	

DESCRIPTION	DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

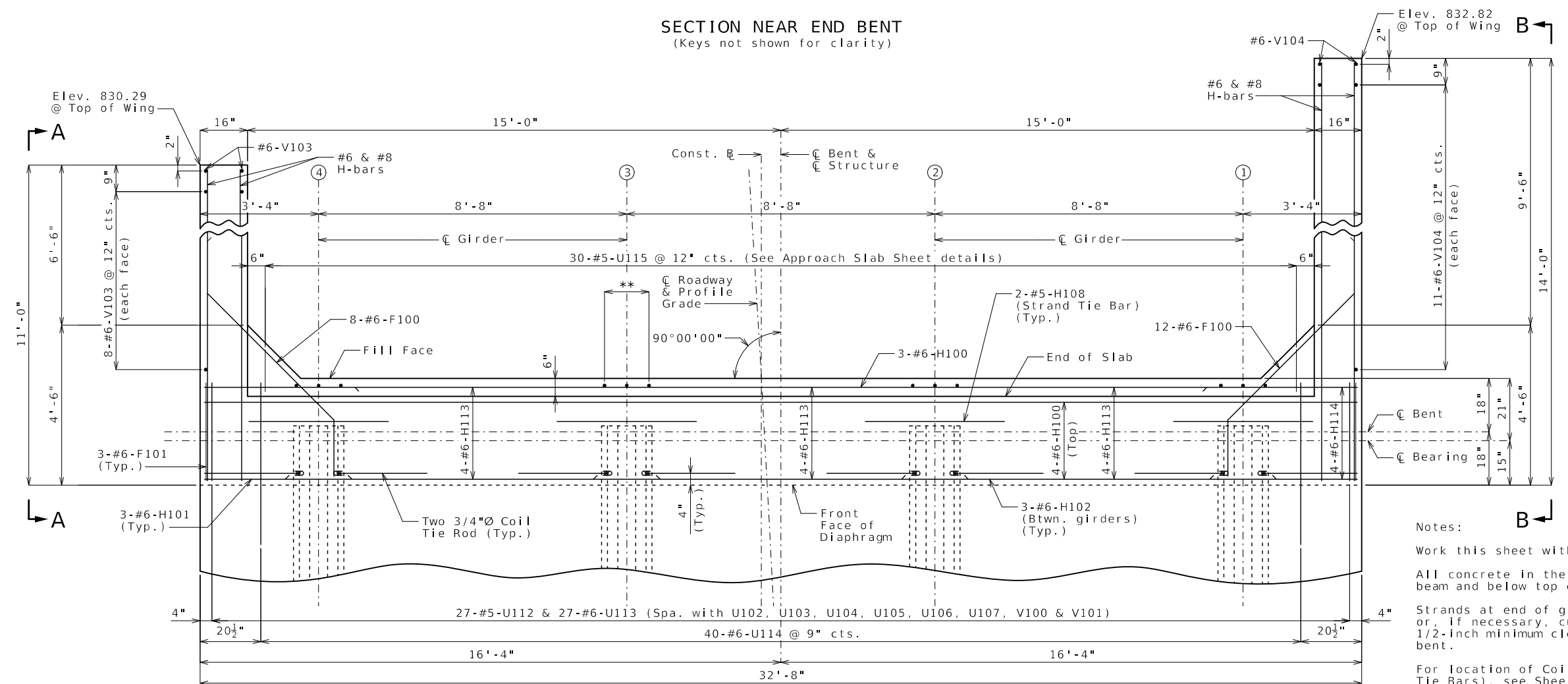
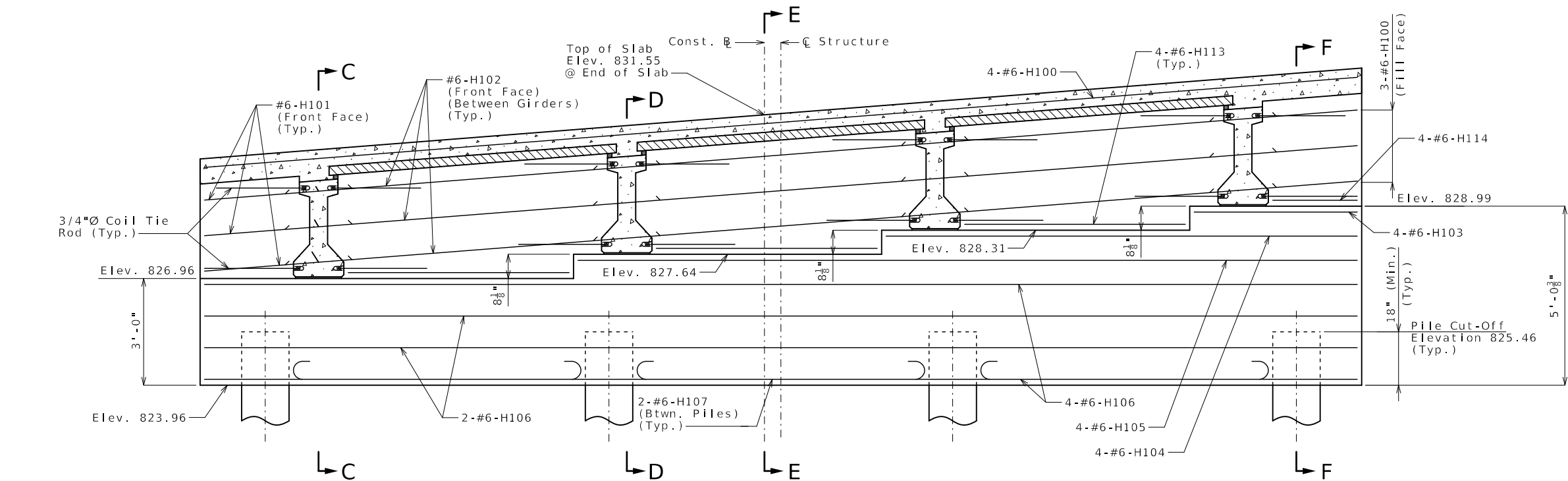
105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-273-6636)



Keys not shown for clarity

All U bars and pairs of V bars shall be placed parallel to centerline of structure.

These quantities are included in the Estimated Quantities table on Sheet No. 2.



** 3-#6-V102 @ 9" cts. (Typ.)

Notes:

Work this sheet with Sheets No. 5 & 7.

All concrete in the end bent above top of beam and below top of slab shall be Class B-2.

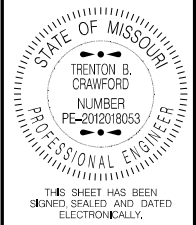
Strands at end of girders shall be field bent or, if necessary, cut in field to maintain 1 1/2-inch minimum clearance to fill face of end bent.

For location of Coil Ties & #5-H108 (Strand Tie Bars), see Sheet No. 14.

The #6-F100 bars shall be bent in field to clear girders.

For details of bridge approach slab, see Sheet No. 25.

For details of vertical drain at end bent, see Sheet No. 8.



DATE PREPARED
12/17/2025

ROUTE	STATE
U	MO

DISTRICT BR	SHEET NO. 6
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COUNTY
CALDWELL

JOB NO.
JNW0144

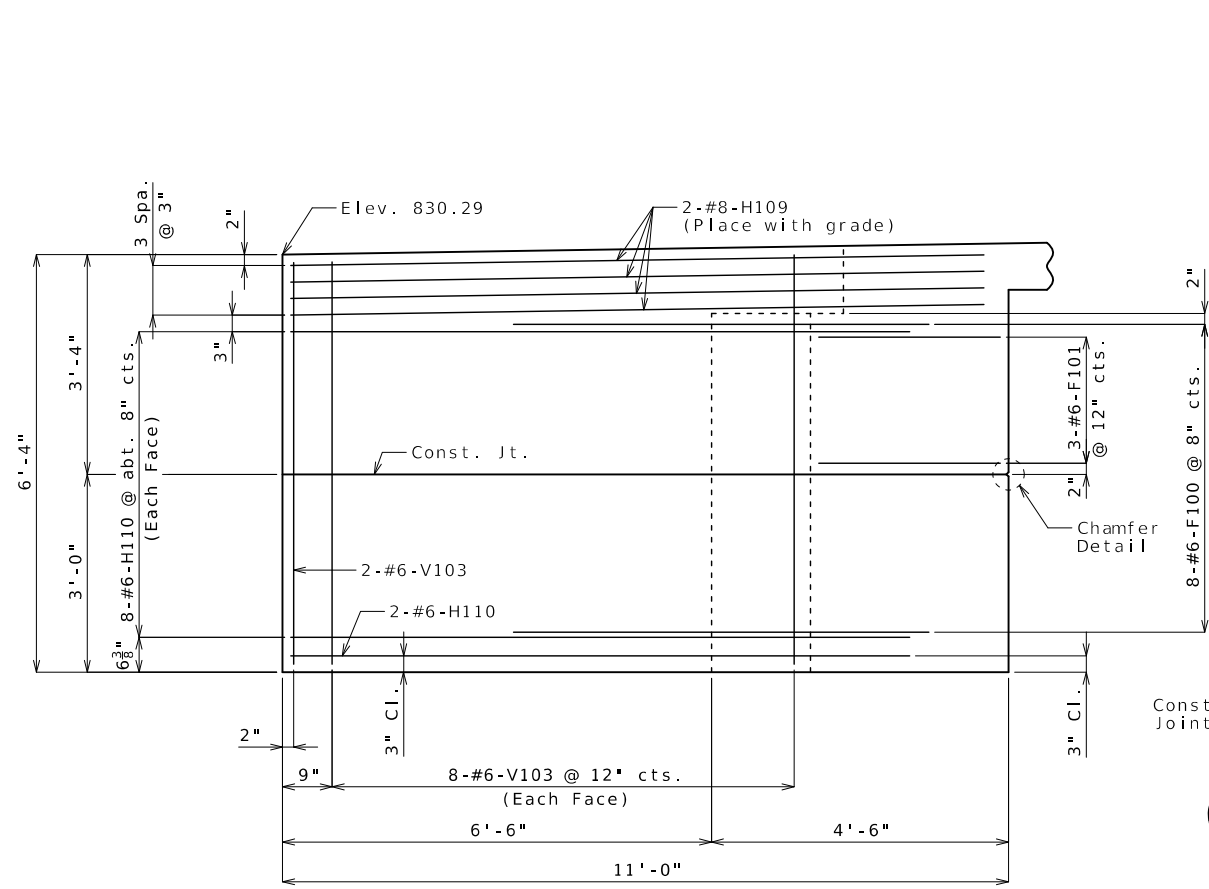
CONTRACT ID.

PROJECT NO.

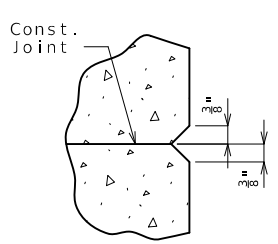
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A9660[illegible]

MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION

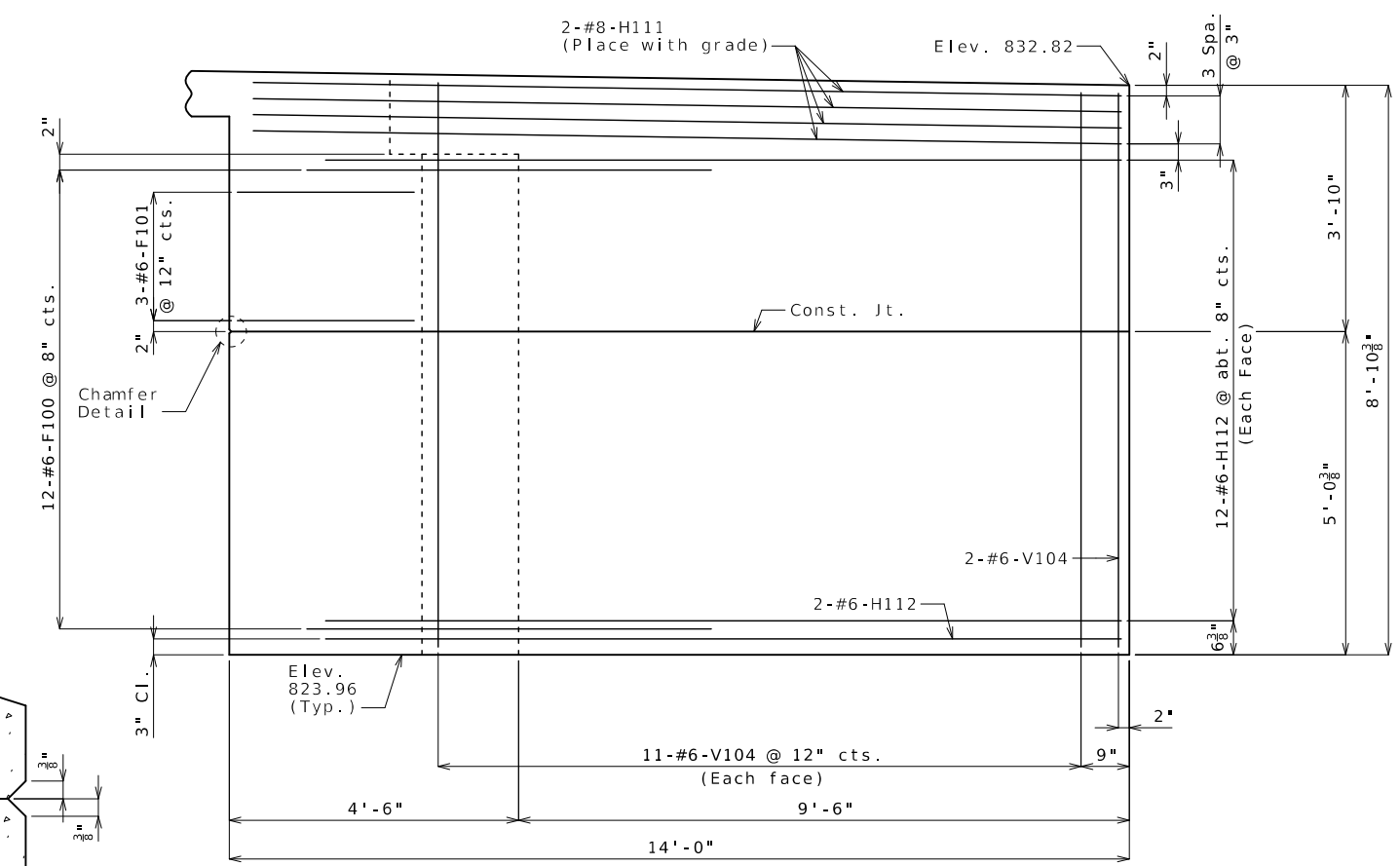
105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)



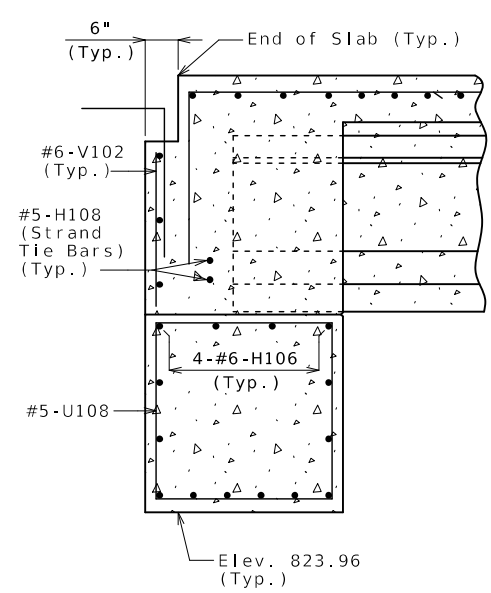
ELEVATION A-A



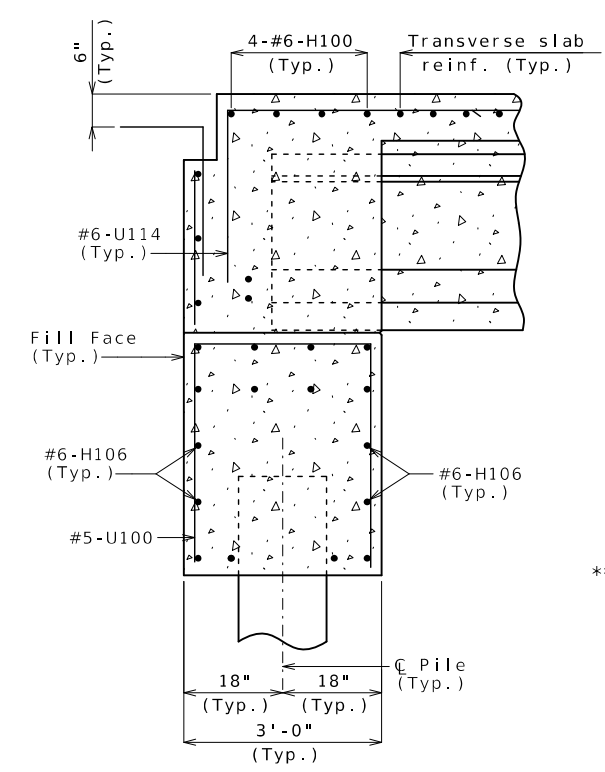
CHAMFER DETAIL



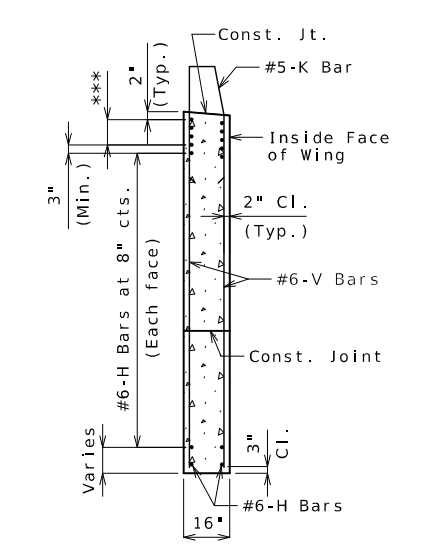
ELEVATION B-B



SECTION C-C



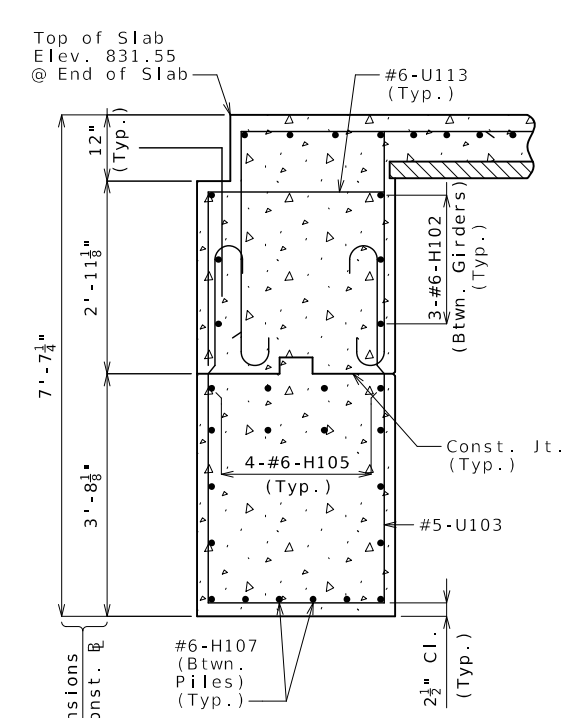
SECTION D-D



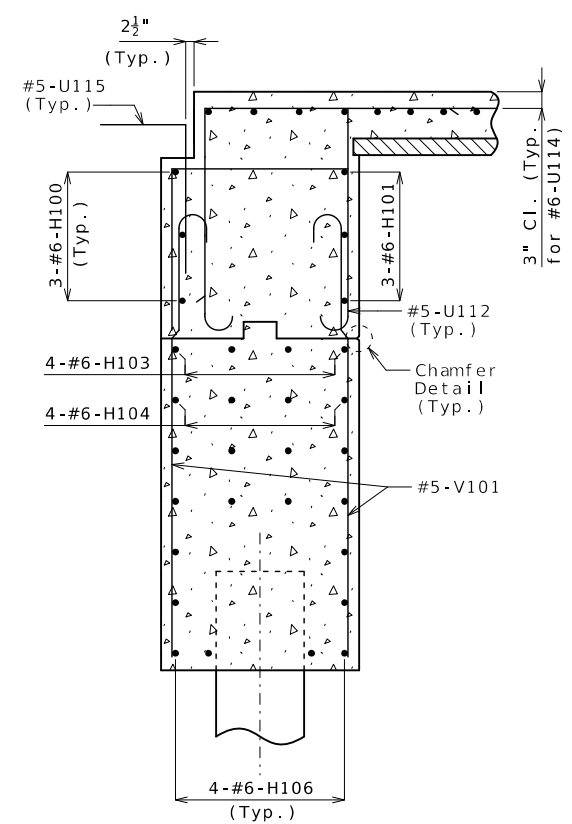
TYPICAL SECTION THRU WING

*** #8-H Bars at 3" cts. (Each face) (Place with grade)
(Left wing shown, right wing similar)

Notes:
Work this sheet with Sheets No. 5 & 6.
For reinforcement of the barrier, see Sheets No. 23 & 24.



SECTION E-E



SECTION F-F

END BENT NO. 1

Detailed Sep. 2025
Checked Oct. 2025

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 7 of 30

STATE OF MISSOURI
TRENTON B. CRAWFORD
NUMBER PE-2012018053
PROFESSIONAL ENGINEER

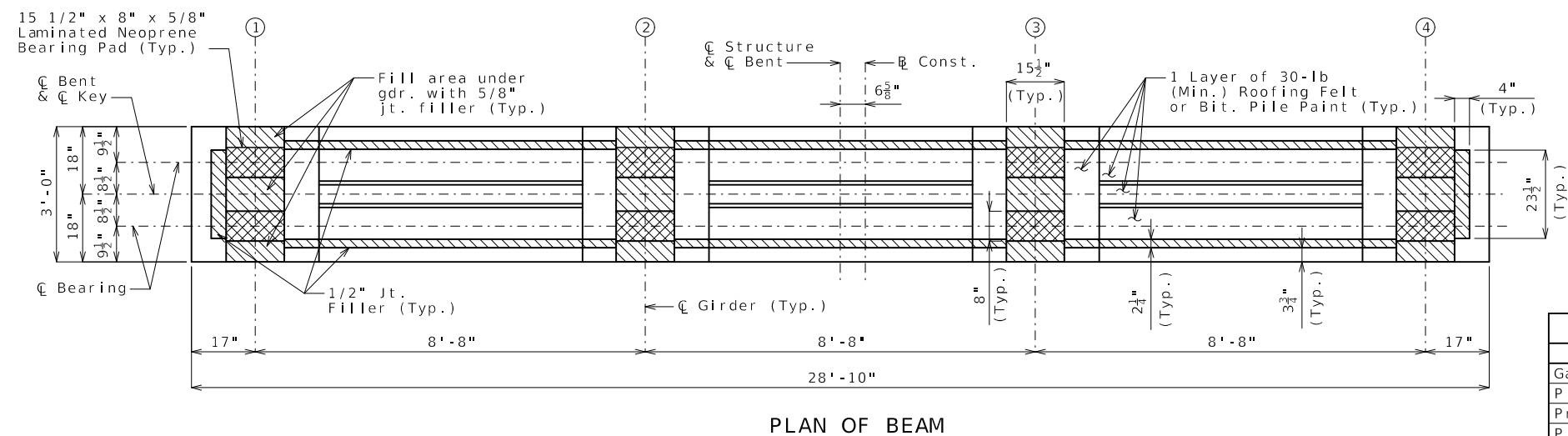
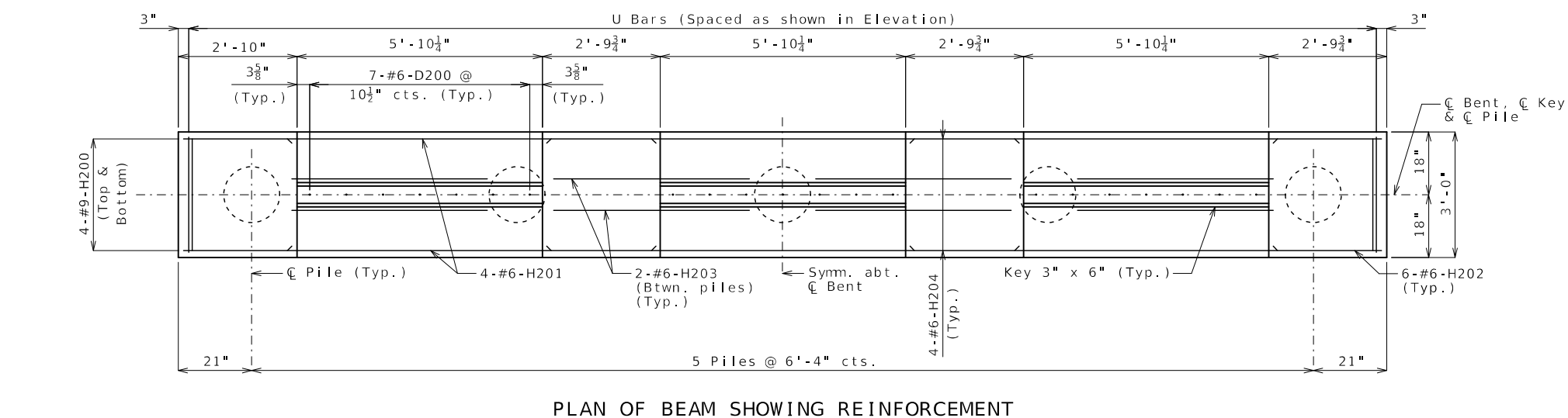
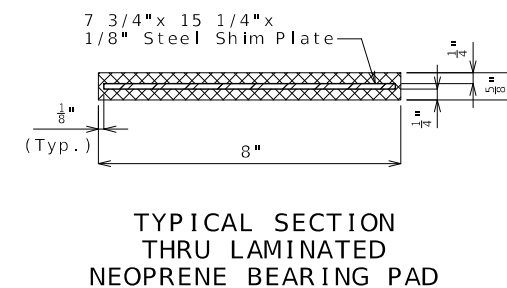
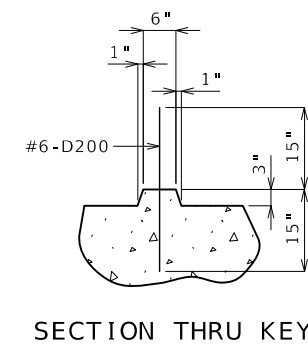
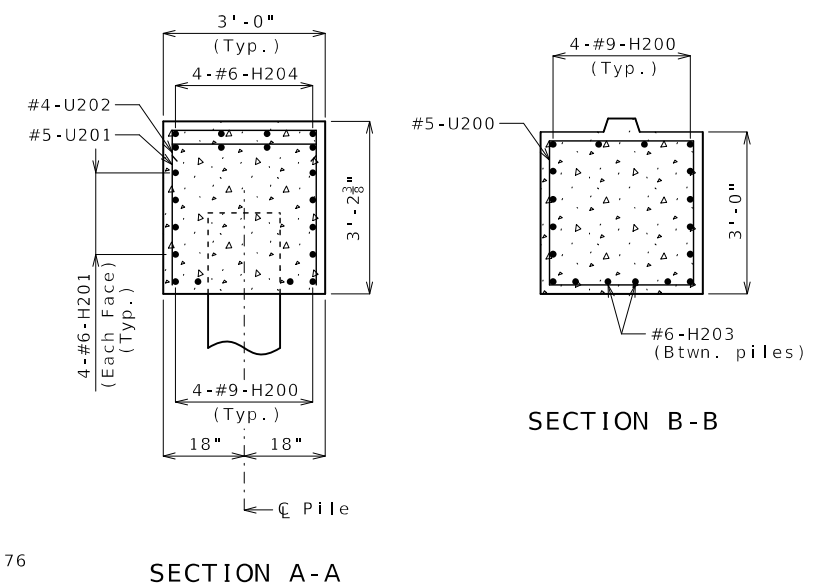
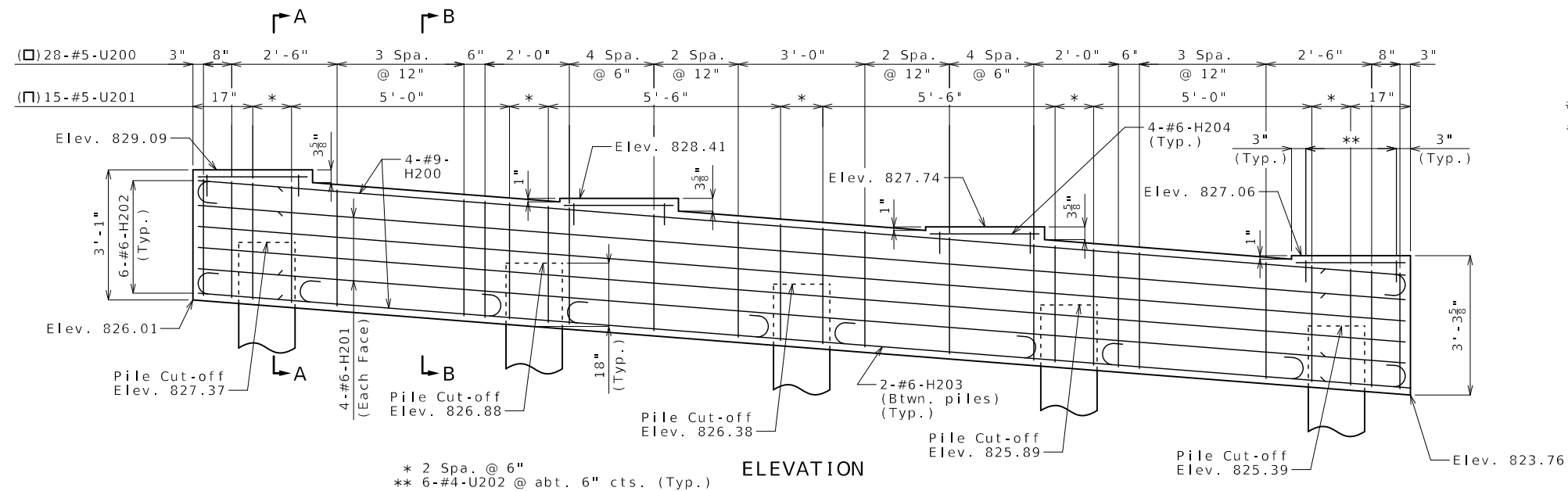
THIS SHEET HAS BEEN
SIGNED, SEALED AND DATED
ELECTRONICALLY.

DATE PREPARED 12/17/2025	
ROUTE U	STATE MO
DISTRICT BR	SHEET NO. 7
COUNTY CALDWELL	
JOB NO. JNW0144	
CONTRACT ID.	
PROJECT NO.	
BRIDGE NO. A9660	

DATE	DESCRIPTION

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-273-6636)

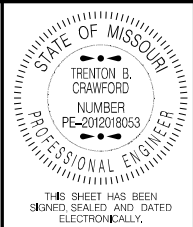


Notes:

For steps 2 inches or more, use 2 1/4 x 1/2 inch joint filler up vertical face.

Reinforcing steel shall be shifted to clear piles.
U bars shall clear piles by at least 1 1/2 inches.

Substructure Quantity Table for Int. Bent No. 2		
Item		Quantity
Galvanized Cast-In-Place Concrete Piles (16 in.)	linear foot	210
Pile Wave Analysis	each	1
Pre-Bore for Piling	linear foot	55
Pile Point Reinforcement	each	5
Class B Concrete (Substructure)	cu. yard	9.6
Reinforcing Steel (Bridges)	pound	2090



DATE PREPARED
12/17/2025

ROUTE	STATE
LI	MO

U	MO
DISTRICT	SHEET NO.
BR	9

COUNTY

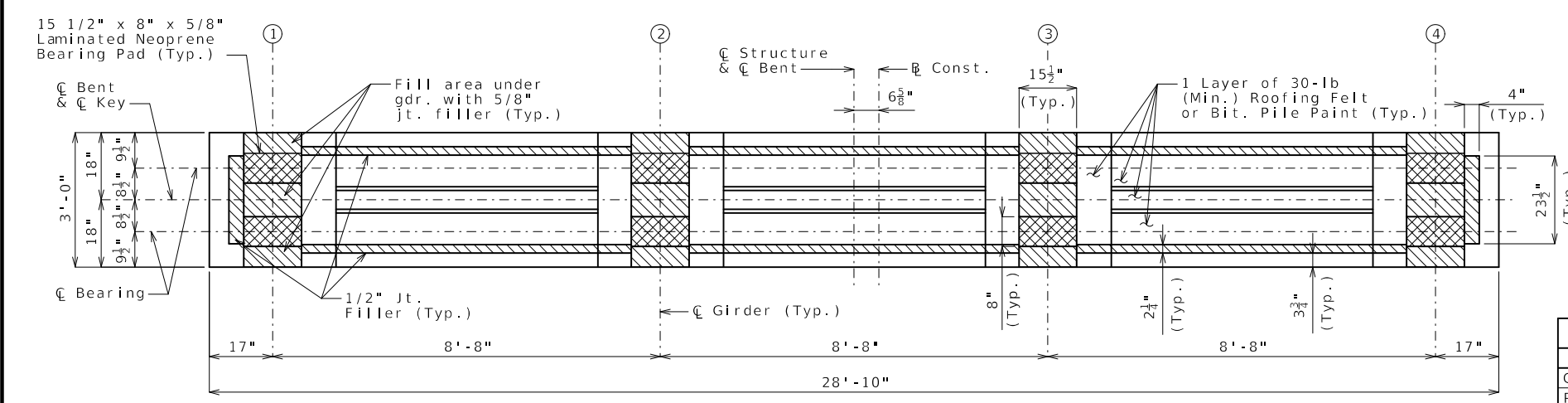
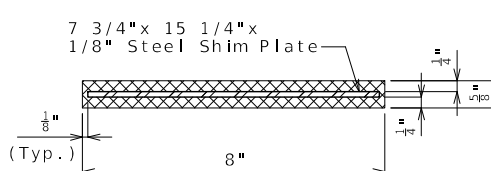
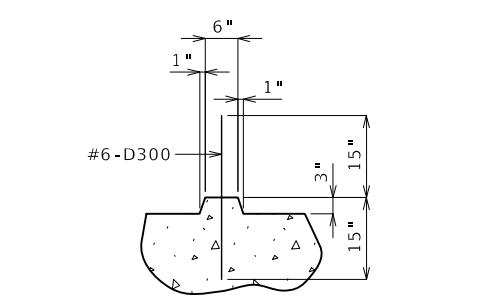
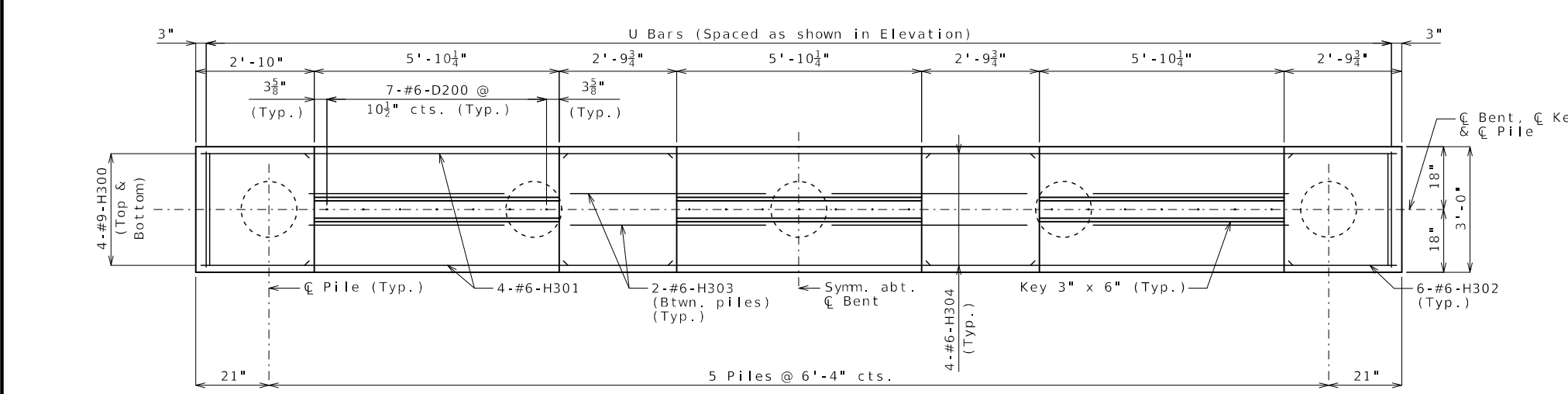
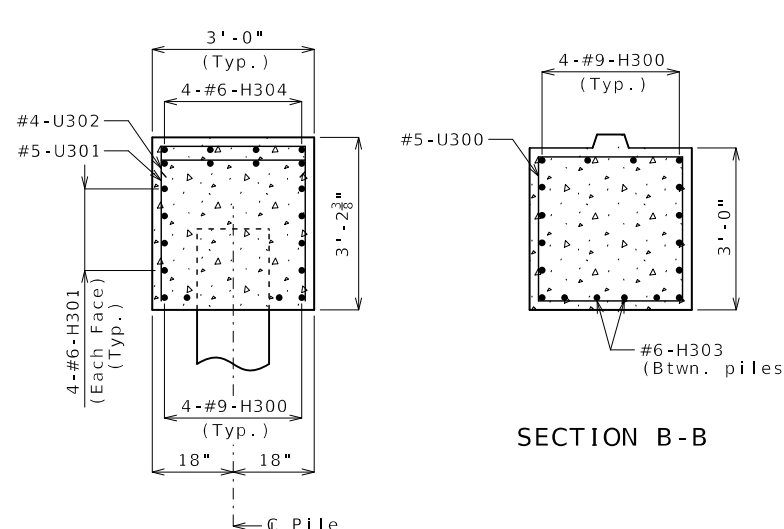
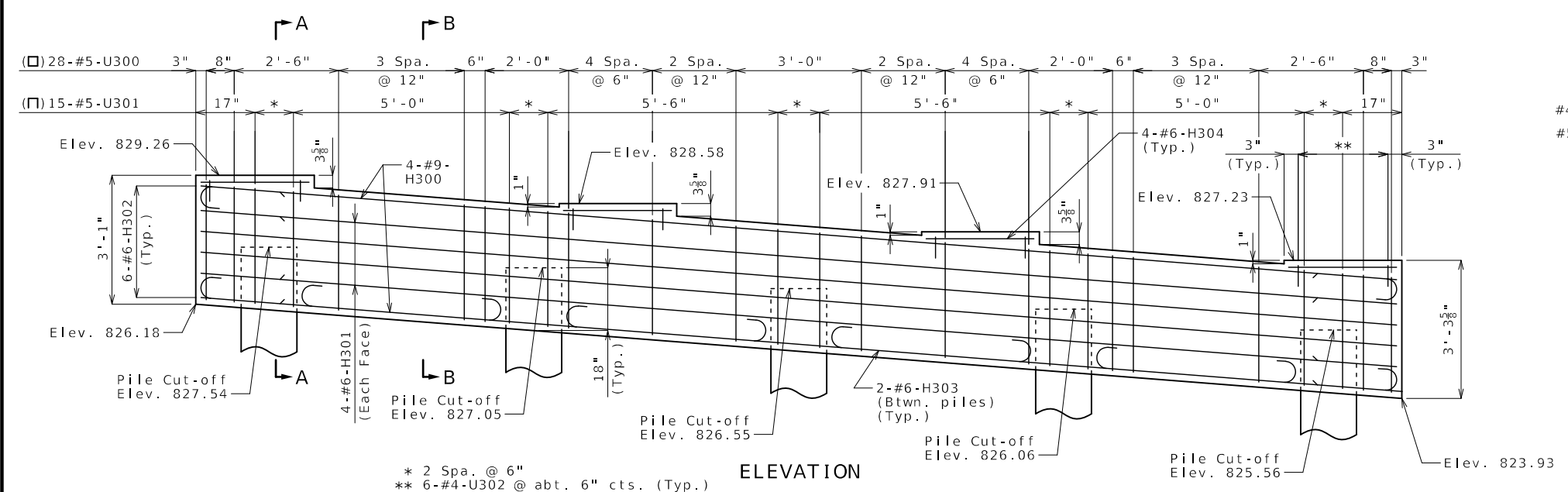
Caldwell

JOB NO.
INW0144

PROJECT NO.

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Notes:

For steps 2 inches or more, use 2 1/4 x 1/2 inch joint filler up vertical face.

Reinforcing steel shall be shifted to clear piles. U bars shall clear piles by at least 1 1/2 inches.

Substructure Quantity Table for Int. Bent No. 3			
Item			Quantity
Galvanized Cast-In-Place Concrete Piles (16 in.)	linear foot	210	
Pile Wave Analysis	each	1	
Pre-Bore for Piling	linear foot	80	
Pile Point Reinforcement	each	5	
Class B Concrete (Substructure)	cu. yard	9.6	
Reinforcing Steel (Bridges)	pound	2090	

Detailed Oct. 2025
Checked Oct. 2025

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 10 of 30

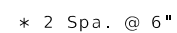
STATE OF MISSOURI
TRENTON B. CRAWFORD
NUMBER PE-2012018053
PROFESSIONAL ENGINEER

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DATE PREPARED 12/17/2025	
ROUTE U	STATE MO
DISTRICT BR	SHEET NO. 10
COUNTY CALDWELL	
JOB NO. JNW0144	
CONTRACT ID.	
PROJECT NO.	
BRIDGE NO. A9660	
DESCRIPTION	DATE

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PLAN OF BEAM SHOWING REINFORCEMENT

Keys not shown for clarity

Notes:

Work this sheet with Sheets No. 12 & 13.

For details of Vertical Drain at End Bent, see Sheet No. 8.

Reinforcing steel shall be shifted to clear piles. U bars shall clear piles by at least 1 1/2 inches.

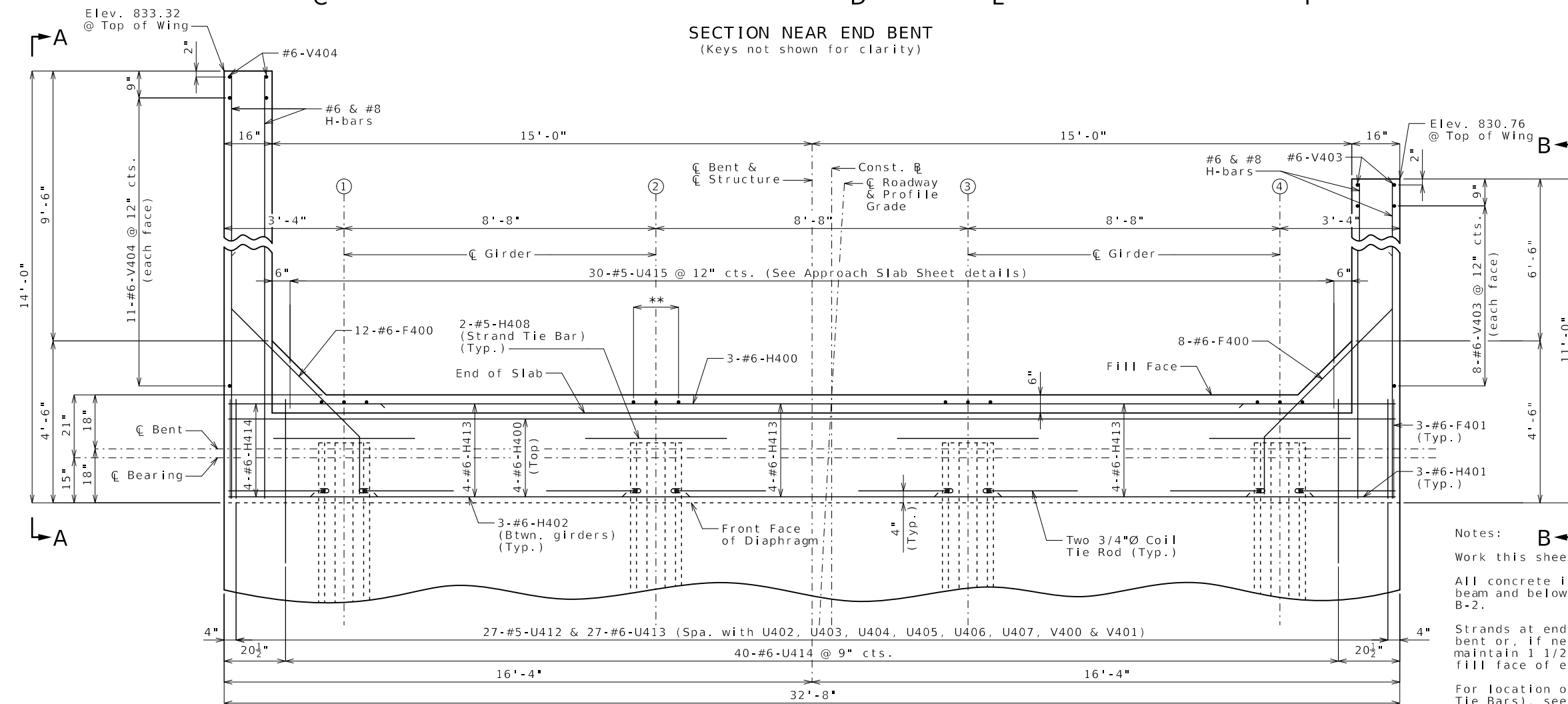
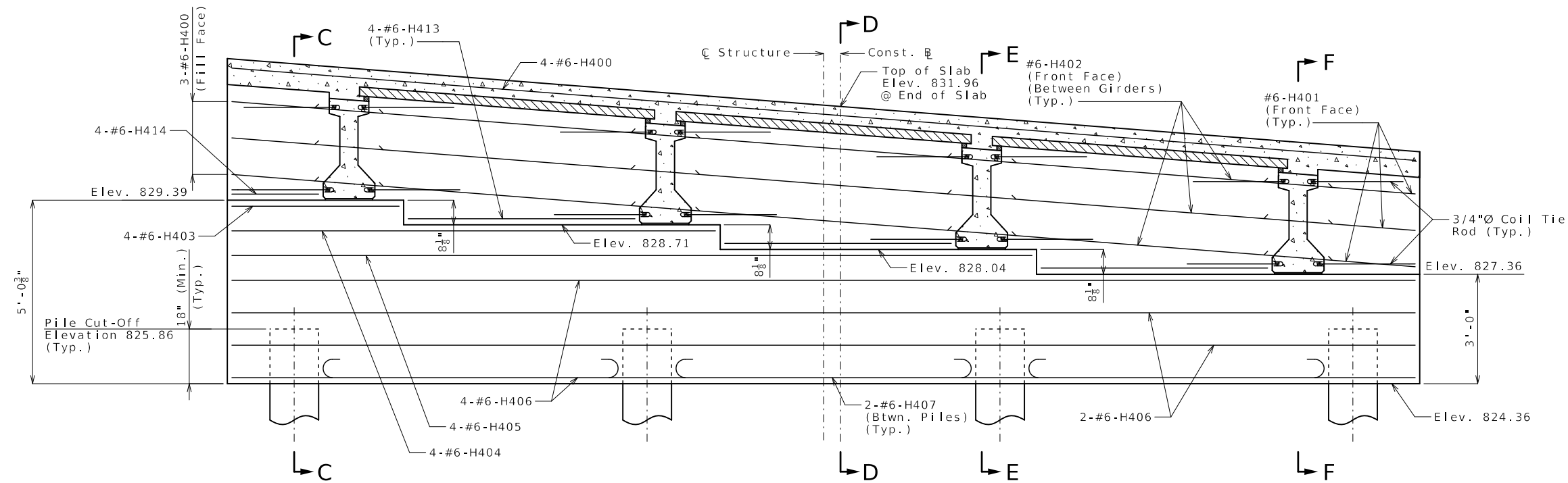
All U bars and pairs of V bars shall be placed parallel to centerline of structure.

END BENT NO. 4

These quantities are included in the Estimated Quantities table on Sheet No. 2.



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** 3-#6-V402 @ 9" cts. (Typ.)

Notes:

Work this sheet with Sheets No. 11 & 13.

All concrete in the end bent above top of beam and below top of slab shall be Class B-2.

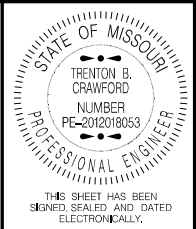
Strands at end of girders shall be field bent or, if necessary, cut in field to maintain 1 1/2-inch minimum clearance to fill face of end bent.

For location of Coil Ties & #5-H407 (Strand Tie Bars), see Sheet No. 14.

The #6-F400 bars shall be bent in field to clear girders.

For details of bridge approach slab, see
Sheet No. 25.

For details of vertical drain at end bent,
see Sheet No. 8.



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ROUTE	STATE
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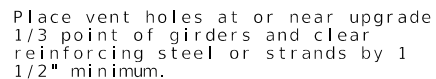
PROJECT NO.

BRIDGE NO.
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+ Indicates prestressing strand. ○ Indicates cut & shop bend with 2'-6" projection.



Exclude coil tie at exterior face
of exterior girders except at
integral end bents.

All dimensions are out to out.

Hooks and bends shall be in accordance with the CRSI Manual of Standard Practice for Detailing Reinforced Concrete Structures, Stirrup and Tie Dimensions.

Actual bar lengths are measured along centerline of bar to the nearest inch.

Minimum clearance to reinforcing shall be one inch.

All reinforcement shall be ASTM A615 OR A706
Grade 60.

The two D1 bars may be furnished as one bar at the fabricator's option.

All B1 bars shall be epoxy coated.

General Notes:

Concrete for prestressed girders shall be Class A-1 with $f'_c = 7,000$ psi and $f'_{ci} = 5,000$ psi.

Use 10 strands, 1/2"Ø Grade 270, with an initial prestress force of 310 kips.

Pretensioned members shall be in accordance with Sec 1029.

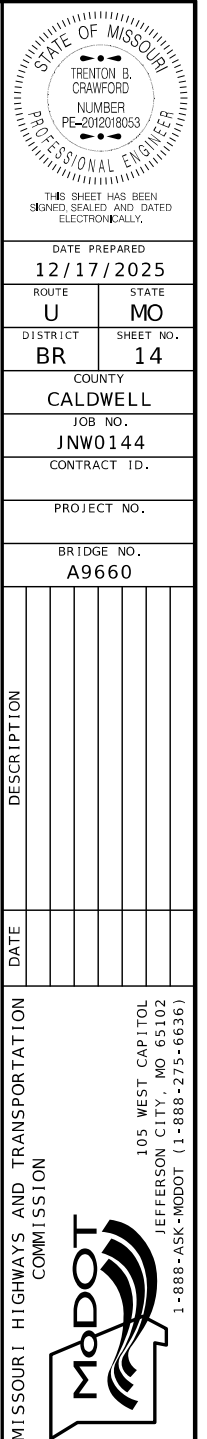
Fabricator shall be responsible for location and design of lifting devices.

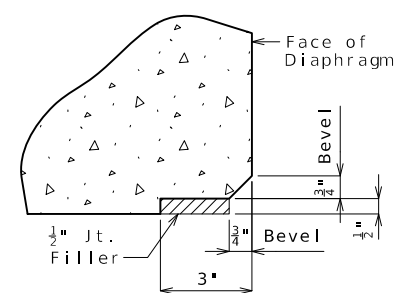
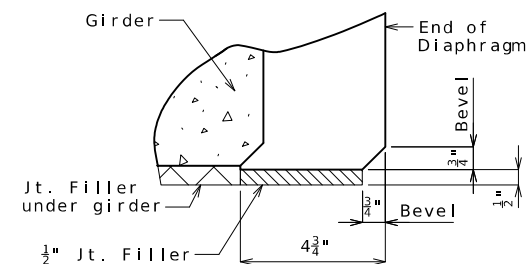
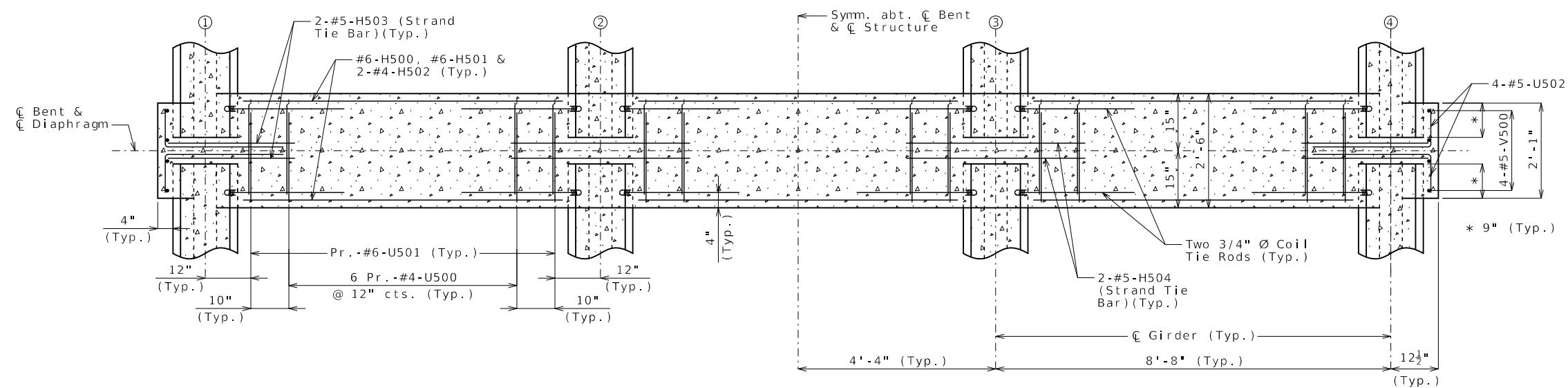
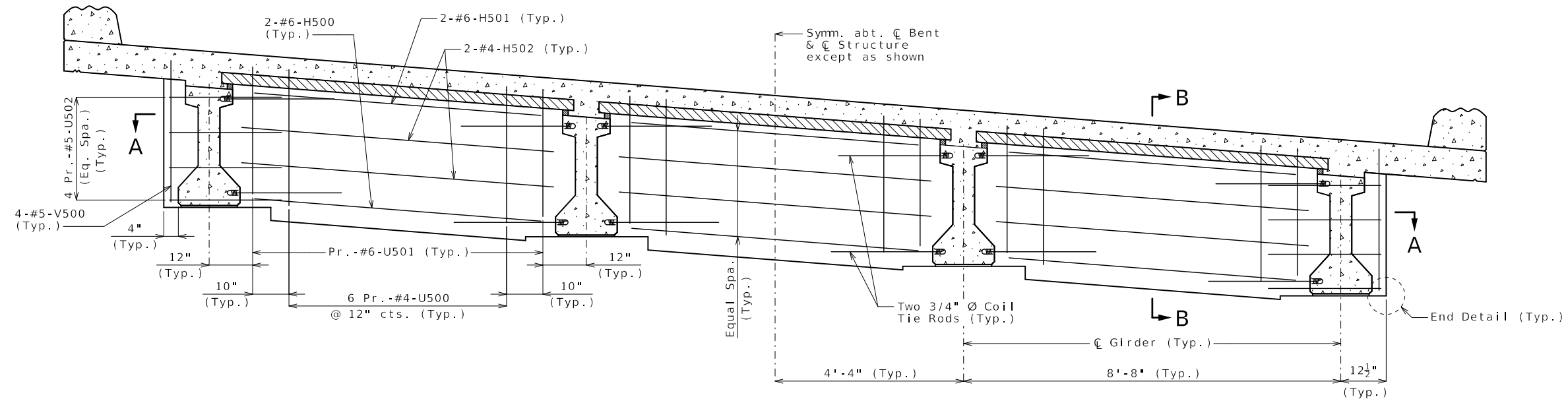
Exterior and interior girders are the same except: coil ties, coil inserts for slab drains.

For Girder Camber Diagram, see Sheet No. 19.

For location of coil inserts at slab drains,
see Sheet No. 18.

For location of coil ties at concrete bent diaphragms, see Sheets No. 6, 12 & 15.



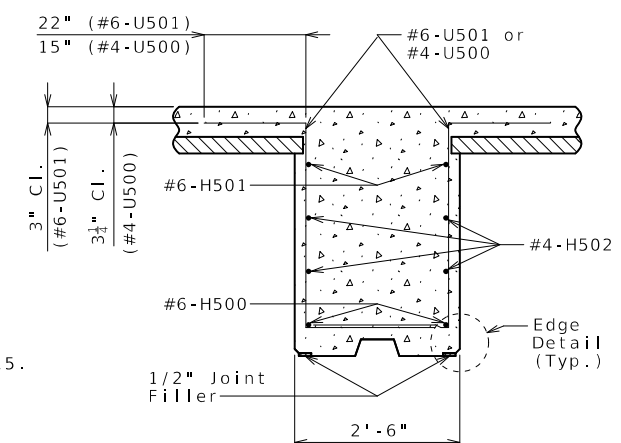


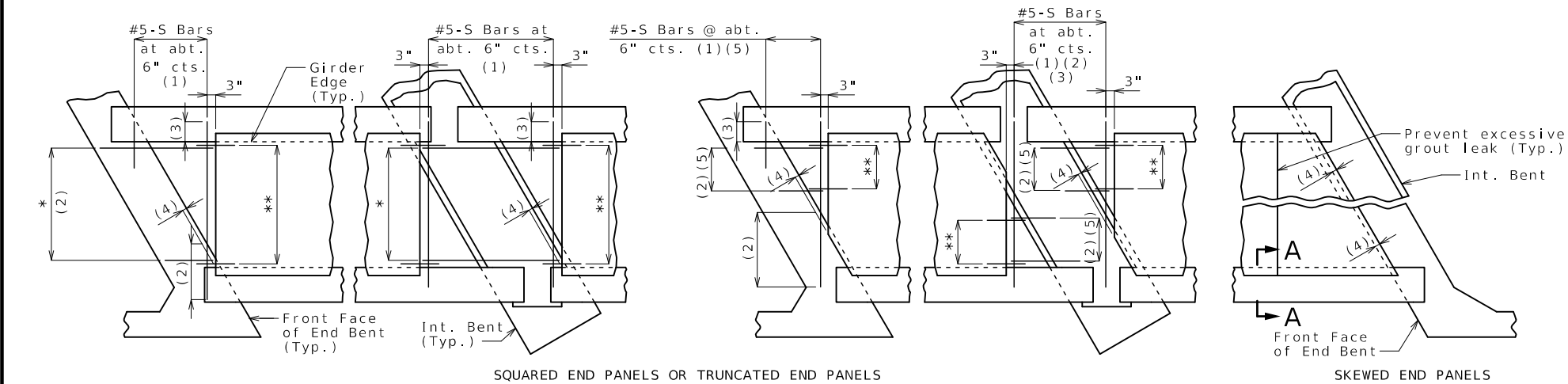
Notes:

For location of Strand Tie Bars, see Sheets No. 14 & 15.

For location of details of Coil Tie Rods, see Sheets No. 14 & 15.

Diaphragms at intermediate bents shall be built vertical.



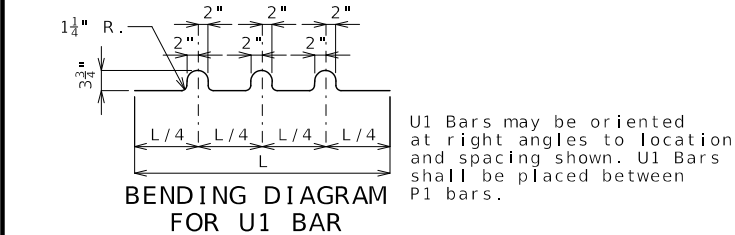


SQUARED END PANELS OR TRUNCATED END PANELS

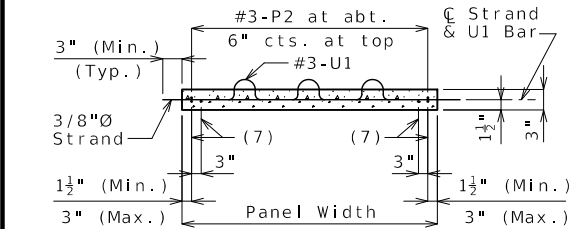
SKEWED END PANELS

PLAN SHOWING PANEL PLACEMENT

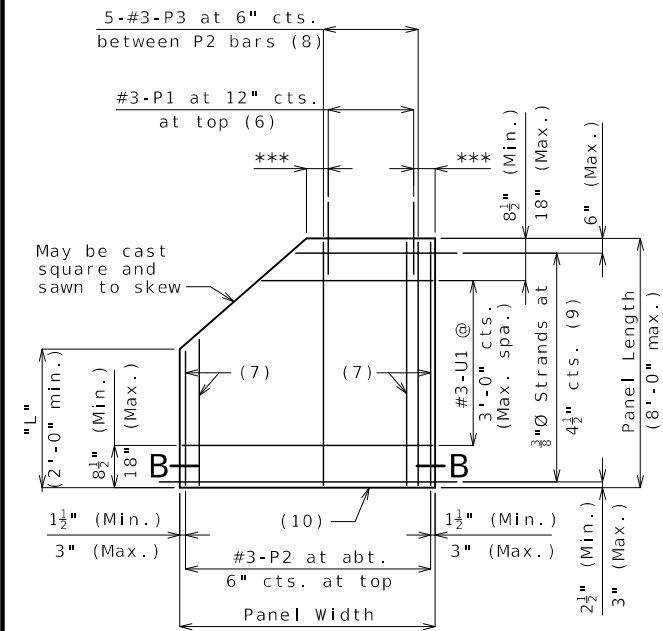
* #5-S Bars at abt. 9" cts. (1)
** #3-P1 at 12" cts. (End panels only)



BENDING DIAGRAM FOR U1 BAR

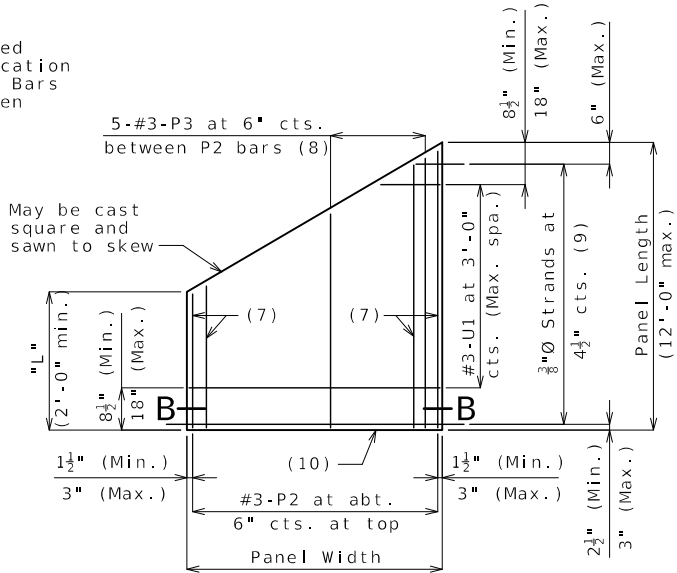


SECTION B-B

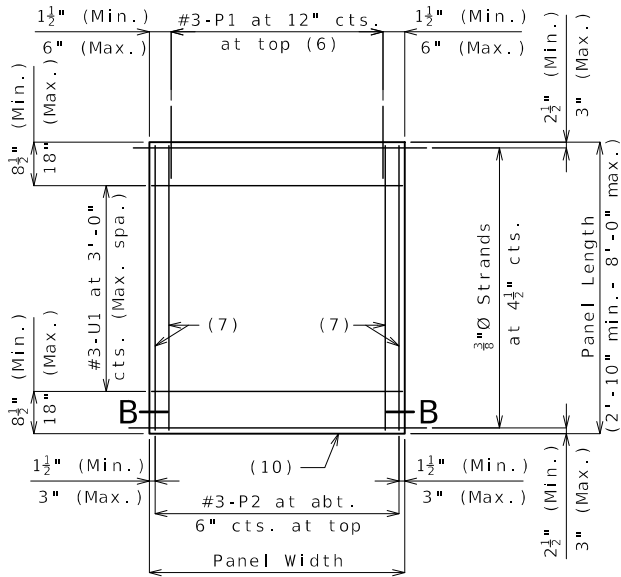


PLAN OF OPTIONAL TRUNCATED END PANEL

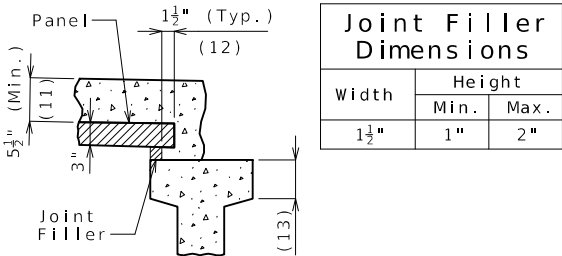
*** 3" (Min.), 6" (Max.)



PLAN OF OPTIONAL SKEWED END PANEL



PLAN OF SQUARED PANEL



SECTION A-A

Joint Filler Dimensions		
Width	Height	
	Min.	Max.
1 1/2"	1"	2"

Reference Notes:

- Plan of Panel Placement:
- (1) S-bars shown are bottom steel in slab between panels and used with squared and truncated end panels only.
 - (2) Extend S-bars 18 inches beyond the front face of end bents and int. bents for squared and truncated end panels only.
 - (3) Extend S-bars 9 inches beyond edge of girder (Typ.).
 - (4) End panels shall be dimensioned 1/2" min. to 1 1/2" max. from the inside face of diaphragm.
 - (5) For truncated end panels, use a min. of #5-S bars at 6" crossings in openings, or min. 4x4-W7xW7.

- Plans of Panels:
- (6) For end panels only, P1 bars shall be 2'-0" in length and embedded 12". P1 bars will not be required for panels at squared integral end bents.
 - (7) #3-P2 bars near edge of panel at bottom (under strands).
 - (8) Use #3-P3 bars if panel is skewed 45° or greater.
 - (9) Any strand 2'-0" or shorter shall have a #4 reinforcing bar on each side of it, centered between strands. Strands 2'-0" or shorter may then be debonded at the fabricator's option.
 - (10) Optional 1/2" x 45° Chamfer one or both sides at bottom.
- Section A-A:
- (11) Slab thickness over prestressed panels varies due to girder camber. In order to maintain minimum slab thickness, it may be necessary to raise the grade uniformly throughout the structure. No payment will be made for additional labor or materials required for necessary grade adjustment.
 - (12) Contractor shall ensure proper consolidation under and between panels.
 - (13) At the contractor's option, the variation in slab thickness over prestressed panels may be eliminated or reduced by increasing and varying the girder top flange thickness. Dimensions shall be shown on the shop drawings.

PRESTRESSED PANELS

General Notes:

Prestressed Panels:
Concrete for prestressed panels shall be Class A-1 with $f'c = 6,000$ psi, $f'ci = 4,000$ psi.

The top surface of all panels shall receive a scored finish with a depth of scoring of 1/8" perpendicular to the prestressing strands in the panels.

Prestressing tendons shall be high-tensile strength, uncoated, seven-wire, low-relaxation strands for prestressed concrete in accordance with AASHTO M 203 Grade 270, with nominal diameter of strand = 3/8" and nominal area = 0.085 sq.in. and minimum ultimate strength = 22.95 kips (270 ksi). Larger strands may be used with the same spacing and initial tension.

Initial prestressing force = 17.2 kips/strand.

The method and sequence of releasing the strands shall be shown on the shop drawings.

Suitable anchorage devices for lifting panels may be cast in panels, provided the devices are shown on the shop drawings and approved by the engineer. Panel lengths shall be determined by the contractor and shown on the shop drawings.

When squared end panels are used at skewed bents, the skewed portion shall be cast full depth. No separate payment will be made for additional concrete and reinforcing required.

Support from diaphragm forms is required under the optional skewed end until cast-in-place concrete has reached 3,000 psi compressive strength.

Prestressed panels shall be brought to saturated surface-dry (SSD) condition just prior to the deck pour. There shall be no free standing water on the panels or in the area to be cast.

The prestressed panel quantities are not included in the table of estimated quantities for the slab.

Reinforcing Steel:
All dimensions are out to out.

Hooks and bends shall be in accordance with the CRSI Manual of Standard Practice for Detailing Reinforced Concrete Structures, Stirrup and Tie Dimensions.

Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise shown.

If U1 bars interfere with placement of slab steel, U1 loops may be bent over, as necessary, to clear slab steel.

Deformed welded wire reinforcement (WWR) providing a minimum area of reinforcing perpendicular to strands of 0.22 sq in./ft, with spacing parallel to strands sufficient to ensure proper handling, may be used in lieu of the #3-P2 bars shown. Wire diameter shall not be larger than 0.375 inch. The above alternative reinforcement criteria may be used in lieu of the #3-P3 bars, when required, and placed over a width not less than 2 feet.

The following reinforcing steel shall be tied securely to the strands with the following maximum spacing in each direction:
#3-P2 bars at 16 inches.
WWR at 24 inches.

The #3-U1 bars shall be tied securely to #3-P2 bars, to WWR or to strands (when placed between P1 bars) at about 3-foot centers.

Minimum reinforcement steel length shall be 2'-0".

All reinforcement other than prestressing strands shall be epoxy coated.

Precast panels may be in contact with stirrup reinforcing in diaphragms.

S-bars are not listed in the bill of reinforcing.

Cost of S-bars will be considered completely covered by the contract unit price for the slab.

Joint Filler:
Joint filler shall be preformed fiber expansion joint material in accordance with Sec 1057 or expanded or extruded polystyrene bedding material in accordance with Sec 1073.

Use Slab Haunching Diagram on Sheet No. 19 for determining thickness of joint filler within the limits noted in the table of Joint Filler Dimensions.

Thicker material may be used on one or both sides of the girder to reduce cast-in-place concrete thickness to within tolerances.

The same thickness of preformed fiber expansion joint material shall be used under any one edge of any panel except at locations where top flange thickness may be stepped. The maximum change in thickness between adjacent panels shall be 1/2 inch. The polystyrene bedding material may be cut with a transition to match haunch height above top of flange.

Joint filler shall be glued to the girder. When thickness exceeds 1 1/2 inches, the joint filler shall be glued top and bottom. The glue used shall be the type recommended by the joint filler manufacturer.

Edges of panels shall be uniformly seated on the joint filler before slab reinforcement is placed.

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TRENTON B. CRAWFORD
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PE-2012018053
PROFESSIONAL ENGINEER
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JNW0144
CONTRACT ID.

PROJECT NO.

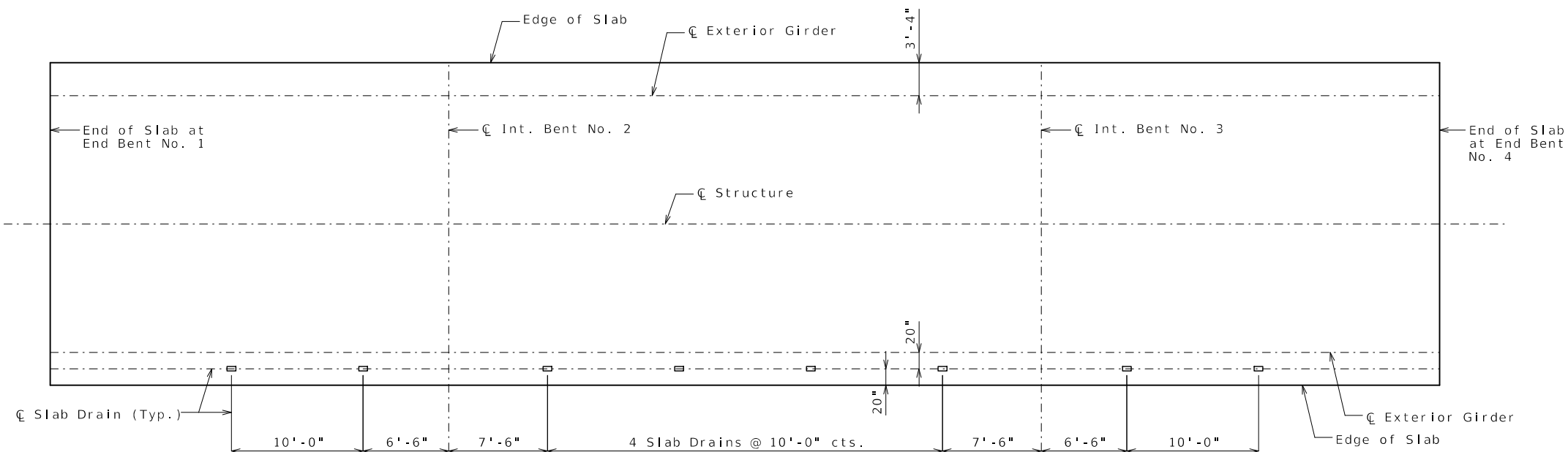
BRIDGE NO.
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DESCRIPTION

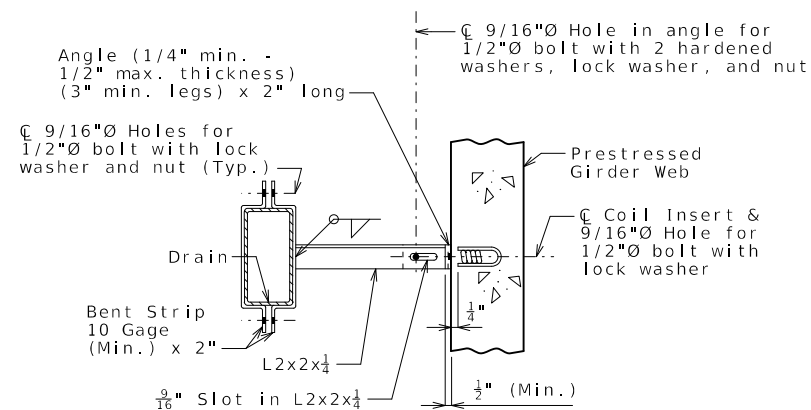
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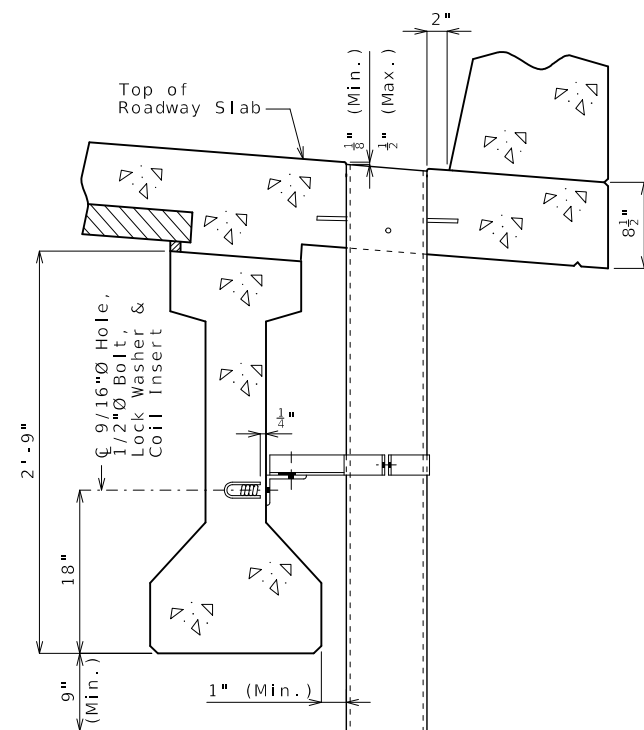
105 WEST CAPITOL
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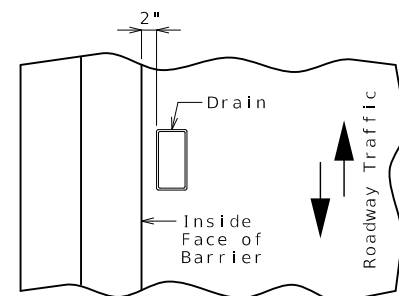
PLAN OF SLAB SHOWING SLAB DRAIN LOCATIONS



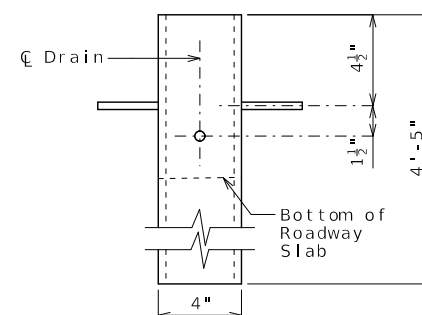
PART SECTION SHOWING BRACKET ASSEMBLY



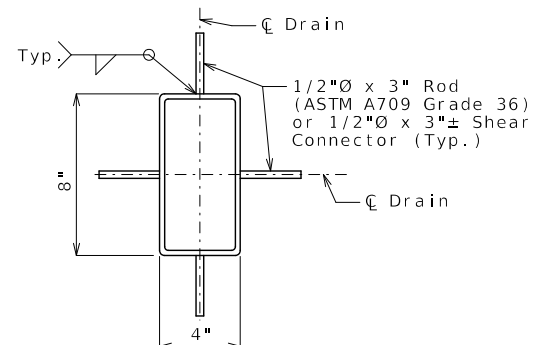
PART SECTION NEAR DRAIN



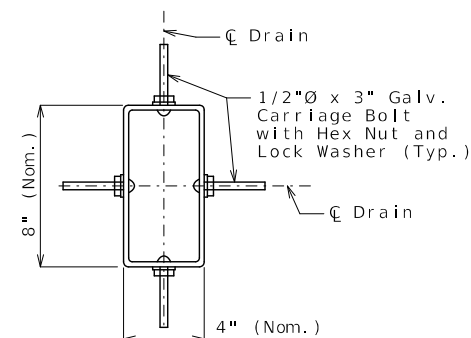
PART PLAN OF SLAB AT DRAIN



ELEVATION OF DRAIN



PLAN OF STEEL DRAIN OPTION



PLAN OF FRP DRAIN OPTION

General Notes:

Contractor shall have the option to construct either steel or FRP slab drains. All drains shall be of same type.

Slab drain bracket assembly shall be ASTM A709 Grade 36 steel.

Locate drains in slab by dimensions shown in Part Section Near Drain.

Reinforcing steel shall be shifted to clear drains.

The coil inserts and bracket assembly shall be galvanized in accordance with ASTM A123.

All bolts, hardened washers, lock washers and nuts shall be galvanized in accordance with AASHTO M 232 (ASTM A153), Class C.

All 1/2"Ø bolts shall be ASTM A307.

Shop drawings will not be required for the slab drains and the bracket assembly.

The coil insert required for the bracket assembly attachment shall be located on the prestressed girder shop drawings.

Coil inserts shall have a concrete pull-out strength (ultimate load) of at least 2,500 pounds in 5,000 psi concrete.

The bolt required to attach the slab drain bracket assembly to the prestressed girder web shall be supplied by the prestressed girder fabricator.

Notes for Steel Drain:

Slab drains may be fabricated of either 1/4" welded sheets of ASTM A709 Grade 36 steel or from 1/4" structural steel tubing ASTM A500 or A501.

Outside dimensions of drains are 8" x 4".

The drains shall be galvanized in accordance with ASTM A123.

Notes for FRP Drain:

Drains shall be machine filament-wound thermosetting resin tubing meeting the requirements of ASTM D2996 with the following exceptions:

Shape of drains shall be rectangular with outside nominal dimensions of 8" x 4".

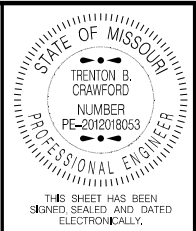
Minimum reinforced wall thickness shall be 1/4 inch.

The resin used shall be ultraviolet (UV) resistant and/or have UV inhibitors mixed throughout. Drains may have an exterior coating for additional UV resistance.

The color of the slab drain shall be gray (Federal Standard #26373). The color shall be uniform throughout the resin and any coating used.

The combination of materials used in the manufacture of the drains shall be tested for long term performance in accordance with ASTM D4329 Cycle A. The representative material shall withstand at least 500 hours of testing with only minor discoloration and without any physical deterioration. The contractor shall furnish the results of the required ultraviolet testing prior to acceptance of the slab drains.

At the contractor's option, drains may be field cut. The method of cutting FRP slab drain shall be as recommended by the manufacturer to ensure a smooth, chip free cut.

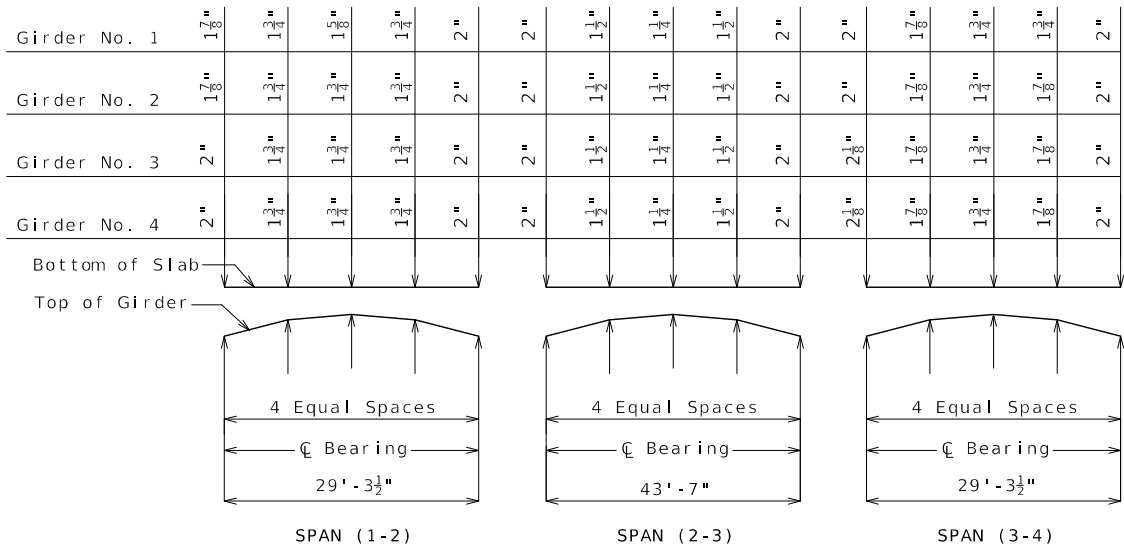


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DISTRICT	SHEET NO.
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COUNTY	
CALDWELL	
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BRIDGE NO. A9660

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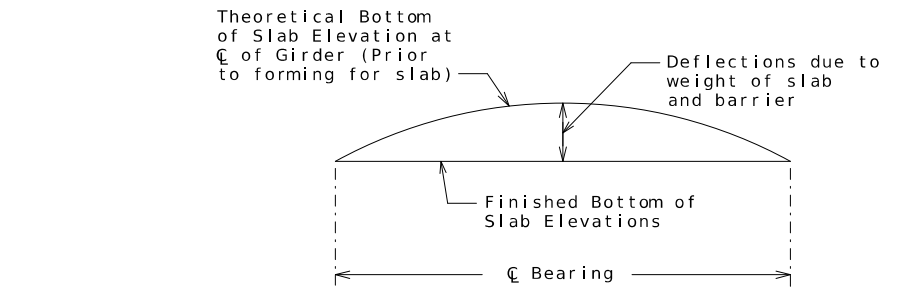
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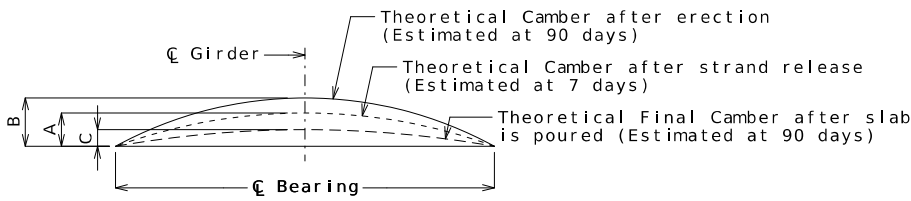
THEORETICAL SLAB HAUNCHING DIAGRAM (ESTIMATED AT 90 DAYS)

If girder camber is different from that shown in the camber diagram, in order to maintain minimum slab thickness, an adjustment of the slab haunches, an increase in slab thickness or a raise in grade uniformly throughout the structure shall be necessary. No payment will be made for additional labor or materials required for variation in haunching, slab thickness or grade adjustment.

Concrete in the slab haunches is included in the Estimated Quantities for Slab on Concrete I-Girder.



TYPICAL SLAB ELEVATIONS DIAGRAM



Girder	Span (1-2)			Span (2-3)			Span (3-4)		
	A	B	C	A	B	C	A	B	C
Exterior	1/4"	3/8"	1/4"	3/8"	1 1/8"	3/4"	1/4"	3/8"	1/4"
Interior	1/4"	3/8"	1/4"	3/8"	1 1/8"	3/4"	1/4"	3/8"	1/4"

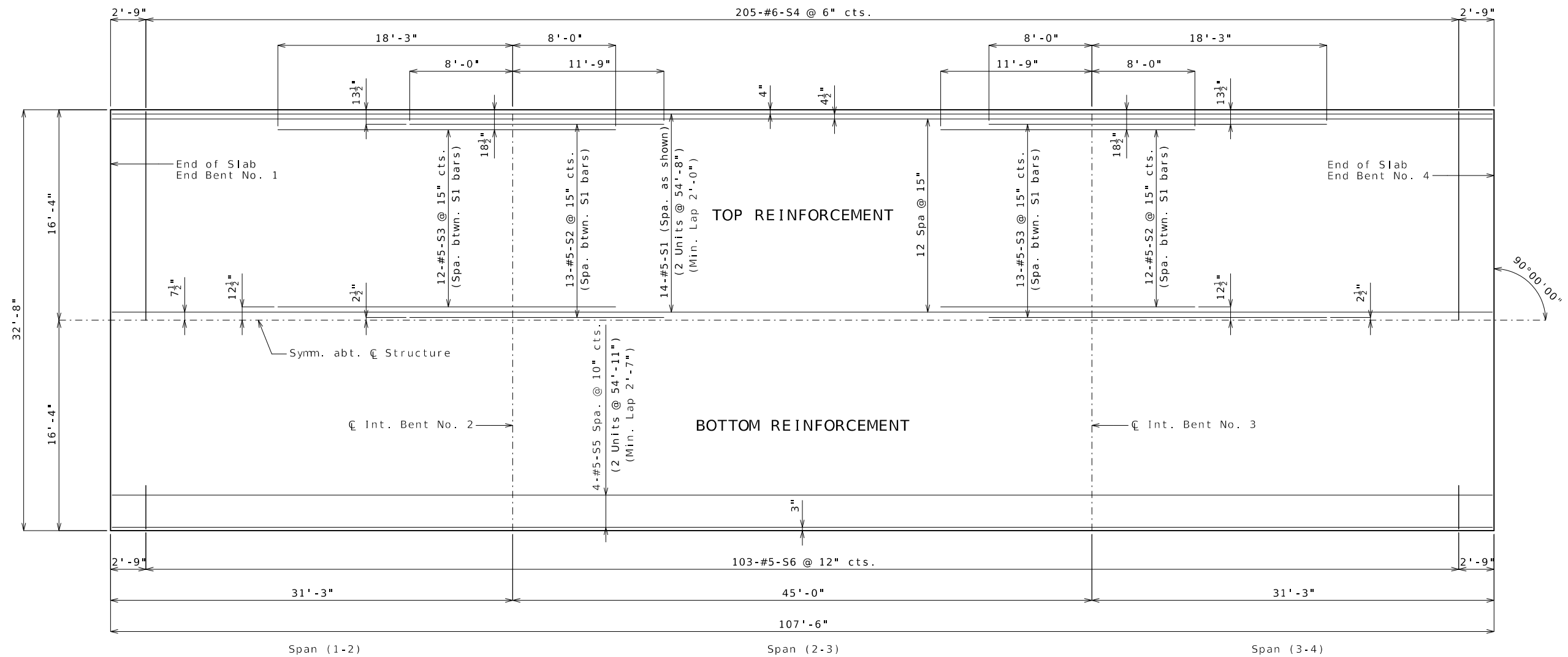
GIRDER CAMBER DIAGRAM

Conversion Factors for Girder Camber (Estimated at 90 days):

0.25 pt. = 0.7125 x 0.5 pt.

Theoretical Bottom of Slab Elevations at Centerline of Girder (Prior to forming for slab) (Estimated at 90 days)															
Girder Number	Span (1-2) (29'-3½" CL Brg. - CL Brg.)					Span (2-3) (43'-7" CL Brg. - CL Brg.)					Span (3-4) (29'-3½" CL Brg. - CL Brg.)				
	CL Brg.	.25	.50	.75	CL Brg.	CL Brg.	.25	.50	.75	CL Brg.	CL Brg.	.25	.50	.75	CL Brg.
1	831.90	831.93	831.96	831.99	832.01	832.02	832.08	832.13	832.17	832.19	832.19	832.22	832.25	832.28	832.30
2	831.22	831.26	831.29	831.31	831.34	831.34	831.41	831.46	831.49	831.51	831.52	831.55	831.58	831.61	831.63
3	830.55	830.58	830.61	830.64	830.66	830.67	830.73	830.79	830.82	830.83	830.84	830.87	830.90	830.93	830.95
4	829.87	829.91	829.94	829.96	829.99	829.99	830.06	830.11	830.14	830.16	830.16	830.20	830.23	830.25	830.28

Elevations are based on a constant slab thickness of 8 1/2" and include allowance for theoretical dead load deflections due to weight of slab (including precast panel) and barrier.



Notes:

Longitudinal slab dimensions are measured horizontally.

For Section Thru Slab and Slab Pouring Sequence, see Sheet No. 21.

For Details and Reinforcement of Type D Barrier not shown, see Sheets No. 22, 23 & 24.

For Theoretical Slab Haunching Diagram, see Sheet No. 19.

For Theoretical Bottom of Slab Elevations, see Sheet No. 19.

For details and locations of Slab Drains, see Sheet No. 18.

For details of Precast Prestressed Panels, see Sheet No. 17.

PLAN OF SLAB SHOWING REINFORCEMENT



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12/17/2025

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JNW0144

CONTRACT ID.

PROJECT NO.

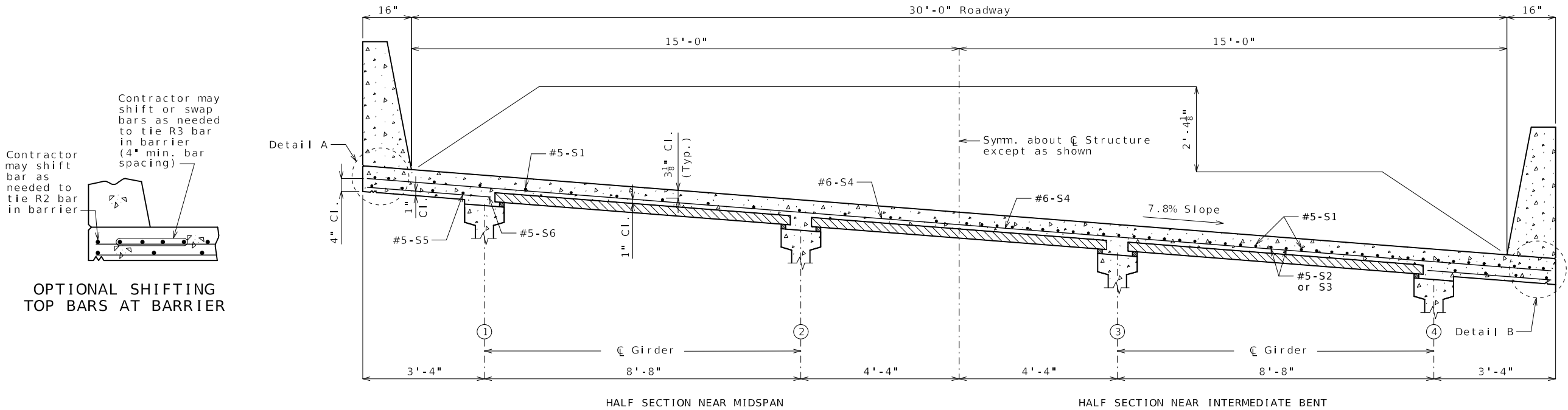
BRIDGE NO.
A9660

DESCRIPTION	DATE

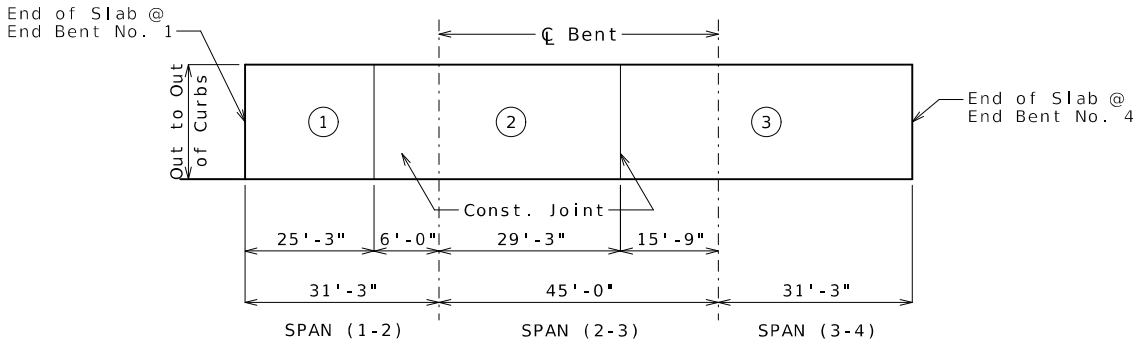
MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION

105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-273-6636)

MoDOT



SECTION THRU SLAB

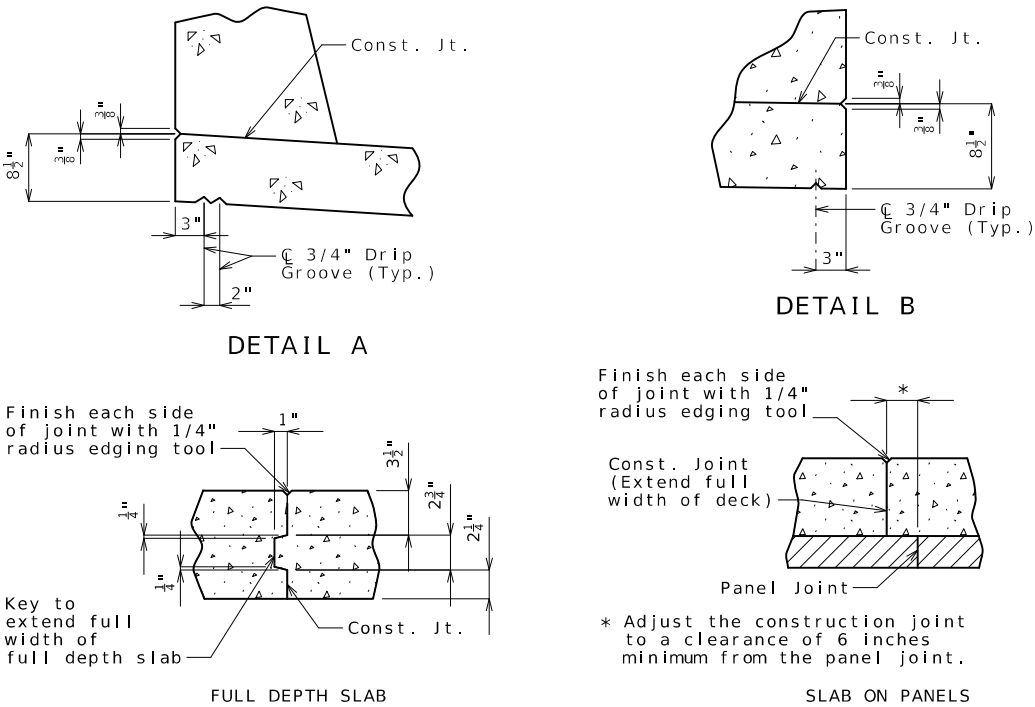


	Sequence of Pours			Min. Rate of Pour Cu. Yds./Hr.
	Direction			With Retarder
Basic Sequence	1	2	3	25
	End to 2	1 to 3	2 to End	
Alternate pours to the basic sequence are subject to the approval of the engineer in accordance with Sec 703.				
Alternate A Pours	1 + 2		3	25
	End to 3		2 to End	
Alternate B Pours	1 + 2 + 3			25
	End to End			

The contractor shall furnish an approved retarder to retard the set of the concrete to 2.5 hours, and shall pour and satisfactorily finish the slab pours at the rate given.

The concrete diaphragm at the intermediate bents and integral end bents shall be poured a minimum of 30 minutes and a maximum of 2 hours before the slab is poured.

SLAB POURING SEQUENCE



SLAB CONSTRUCTION JOINT

- Notes:
- For details of Precast Prestressed Panels, see Sheet No. 17.
 - For details and reinforcement of barrier not shown, see Sheets No. 22, 23 & 24.
 - For Theoretical Bottom of Slab Elevations, Girder Camber Diagram and Theoretical Slab Haunching Diagram, see Sheet No. 19.
 - For Plan of Slab Showing Reinforcement, see Sheet No. 20.

SLAB DETAILS

Detailed Aug. 2025
Checked Oct. 2025

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 21 of 30

STATE OF MISSOURI
TRENTON B. CRAWFORD
NUMBER
PE-2012018053
PROFESSIONAL ENGINEER

THIS SHEET HAS BEEN
SIGNED, SEALED AND DATED
ELECTRONICALLY.

DATE PREPARED
12/17/2025

ROUTE
U
DISTRICT
BR

STATE
MO
SHEET NO.
21

COUNTY
CALDWELL

JOB NO.
JNW0144

CONTRACT ID.

PROJECT NO.

BRIDGE NO.
A9660

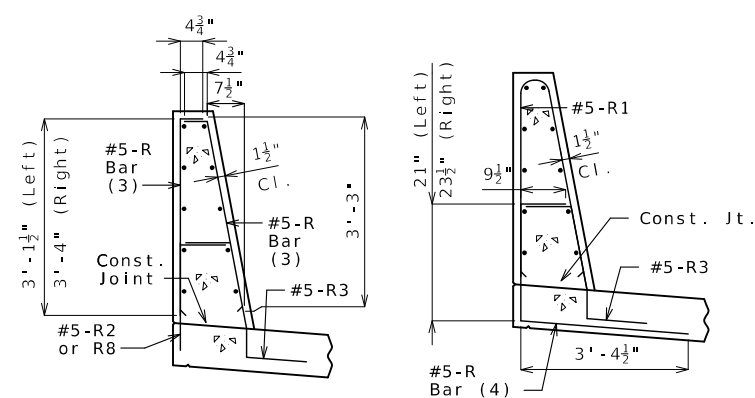
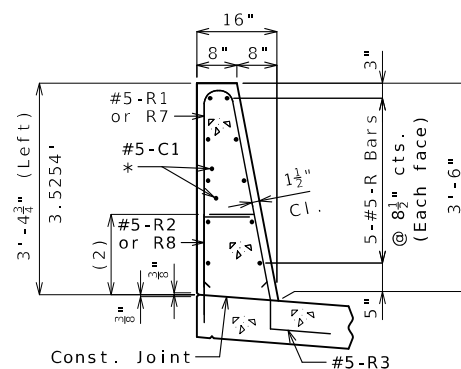
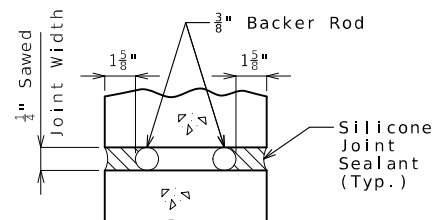
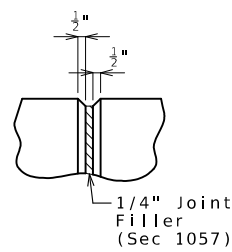
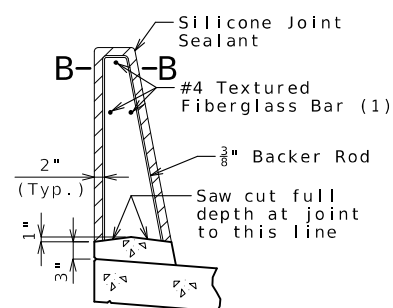
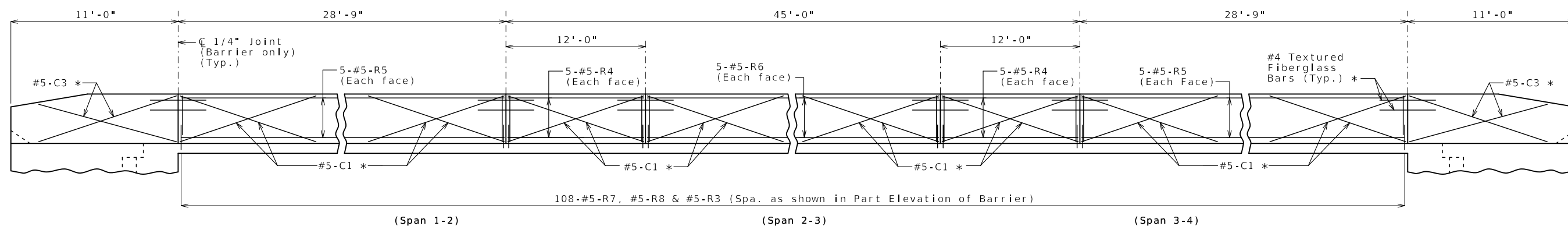
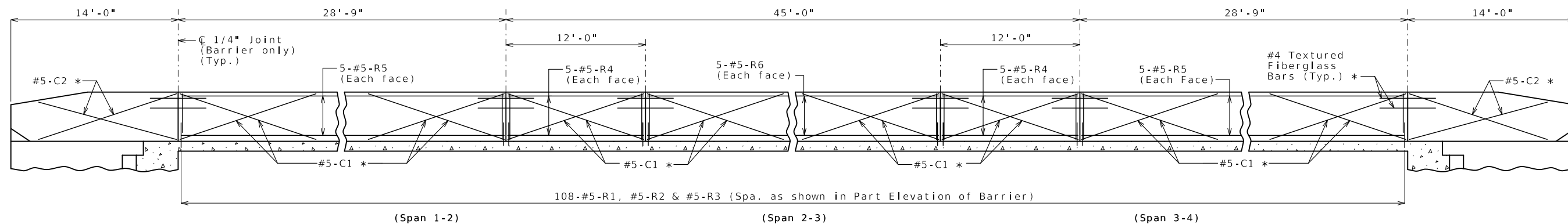
DESCRIPTION

DATE

MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION

105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-273-6636)

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General Notes:

* Slip-formed option only.

Conventional forming or slip forming may be used. Saw cut joints may be used with conventional forming.

Top of barrier shall be built parallel to grade and barrier joints (except at end bents) normal to grade.

All exposed edges of barrier shall have either a 1/2-inch radius or a 3/8-inch bevel, unless otherwise noted.

Payment for all concrete and reinforcement, complete in place, will be considered completely covered by the contract unit price for Type D Barrier per linear foot.

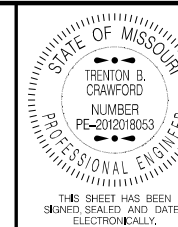
Concrete in barrier shall be Class B-1.

Measurement of barrier is to the nearest linear foot for each structure, measured along the outside top of slab from end of wing to end of wing.

Concrete traffic barrier delineators shall be placed on top of the barrier as shown on Missouri Standard Plan 617.10 and in accordance with Sec 617. Delineators on bridges with two-lane, two-way traffic shall have retroreflective sheeting on both sides. Concrete traffic barrier delineators will be considered completely covered by the contract unit price for Type D Barrier.

Joint sealant and backer rods shall be in accordance with Sec 717 for silicone joint sealant for saw cut and formed joints.

For slip-formed option, both sides of barrier shall have a vertically broomed finish and the top shall have a transversely broomed finish.



DATE PREPARED _____

12/17/20

ROUTE	ST
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BR	2

COUNTY
CALDWELL

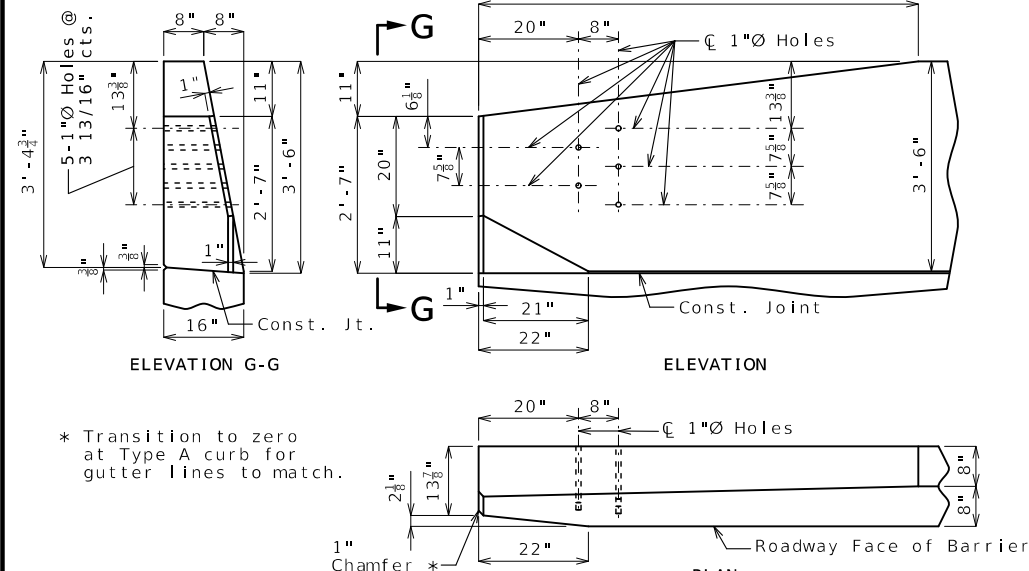
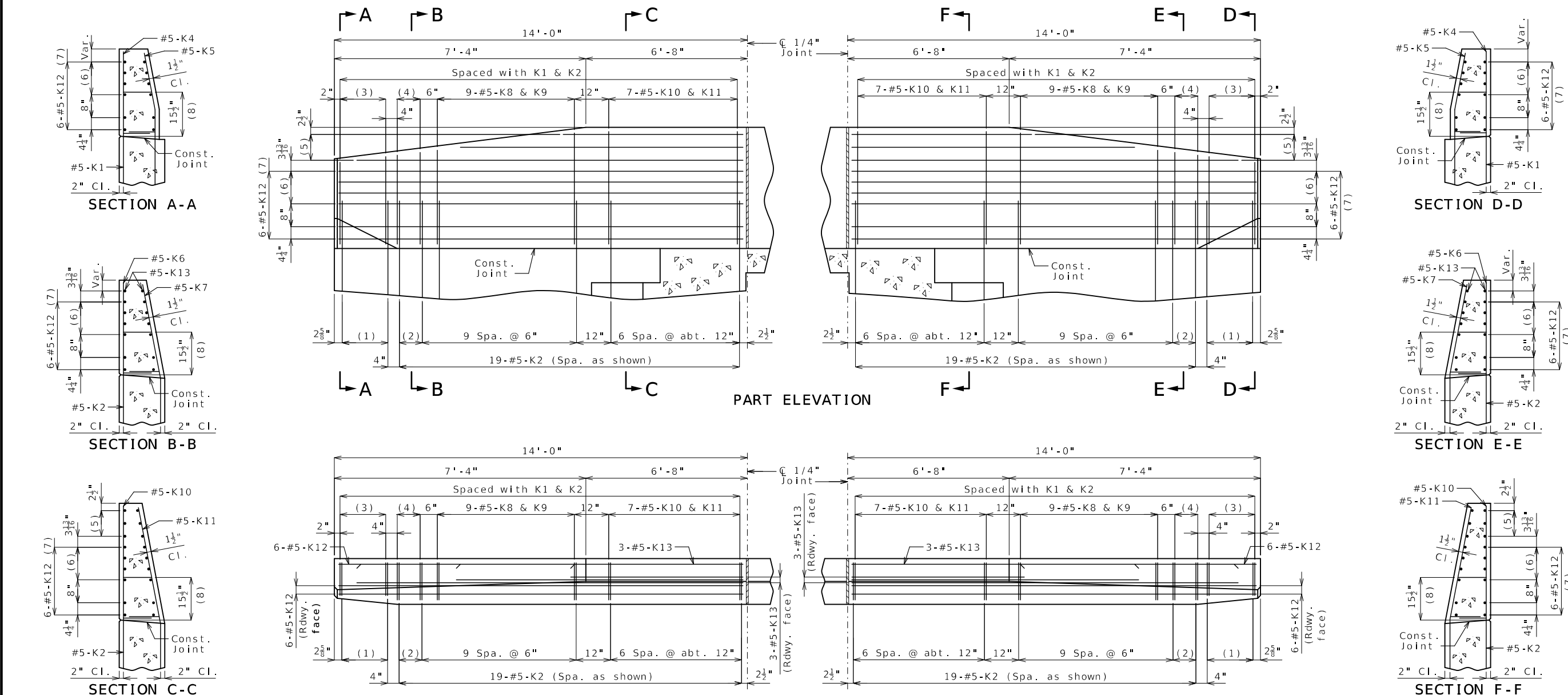
JOB NO.
INW0144

CONTRACT ID

PROJECT NO. _____

BRIDGE NO.
A9660[illegible]MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION

105 WEST CAPITOL
JEFFERSON CITY, MO 65102



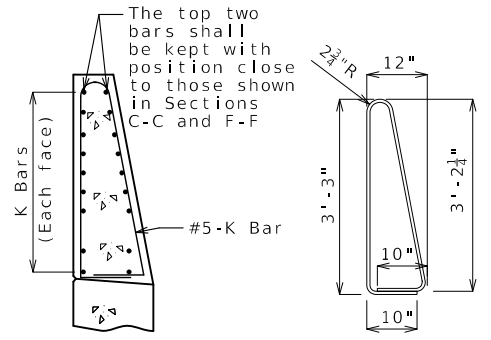
* Transition to zero at Type A curb for gutter lines to match.

General Notes:

Concrete traffic barrier delineators shall be placed on top of the barrier as shown on Missouri Standard Plan 617.10 and in accordance with Sec 617. Delineators on bridges with two-lane, two-way traffic shall have retroreflective sheeting on both sides. Concrete traffic barrier delineators will be considered completely covered by the contract unit price for Type D Barrier.

Reinforcing Steel:

Minimum clearance to reinforcing steel shall be 1 1/2" except as shown for bars embedded into end bent.



K10-K11 BAR PERMISSIBLE ALTERNATE SHAPE

(Other K bars not shown for clarity)

The K10-K11 bar combination may be furnished as one bar as shown, at the contractor's option.

All dimensions are out to out.

STATE OF MISSOURI
TRENTON B. CRAWFORD
NUMBER
PE-2012018053
PROFESSIONAL ENGINEER
THIS SHEET HAS BEEN
SIGNED, SEALED AND DATED
ELECTRONICALLY.

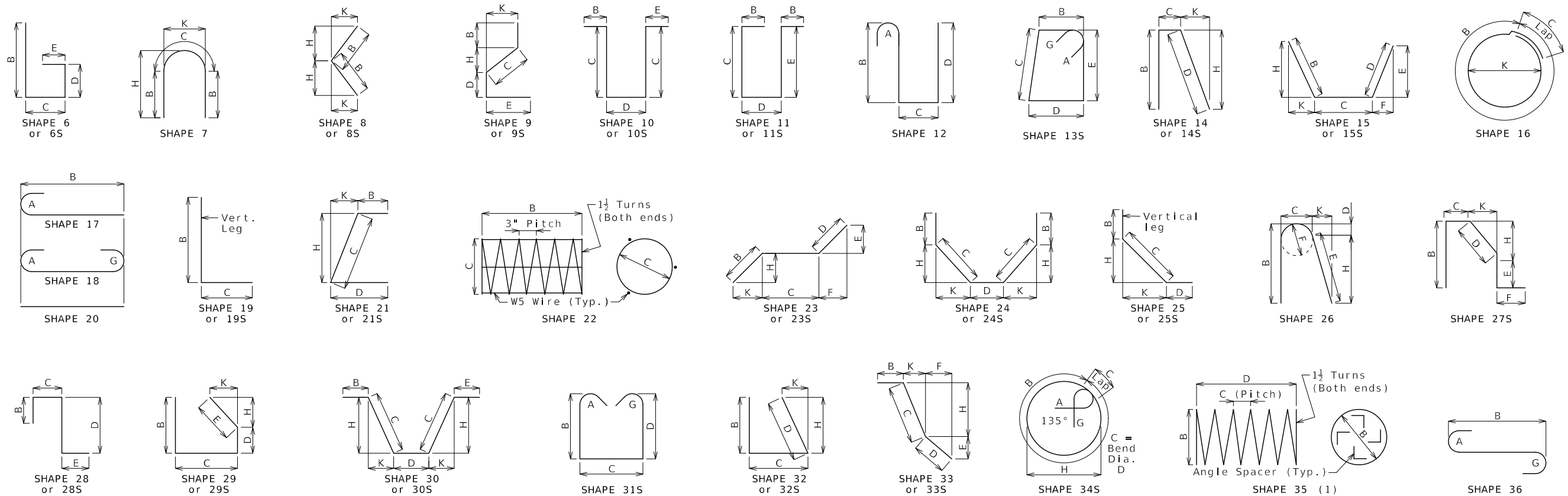
DATE PREPARED
12/17/2025
ROUTE
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SHEET NO.
23
COUNTY
CALDWELL
JOB NO.
JNW0144
CONTRACT ID.
PROJECT NO.
BRIDGE NO.
A9660

DESCRIPTION

DATE

MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION

105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-273-6636)



Finished Bend Diameters D and Hook Dimensions

Standard Pin Bend Shapes

Size	Case	D	A or G		J
			90°	180°	180°
#4	1	3"	8"	6"	4"
#5	1	3 $\frac{3}{4}$ "	10"	7"	5"
#6	1	4 $\frac{1}{2}$ "	12"	8 $\frac{3}{4}$ "	6"
#7	2	5 $\frac{1}{4}$ "	14"	9 $\frac{1}{4}$ "	7"
	3	7"	15"	11 $\frac{1}{2}$ "	8 $\frac{3}{4}$ "
#8	2	6"	16"	11"	8"
	3	8"	17"	13 $\frac{1}{4}$ "	10"
#9	1	9 $\frac{1}{4}$ "	19 $\frac{1}{2}$ "	15 $\frac{1}{2}$ "	11 $\frac{3}{4}$ "
#10	1	10 $\frac{3}{4}$ "	22"	17 $\frac{1}{2}$ "	13 $\frac{1}{4}$ "
#11	1	12"	24 $\frac{1}{2}$ "	19 $\frac{1}{2}$ "	14 $\frac{7}{8}$ "
#14	1	18 $\frac{1}{4}$ "	31 $\frac{1}{4}$ "	27 $\frac{1}{2}$ "	21 $\frac{5}{8}$ "
#18	1	24"	41 $\frac{1}{2}$ "	36 $\frac{1}{4}$ "	28 $\frac{1}{2}$ "

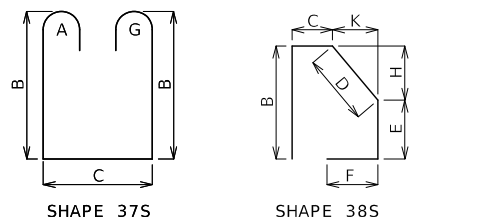
Stirrup Pin Bend Shapes (S)

Size	Case	D	A or G			H	J
			90°	135°	180°	135°	180°
#4	2	2"	4 $\frac{1}{2}$ "	4 $\frac{1}{2}$ "	5"	2 $\frac{7}{8}$ "	3"
	3	3"	5"	5 $\frac{1}{4}$ "	6"	3"	4"
#5	2	2 $\frac{1}{2}$ "	5 $\frac{3}{4}$ "	5 $\frac{3}{4}$ "	5 $\frac{3}{4}$ "	3 $\frac{5}{8}$ "	3 $\frac{3}{4}$ "
	3	3 $\frac{3}{4}$ "	6 $\frac{3}{4}$ "	6 $\frac{3}{4}$ "	7"	3 $\frac{7}{8}$ "	5"
#6	1	4 $\frac{1}{2}$ "	12"	7 $\frac{3}{4}$ "	8 $\frac{3}{4}$ "	4 $\frac{5}{8}$ "	6"

6d for #4 & #5,
12d for #6

Applicable for all grades of steel.

Case 1 applies to all reinforcement. Case 2 applies to all reinforcement except for galvanized bars. Case 3 applies to galvanized bars only.



BENDING DIAGRAMS

All dimensions are out to out.

Shapes ending with an S shall be bent in accordance with stirrup pin bend shapes.

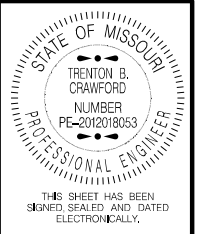
Unless otherwise noted,
finished bending diameter
D is the same for all
bends of a shape.

(1) Shall be a deformed or plain spiral bar or wire.

Four angle or channel spacers are required for each column spiral. Spacers are to be placed on inside of spirals. Length and weight of column spirals do not include splices or spacers.

	Reinforcing Steel Totals (Pounds)							
		Substructure		Superstructure			Entire Bridge	
	Size	Plain	Epoxy	Slab	Barrier	Slip Form	Plain	Epoxy
By Size	W5	0	0	0	0	0	0	0
	4	168	0	690	0	0	168	690
	5	926	0	9,102	7,093	482	926	16,677
	6	1,396	0	17,780	0	0	1,396	17,780
	7	0	0	0	0	0	0	0
	8	0	0	1,024	0	0	0	1,024
	9	1,696	0	0	0	0	1,696	0
	10	0	0	0	0	0	0	0
	11	0	0	0	0	0	0	0
	14	0	0	0	0	0	0	0
	18	0	0	0	0	0	0	0
By Type		4,186	0	28,596	7,093	482	4,186	36,171

All superstructure reinforcing steel shall be epoxy coated unless otherwise specified.



DATE PREPARED	
12/17/2025	
ROUTE	STATE
U	MO
DISTRICT	SHEET NO.
BR	26
COUNTY	
CALDWELL	
JOB NO.	
JNW0144	
CONTRACT ID.	
PROJECT NO.	
BRIDGE NO.	
A9660	

[illegible]

MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION

105 WEST CAPITAL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)

Bill of Reinforcing Steel																
No. Req.	Size/ Mark	Location	Codes			Dimensions								Nom. Length	Actual Length	Weight
			C	SH	V	ft	in.	ft	in.	ft	in.	ft	in.	ft	in.	lb
		Substructure														
		Int Bent 2														
21	6 D200	BEAM		20		2	6.00							2	6	79
8	9 H200	BEAM		18		28	8.00							31	2	848
8	6 H201	BEAM		20		28	8.00							28	8	344
12	6 H202	BEAM		10S				12.00	2	9.00				4	9	80
8	6 H203	BEAM		18		4	9.00							6	1	73
16	6 H204	BEAM		20		2	6.00							2	6	60
8	4 P200	CECIP PILE		34S		3	1.75	2.00				12.00		4	1	21
28	5 U200	BEAM		13S		2	9.00	2	9.00	2	9.00	2	9.00	11	11	338
15	5 U201	BEAM		10S				2	9.00	2	9.00			8	3	125
24	4 U202	BEAM		10S				8.25	2	9.00				4	2	63
6	6 V200	CECIP PILE		17		6	3.00							6	11	62
		Int Bent 3														
21	6 D300	BEAM		20		2	6.00							2	6	79
8	9 H300	BEAM		18		28	8.00							31	2	848
8	6 H301	BEAM		20		28	8.00							28	8	344
12	6 H302	BEAM		10S				12.00	2	9.00				4	9	80
8	6 H303	BEAM		18		4	9.00							6	1	73
16	6 H304	BEAM		20		2	6.00							2	6	60
8	4 P300	CECIP PILE		34S		3	1.75	2.00				12.00		4	1	21
28	5 U300	BEAM		13S		2	9.00	2	9.00	2	9.00	2	9.00	11	11	338
15	5 U301	BEAM		10S				2	9.00	2	9.00			8	3	125
24	4 U302	BEAM		10S				8.25	2	9.00				4	2	63
6	6 V300	CECIP PILE		17		6	3.00							6	11	62
		Superstructure														
		End Bent 1														
20	6 F100	WING BRACE	E	23S		20.00	5	1.00	20.00	14.25	14.00	14.25	14.00	8	5	250
4	6 F101	DIAPHRAGM	E	19S		4	9.00	2	9.00					7	6	44
7	6 H100	DIAPHRAGM	E	20		32	5.00							32	5	341
6	6 H101	DIAPHRAGM	E	20		2	4.00							2	4	21
9	6 H102	DIAPHRAGM	E	20		7	0.00							7		95
4	6 H103	BEAM	E	20		4	7.00							4	7	28
4	6 H104	BEAM	E	20		13	3.00							13	3	80
4	6 H105	BEAM	E	20		21	11.00							21	11	132
12	6 H106	BEAM	E	20		32	5.00							32	5	584
6	6 H107	BEAM	E	18		8	1.00							9	5	85
8	5 H108	STRAND TIE	E	20		3	11.00							3	11	33
8	8 H109	WING	E	20		10	6.00							10	6	224
18	6 H110	WING	E	20		9	0.00							9		243
8	8 H111	WING	E	20		13	6.00							13	6	288
26	6 H112	WING	E	20		12	0.00							12		469
12	6 H113	DIAPHRAGM	E	20		6	2.00							6	2	111
4	6 H114	DIAPHRAGM	E	20		2	4.00							2	4	14
8	4 P100	CECIP PILE	E	34S		3	1.75	2.00				12.00		4	1	21
2	4 U100	BEAM	E	10S			3	4.00	2	9.00				9	5	12
2	4 U101	BEAM	E	10S			4	0.25	2	9.00				10	10	14
1	5 U102	BEAM/DIAPHRAGM	E	37S		4	7.50	2	9.00					13	12	13
7	5 U103	BEAM/DIAPHRAGM	E	37S		5	5.00	2	9.00					14	7	105
6	5 U104	BEAM/DIAPHRAGM	E	37S		6	0.50	2	9.00					15	10	98
1	5 U105	BEAM/DIAPHRAGM	E	37S		6	2.00	2	9.00					16	1	17
6	5 U106	BEAM/DIAPHRAGM	E	37S		6	9.00	2	9.00					17	3	106
2	5 U107	BEAM/DIAPHRAGM	E	37S		7	2.00	2	9.00					18	1	37
3	4 U108	BEAM	E	13S		2	9.00	2	8.00	2	9.00	2	8.00	11	7	23
2	4 U109	BEAM	E	13S		2	9.00	3	4.00	2	9.00	3	4.00	12	11	17
2	4 U110	BEAM	E	13S		2	9.00	4	0.25	2	9.00	4	0.25	14	4	19
3	4 U111	BEAM	E	13S		2	9.00	4	8.25	2	9.00	4	8.25	15	8	31
27	5 U112	DIAPHRAGM	E	37S		3	2.50	2	3.00					9	8	265
27	6 U113	DIAPHRAGM	E	19S		2	2.00	2	9.00					4	11	193

Nominal lengths are based on out to out dimensions shown in bending diagrams and are listed to the nearest inch for fabricator's use. Actual lengths are measured along centerline bar to the nearest inch. Weights are based on actual lengths.

For bending diagrams and steel reinforcing totals, see Sheet No. 26.

Detailed Nov. 2025
Checked Nov. 2025

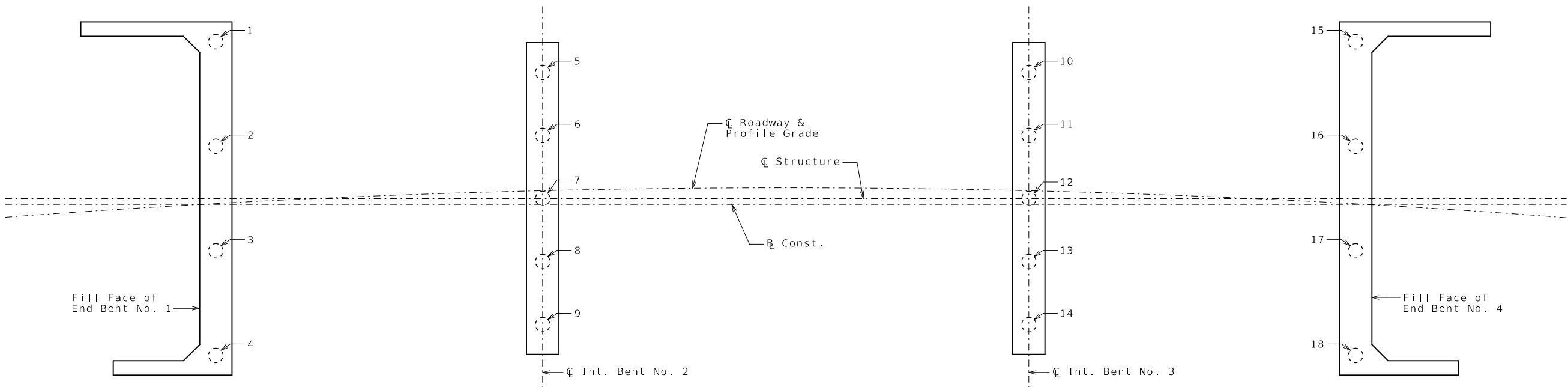
All bars shall be Reinforcing steel
(ASTM A615 Grade 60) fy= 60,000 psi

BILL OF REINFORCING STEEL

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 27 of 30

Bill of Reinforcing Steel																			
No. Req.	Size/ Mark	Location	Codes			Dimensions								Nom. Length ft in.	Actual Length ft in.	Weight lb			
						B ft in.	C ft in.	D ft in.	E ft in.	F ft in.	H ft in.	K ft in.							
40	6 U114	DIAPHRAGM	E 19S	V	2	7.00	4	2.50							6	10	6	8	401
30	5 U115	APPROACH TIE	E 19S		2	0.00		15.00							3	3	3	2	99
4	5 V100	BEAM/DIAPHRAGM	E 17		5	0.00									5	7	5	7	23
4	5 V101	BEAM/DIAPHRAGM	E 17		7	1.00									7	8	7	8	32
12	6 V102	DIAPHRAGM	E 20		2	4.00									2	4	2	4	42
18	6 V103	WING	E 20		6	1.00									6	1	6	1	164
24	6 V104	WING	E 20		8	7.00									8	7	8	7	309
6	6 V105	CECIP PILE	E 17		6	3.00									6	11	6	11	62
		End Bent 4																	
20	6 F400	WING BRACE	E 23S			20.00	5	1.00	20.00	14.25	14.00	14.25	14.00		8	5	8	4	250
4	6 F401	DIAPHRAGM	E 19S		4	9.00	2	9.00							7	6	7	4	44
7	6 H400	DIAPHRAGM	E 20		32	5.00									32	5	32	5	341
6	6 H401	DIAPHRAGM	E 20		2	4.00									2	4	2	4	21
9	6 H402	DIAPHRAGM	E 20		7	0.00									7		7		95
4	6 H403	BEAM	E 20		4	7.00									4	7	4	7	28
4	6 H404	BEAM	E 20		13	3.00									13	3	13	3	80
4	6 H405	BEAM	E 20		21	11.00									21	11	21	11	132
12	6 H406	BEAM	E 20		32	5.00									32	5	32	5	584
6	6 H407	BEAM	E 18		8	1.00									9	5	9	5	85
8	5 H408	STRAND TIE	E 20		3	11.00									3	11	3	11	33
8	8 H409	WING	E 20		10	6.00									10	6	10	6	224
18	6 H410	WING	E 20		9	0.00									9		9		243
8	8 H411	WING	E 20		13	6.00									13	6	13	6	288
26	6 H412	WING	E 20		12	0.00									12		12		469
12	6 H413	DIAPHRAGM	E 20		6	2.00									6	2	6	2	111
4	6 H414	DIAPHRAGM	E 20		2	4.00									2	4	2	4	14
8	4 P400	CECIP PILE	E 34S		3	1.75	2.00					12.00			4	1	3	11	21
2	4 U400	BEAM	E 10S				3	4.00	2	9.00					9	5	9	3	12
2	4 U401	BEAM	E 10S				4	0.25	2	9.00					10	10	10	7	14
1	5 U402	BEAM/DIAPHRAGM	E 37S		4	7.50	2	9.00							13		12	9	13
7	5 U403	BEAM/DIAPHRAGM	E 37S		5	5.00	2	9.00							14	7	14	4	105
6	5 U404	BEAM/DIAPHRAGM	E 37S		6	0.50	2	9.00							15	10	15	7	98
1	5 U405	BEAM/DIAPHRAGM	E 37S		6	2.00	2	9.00							16	1	15	10	17
6	5 U406	BEAM/DIAPHRAGM	E 37S		6	9.00	2	9.00							17	3	17		106
2	5 U407	BEAM/DIAPHRAGM	E 37S		7	2.00	2	9.00							18	1	17	10	37
3	4 U408	BEAM	E 13S		2	9.00	2	8.00	2	9.00	2	8.00			11	7	11	4	23
2	4 U409	BEAM	E 13S		2	9.00	3	4.00	2	9.00	3	4.00			12	11	12	8	17
2	4 U410	BEAM	E 13S		2	9.00	4	0.25	2	9.00	4	0.25			14	4	14		19
3	4 U411	BEAM	E 13S		2	9.00	4	8.25	2	9.00	4	8.25			15	8	15	4	31
27	5 U412	DIAPHRAGM	E 37S		3	2.50	2	3.00							9	8	9	5	265
27	6 U413	DIAPHRAGM	E 19S		2	2.00	2	9.00							4	11	4	9	193
40	6 U414	DIAPHRAGM	E 19S		2	7.00	4	2.50							6	10	6	8	401
30	5 U415	APPROACH TIE	E 19S		2	0.00		15.00							3	3	3	2	99
4	5 V400	BEAM/DIAPHRAGM	E 17		5	0.00									5	7	5	7	23
4	5 V401	BEAM/DIAPHRAGM	E 17		7	1.00									7	8	7	8	32
12	6 V402	DIAPHRAGM	E 20		2	4.00									2	4	2	4	42
18	6 V403	WING	E 20		6	1.00									6	1	6	1	164
24	6 V404	WING	E 20		8	7.00									8	7	8	7	309
6	6 V405	CECIP PILE	E 17		6	3.00									6	11	6	11	62
		Int Diaphragms																	
12	6 H500	DIAPHRAGM	E 20		6	3.00									6	3	6	3	113
12	6 H501	DIAPHRAGM	E 20		6	7.00									6	7	6	7	119
24	4 H502	DIAPHRAGM	E 20		7	2.00									7	2	7	2	115
8	5 H503	STRAND TIE	E 20		2	10.00									2	10	2	10	24
8	5 H504	STRAND TIE	E 20		3	11.00									3	11	3	11	33
72	4 U500	DIAPHRAGM	E 28S				15.00	3	2.00	2	0.00				6	5	6	3	301
24	6 U501	DIAPHRAGM	E 28S				22.00	2	11.00	2	0.00				6	9	6	5	231
32	5 U502	DIAPHRAGM	E 19S		2	7.50	10.00								3	6	3	4	111
16	5 V500	DIAPHRAGM	E 20		3	3.00									3	3	3	3	54
		Slab																	
56	5 S1	SLAB	E 20		54	8.00									54	8	54	8	3,193
50	5 S2	SLAB	E 20		19	9.00									19	9	19	9	1,030
50	5 S3	SLAB	E 20		26	3.00									26	3	26	3	1,369
205	6 S4	SLAB	E 20		32	5.00									32	5	32	5	9,981
16	5 S5	SLAB	E 20		54	11.00									54	11	54	11	916



PART PLAN SHOWING PILE NUMBERING FOR RECORDING AS-BUILT PILE DATA

As-Built Pile Data			
Pile No.	Length in Place (ft)	Computed Nominal Axial Compressive Resistance (kips)	Remarks
End Bent No. 1			
1			
2			
3			
4			
Int. Bent No. 2			
5			
6			
7			
8			
9			

As-Built Pile Data			
Pile No.	Length in Place (ft)	Computed Nominal Axial Compressive Resistance (kips)	Remarks
Int. Bent No. 3			
10			
11			
12			
13			
14			
End Bent No. 4			
15			
16			
17			
18			

Note:
Indicate in remarks column:
A. Pile type and grade
B. Batter
C. Driven to practical refusal

This sheet to be completed by MoDOT construction personnel.

STATE OF MISSOURI
TRENTON B. CRAWFORD
NUMBER PE-2012018053
PROFESSIONAL ENGINEER
THIS SHEET HAS BEEN
SIGNED, SEALED AND DATED
ELECTRONICALLY.

DATE PREPARED
12/17/2025

ROUTE U STATE MO

DISTRICT BR SHEET NO. 29

COUNTY
CALDWELL

JOB NO.
JNW0144

CONTRACT ID.

PROJECT NO.

BRIDGE NO.
A9660

DESCRIPTION									
DATE									

MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION

105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-273-6636)

